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J.1 – Summary of Geotechnical Design Parameters



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

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Problem Statement

Summarize the geotechnical parameters for the various landfill and in-situ materials present at the Zion Landfill.

Background

The material strengths of the geologic units described in the following text are derived from laboratory testing that has been conducted using on-site material. The geotechnical properties of the constructed earthen layers (e.g. recompacted soil liner, granular soil layer) are based on site-specific laboratory testing and properties of similar materials. The geotechnical properties of waste are based on published data. Test results of laboratory data used to support this summary are summarized in the attached tables. Laboratory test results are included in **Appendix I**.

As geosynthetics are manufactured products, they can be designed and/or selected to meet strength requirements. The geosynthetic interface shear strengths are calculated values based on the Zion Landfill configuration that have been determined to be appropriate, as further described in **Appendix J.2**.

Given

- Hydrogeologic and Design Drawings contained in this Application.
- Section 2.2** Hydrogeologic Investigation, in this Application.
- Boring Logs contained in this Application.
- Laboratory Test Results contained in this Application (please refer to the **Appendix I**).
- Bray, Jonathan D., Zekkos, Dimitrios, Kavazanjian, Edward Jr., Athanasopopoulos, George A., and Riemer, Michael F., Shear Strength of Municipal Solid Waste, Journal of Geotechnical and Geoenvironmental Engineering, June, 2009 (please refer to attached pages).
- Geotechdata.info, Angle of friction, <http://geotechdata.info/parameter/cohesion> (as of December 15, 2013) (please see attached pages).
- Geotechdata.info, Cohesion, <http://geotechdata.info/parameter/cohesion> (as of December 15, 2013) (please see attached pages).
- Coduto, Donald P., Geotechnical Engineering, Principles and Practices (please refer to attached pages).
- Vilar, O.M., and Carvalho, M.F., Shear Strength and Consolidation Properties of Municipal Solid Waste, International Workshop "Hydro-Physico-Mechanics of Landfill" Grenoble 1 University, France, 21-22 March 2005 (please refer to attached pages).
- Babu, G.L. Sivakumar, Reddy, Krishna R., Chouksey, Sandeep K., Constitutive Model for Municipal Solid Waste Incorporating Mechanical Creep and Biodegradation-Induced Compression, 2009.



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- Kavasanjian, Edward, The Impact of Degradation on MSW Shear Strength, Conference Proceeding Paper from the GeoCongress 2008: Geotechnics of Waste Management and Remediation (GSP177), MSW Shear Strength (please refer to attached pages).
- Compression Indices for Full-Scale Landfills*, July 8, 2008, completed by C.Bareither (please refer to attached pages).
- USGS National Seismic Hazard Mapping Project found on the internet/online at <http://earthquake.usgs.gov/hazards/> (please see attached pages).
- Computer model SLIDE

Assumptions*Layer Configuration*

The Horizontal and Vertical Expansion areas will include the various components detailed in **Table J.1-1** (system layers are listed from top to bottom).

Table J.1-1 Zion Landfill – Site 2 North Expansion Layer Configuration Overview		
Component	Horizontal Expansion	Existing Landfill / Vertical Expansion
Final Cover System	<ul style="list-style-type: none"> • 3-foot protective soil cover (6-inch vegetative soil layer and 2.5-foot protective soil layer) • 6-oz/yd² double-sided geocomposite drainage layer • 40-mil textured LLDPE geomembrane • 2-foot compacted low permeable soil layer soil layer ($k \leq 1 \times 10^{-5}$ cm/sec) 	<ul style="list-style-type: none"> • 3-foot protective soil cover (6-inch vegetative soil layer and 2.5-foot protective soil layer) • 6-oz/yd² double-sided geocomposite drainage layer • 40-mil textured LLDPE geomembrane • 2-foot compacted low permeable soil layer ($k \leq 5 \times 10^{-7}$ cm/sec)
Waste	<ul style="list-style-type: none"> • A maximum waste column height of approximately 198 feet occurs in the proposed horizontal expansion area of the landfill beneath the final landform elevation of approximately 896 feet MSL. 	<ul style="list-style-type: none"> • A maximum waste column height of approximately 206 feet occurs in the proposed vertical expansion area of the landfill beneath the final landform elevation of approximately 896 feet MSL.
Leachate Collection System (LCS) / Liner System ¹	<ul style="list-style-type: none"> • 8-oz/yd² non-woven geotextile filter • 1-foot granular drainage layer ($d < 1.0$ inches, $k \geq 1 \times 10^{-1}$ cm/sec) • 12-oz/yd² non-woven geotextile cushion • 60-mil HDPE textured geomembrane • 5-foot low permeable earth liner ($k \leq 1 \times 10^{-7}$ cm/sec) 	<p><u>Cell 6:</u></p> <ul style="list-style-type: none"> • 6-oz/yd² non-woven geotextile filter on floor • 1-foot granular drainage layer ($d < 0.187$-inch, $k \geq 1 \times 10^{-2}$ cm/sec) • 60-mil HDPE geomembrane (textured on sideslopes, smooth on floor) • 5-foot low permeable earth liner ($k \leq 1 \times 10^{-7}$ cm/sec) <p><u>Cells 7 and 9:</u></p> <ul style="list-style-type: none"> • 6-oz/yd² nonwoven geotextile filter • 1-foot granular drainage layer ($D < 1.0$-inch, $k \geq 1 \times 10^{-1}$ cm/sec) • 12-oz/yd² nonwoven geotextile cushion • 60-mil HDPE textured geomembrane • 5-foot low permeable earth liner ($k \leq 1 \times 10^{-7}$ cm/sec)



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<p>LCS Pipe Trench / Liner System</p>	<ul style="list-style-type: none"> • 8-oz/yd² non-woven geotextile filter • 24-inch coarse aggregate (D ≤ 1.5-inches) • 12-oz/yd² non-woven geotextile cushion • 60-mil HDPE textured geomembrane • 5-foot low permeable earth liner 	<p><u>Cell 6:</u></p> <ul style="list-style-type: none"> • 6-oz/yd² non-woven geotextile filter • 6-inches intermediate aggregate (D < 0.5-inches) • 1.5-feet coarse aggregate (D ≤ 1.5-inches) • 12-oz/yd² non-woven geotextile • 60-mil HDPE smooth geomembrane • 5-foot low permeable earth liner ($k \leq 1 \times 10^{-7}$ cm/sec) <p><u>Cells 7 and 9:</u></p> <ul style="list-style-type: none"> • 6-oz/yd² non-woven geotextile filter • 2-feet coarse aggregate (D ≤ 1.5-inches) • 12-oz/yd² non-woven geotextile cushion • 60-mil HDPE textured geomembrane • 5-foot low permeable earth liner ($k \leq 1 \times 10^{-7}$ cm/sec)
<p>In-Situ Underlying Geologic Units</p>	<ul style="list-style-type: none"> • Wadsworth Formation • Shallow Drift Aquifer (combination sands, silts, and clays) 	<ul style="list-style-type: none"> • Wadsworth Formation • Shallow Drift Aquifer (combination sands, silts, and clays)
<p>Notes:</p> <p>1. The “Leachate Collection System (LCS)/Liner System” and “LCS Pipe Trench/Liner System” identified for the existing landfill/vertical expansion area reflects the constructed landfill configuration. Constructed landfill configurations do not necessarily match what was previously permitted.</p>		



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Summary of Geologic Unit and Constructed Layer Properties

A summary of the material unit weights and shear strength values for all landfill layers and geologic units is presented in the table below. These values are used in the geotechnical calculations presented in **Appendices J.2** through **J.5**. All values are considered conservative, and are derived from laboratory test data, field test data, and test data for similar materials at other landfill project sites. Laboratory testing results are summarized in the attached tables. Supporting test reports are provided in **Appendix I**.

Table J.1-2 Zion Landfill – Site 2 North Expansion Summary of Material Unit Weights and Shear Strength							
Layer Description	Dry Unit Weight " γ_{dry} " (pcf)	Total Unit Weight " γ_{total} " (pcf)	Saturated Unit Weight " $\gamma_{saturated}$ " (pcf)	Shear Strength Short-Term Conditions ¹		Shear Strength Long-Term Conditions ²	
				Cohesion c (psf)	Friction Angle ϕ' (degrees)	Cohesion c' (psf)	Friction Angle ϕ' (degrees)
In Situ / Foundation Soils Beneath Landfill & Outside Landfill Footprint							
Wadsworth Till	118.4	136.6	137.8	1,465	11.8	1,000	14.3
Shallow Drift Aquifer ³	104.8	123.3	129.8	-	-	-	-
Landfill Layers:							
Final Cover Soils ⁴	106.7	121.5	130.3	1,465	11.8	0	34.3
Waste Fill ⁵	75.0	75.0	75.0	0	33	0	30
LCS Granular Drainage Layer ⁶	125.0	126.0	130.0	0	30	0	30
Low Permeable Earth Liner ⁴	112.6	128.2	134.1	1,465	11.8	0	34.3

Notes:

1. Shear strength values for short-term conditions of the Wadsworth Till, Final Cover Soils, Low Permeable Earth Liner are derived from the unconsolidated-undrained triaxial shear strength Mohr circles (see attached figures). It is assumed these conditions occur during initial landfill cell development and interim waste fill heights / active landfill cell phase. A summary of the test results are presented in the attached Tables and the complete laboratory test results are provided in **Appendix I**. The Mohr circles are also provided in the attached pages.
2. Shear strength values for long-term conditions of the Final Cover Soils and Low Permeable Earth Liner are conservatively derived from the Mohr circles of the effective stress, consolidated-undrained triaxial shear strength tests. The long-term shear strength value assumed for the Wadsworth Till is derived from the Mohr circles of the total stress, consolidated-undrained triaxial shear strength tests. A summary of the test results are presented in the attached Tables and the complete laboratory test results are provided in **Appendix I**. The Mohr circles are also provided in the attached pages.
3. The Shallow Drift Aquifer, Lower Till, Basal Drift, and Bedrock units are significantly lower than the proposed landfill base and therefore were not considered in the geotechnical analyses.
4. The unit weights of the Final Cover Soils and the Low Permeable Earth Liner are derived from the results of Standard Proctor tests performed on the Wadsworth Till soils. It was assumed that the Final Cover Soils and Earth Liner Soils will be compacted to 90% and 95% of the Standard Proctor test results, respectively, and therefore the unit weights of the Final Cover Soils and Low Permeable Earth Liner are based on these corresponding values. A summary of the Standard Proctor test results are presented on Table 3 in the attached pages. The complete Standard Proctor laboratory test reports are provided in **Appendix I**. Modified Proctor testing was not analyzed for hydraulic conductivity testing or shear strength evaluations. In the event that Modified Proctors are intended to be used during construction, shear strength values should be evaluated prior to use to ensure that the strength parameters fall within the acceptable ranges identified within this Appendix. While direct comparisons cannot be made, 90% and 95% of the Standard Proctor (for the Final Cover Soil and Low Permeable Earth Liner, respectively) generally produce similar compactive efforts as 85% and 90% of the Modified Proctors, respectively. It should be noted that the unit weights of the Final Cover Soil and Low Permeable Earth Liner in the Site 2 North Expansion are as conservative as or more conservative than the values used for the Zion Site 2 East Expansion completed in 2011 by Weaver Boos Consultants, Inc.
5. The unit weight of the waste fill is based on an average value reported in published technical literature (see attached pages). The shear strength values of the waste fill are based on bilinear shear strength envelopes results for MSW waste in conventional landfills described in Eid et. al (2000). It should be noted that the unit weights and shear strength values of the Waste for the Site 2 North Expansion are as conservative as or more conservative than the values used for the Zion Site 2 East Expansion completed in 2011 by Weaver Boos Consultants, Inc.
6. The unit weights of the LCS Granular Drainage Layer are based on the unit weights of similar materials used at other landfills. It should be noted that the unit weights and shear strength values of the LCS granular drainage layer for the Site 2 North Expansion are as conservative as or more conservative than the values used for the Zion Site 2 East Expansion completed in 2011 by Weaver Boos Consultants, Inc.



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Summary of Consolidation Parameters

The consolidation parameters presented in the following table were used to calculate the primary and secondary settlement of the waste mass and compressible soil layers (i.e., final cover, low permeable earth liner, and Wadsworth Till foundation soil) due to compression at the landfill.

- Waste Fill.** The consolidation parameters for the waste fill are based on average data reported in published technical literature (see attached pages).
- Constructed Earthen Layers and Wadsworth Till Foundation.** The consolidation parameters for the landfill soil layers (i.e., final cover, and low permeable earth liner) and the Wadsworth Till are derived from the results of consolidation tests performed on representative samples of the on-site Wadsworth Till soils. The overconsolidation ratio (OCR) calculated for the samples tested ranged from approximately 0.82 to 1.52. The average OCR value was determined to be 1.25. Sample B-02-18 had a calculated OCR value of 0.82 and was therefore assumed to be disturbed and not included in the calculated average values reported in Table 6 (see attached pages). The on-site Wadsworth Till soils were determined to be normally consolidated. Note the settlement calculations assume normally consolidated soil conditions. A summary of the consolidation test data including pre-consolidation stresses, existing overburden stresses, OCRs, and sample locations/depths are reported on Table 9 in the attached pages. Additionally, the complete consolidation test reports are provided in **Appendix I**.

Layer Description	Compression Index C_c (C'_c)	Void Ratio e_o	Recompression Index C_r	Secondary Compression Index C_{α} (C'_{α})
Waste Fill	- (0.172)	0.650	--	0.084 (0.051)
Final Cover Low Permeable Earth Liner Wadsworth Till	0.08 -	0.432	0.021	0.0032 -

Seismic Impact Zone

A seismic impact zone is defined as an area with a 10% or greater probability that the maximum horizontal acceleration in lithified earth material, expressed as a percentage of the earth's gravitational pull, will exceed 0.10g in 250 years. Maximum horizontal acceleration is defined as the maximum expected horizontal acceleration depicted on a seismic hazard map, with a 90% or greater probability that the acceleration will not be exceeded in 250 years. From the United State Geologic Survey (USGS) Earthquake Hazards Program - National Seismic Hazard Mapping website, the seismic coefficient for the landfill site area was determined to be 0.0461g (please refer to attached pages).



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Water Surface Assumed in Geotechnical Stability Calculations

A liquid level representing a 1-foot leachate level was assumed to be at the top of the LCS drainage layer along the entire landfill base. The groundwater elevation was conservatively assumed to be 5-feet below the existing ground surface outside of the landfill waste boundary.

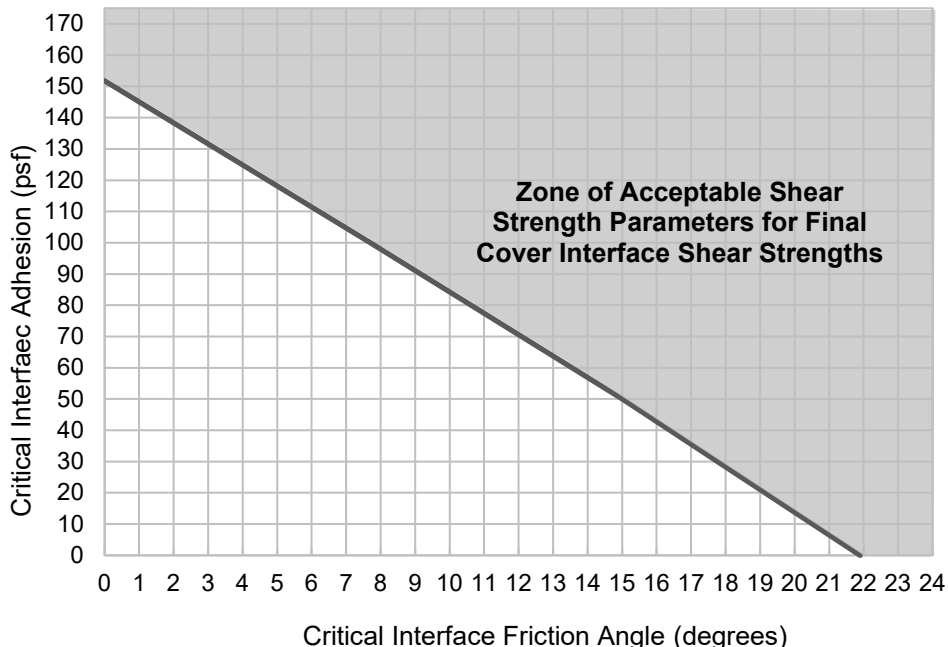
Summary of Geosynthetic Material Interfaces

Geosynthetics are used in both the existing and proposed designs to provide engineered barriers (geomembranes), drainage medium (geocomposites), and filtration and cushioning (geotextiles). Calculations have been completed as part of this appendix to identify the range of acceptable shear strength values that must be met to ensure minimum acceptable safety factors of 1.5 for static conditions and 1.3 for seismic conditions during all phases of the landfill life (during construction, closure, and post closure).

Calculation J.2-A identifies the lowest peak shear strength that produces acceptable factors of safety for any potential interface within the final cover. The most critical failure surface will be located between one of the following interfaces:

- Low permeable soil layer vs. textured geomembrane;
- Textured geomembrane vs. geocomposite; or
- Geocomposite vs. protective soil cover layer.

Acceptable values for these interfaces are shown below. It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in this calculation. Interface friction test results for previous final cover construction events is attached demonstrating existing construction achieves the required acceptable values.





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Calculation J.2-B through J.2-E identifies the lowest peak shear strength that produces acceptable factors of safety for any potential interface within the liner and leachate collection system through all stages of operations, including prior to waste placement, during operations, and after closure. The interfaces that are considered include the following:

- Geotextile to granular drainage layer;
- Geotextile to geomembrane; or
- Geomembrane to low permeable earth liner.

Acceptable peak shear strength values for these interfaces are shown in the following chart in the gray shaded area. It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in geotechnical evaluations. Interface friction test results for previous liner construction events is attached demonstrating existing construction achieves the required acceptable values.

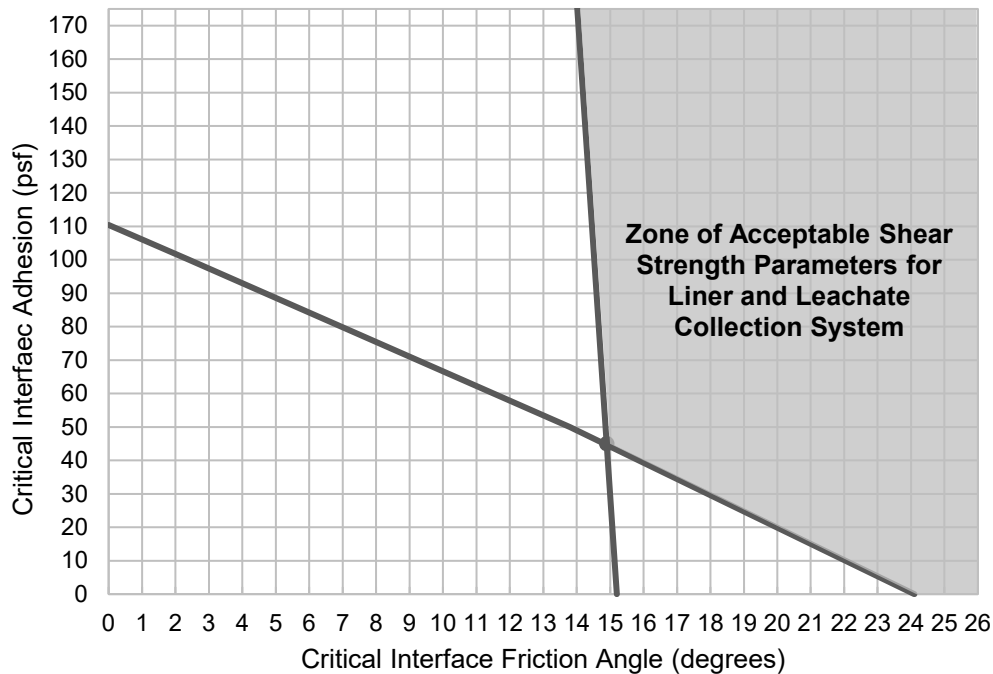


TABLE 1
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF GEOTECHNICAL LABORATORY TEST RESULTS FOR WADSWORTH TILL

Boring No.	Sample No.	Depth BGS (feet)	USCS Soil Class.	Soil	Grain Size Analysis: ASTM C136 & ASTM D1140				Atterberg Limits: ASTM D4318			Properties Below Taken from Hydraulic Conductivity Test Data (ASTM D5084) or Specific Gravity Test Data (ASTM D854)							Hydr. Cond. "k" (cm/sec)		
					Gravel (%)	Sand (%)	Silt (%)	Clay (%)	PL	LL	PI	Specific Gravity "G _s "	Void Ratio "e"	Porosity "n"	Saturation "S" (%)	Moisture Content "w" (%)	Dry Unit Weight "γ _{dry} " (pcf)	Total Unit Weight "γ _{total} " (pcf)		Sat. Unit Weight "γ _{sat} " (pcf)	
B-01-18	ST-35	68.0 - 70.0'	CL	Grey CLAY	-	-	-	-	-	-	-	2.750	0.42	0.30	98.0	15.0	120.7	138.9	139.3	2.38E-08	
B-03-18	ST-34	66.0 - 68.0'	CL	Dark Grey Silty CLAY	-	-	-	-	-	-	-	2.750	0.38	0.28	92.0	12.7	124.3	140.2	141.5	4.77E-08	
B-05-18	ST-37	72.0 - 74.0'	CL	Brown-Grey CLAY	-	-	-	-	-	-	-	2.750	0.67	0.40	99.0	24.3	102.6	127.5	127.8	4.85E-08	
B-10-18	ST-30	58.0 - 60.0'	CL-ML	Grey Silty CLAY with thin layers of dry SILT	-	-	-	-	-	-	-	2.750	0.45	0.31	87.0	14.3	118.1	135.0	137.6	9.84E-08	
B-11-18	ST-29	56.0 - 58.0'	CL	Grey CLAY	-	-	-	-	-	-	-	2.750	0.46	0.31	100.0	16.6	117.9	137.5	137.4	3.63E-08	
B-14-18	ST-33	64.0 - 66.0'	CL	Grey Silty CLAY	-	-	-	-	-	-	-	2.750	0.35	0.26	84.0	10.8	126.9	140.6	143.2	3.65E-08	
B-01-18	SS-23	44.0 - 46.0'	CL	Grey Lean CLAY with Sand	2.2	19.1	51.8	26.9	11	26	15	-	-	-	-	-	-	-	-	-	
B-01-18	SS-27	52.0 - 54.0'	CL	Light Grey Lean CLAY	0.8	5.5	52.9	40.8	12	34	22	-	-	-	-	-	-	-	-	-	
B-02-18	SS-30	58.0 - 60.0'	CL	Light Brown Lean CLAY	0.2	7.7	55.9	36.2	12	30	18	-	-	-	-	-	-	-	-	-	
B-02-18	SS-33	64.0 - 66.0'	CL	Grey Silty Lean CLAY with Sand	1.3	18.1	58.5	22.1	10	21	11	-	-	-	-	-	-	-	-	-	
B-03-18	SS-29	56.0 - 58.0'	CL	Grey Lean CLAY	1.7	7.7	55.6	35.0	15	30	15	-	-	-	-	-	-	-	-	-	
B-03-18	SS-32	62.0 - 64.0'	CL	Grey Silty Lean CLAY	0.9	6.4	52.2	40.5	15	32	17	-	-	-	-	-	-	-	-	-	
B-04-18	SS-20	38.0 - 40.0'	CL	Greay Sandy Lean CLAY	9.4	27.4	51.2	12.0	11	19	8	-	-	-	-	-	-	-	-	-	
B-04-18	SS-23	44.0 - 46.0'	CL	Light Grey Lean CLAY with Sand	2.9	19.8	48.0	29.3	11	29	18	-	-	-	-	-	-	-	-	-	
B-05-18	SS-31	60.0 - 62.0'	CL	Light Grey Lean CLAY	0.0	12.3	52.9	34.8	13	28	15	-	-	-	-	-	-	-	-	-	
B-06-18	SS-31	60.0 - 62.0'	CL	Grey Lean CLAY	1.1	11.2	52.3	35.4	12	33	21	-	-	-	-	-	-	-	-	-	
B-07-18	SS-15	28.0 - 30.0'	CL	Light Brown Lean CLAY	0.9	12.5	64.7	21.9	14	23	9	-	-	-	-	-	-	-	-	-	
B-07-18	SS-24	46.0 - 48.0'	CL	Grey Lean CLAY with Sand	1.4	18.0	56.8	23.8	13	27	14	-	-	-	-	-	-	-	-	-	
B-08-18	SS-30	58.0 - 60.0'	CL	Brown, Grey Lean CLAY with Sand	0.6	19.1	53.2	27.1	13	28	15	-	-	-	-	-	-	-	-	-	
B-09-18	SS-22	42.0 - 44.0'	CL-ML	Grey Silty CLAY with Sand	1.0	27.4	62.0	9.6	14	18	4	-	-	-	-	-	-	-	-	-	
B-09-18	SS-28	54.0 - 56.0'	CL	Grey Lean CLAY with SAND	2.0	17.2	52.1	28.7	14	27	13	-	-	-	-	-	-	-	-	-	
B-10-18	SS-23	44.0 - 46.0'	CL	Brown-Grey Lean CLAY with Sand	2.6	17.6	49.7	30.1	14	26	12	-	-	-	-	-	-	-	-	-	
B-10-18	SS-28	54.0 - 56.0'	CL	Grey Lean CLAY with SAND	3.2	16.6	49.7	30.5	14	28	14	-	-	-	-	-	-	-	-	-	
B-11-18	SS-22	42.0 - 44.0'	CL-ML	Grey Sandy Silty CLAY	3.1	36.2	49.4	11.3	12	17	5	-	-	-	-	-	-	-	-	-	
B-11-18	SS-28	54.0 - 56.0'	CL	Grey Lean CLAY with Sand	0.9	19.2	50.4	29.5	14	27	13	-	-	-	-	-	-	-	-	-	
B-12-18	SS-27	52.0 - 54.0'	CL	Grey Lean CLAY with Sand	4.4	18.1	49.2	28.3	13	25	12	-	-	-	-	-	-	-	-	-	
B-12-18	SS-32	62.0 - 64.0'	CL	Grey Lean CLAY with Sand	3.6	17.4	55.7	23.3	12	25	13	-	-	-	-	-	-	-	-	-	
B-13-18	SS-17	32.0 - 34.0'	CL-ML	Grey Silty CLAY	0.0	1.5	79.5	19.0	15	22	7	-	-	-	-	-	-	-	-	-	
B-13-18	SS-26	50.0 - 52.0'	CL	Grey Lean CLAY with Sand	5.5	17.8	50.8	25.9	14	26	12	-	-	-	-	-	-	-	-	-	
B-14-18	SS-26	50.0 - 52.0'	CL	Grey Lean CLAY	0.0	0.4	49.7	49.9	18	40	22	-	-	-	-	-	-	-	-	-	
B-14-18	SS-31	60.0 - 62.0'	CL-ML	Brownish Grey Sandy Silty CLAY	0.6	41.6	43.8	14.0	12	18	6	-	-	-	-	-	-	-	-	-	
B-15-18	SS-24	46.0 - 48.0'	CL	Greay Sandy Lean CLAY	6.0	25.2	42.9	25.9	14	28	14	-	-	-	-	-	-	-	-	-	
B-15-18	SS-29	56.0 - 58.0'	CL	Grey Lean CLAY	0.0	1.5	54.9	43.6	15	32	17	-	-	-	-	-	-	-	-	-	
					Minimum:	0.0	0.4	42.9	9.6	10.0	17.0	4.0	2.750	0.35	0.26	84.0	10.8	102.6	127.5	127.8	2.38E-08
					Maximum:	9.4	41.6	79.5	49.9	18.0	40.0	22.0	2.750	0.67	0.40	100.0	24.3	126.9	140.6	143.2	9.84E-08
					Average:	2.1	16.4	53.5	28.0	13.2	26.6	13.4	2.750	0.46	0.31	93.3	15.6	118.4	136.6	137.8	4.85E-08

Note:

1) Values in cells shaded in gray were calculated using the soil density relationships below.

Soil Density Relationships:

$$\gamma_{dry} = [(G_s \times \gamma_w) / (1+e)]$$

$$\gamma_{total} = [(G_s + Se) \times \gamma_w] / (1+e)$$

$$\gamma_{total} = \gamma_d \times (1 + (w\% / 100))$$

$$\gamma_{sat} = [(G_s + e) \times \gamma_w] / (1+e)$$

$$S\% = (w \times G_s) / e$$

$$n = e / (1+e)$$

$$e = n / (1-n)$$

TABLE 2
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF GEOTECHNICAL LABORATORY TEST RESULTS FOR SHALLOW DRIFT AQUIFER

Boring No.	Sample No.	Depth BGS (feet)	USCS Soil Class.	Soil	Properties Below Taken from Hydraulic Conductivity Test Data (ASTM D5084) or Specific Gravity Test Data (ASTM D854)											
					Grain Size Analysis: ASTM C136 & ASTM D1140				Specific Gravity "G _s "	Void Ratio "e"	Porosity "n"	Saturation "S" (%)	Moisture Content "w" (%)	Dry Unit Weight "γ _{dry} " (pcf)	Total Unit Weight "γ _{total} " (pcf)	Sat. Unit Weight "γ _{sat} " (pcf)
					Gravel (%)	Sand (%)	Silt (%)	Clay (%)								
B-05-18	SS-59	116.0 - 118.0'	SM	Grey Silty SAND	0.0	77.6	20.6	1.8	2.789	0.76	0.43	63.0	17.1	99.0	116.0	125.9
B-06-18	SS-53	104.0 - 106.0'	SM	Grey Silty SAND	4.4	82.7	12.3	0.6	2.814	0.53	0.35	75.0	14.2	114.5	130.8	136.2
B-08-18	SS-61	120.0 - 122.0'	SM	Grey Silty SAND	0.0	81.7	16.3	2.0	2.790	0.73	0.42	84.0	22.0	100.8	123.0	127.2
Minimum:					0.0	77.6	12.3	0.6	2.789	0.53	0.35	63.0	14.2	99.0	116.0	125.9
Maximum:					4.4	82.7	20.6	2.0	2.814	0.76	0.43	84.0	22.0	114.5	130.8	136.2
Average:					1.5	80.7	16.4	1.5	2.798	0.67	0.40	74.0	17.8	104.8	123.3	129.8

Soil Density Relationships:

$$\gamma_{dry} = [(G_s \times \gamma_w)] / (1+e)$$

$$\gamma_{total} = [(G_s + Se) \times \gamma_w] / (1+e)$$

$$\gamma_{total} = \gamma_d \times (1 + (w\% / 100))$$

$$\gamma_{sat} = [(G_s + e) \times \gamma_w] / (1+e)$$

$$S\% = (w \times G_s) / e$$

$$n = e / (1+e)$$

$$e = n / (1-n)$$

TABLE 3
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF SOIL PROPERTIES AND STANDARD PROCTOR TEST DATA - WADSWORTH FORMATION

Boring Location	Depth BGS (feet)	USCS Soil Class.	Standard Proctor Test Results (ASTM D698)		95% of Standard Proctor			90% of Standard Proctor		
			$\gamma_{dry-max}$ (pcf)	MC_{opt} (%)	γ_{dry} (pcf)	γ_{total} (pcf)	$\gamma_{saturated}$ (pcf)	γ_{dry} (pcf)	γ_{total} (pcf)	$\gamma_{saturated}$ (pcf)
B-01-18 Brown and Grey CLAY	6.0 - 79.0'	CL	118.1	14.5	112.2	128.5	133.8	106.3	121.7	130.0
B-05-18 Grey CLAY	16.0 - 115.5'	CL	119	13.2	113.1	128.0	134.3	107.1	121.2	130.6
MINIMUM:			118.1	13.2	112.2	128.0	133.8	106.3	121.2	130.0
MAXIMUM:			119	14.5	113.1	128.5	134.3	107.1	121.7	130.6
AVERAGE:			118.6	13.9	112.6	128.2	134.1	106.7	121.5	130.3

Notes:

1) Values in cells shaded in gray were calculated using the soil density relationships below.

Soil Density Relationships:

$$\gamma_{total} = \gamma_d \times (1 + (w\% / 100))$$

$$\gamma_{sat} = [(G_s + e) \times \gamma_w] / (1+e)$$

TABLE 4
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF REMOLDED PERMEABILITY TEST RESULTS FOR REMOLDED WADSWORTH TILL SAMPLES

Boring No.	Depth BGS (feet)	Description	Hydraulic Conductivity Test Data (ASTM D5084)								
			Specific Gravity "G _s "	Void Ratio "e"	Porosity "n"	Saturation "S" (%)	Moisture Content "w" (%)	Dry Unit Weight "γ _{dry} " (pcf)	Total Unit Weight "γ _{total} " (pcf)	Sat. Unit Weight "γ _{sat} " (pcf)	Hydr. Cond. "k" (cm/sec)
B-01-18	6'-79'	Opt+ 0%	2.750	0.49	0.33	78	14.0	114.9	131.0	135.5	2.13E-07
		Opt+ 2%	2.750	0.50	0.33	88	16.0	114.4	132.8	135.2	3.05E-08
		Opt+ 4%	2.750	0.51	0.34	99	18.2	113.9	134.7	134.9	1.95E-08
B-05-18	16'-115.5'	Opt+ 0%	2.750	0.50	0.33	71	12.9	114.6	129.3	135.3	1.73E-07
		Opt+ 2%	2.750	0.47	0.32	87	15.0	116.5	134.0	136.6	3.53E-08
		Opt+ 4%	2.750	0.49	0.33	97	17.4	115.0	135.0	135.6	1.54E-08
MINIMUM:			2.750	0.47	0.32	71	12.9	113.9	129.3	134.9	1.54E-08
MAXIMUM:			2.750	0.51	0.34	99	18.2	116.5	135.0	136.6	2.13E-07
AVERAGE:			2.750	0.49	0.33	87	15.6	114.9	132.8	135.5	4.78E-08

Note:

1) Values in cells shaded in gray were calculated using the soil density relationships below.

Soil Density Relationships:

$$\gamma_{dry} = [(G_s \times \gamma_w) / (1+e)]$$

$$\gamma_{total} = [(G_s + Se) \times \gamma_w] / (1+e)$$

$$\gamma_{total} = \gamma_d \times (1 + (w\% / 100))$$

$$\gamma_{sat} = [(G_s + e) \times \gamma_w] / (1+e)$$

$$S\% = (w \times G_s) / e$$

$$n = e / (1+e)$$

$$e = n / (1-n)$$

**TABLE 5
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF TRIAXIAL SHEAR STRENGTH TEST DATA - WADSWORTH FORMATION**

Boring Location	Sample No.	Depth (Feet MSL)	Soil	Mohr Circles			Total (Undrained) Shear Strength		Effective (Drained) Shear Strength			
				Failure Stress	1	2	3	Cohesion (C)	Friction Angle (ϕ)	Cohesion (C')	Friction Angle (ϕ')	
Consolidated Undrained - Triaxial Shear Strength: ASTM D2850												
B-01-18	ST-31	60.0 - 62.0'	Brown-Grey CLAY	σ_1 :	4,752 psf	8,496 psf	14,976 psf	752 psf	24.9 deg	508 psf	33.2 deg	
				σ_3 :	864 psf	2,016 psf	3,888 psf					
B-12-18	ST-34	66.0 - 68.0'	Grey Silty CLAY	σ_1 :	9,173 psf	11,750 psf	12,845 psf	2,740 psf	12.1 deg	48 psf	37.2 deg	
				σ_3 :	2,218 psf	2,851 psf	3,125 psf					
B-13-18	ST-29	56.0-58.0'	Grey Silty CLAY	σ_1 :	6,106 psf	9,202 psf	9,821 psf	1,974 psf	11.1 deg	726 psf	29.5 deg	
				σ_3 :	1,253 psf	2,174 psf	2,578 psf					
								MINIMUM:	752 psf	11.1 deg	48 psf	29.5 deg
								MAXIMUM:	2,740 psf	24.9 deg	726 psf	37.2 deg
								AVERAGE:	1,822 psf	16.0 deg	427 psf	33.3 deg
Unconsolidated Undrained - Triaxial Shear Strength: ASTM D4767												
B-06-18	ST-38	74.0 - 76.0'	Brown-Grey CLAY	σ_1 :	8,640 psf	12,816 psf	21,168 psf	1,519 psf	20.7 deg			
				σ_3 :	2,016 psf	4,032 psf	8,064 psf					
B-07-18	ST-29	56.0 - 58.0'	Brown-Grey Silty CLAY	σ_1 :	7,344 psf	10,368 psf	15,120 psf	2,481 psf	6.0 deg			
				σ_3 :	2,016 psf	3,312 psf	8,064 psf					
B-15-18	ST-30	58.0 - 60.0'	Brown-Grey CLAY	σ_1 :	8,064 psf	10,656 psf	15,840 psf	2,344 psf	7.8 deg			
				σ_3 :	2,016 psf	4,032 psf	8,064 psf					
								MINIMUM:	1,519 psf	6.0 deg		
								MAXIMUM:	2,481 psf	20.7 deg		
								AVERAGE:	2,115 psf	11.5 deg		

TABLE 6
ZION LANDFILL SITE 2 NORTH EXPANSION
SUMMARY OF 2018 CONSOLIDATION PARAMETERS

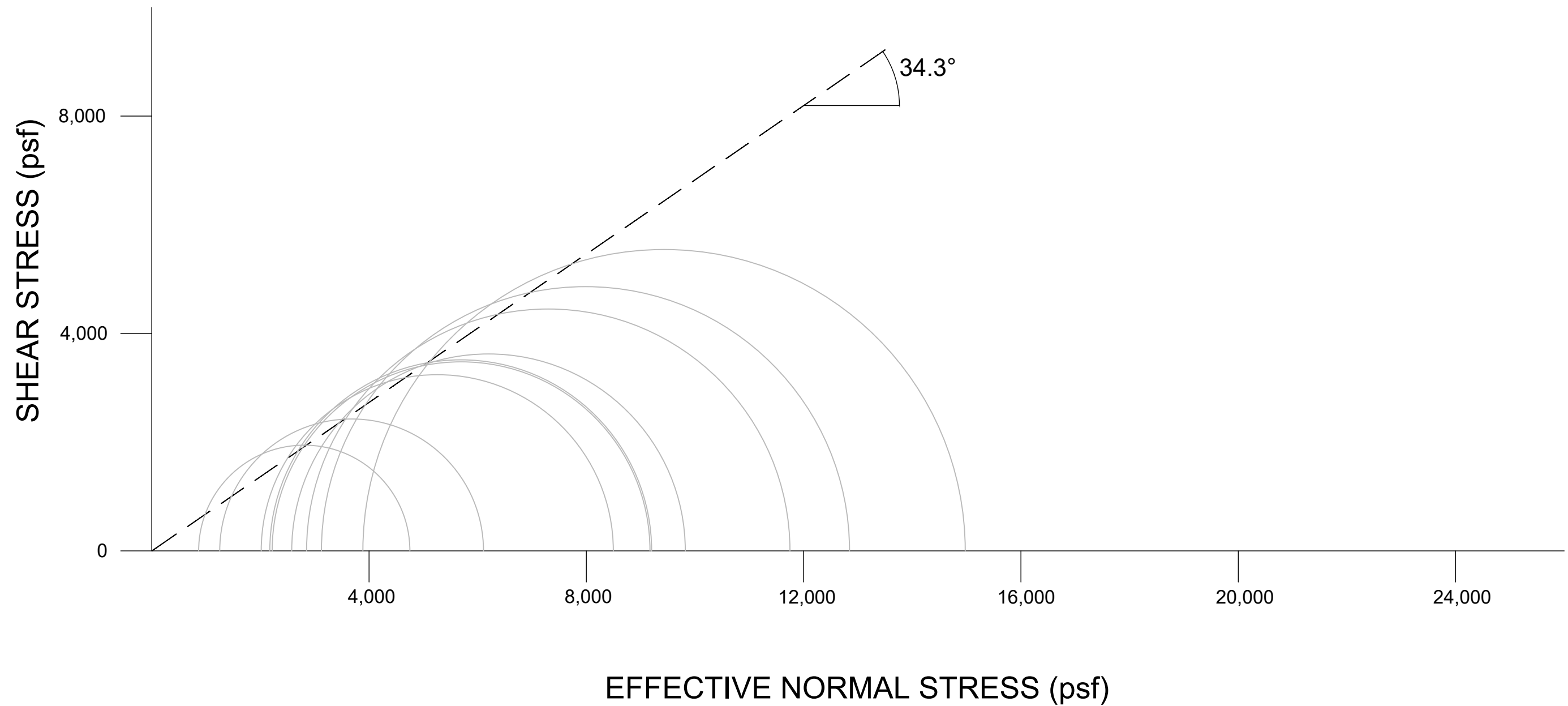
Geologic Unit	Specific Gravity G_s	Dry Unit Weight ¹ γ_{dry} (pcf)	Moisture Content %	Saturation (%)	Total Unit Weight ¹ γ_{total} (pcf)	Saturated Unit Weight ¹ γ_{sat} (pcf)	Pre-Consolidation Stress σ'_{pc} (psf)	Existing Effective Overburden Stress σ'_{vo} (psf)	OCR	Void Ratio (e_o)	Compression Index (C_c)	Recompression Index (C_r)	Secondary Compression Index (C_α)
Clay: B-02-18, ST-29 (56.0 - 58.0' bgs)	2.750	124.0	13.3	94.8	140.5	141.2	5,600	6,870	0.82	0.385	0.09	0.009	0.0036
Clay: B-04-18, ST-27 (52.0 - 54.0' bgs)	2.750	115.5	16.8	95.1	134.9	135.9	8,400	5,528	1.52	0.486	0.09	0.015	0.0036
Clay B-09-18, ST-31 (60.0 - 62.0' bgs)	2.750	124.6	13.2	95.7	141.0	141.6	7,000	7,097	0.99	0.378	0.07	0.027	0.0028
Averages:	2.750	120.1	15.0	95.4	138.0	138.8	7,700	6313	1.25	0.432	0.08	0.021	0.0032

Notes:


1. B-02-18 was assumed to be disturbed and was not included in the averages values reported in the table.
2. Consolidation tests were performed on representative soil samples taken at the borings/sample locations noted above.
3. The pre-consolidation stresses reported above were derived from the consolidation test data plots provided the attached pages.
4. The existing overburden stresses were calculated using the unit weight and the sample depth intervals reported above.

Mohr Circle Plots

EFFECTIVE STRESS CONSOLIDATED-UNDRAINED (CU) TRIAXIAL TEST RESULTS



REV. NO.	DATE	DESCRIPTION



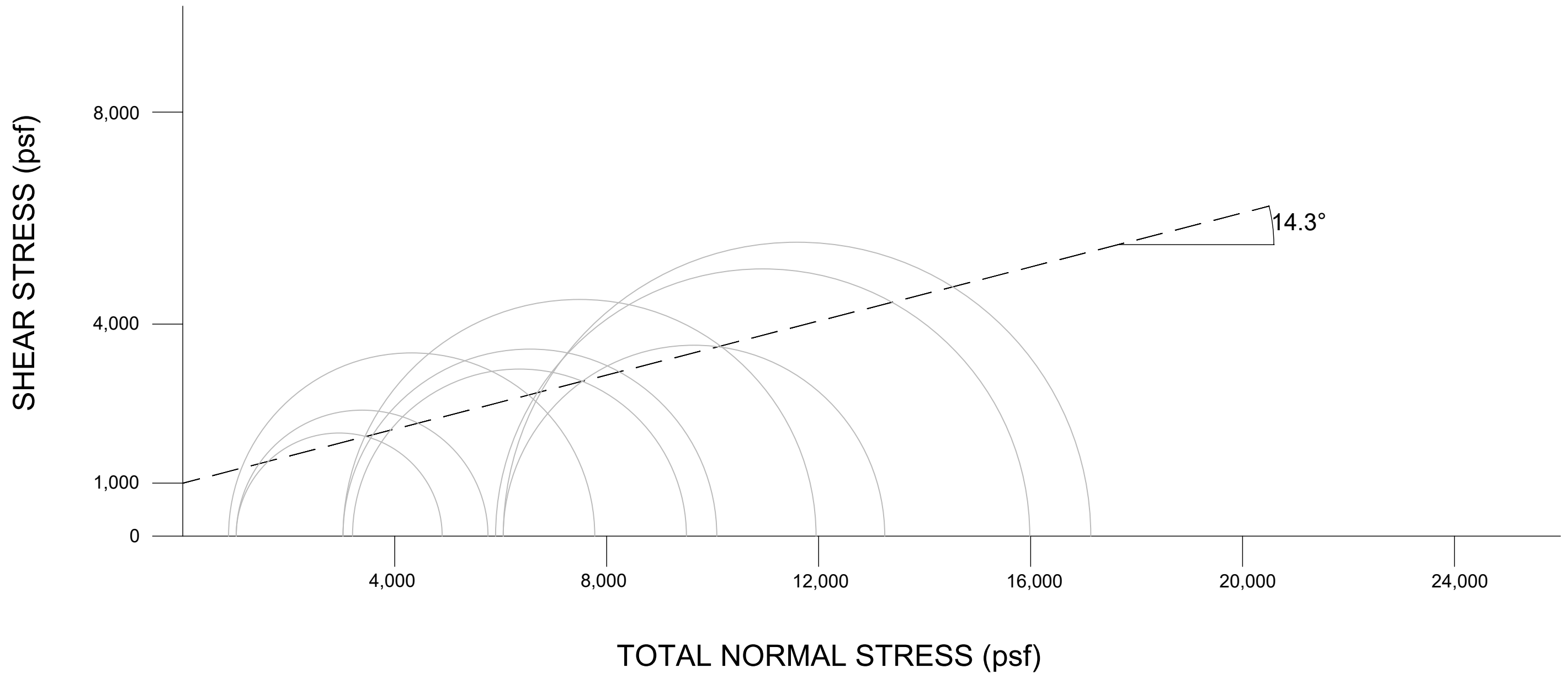
APTIM Environmental & Infrastructure, LLC

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ZION LANDFILL - SITE 2 NORTH EXPANSION MOHR CIRCLES			
EFFECTIVE SHEAR STRENGTH RESULTS CU TRIAXIAL SHEAR STRENGTH TESTS			
DRAWN BY:	ORC	APPROVED BY:	DAM
PROJ. NO.:	631020105	DATE:	FEBRUARY 2020

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TOTAL STRESS CONSOLIDATED-UNDRAINED (CU) TRIAXIAL TEST RESULTS



REV. NO.	DATE	DESCRIPTION



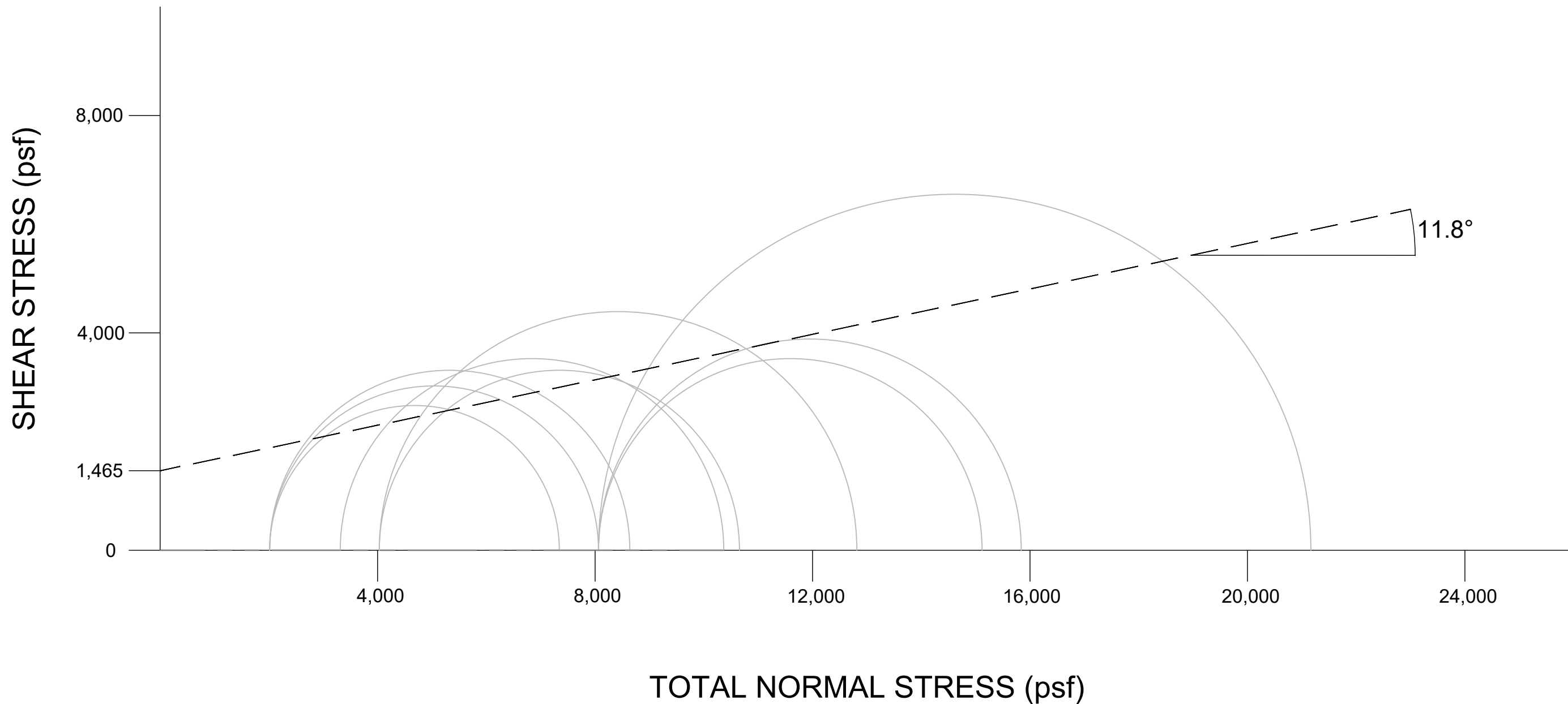
APTIM Environmental & Infrastructure, LLC

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ZION LANDFILL - SITE 2 NORTH EXPANSION MOHR CIRCLES			
TOTAL SHEAR STRENGTH RESULTS CU TRIAXIAL SHEAR STRENGTH TESTS			
DRAWN BY:	ORC	APPROVED BY:	DAM
PROJ. NO.:	631020105	DATE:	FEBRUARY 2020

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UNCONSOLIDATED-UNDRAINED (UU) TRIAXIAL TEST RESULTS



REV. NO.	DATE	DESCRIPTION



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**ZION LANDFILL - SITE 2 NORTH EXPANSION
MOHR CIRCLES**

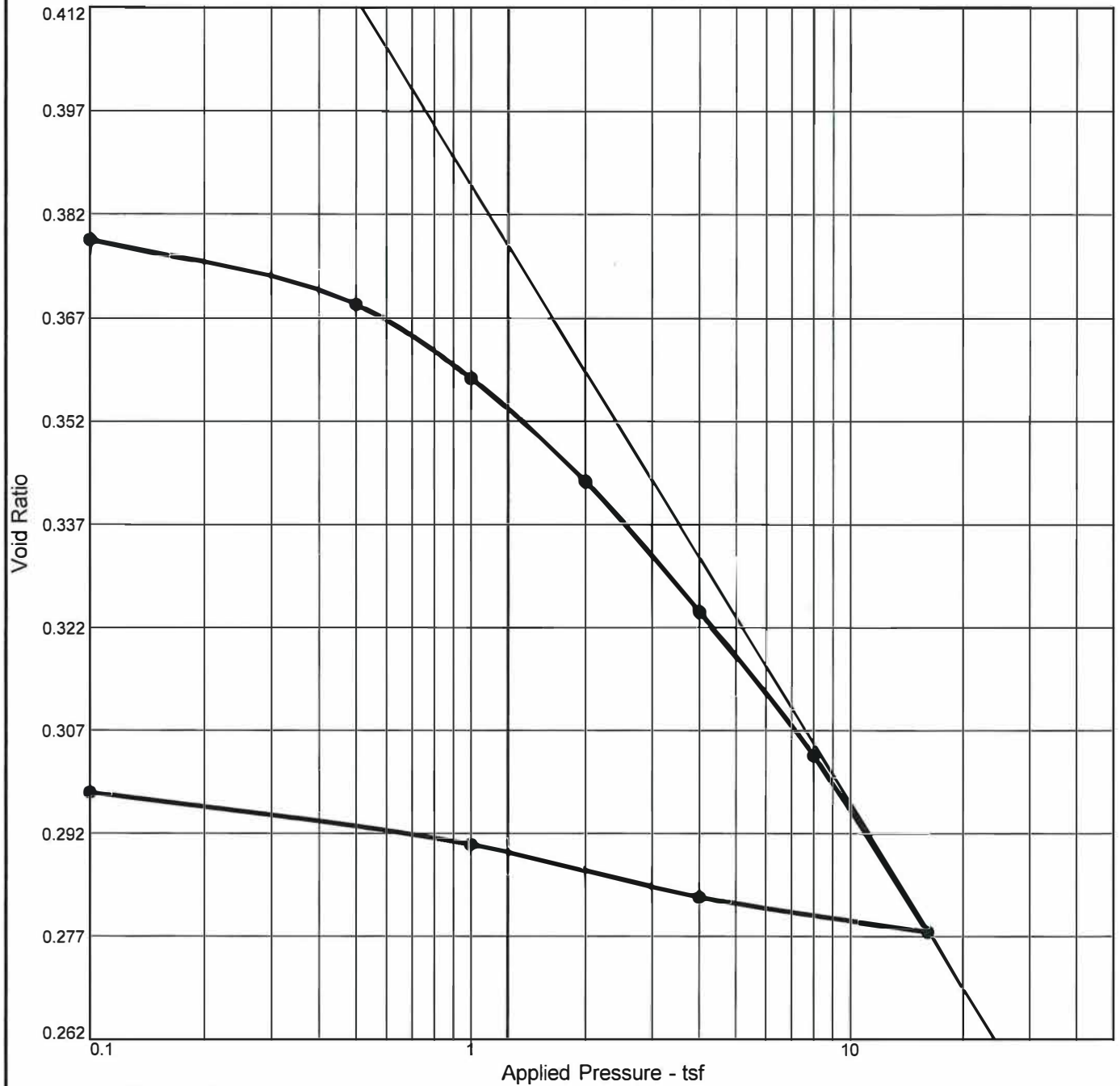
**TOTAL SHEAR STRENGTH RESULTS
UU TRIAXIAL SHEAR STRENGTH TESTS**

DRAWN BY: ORC APPROVED BY: DAM PROJ. NO.: 631020105 DATE: FEBRUARY 2020

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Consolidation Test Data Plots

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P _C (tsf)	C _c	C _r	Initial Void Ratio
Saturation	Moisture									
94.8 %	13.3 %	124.0			2.75		2.8	0.09		0.385

MATERIAL DESCRIPTION	USCS	AASHTO
Brown-Grey Silty CLAY		

<p>Project No. 28287 Client: APTIM</p> <p>Project: Zion Landfill Site 2 Expansion, Aptim #3211</p> <p>Location: B-2 Depth: 56.0- 58.0' Sample Number: ST-29</p> <p style="text-align: center;">Midland Standard Engineering & Testing</p> <p style="text-align: center;">South Elgin, IL</p>	<p>Remarks:</p> <p style="text-align: right;">Figure</p>
---	---

Dial Reading vs. Time

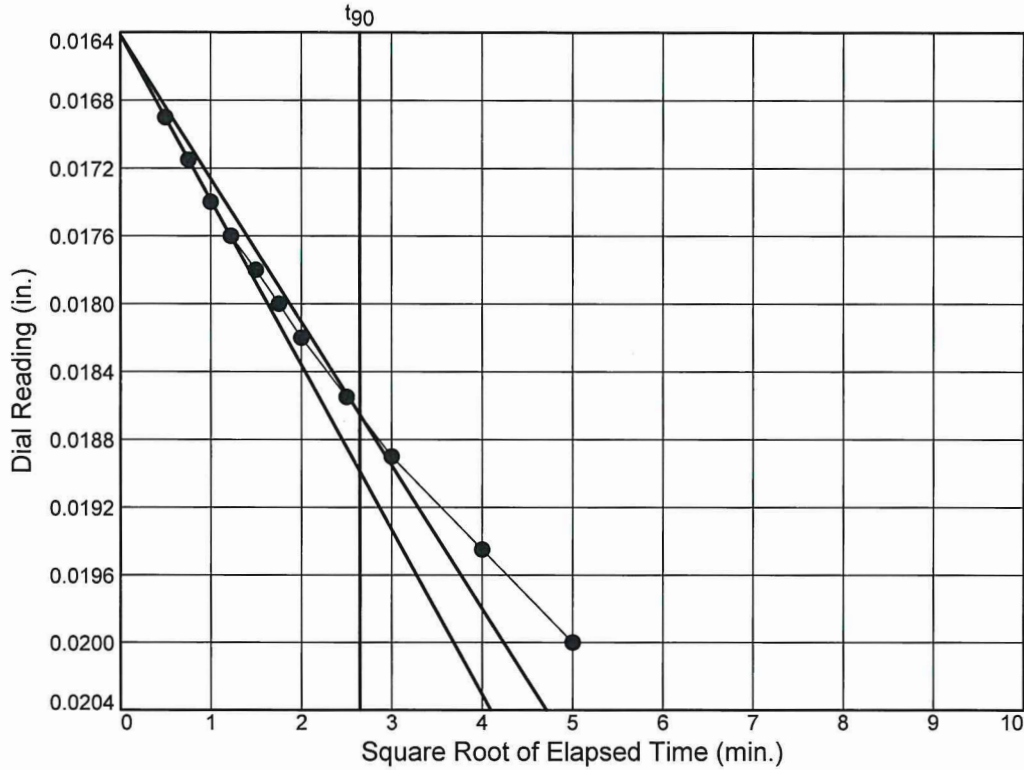
Project No.: 28287

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-2

Depth: 56.0- 58.0'

Sample Number: ST-29



Load No.= 1

Load=0.10 tsf

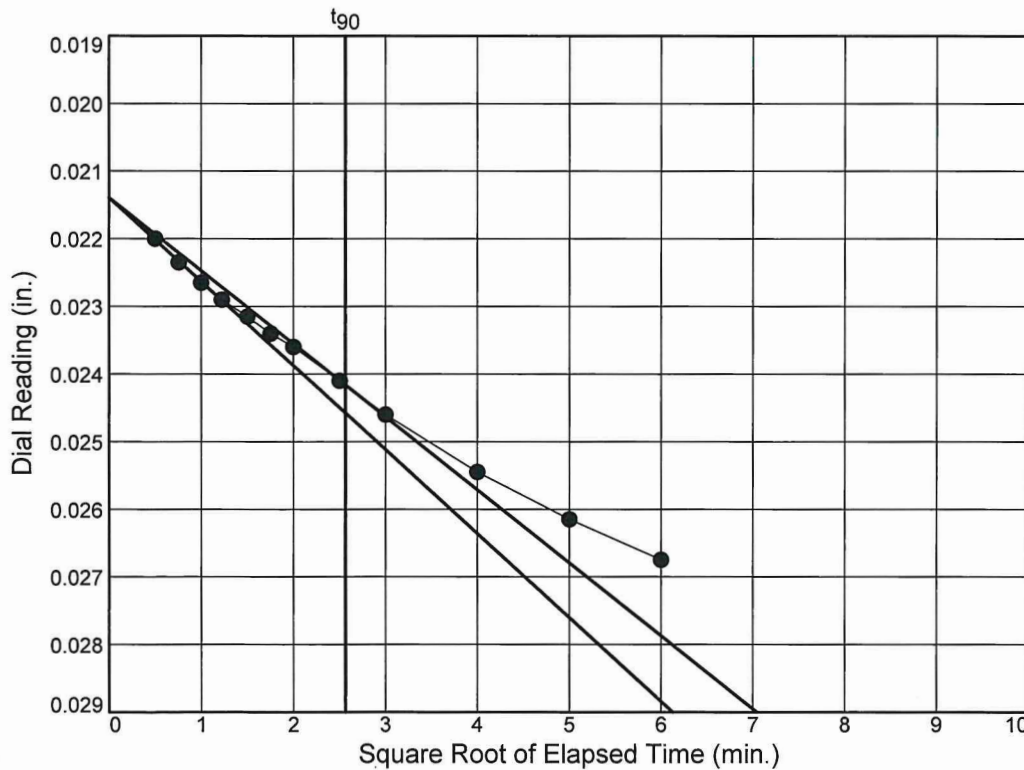
$D_0 = 0.0164$

$D_{90} = 0.0187$

$D_{100} = 0.0189$

$T_{90} = 6.99 \text{ min.}$

$C_v @ T_{90}$
0.302 ft.²/day



Load No.= 2

Load=0.50 tsf

$D_0 = 0.0214$

$D_{90} = 0.0242$

$D_{100} = 0.0245$

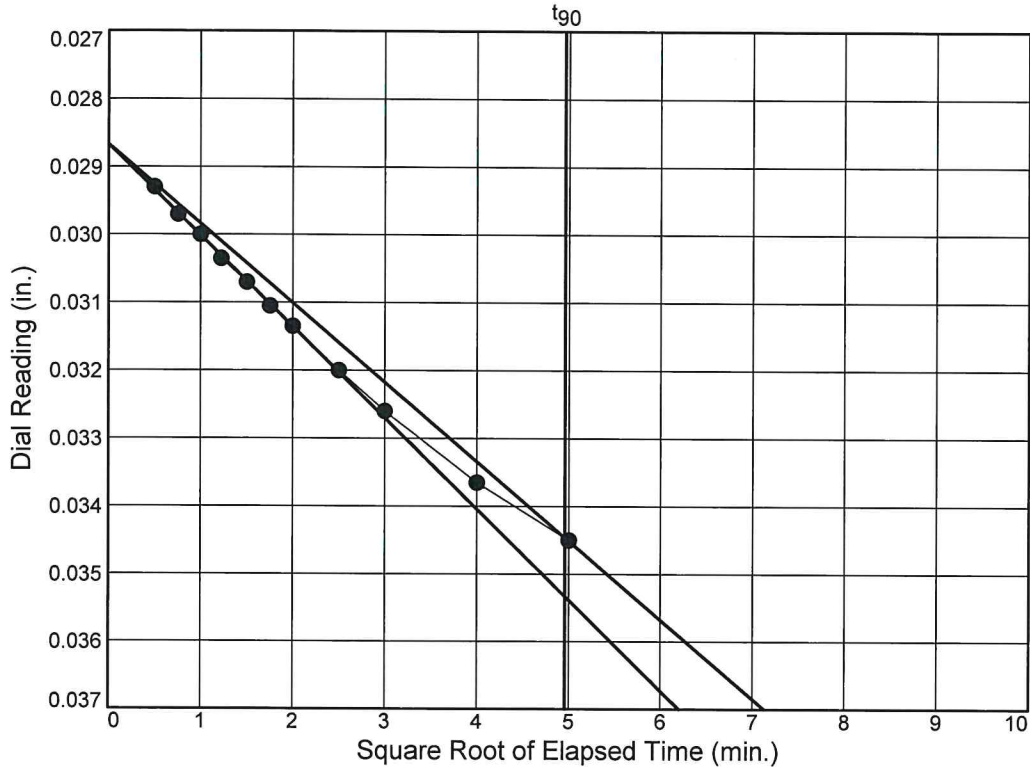
$T_{90} = 6.57 \text{ min.}$

$C_v @ T_{90}$
0.318 ft.²/day

Dial Reading vs. Time

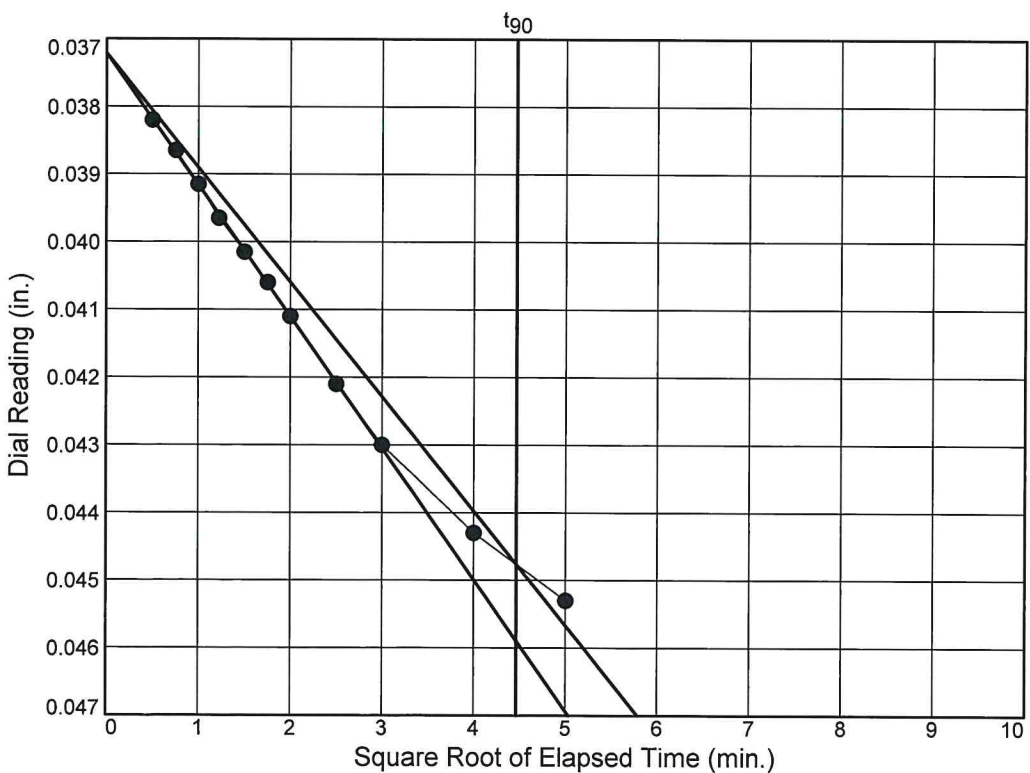
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-2 Depth: 56.0- 58.0' Sample Number: ST-29



Load No.= 3
 Load=1.00 tsf
 $D_0 = 0.0287$
 $D_{90} = 0.0345$
 $D_{100} = 0.0351$
 $T_{90} = 24.61$ min.

$C_v @ T_{90}$
 0.084 ft.²/day



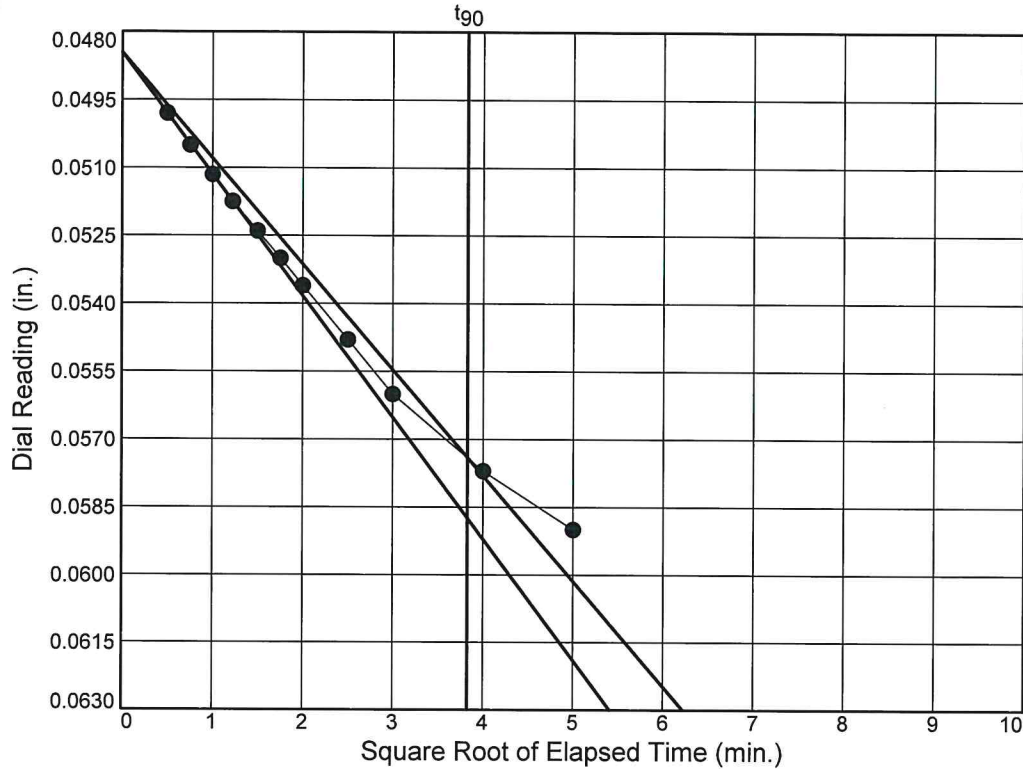
Load No.= 4
 Load=2.00 tsf
 $D_0 = 0.0372$
 $D_{90} = 0.0448$
 $D_{100} = 0.0456$
 $T_{90} = 19.95$ min.

$C_v @ T_{90}$
 0.101 ft.²/day

Dial Reading vs. Time

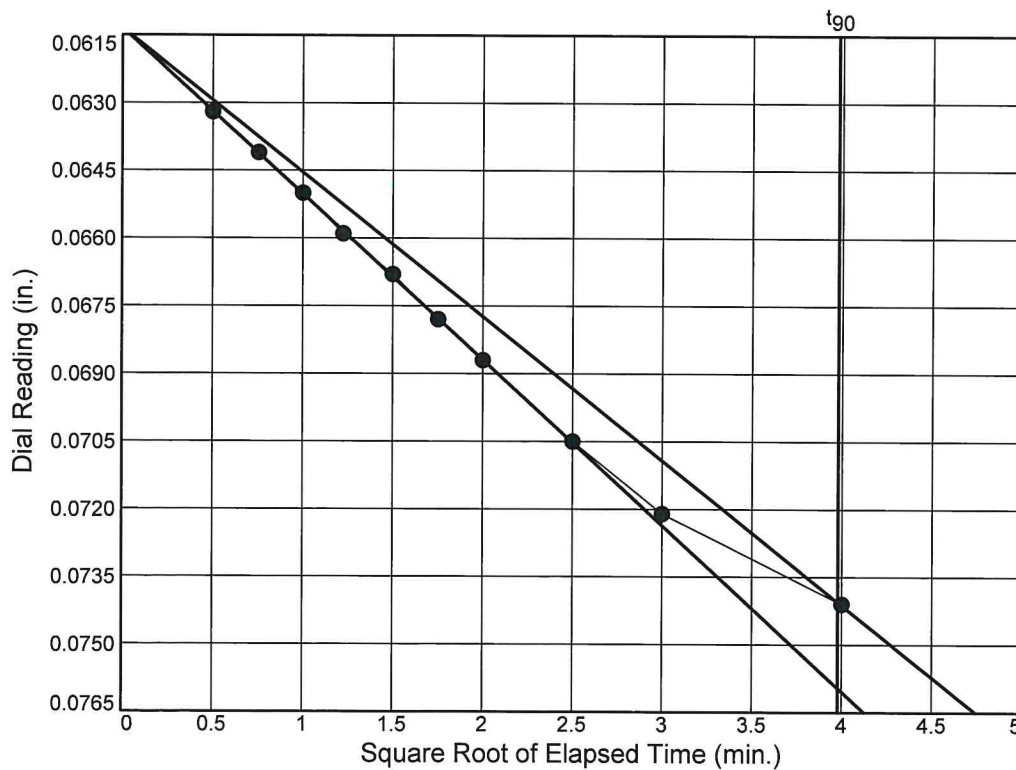
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-2 Depth: 56.0- 58.0' Sample Number: ST-29



Load No.= 5
 Load=4.00 tsf
 $D_0 = 0.0485$
 $D_{90} = 0.0574$
 $D_{100} = 0.0584$
 $T_{90} = 14.66 \text{ min.}$

$C_v @ T_{90}$
 0.134 ft.²/day



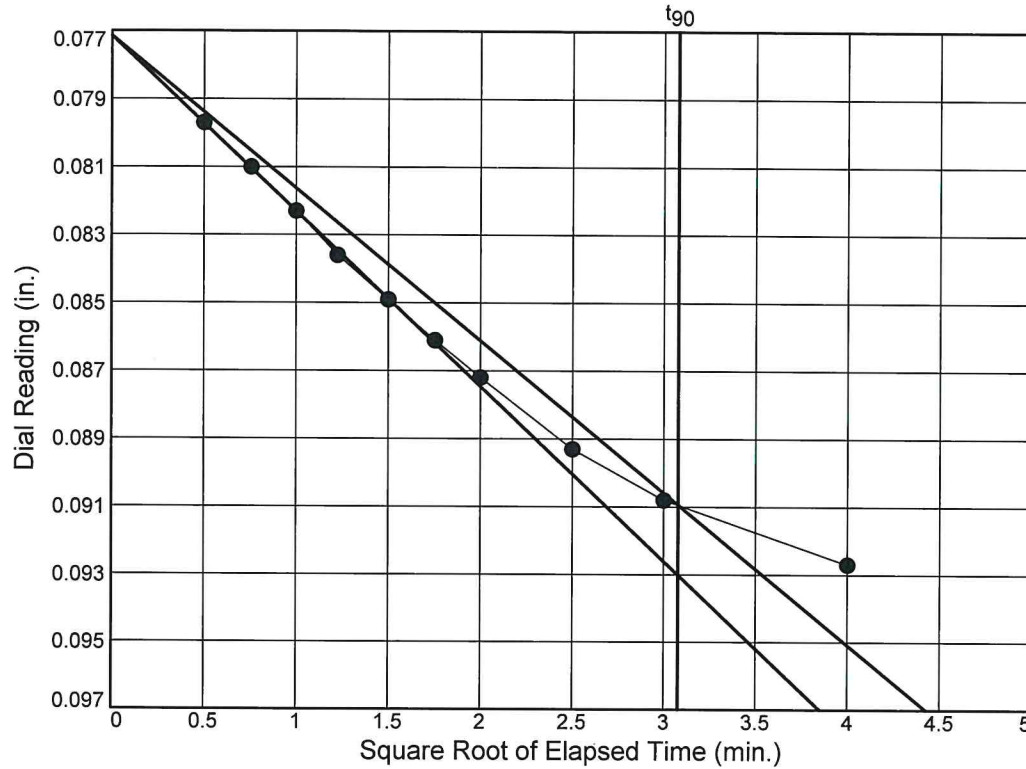
Load No.= 6
 Load=8.00 tsf
 $D_0 = 0.0613$
 $D_{90} = 0.0741$
 $D_{100} = 0.0755$
 $T_{90} = 15.84 \text{ min.}$

$C_v @ T_{90}$
 0.120 ft.²/day

Dial Reading vs. Time

Project No.: 28287
Project: Zion Landfill Site 2 Expansion, Aptim #3211

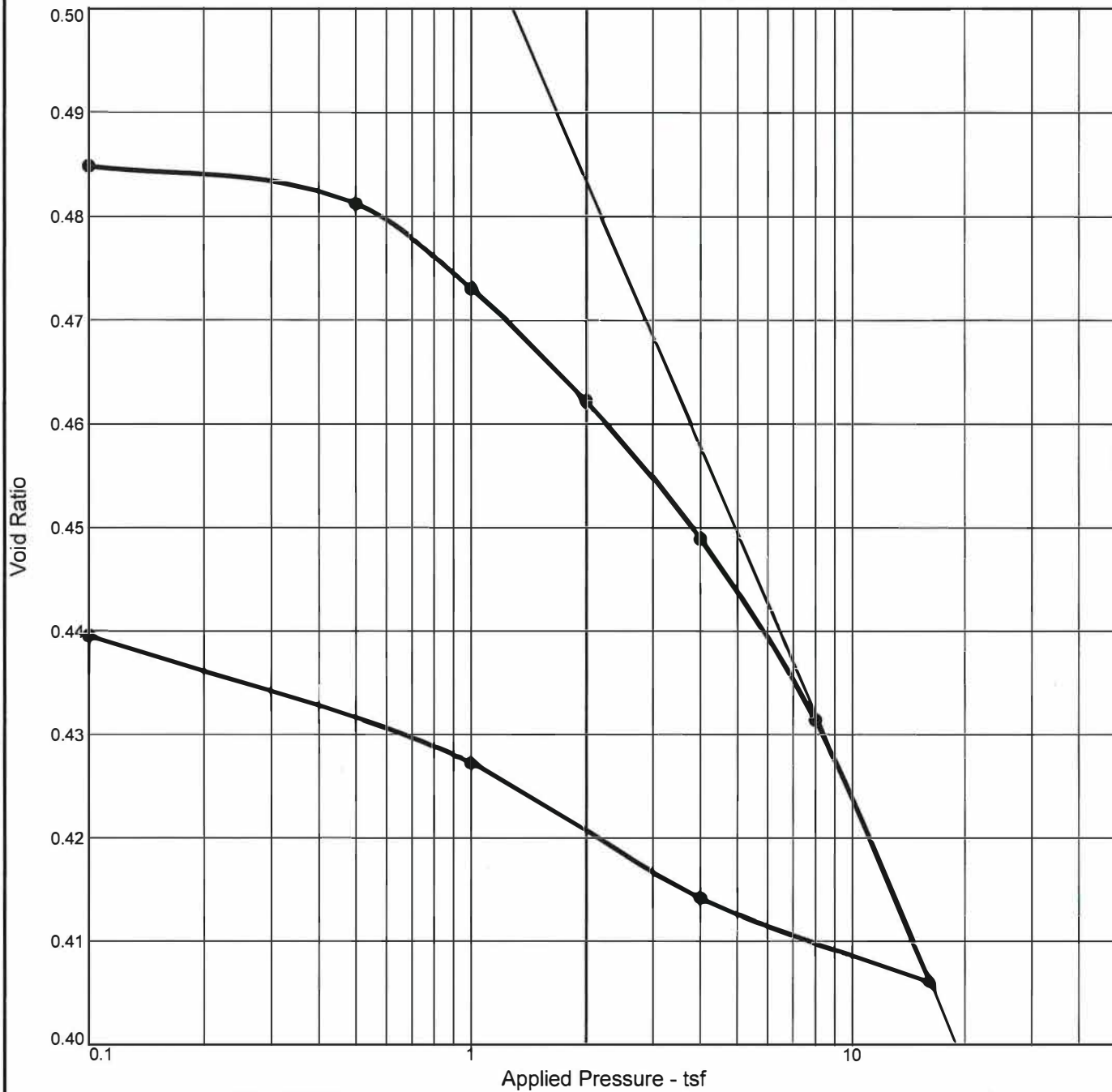
Location: B-2 Depth: 56.0- 58.0' Sample Number: ST-29



Load No.= 7
Load=16.00 tsf
 $D_0 = 0.0771$
 $D_{90} = 0.0909$
 $D_{100} = 0.0925$
 $T_{90} = 9.47 \text{ min.}$

$C_v @ T_{90}$
0.194 ft.²/day

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P _C (tsf)	C _C	C _r	Initial Void Ratio
Saturation	Moisture									
95.1 %	16.8 %	115.5			2.75		4.2	0.09		0.486

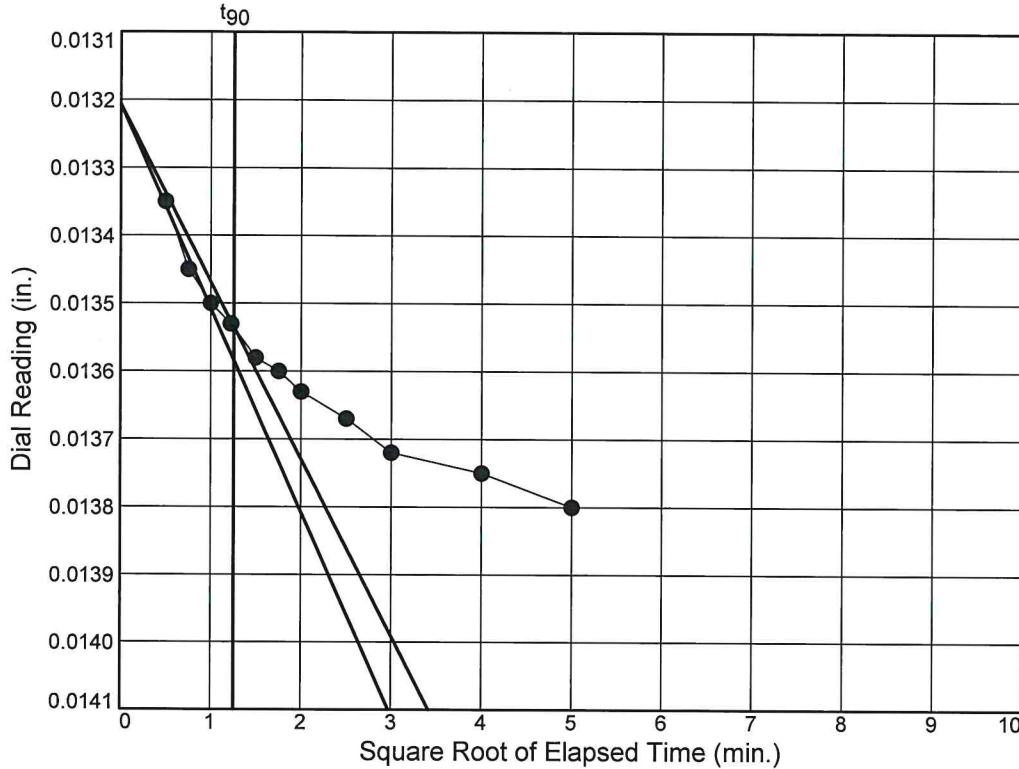
MATERIAL DESCRIPTION								USCS	AASHTO
Brown-grey CLAY									

<p>Project No. 28287 Client: APTIM</p> <p>Project: Zion Landfill Site 2 Expansion, Aptim #3211</p> <p>Location: B-4 Depth: 52.0- 54.0' Sample Number: ST-27</p> <p style="text-align: center;">Midland Standard Engineering & Testing</p> <p style="text-align: center;">South Elgin, IL</p>	<p>Remarks:</p> <p style="text-align: right;">Figure</p>
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Dial Reading vs. Time

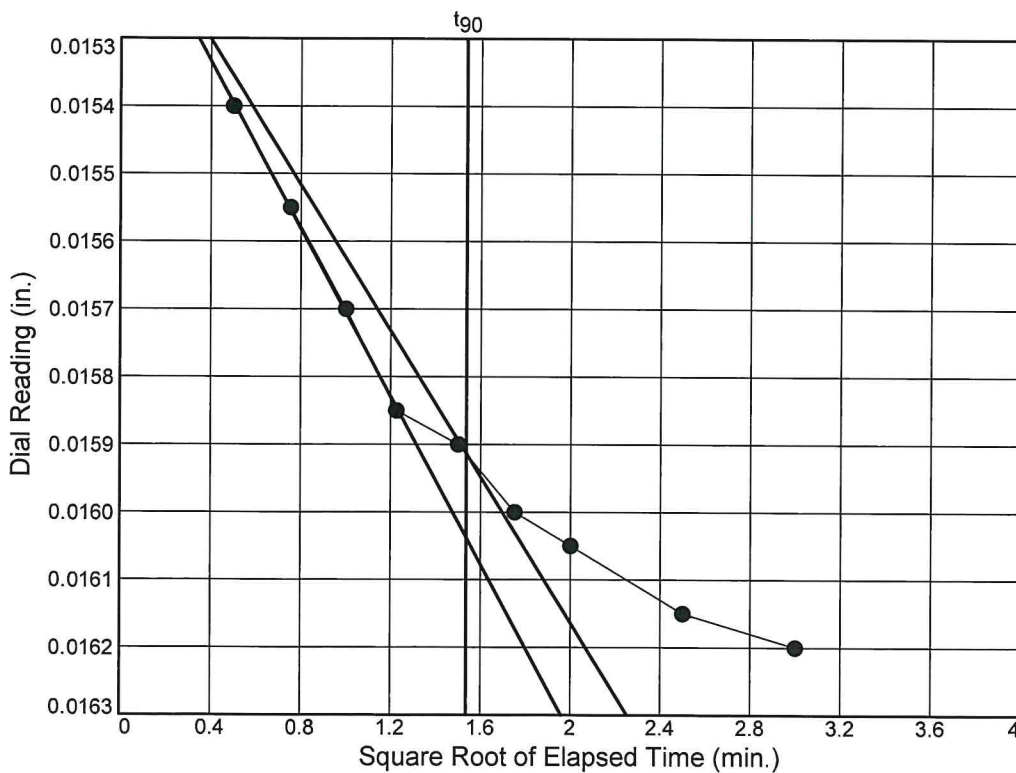
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-4 Depth: 52.0- 54.0' Sample Number: ST-27



Load No.= 1
 Load=0.10 tsf
 $D_0 = 0.0132$
 $D_{90} = 0.0135$
 $D_{100} = 0.0136$
 $T_{90} = 1.58 \text{ min.}$

$C_v @ T_{90}$
 1.348 ft.²/day



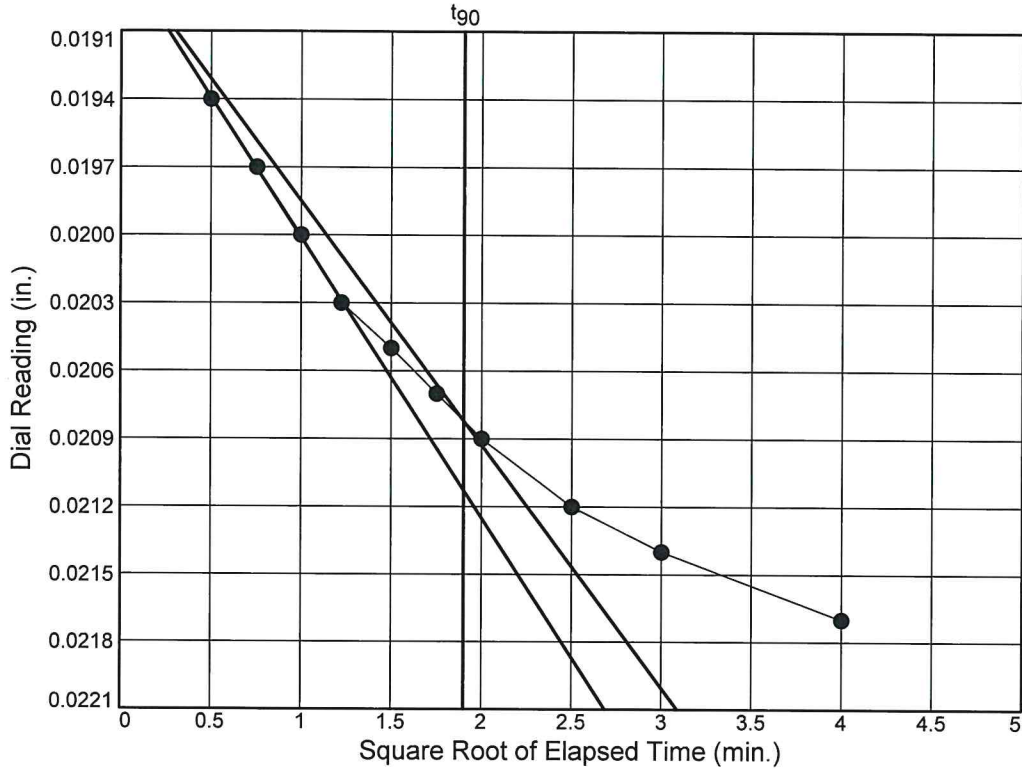
Load No.= 2
 Load=0.50 tsf
 $D_0 = 0.0151$
 $D_{90} = 0.0159$
 $D_{100} = 0.0160$
 $T_{90} = 2.36 \text{ min.}$

$C_v @ T_{90}$
 0.899 ft.²/day

Dial Reading vs. Time

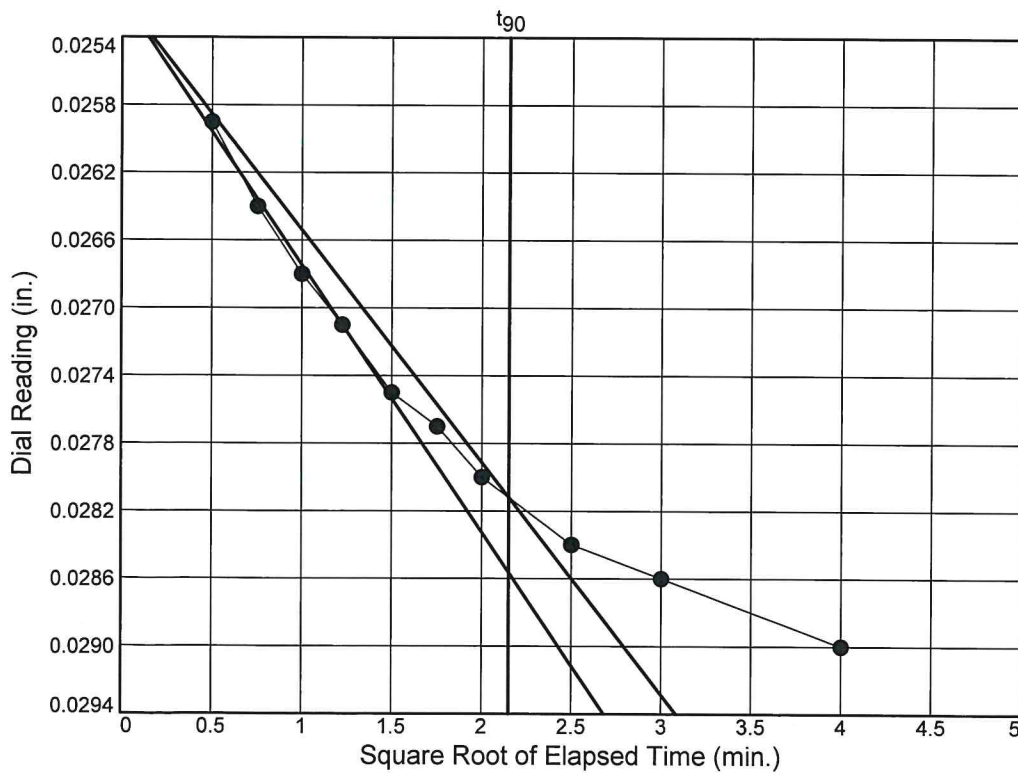
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-4 Depth: 52.0- 54.0' Sample Number: ST-27



Load No.= 3
 Load=1.00 tsf
 $D_0 = 0.0188$
 $D_{90} = 0.0208$
 $D_{100} = 0.0210$
 $T_{90} = 3.62 \text{ min.}$

$C_v @ T_{90}$
 0.582 ft.²/day



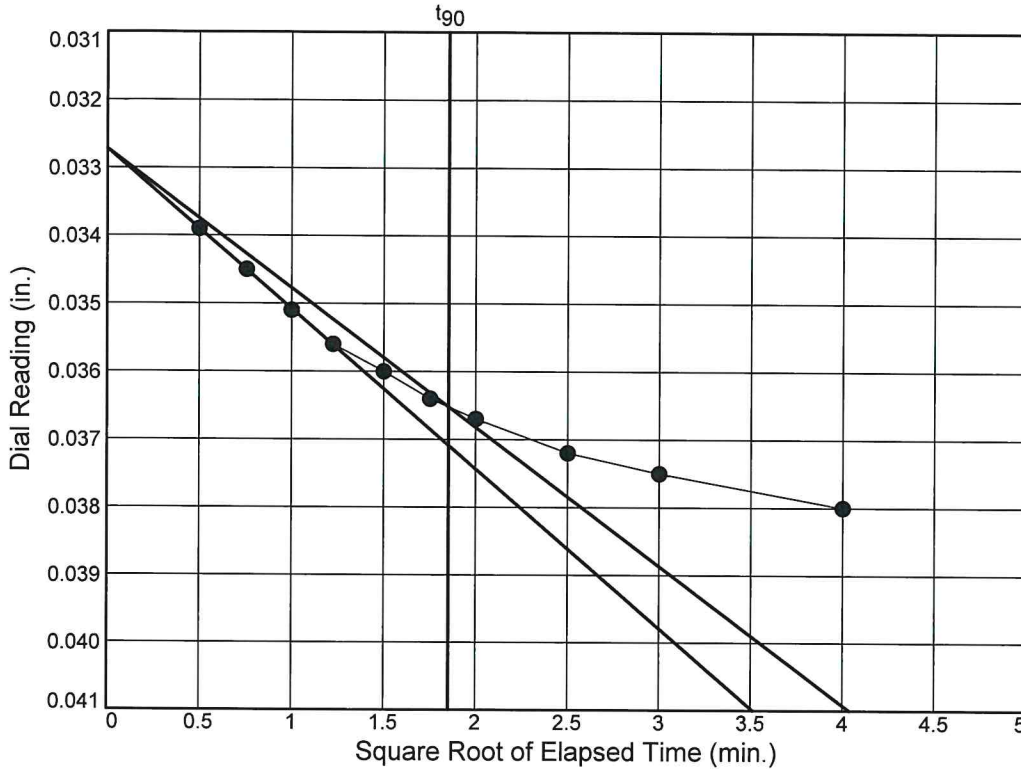
Load No.= 4
 Load=2.00 tsf
 $D_0 = 0.0252$
 $D_{90} = 0.0281$
 $D_{100} = 0.0284$
 $T_{90} = 4.63 \text{ min.}$

$C_v @ T_{90}$
 0.449 ft.²/day

Dial Reading vs. Time

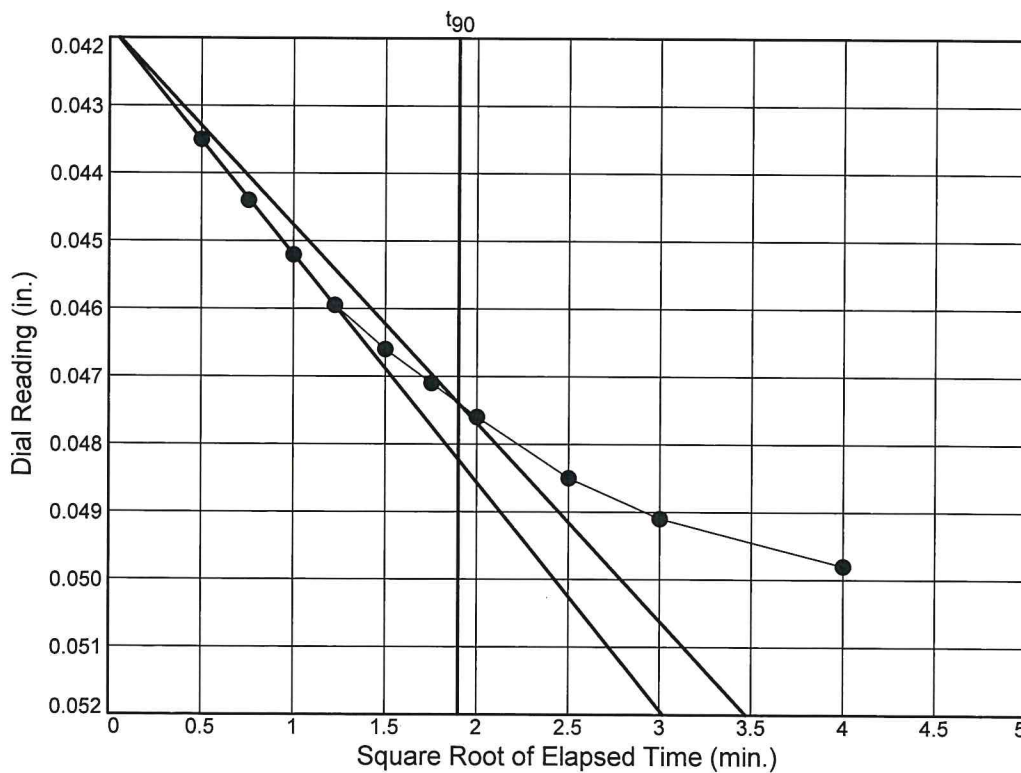
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-4 Depth: 52.0- 54.0' Sample Number: ST-27



Load No.= 5
 Load=4.00 tsf
 $D_0 = 0.0327$
 $D_{90} = 0.0365$
 $D_{100} = 0.0369$
 $T_{90} = 3.44 \text{ min.}$

$C_v @ T_{90}$
 0.594 ft.²/day



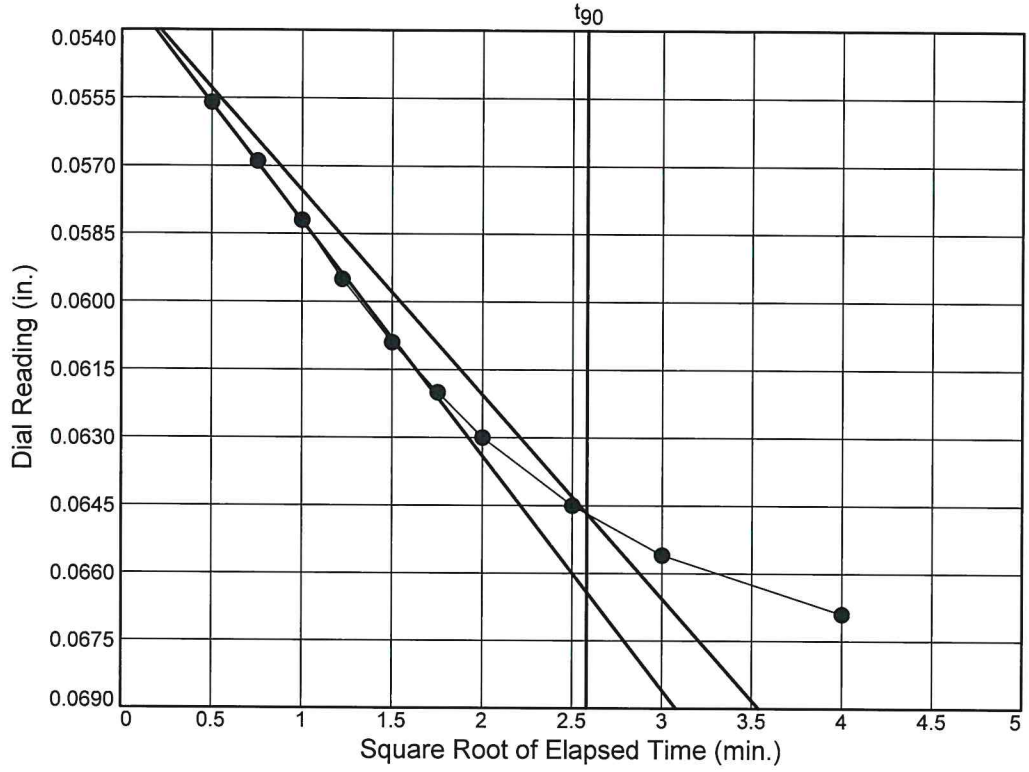
Load No.= 6
 Load=8.00 tsf
 $D_0 = 0.0418$
 $D_{90} = 0.0474$
 $D_{100} = 0.0480$
 $T_{90} = 3.61 \text{ min.}$

$C_v @ T_{90}$
 0.554 ft.²/day

Dial Reading vs. Time

Project No.: 28287
Project: Zion Landfill Site 2 Expansion, Aptim #3211

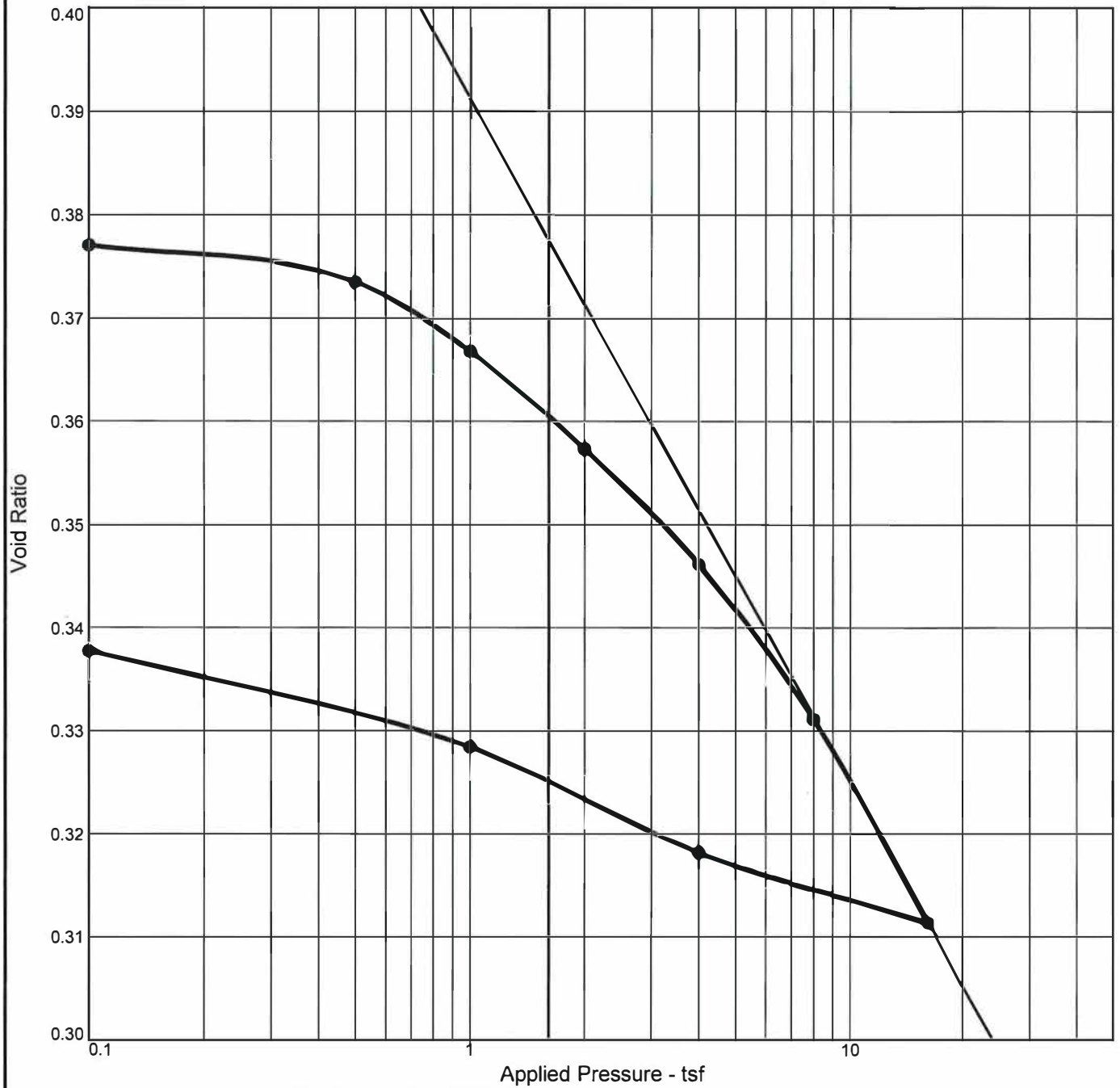
Location: B-4 Depth: 52.0- 54.0' Sample Number: ST-27



Load No.= 7
Load=16.00 tsf
 $D_0 = 0.0530$
 $D_{90} = 0.0647$
 $D_{100} = 0.0660$
 $T_{90} = 6.66 \text{ min.}$

$C_v @ T_{90}$
0.291 ft.²/day

CONSOLIDATION TEST REPORT



Natural		Dry Dens. (pcf)	LL	PI	Sp. Gr.	Overburden (tsf)	P _c (tsf)	C _c	C _r	Initial Void Ratio
Saturation	Moisture									
95.7 %	13.2 %	124.6			2.75		3.5	0.07		0.378

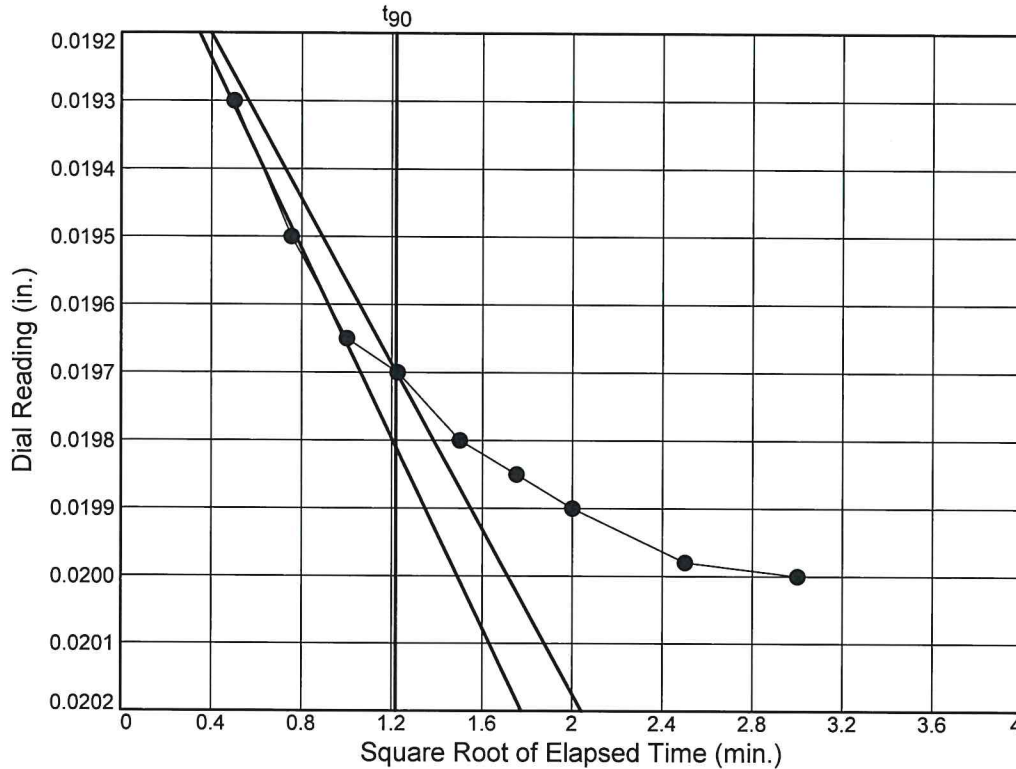
MATERIAL DESCRIPTION								USCS	AASHTO
Brown-grey Silty CLAY									

Project No. 28287 Client: APTIM Project: Zion Landfill Site 2 Expansion, Aptim #3211 Location: B-9 Depth: 60.0- 62.0' Sample Number: ST-31 Midland Standard Engineering & Testing South Elgin, IL	Remarks: <div style="text-align: right;">Figure</div>
---	--

Dial Reading vs. Time

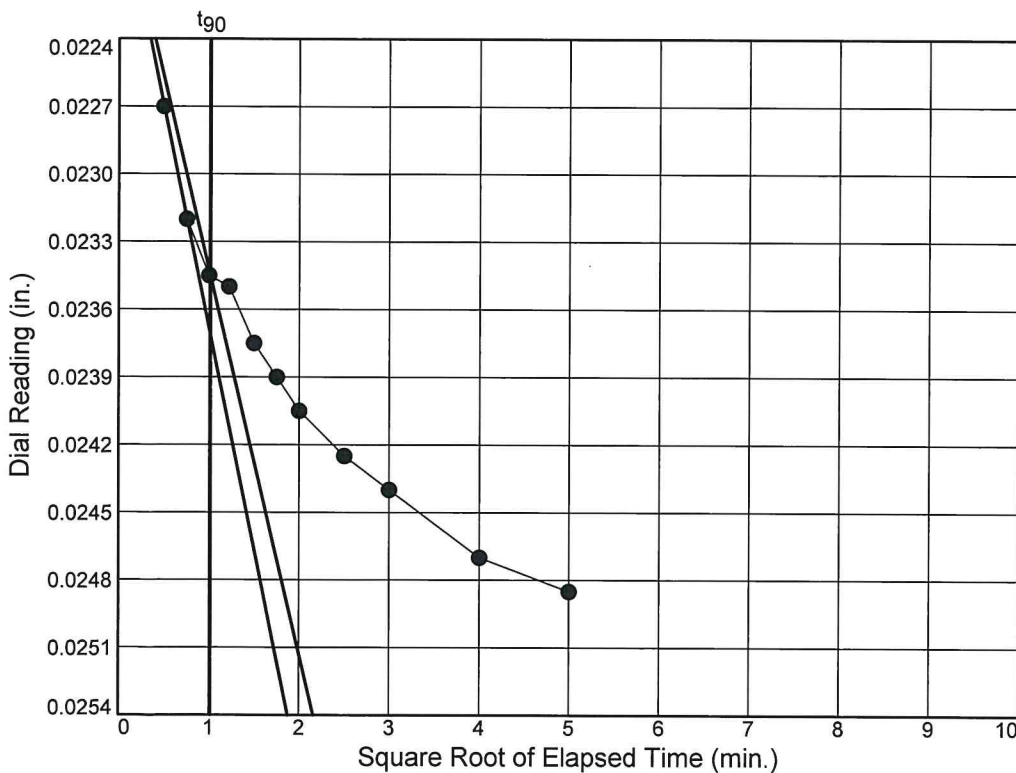
Project No.: 28287
 Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-9 Depth: 60.0- 62.0' Sample Number: ST-31



Load No.= 2
 Load=0.50 tsf
 $D_0 = 0.0190$
 $D_{90} = 0.0197$
 $D_{100} = 0.0198$
 $T_{90} = 1.48 \text{ min.}$

$C_v @ T_{90}$
 1.421 ft.²/day



Load No.= 3
 Load=1.00 tsf
 $D_0 = 0.0217$
 $D_{90} = 0.0235$
 $D_{100} = 0.0236$
 $T_{90} = 1.04 \text{ min.}$

$C_v @ T_{90}$
 2.016 ft.²/day

Dial Reading vs. Time

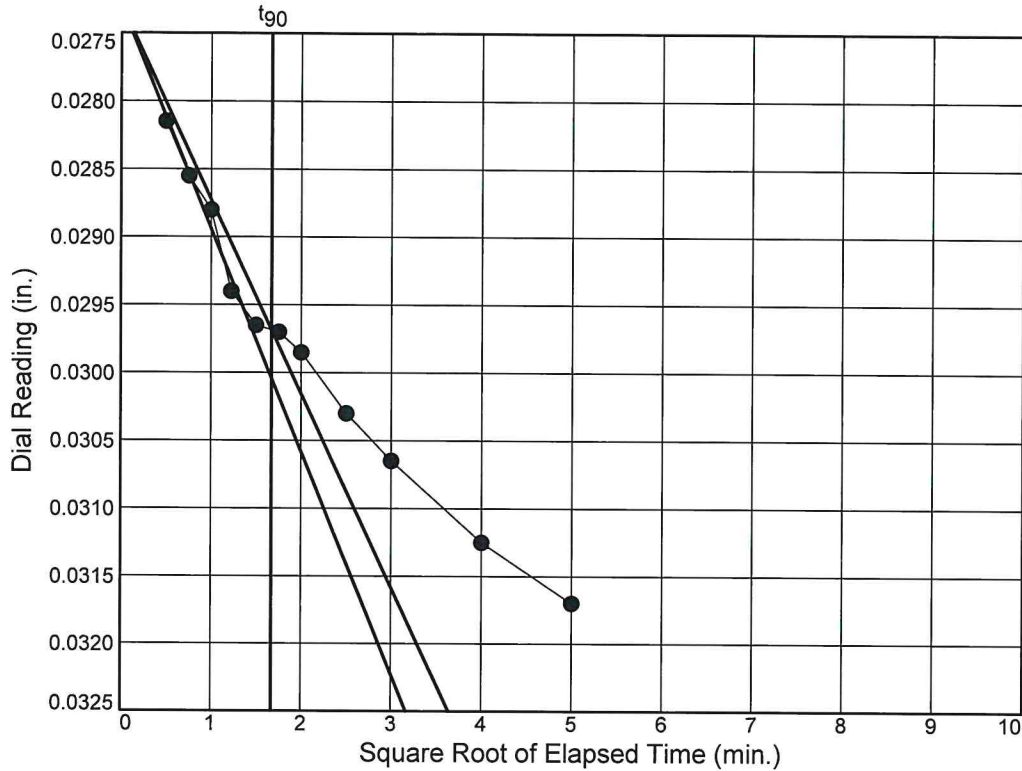
Project No.: 28287

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-9

Depth: 60.0- 62.0'

Sample Number: ST-31



Load No.= 4

Load=2.00 tsf

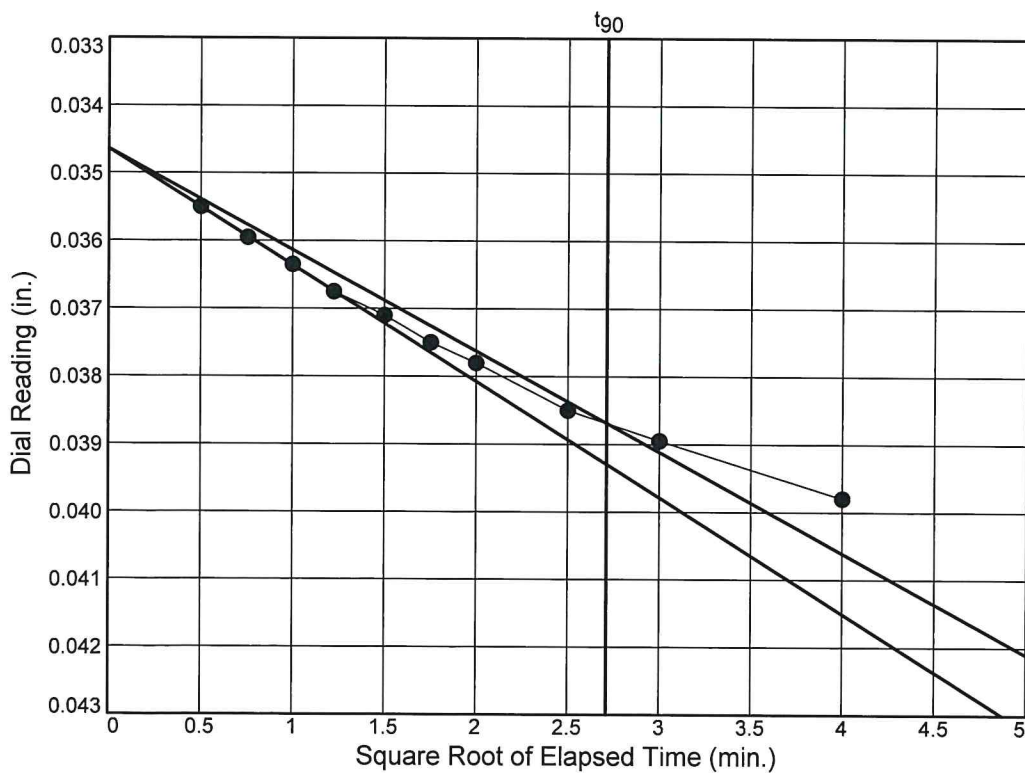
$D_0 = 0.0273$

$D_{90} = 0.0297$

$D_{100} = 0.0299$

$T_{90} = 2.79 \text{ min.}$

$C_v @ T_{90}$
0.740 ft.²/day



Load No.= 5

Load=4.00 tsf

$D_0 = 0.0346$

$D_{90} = 0.0387$

$D_{100} = 0.0391$

$T_{90} = 7.35 \text{ min.}$

$C_v @ T_{90}$
0.277 ft.²/day

Dial Reading vs. Time

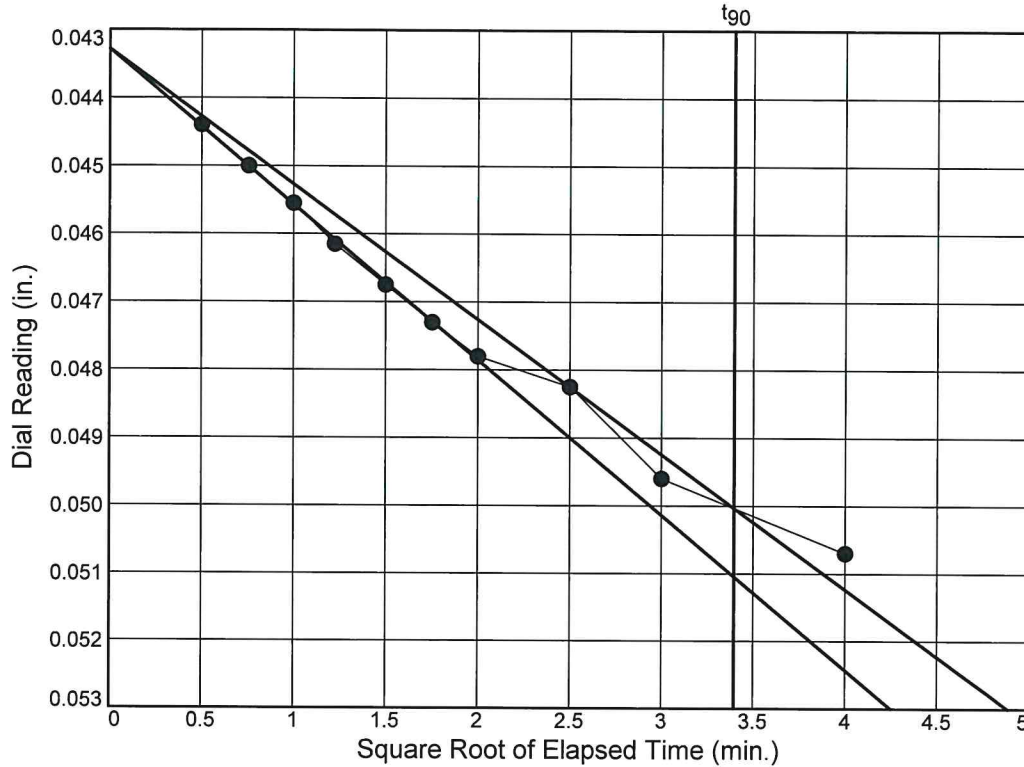
Project No.: 28287

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-9

Depth: 60.0- 62.0'

Sample Number: ST-31



Load No.= 6

Load=8.00 tsf

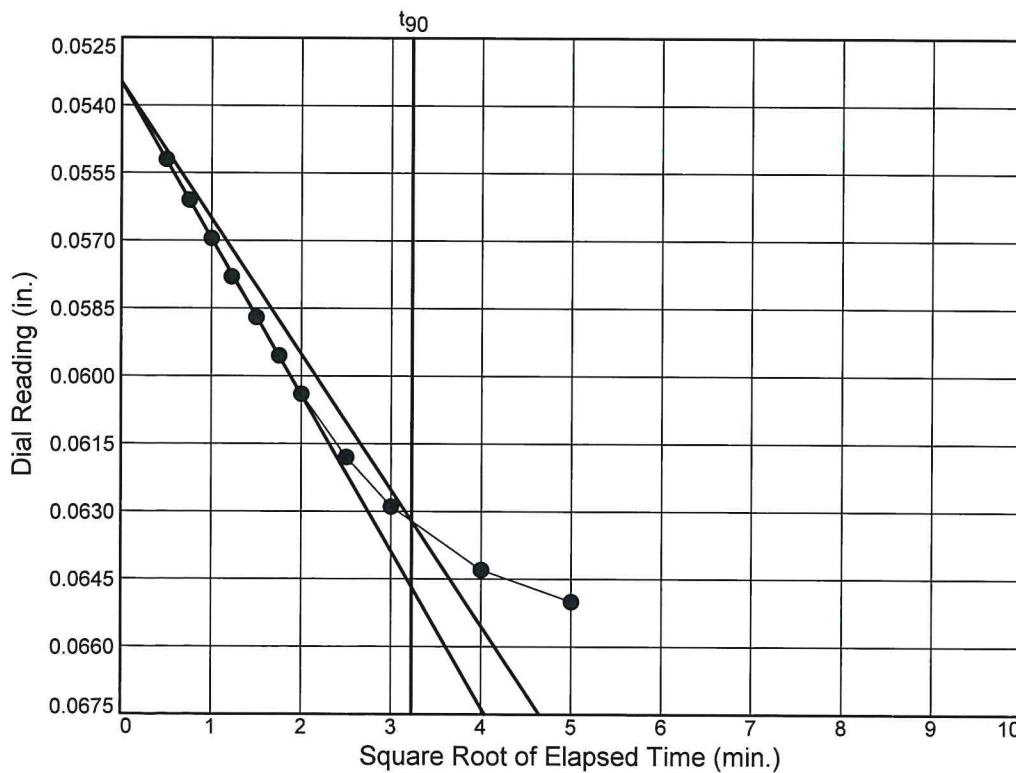
$D_0 = 0.0433$

$D_{90} = 0.0500$

$D_{100} = 0.0508$

$T_{90} = 11.53 \text{ min.}$

$C_v @ T_{90}$
0.173 ft.²/day



Load No.= 7

Load=16.00 tsf

$D_0 = 0.0535$

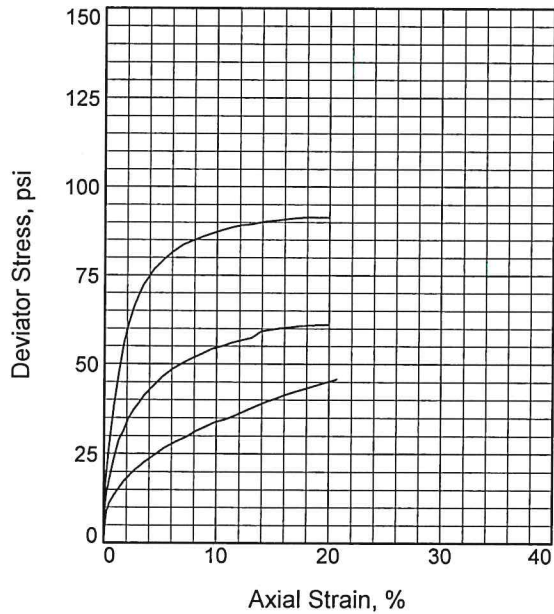
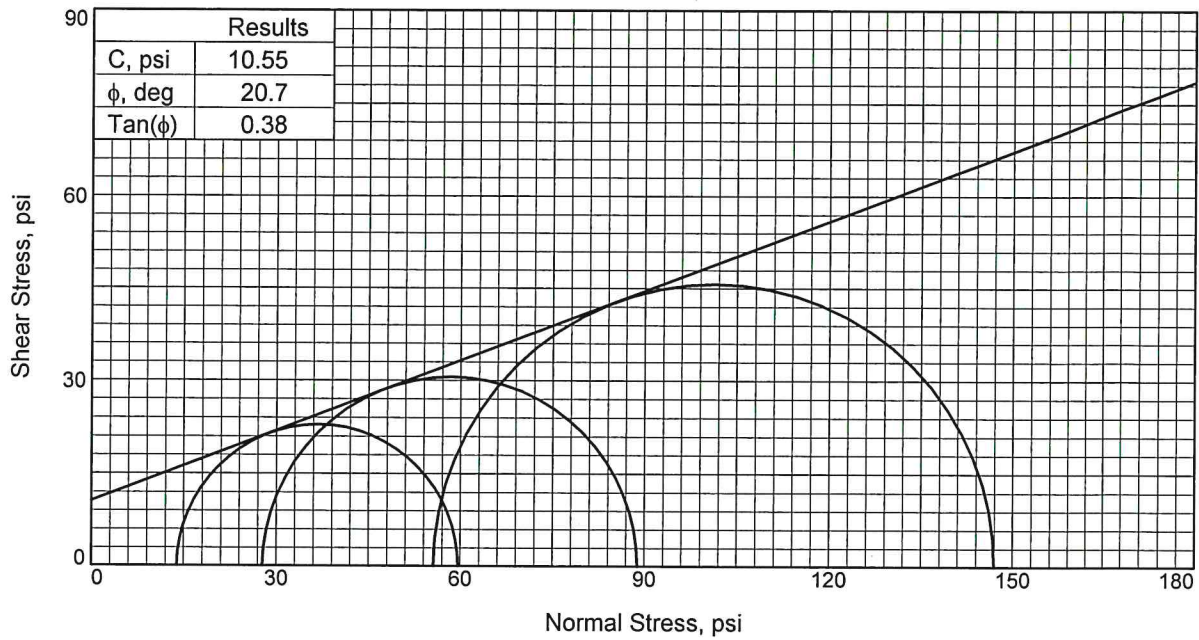
$D_{90} = 0.0632$

$D_{100} = 0.0643$

$T_{90} = 10.42 \text{ min.}$

$C_v @ T_{90}$
0.187 ft.²/day

Unconsolidated-Undrained Triaxial Test Results



Sample No.		1	2	3
Initial	Water Content, %	21.1	22.9	19.8
	Dry Density, pcf	108.6	106.1	110.2
	Saturation, %	99.6	101.8	97.6
	Void Ratio	0.5814	0.6181	0.5581
	Diameter, in.	2.87	2.86	2.86
	Height, in.	5.56	5.75	5.76
At Test	Water Content, %	21.1	22.5	20.3
	Dry Density, pcf	108.6	106.1	110.2
	Saturation, %	100.0	100.0	100.0
	Void Ratio	0.5814	0.6181	0.5581
	Diameter, in.	2.87	2.86	2.86
	Height, in.	5.56	5.75	5.76
Strain rate, in./min.		0.037	0.037	0.037
Back Pressure, psi		68	68	68
Cell Pressure, psi		82	96	123
Fail. Stress, psi		46	61	91
Ult. Stress, psi				
σ_1 Failure, psi		60	89	147
σ_3 Failure, psi		14	28	56

Type of Test:

Unconsolidated Undrained

Sample Type: Shelby Tube

Description: Brown-Grey CLAY

Specific Gravity= 2.75

Remarks:

Figure _____

Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-6

Sample Number: ST-38

Depth: 74.0- 76.0'

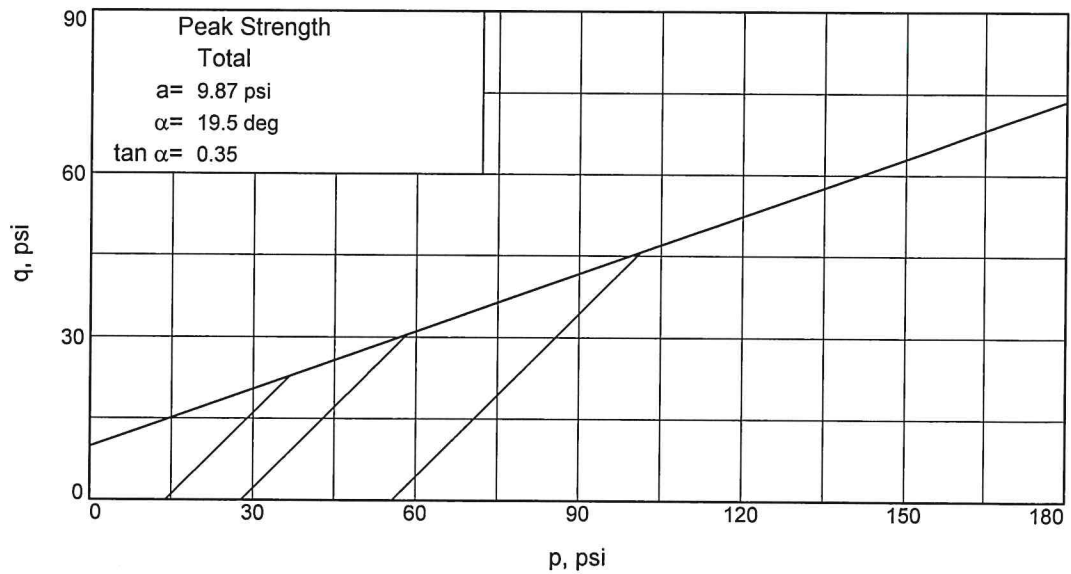
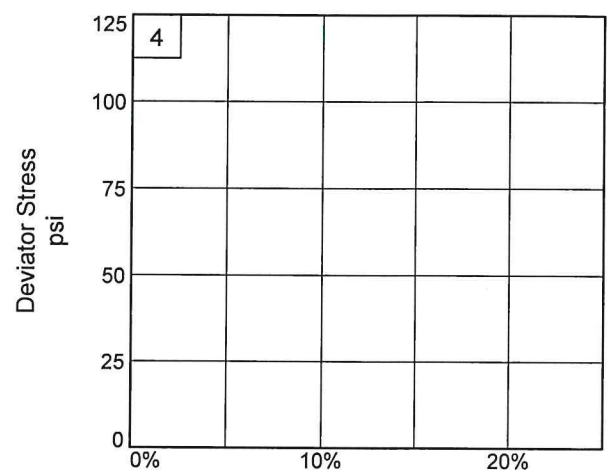
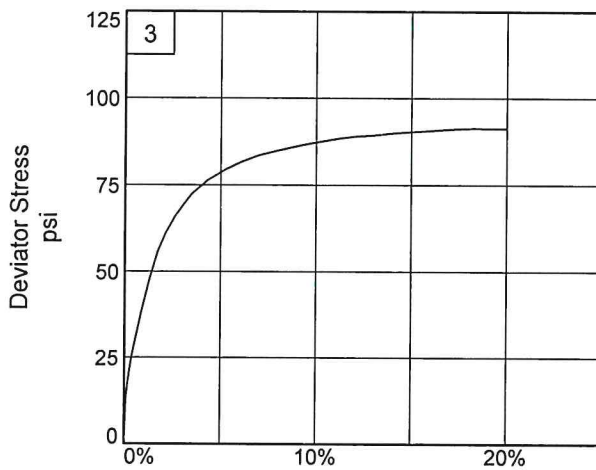
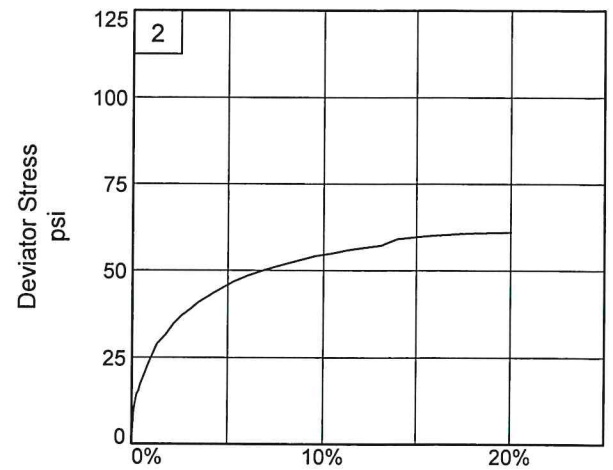
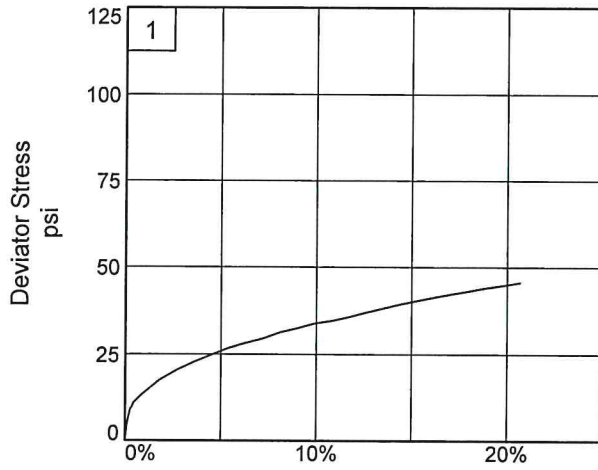
Proj. No.: 28287

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Midland Standard Engineering & Testing
 South Elgin, IL

Tested By: JDS

Checked By: WDP



Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-6

Depth: 74.0- 76.0'

Sample Number: ST-38

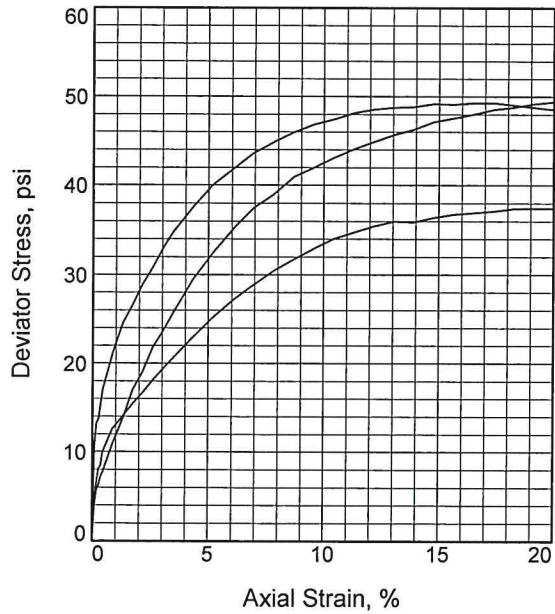
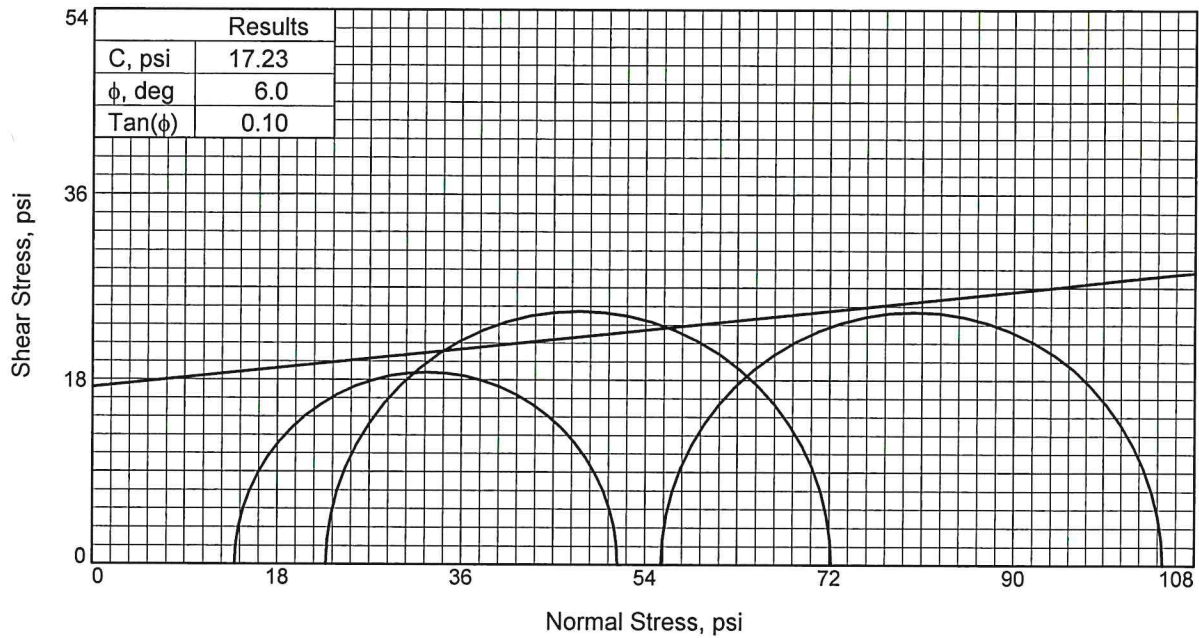
Project No.: 28287

Figure _____

Midland Standard Engineering & Testing

Tested By: JDS

Checked By: WDP



Sample No.		1	2	3
Initial	Water Content, %	15.7	17.3	16.7
	Dry Density, pcf	121.0	117.5	118.3
	Saturation, %	103.1	103.3	101.9
	Void Ratio	0.4188	0.4614	0.4510
	Diameter, in.	2.74	2.85	2.79
At Test	Height, in.	5.75	5.76	5.75
	Water Content, %	0.0	16.8	17.5
	Dry Density, pcf	0.0	117.5	118.3
	Saturation, %	0.0	100.1	106.9
	Void Ratio	N/A	0.4614	0.4510
Diameter, in.		2.74	2.85	2.79
Height, in.		5.75	5.76	5.75
Strain rate, in./min.		0.037	0.037	0.037
Back Pressure, psi		52	57	52
Cell Pressure, psi		65	79	107
Fail. Stress, psi		37	49	49
Ult. Stress, psi				
σ_1 Failure, psi		51	72	105
σ_3 Failure, psi		14	23	56

Type of Test:

Unconsolidated Undrained

Sample Type: Shelby Tube

Description: Brown-grey Silty CLAY

Specific Gravity= 2.75

Remarks:

Figure _____

Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-7

Sample Number: ST-29

Depth: 56.0- 58.0'

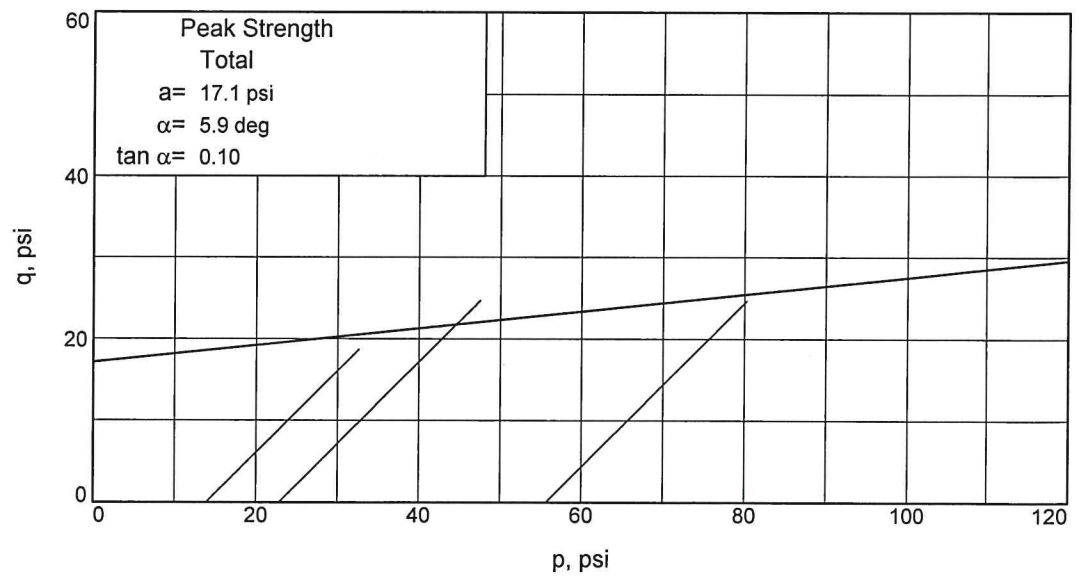
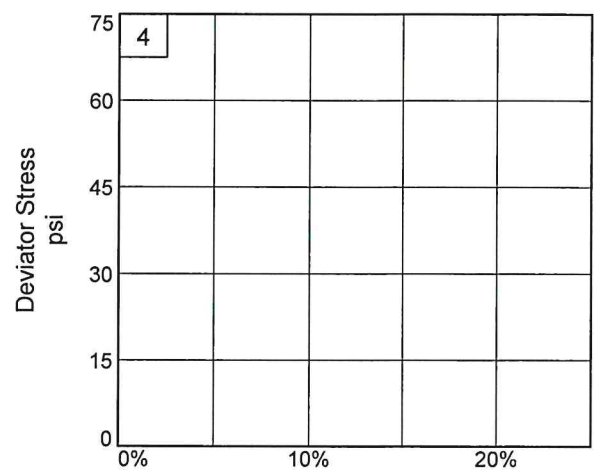
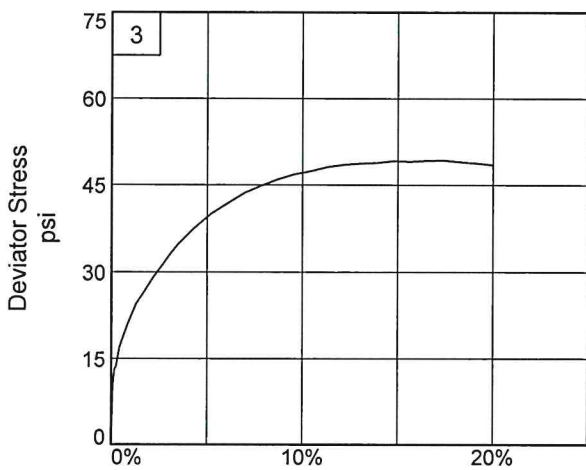
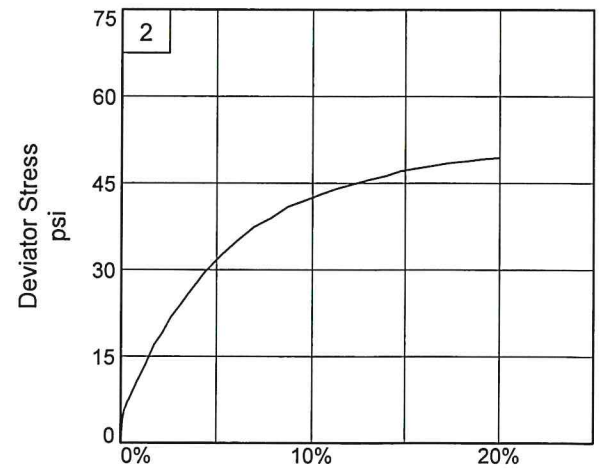
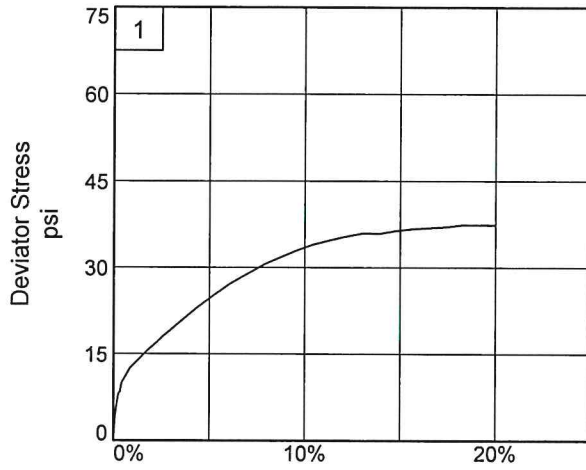
Proj. No.: 28287

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
Midland Standard Engineering & Testing
South Elgin, IL

Tested By: JDS

Checked By: WDP



Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-7 Depth: 56.0- 58.0' Sample Number: ST-29

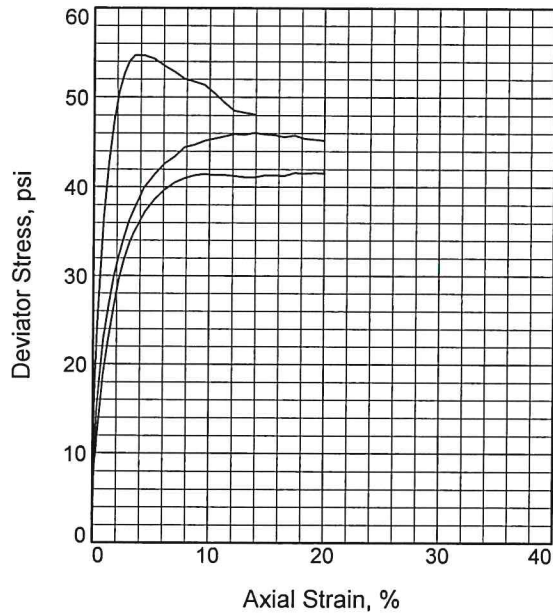
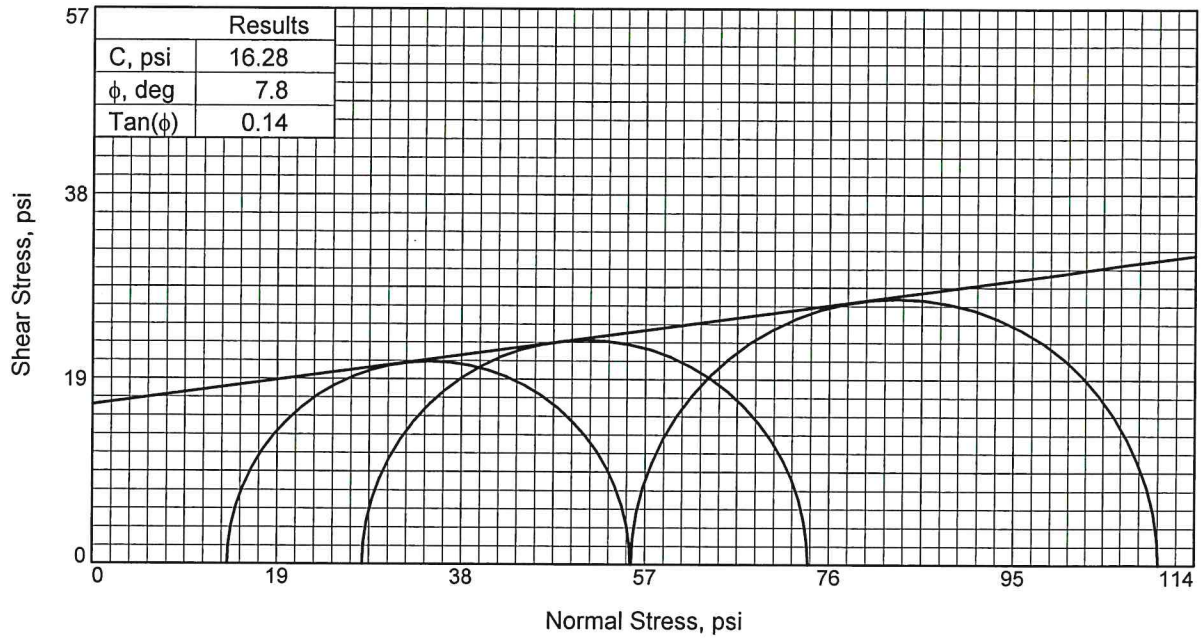
Project No.: 28287

Figure _____

Midland Standard Engineering & Testing

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Sample No.	1	2	3	
Initial	Water Content, %	20.6	27.5	24.5
	Dry Density, pcf	106.6	97.5	103.3
	Saturation, %	93.0	99.3	101.8
	Void Ratio	0.6101	0.7607	0.6612
	Diameter, in.	2.86	2.86	2.84
At Test	Height, in.	5.75	5.75	5.76
	Water Content, %	23.2	27.0	24.2
	Dry Density, pcf	106.6	97.5	103.3
	Saturation, %	104.4	97.5	100.5
	Void Ratio	0.6101	0.7607	0.6612
Strain rate, in./min.	Diameter, in.	2.86	2.86	2.84
	Height, in.	5.75	5.75	5.76
	Back Pressure, psi	0.037	0.037	0.037
Cell Pressure, psi	53	53	53	
Fail. Stress, psi	67	81	109	
Ult. Stress, psi	42	46	55	
σ_1 Failure, psi	56	74	110	
σ_3 Failure, psi	14	28	56	

Type of Test:

Unconsolidated Undrained

Sample Type: Shelby Tube

Description: Brown-grey CLAY

Specific Gravity= 2.750

Remarks:

Figure _____

Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-15

Sample Number: ST-30

Depth: 58.0- 60.0'

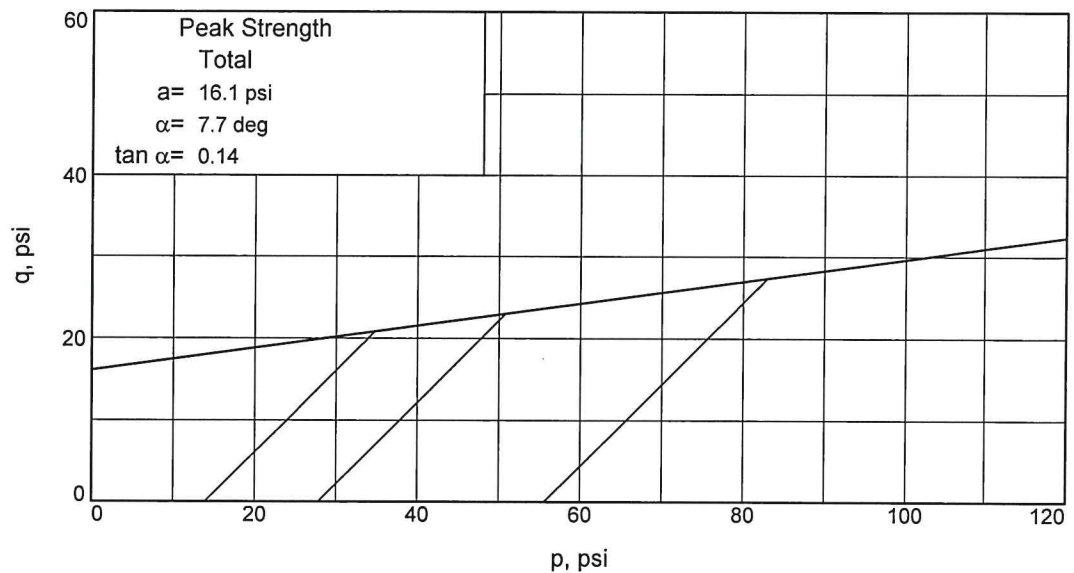
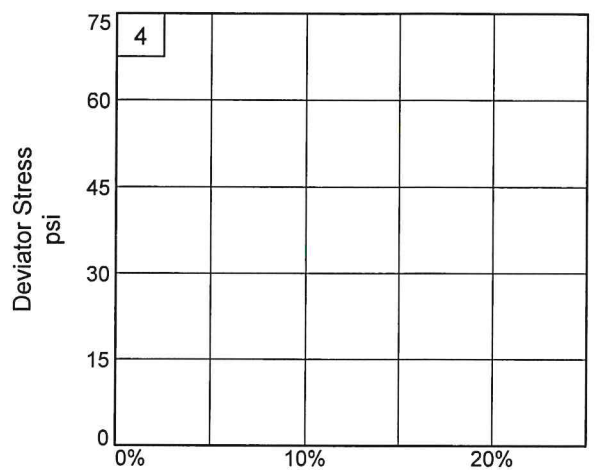
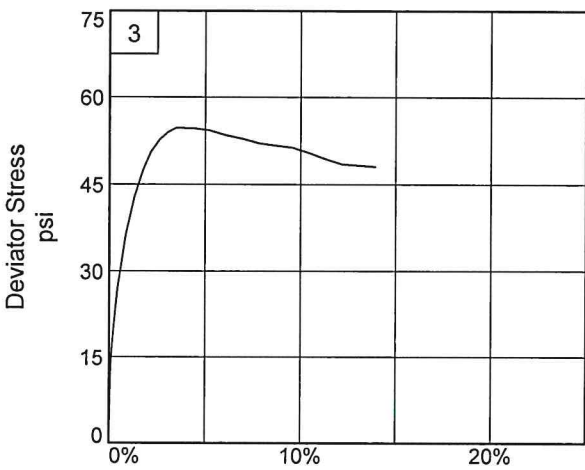
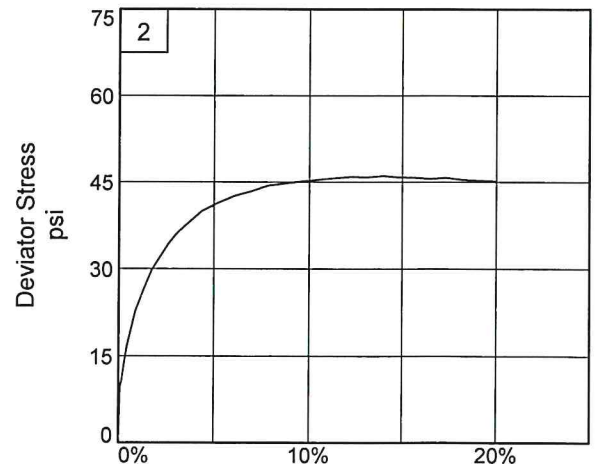
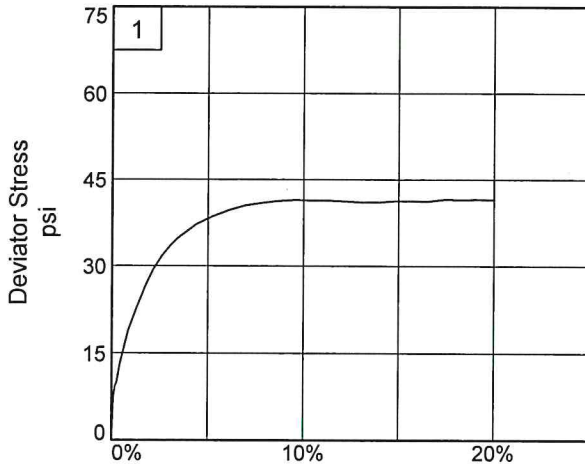
Proj. No.: 28287

Date Sampled:

TRIAXIAL SHEAR TEST REPORT
 Midland Standard Engineering & Testing
 South Elgin, IL

Tested By: JDS

Checked By: WP



Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-15

Depth: 58.0- 60.0'

Sample Number: ST-30

Project No.: 28287

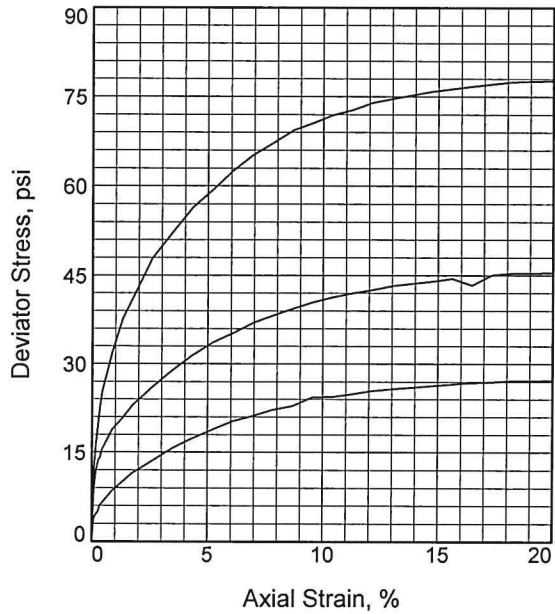
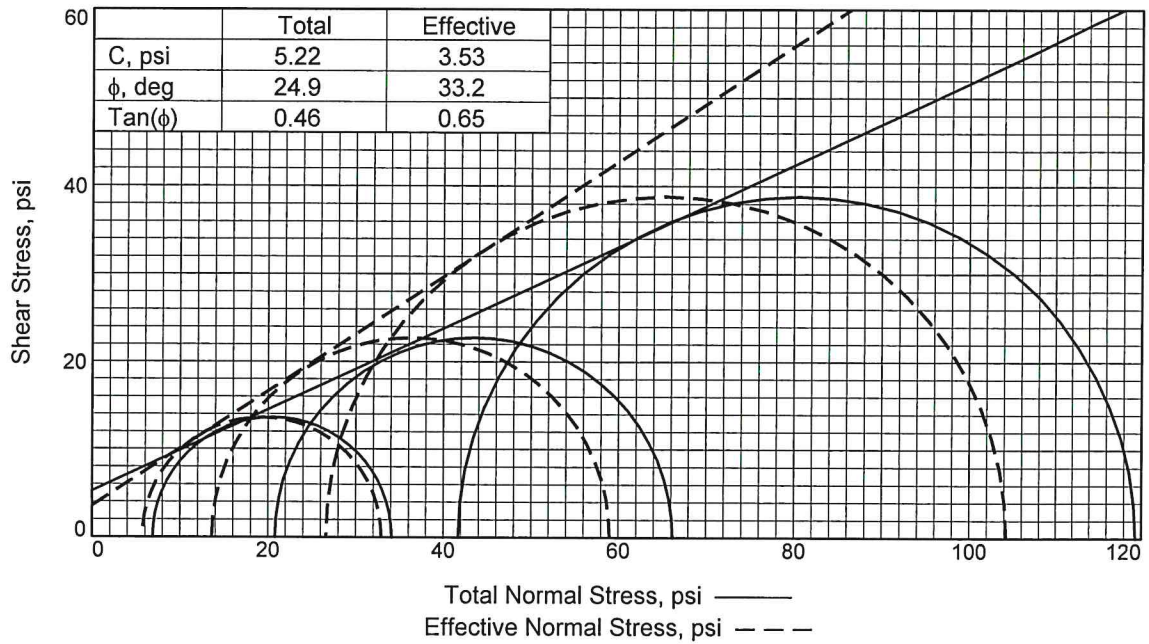
Figure _____

Midland Standard Engineering & Testing

Tested By: JDS

Checked By: WP

Consolidated-Undrained Triaxial Test Results



Sample No.	1	2	3	
Initial	Water Content, %	14.3	13.2	14.8
	Dry Density, pcf	121.6	124.2	120.3
	Saturation, %	95.3	94.8	95.2
	Void Ratio	0.4120	0.3818	0.4269
	Diameter, in.	2.84	2.85	2.82
At Test	Height, in.	5.76	5.77	5.76
	Water Content, %	15.0	13.9	15.5
	Dry Density, pcf	121.6	124.2	120.3
	Saturation, %	100.0	100.0	100.0
	Void Ratio	0.4120	0.3818	0.4269
Strain rate, in./min.	0.037	0.037	0.037	
	Back Pressure, psi	55	55	55
	Cell Pressure, psi	62	76	97
	Fail. Stress, psi	27	45	78
	Total Pore Pr., psi	56	62	70
	Ult. Stress, psi			
	Total Pore Pr., psi			
	$\bar{\sigma}_1$ Failure, psi	33	59	104
	$\bar{\sigma}_3$ Failure, psi	6	14	27

Type of Test:

CU with Pore Pressures

Sample Type: Shelby Tube

Description: Brown-grey CLAY

Specific Gravity= 2.75

Remarks:

Figure _____

Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-01

Sample Number: ST-31

Depth: 60.0- 62.0'

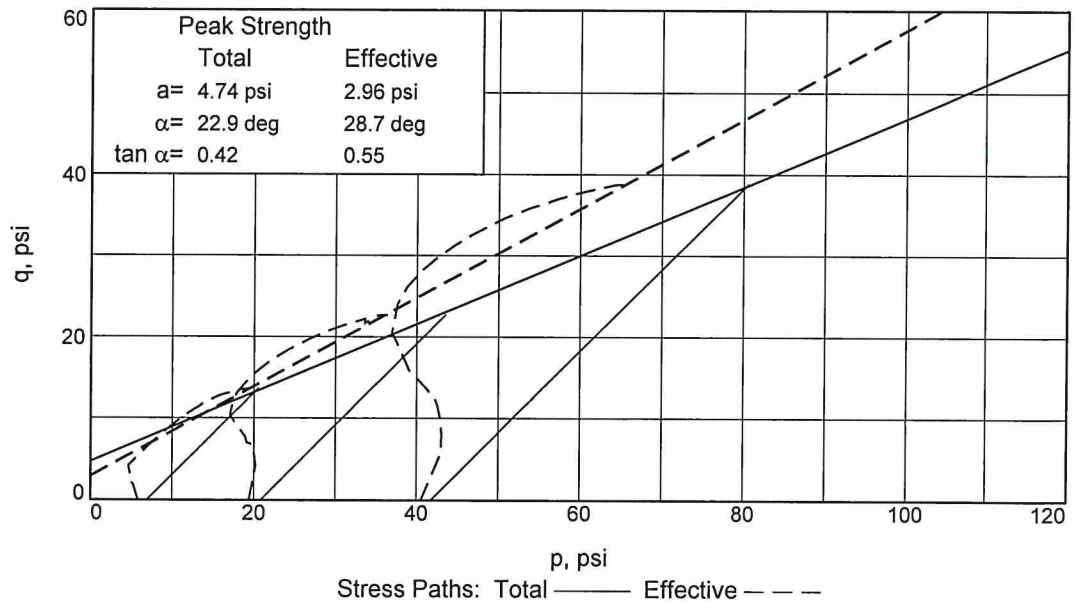
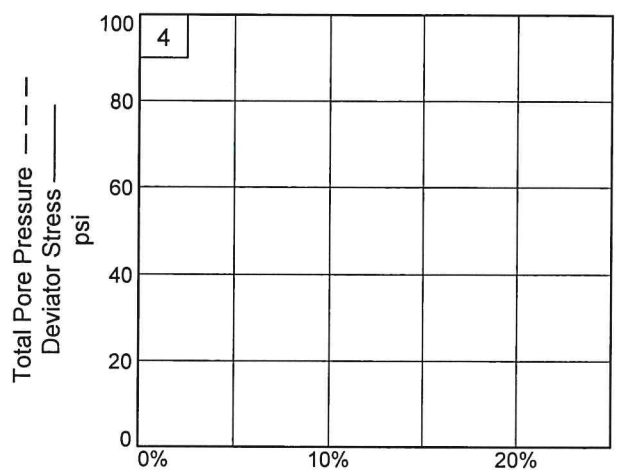
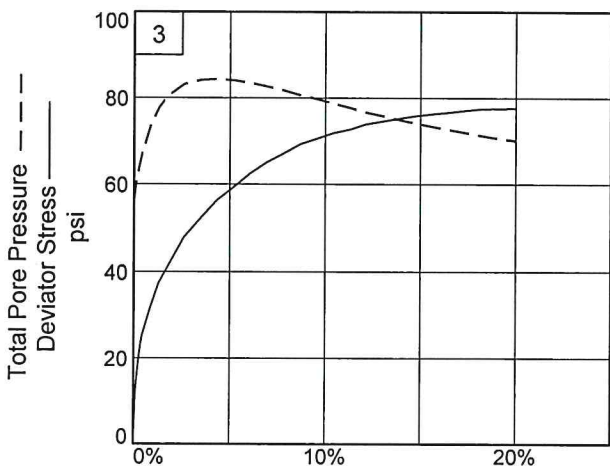
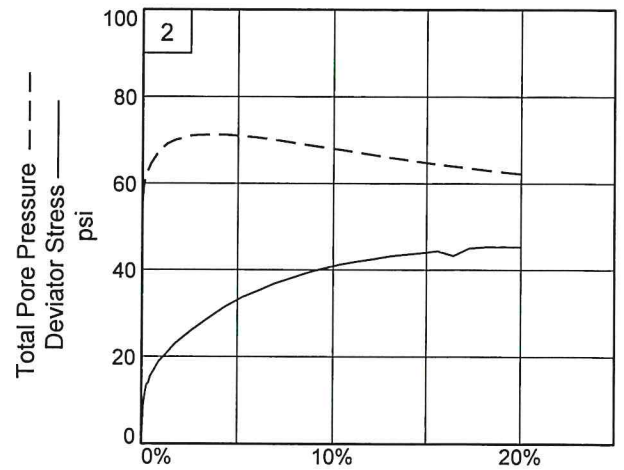
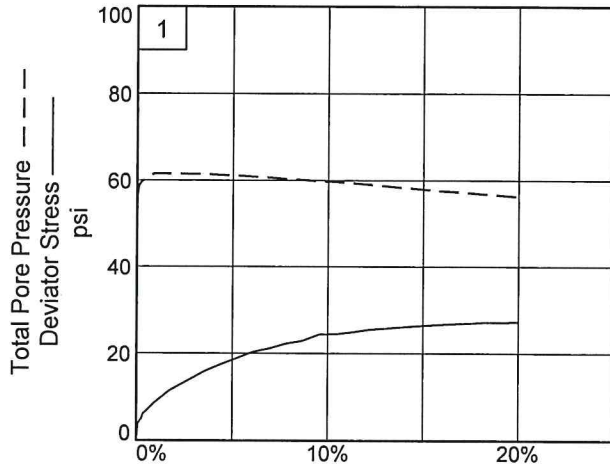
Proj. No.: 28287

Date Sampled: 4/1/19

TRIAXIAL SHEAR TEST REPORT
 Midland Standard Engineering & Testing
 South Elgin, IL

Tested By: JDS

Checked By: WDP



Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-01

Depth: 60.0- 62.0'

Sample Number: ST-31

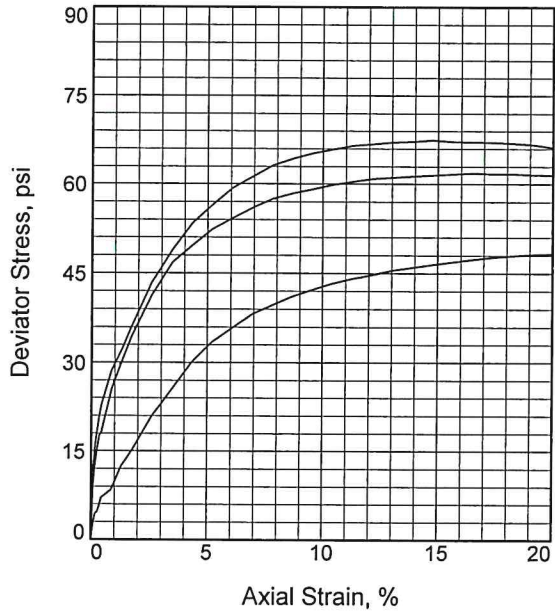
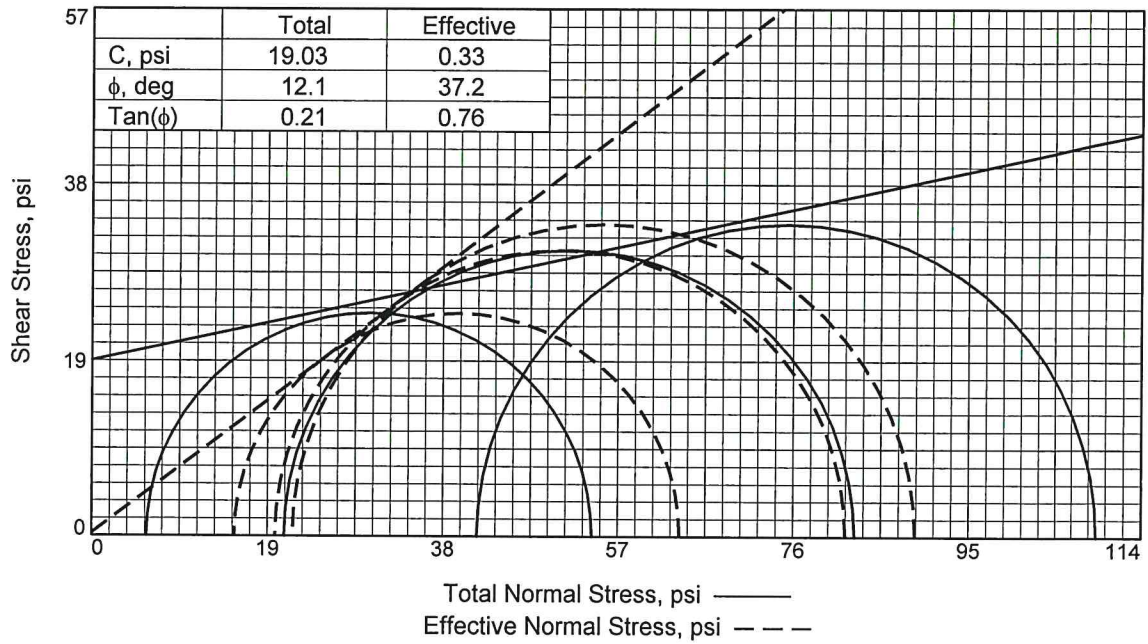
Project No.: 28287

Figure _____

Midland Standard Engineering & Testing

Tested By: JDS

Checked By: WDP



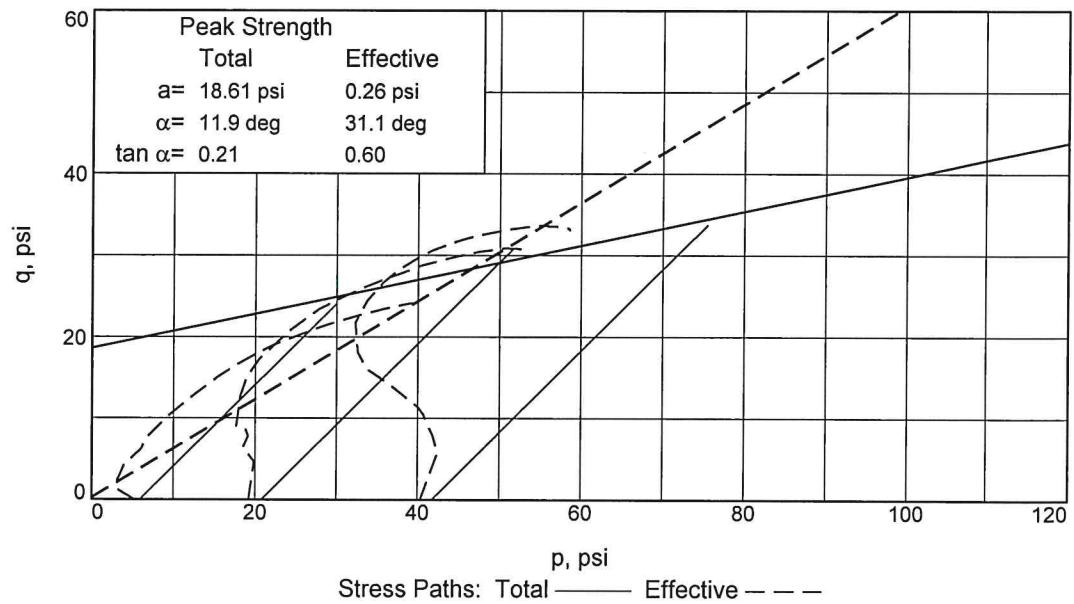
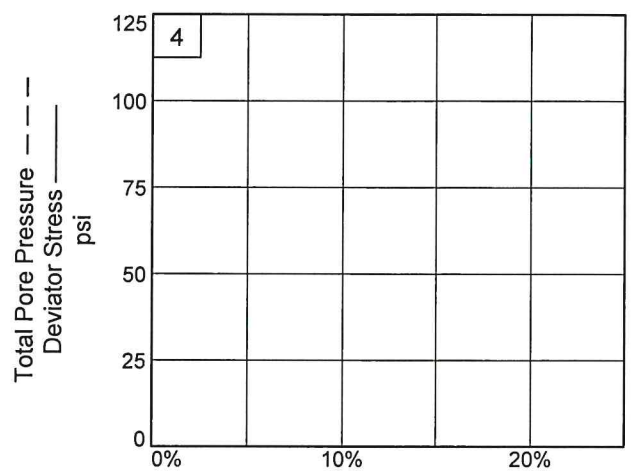
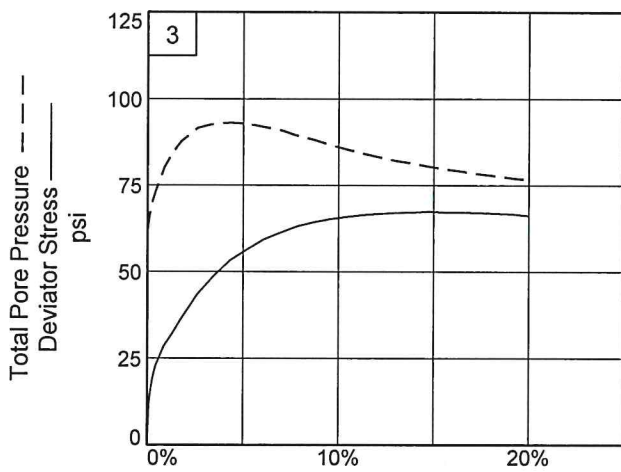
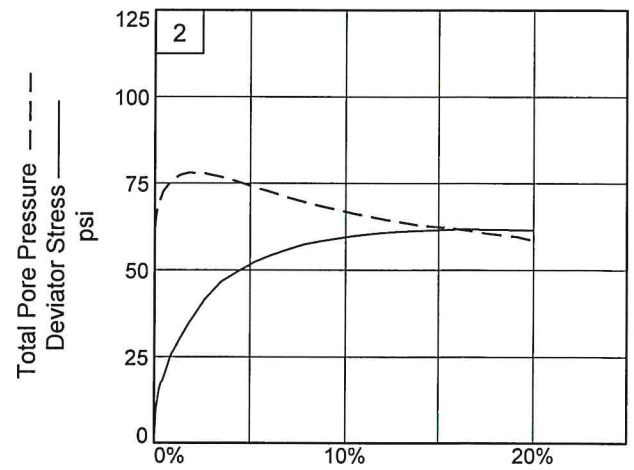
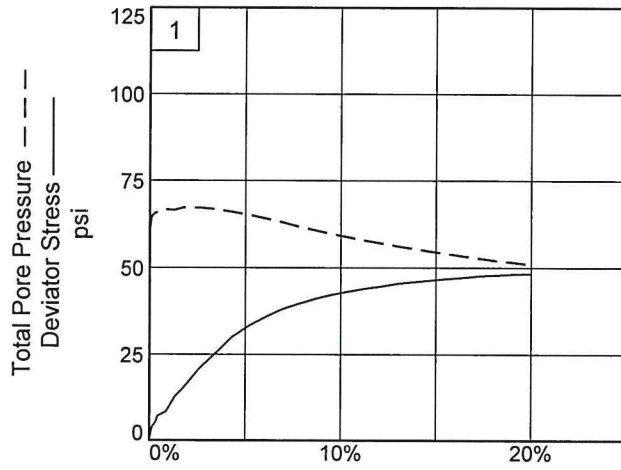
Sample No.		1	2	3
Initial	Water Content, %	15.9	15.3	16.2
	Dry Density, pcf	117.7	118.1	118.8
	Saturation, %	95.1	92.8	99.9
	Void Ratio	0.4589	0.4533	0.4448
	Diameter, in.	2.87	2.84	2.85
At Test	Height, in.	5.76	5.75	5.76
	Water Content, %	16.7	16.5	16.2
	Dry Density, pcf	117.7	118.1	118.8
	Saturation, %	100.0	100.0	100.0
	Void Ratio	0.4589	0.4533	0.4448
	Diameter, in.	2.87	2.84	2.85
	Height, in.	5.76	5.75	5.76
	Strain rate, in./min.	0.037	0.037	0.037
	Back Pressure, psi	60.5	60.5	60.5
	Cell Pressure, psi	66.4	81.3	102.2
	Fail. Stress, psi	48.3	61.8	67.5
	Total Pore Pr., psi	51.0	61.5	80.5
	Ult. Stress, psi			
	Total Pore Pr., psi			
	$\bar{\sigma}_1$ Failure, psi	63.7	81.6	89.2
$\bar{\sigma}_3$ Failure, psi	15.4	19.8	21.7	

Type of Test:
 CU with Pore Pressures
Sample Type: Shelby Tube
Description: Grey Silty CLAY

Specific Gravity= 2.750
Remarks:

Client: APTIM
Project: Zion Landfill Site 2 Expansion, Aptim #3211
Location: B-12
Sample Number: ST-34 **Depth:** 66.0- 68.0'
Proj. No.: 28287 **Date Sampled:**
 TRIAXIAL SHEAR TEST REPORT
 Midland Standard Engineering & Testing
 South Elgin, IL

Figure _____



Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-12

Depth: 66.0- 68.0'

Sample Number: ST-34

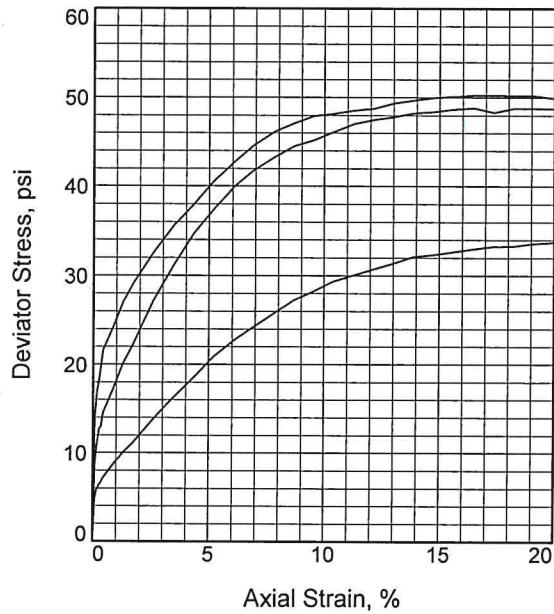
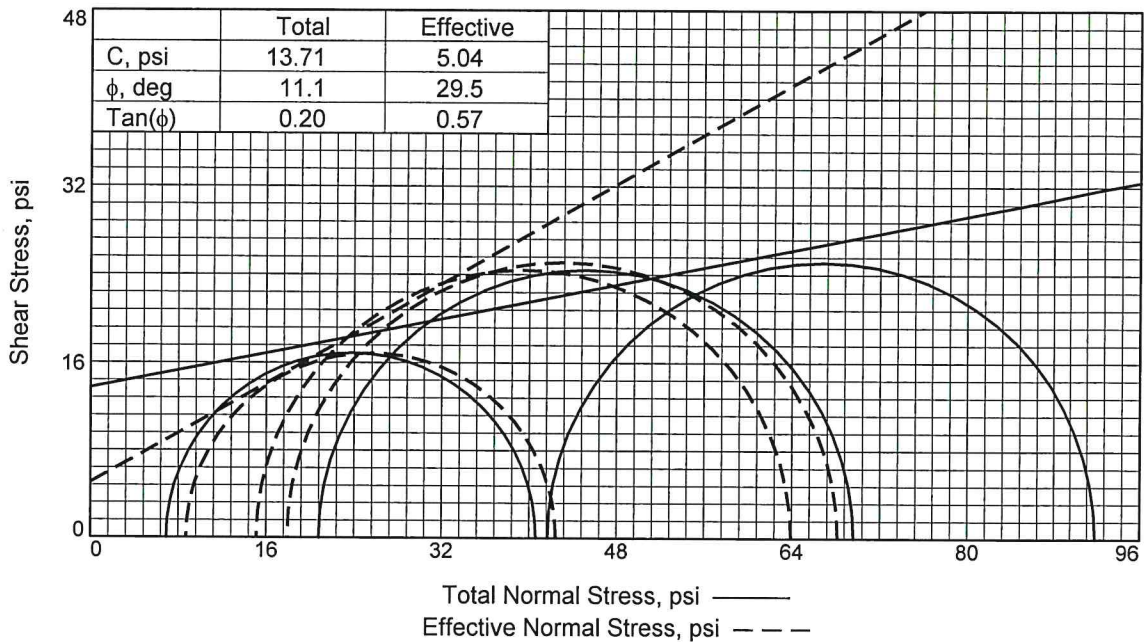
Project No.: 28287

Figure _____

Midland Standard Engineering & Testing

Tested By: JDS

Checked By: WDP



Sample No.	1	2	3	
Initial	Water Content, %	13.9	15.7	13.6
	Dry Density, pcf	123.5	120.6	122.3
	Saturation, %	97.8	102.1	92.4
	Void Ratio	0.3906	0.4239	0.4043
	Diameter, in.	2.86	2.90	2.85
	Height, in.	5.77	5.76	5.76
At Test	Water Content, %	14.2	15.4	14.7
	Dry Density, pcf	123.5	120.6	122.3
	Saturation, %	100.0	100.0	100.0
	Void Ratio	0.3906	0.4239	0.4043
	Diameter, in.	2.86	2.90	2.85
	Height, in.	5.77	5.76	5.76
Strain rate, in./min.	0.037	0.037	0.037	
Back Pressure, psi	51.5	51.5	51.5	
Cell Pressure, psi	58.4	72.3	93.2	
Fail. Stress, psi		33.7	48.8	50.3
	Total Pore Pr., psi	49.7	57.2	75.3
Ult. Stress, psi				
	Total Pore Pr., psi			
$\bar{\sigma}_1$ Failure, psi	42.4	63.9	68.2	
$\bar{\sigma}_3$ Failure, psi	8.7	15.1	17.9	

Type of Test:

CU with Pore Pressures

Sample Type: Shelby Tube

Description: Grey Silty CLAY

Specific Gravity= 2.75

Remarks:

Figure _____

Client: APTIM

Project: Zion Landfill Site 2 Expansion, Aptim #3211

Location: B-13

Sample Number: ST-29

Depth: 56.0- 58.0'

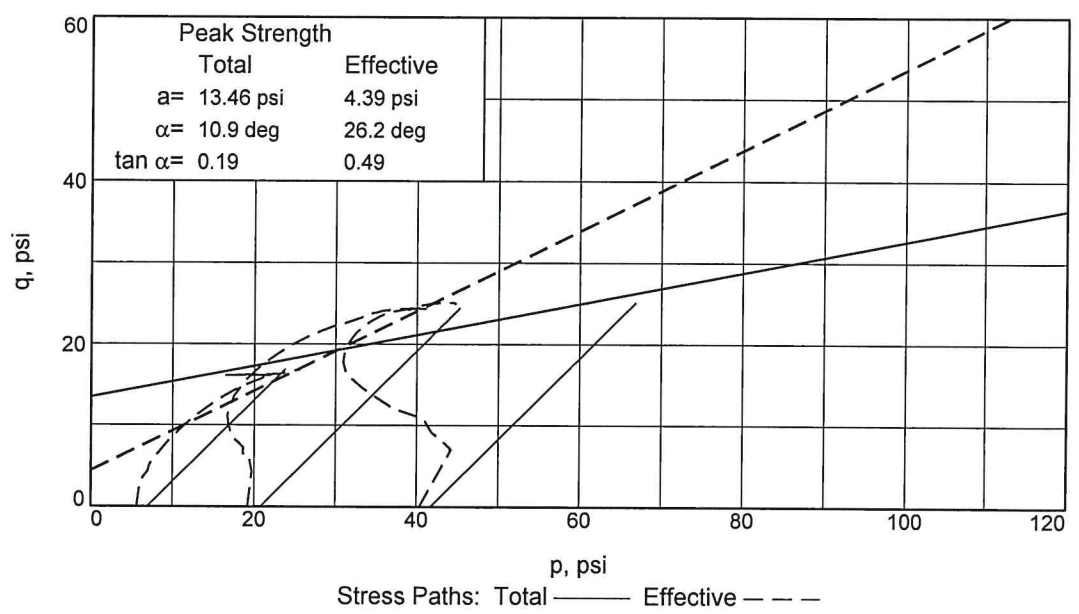
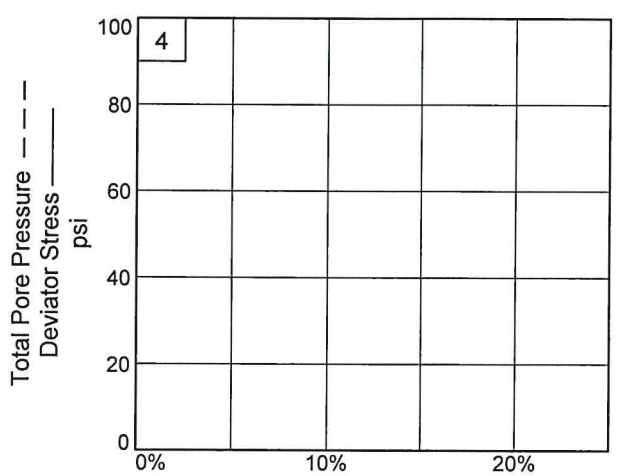
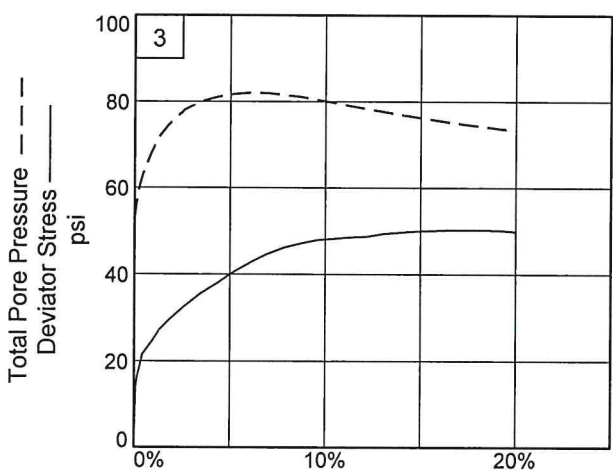
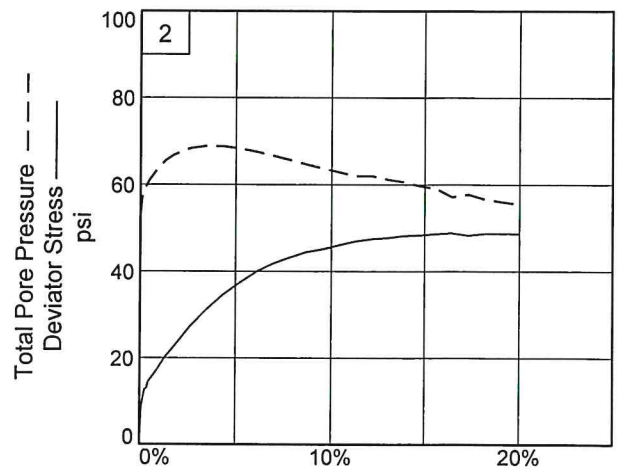
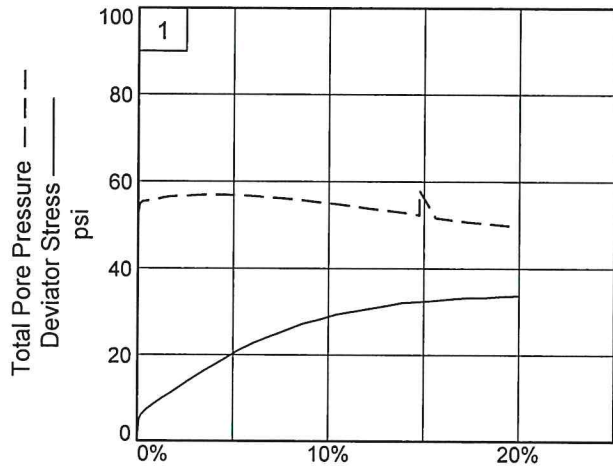
Proj. No.: 28287

Date Sampled: 4/17/19

TRIAXIAL SHEAR TEST REPORT
 Midland Standard Engineering & Testing
 South Elgin, IL

Tested By: JDS

Checked By: WDP



Client: APTIM
 Project: Zion Landfill Site 2 Expansion, Aptim #3211
 Location: B-13 Depth: 56.0- 58.0' Sample Number: ST-29
 Project No.: 28287 Figure _____ Midland Standard Engineering & Testing

Tested By: JDS Checked By: WDP

**USGS Earthquake Hazards Program -
Bedrock Acceleration**

**Interface Friction Test Results -
Previous Final Cover and Liner Construction**



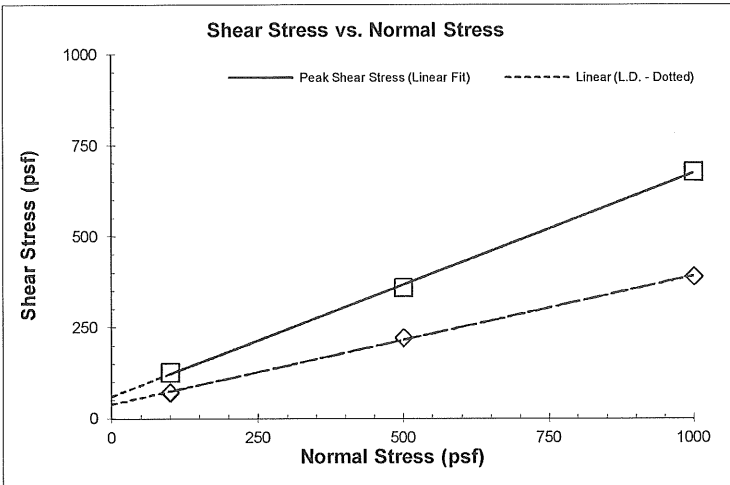
Interface Friction Test Report

Client: **Veolia Environmental Services**
Project: **Zion Landfill, 2012 Cap**
Test Date: 10/30/12-10/30/12

TRI Log#: E2365-93-04
Test Method: ASTM D 5321

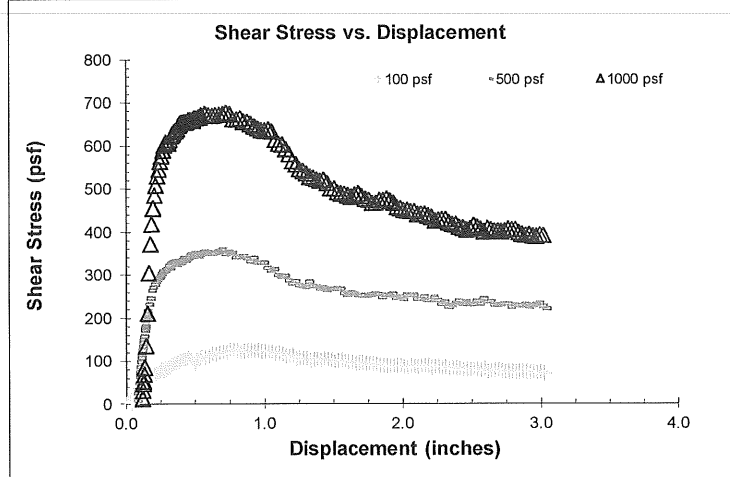
John M. Allen, P.E., 10/30/2012
Quality Review/Date

Tested Interface: GSE Double-sided Geocomposite vs. GSE 40 mil LLDPE Textured Geomembrane



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	31.5	19.4
Y-intercept or Adhesion (psf):	61	40

Shearing occurred at the interface.



Test Conditions	
Upper Box &	GSE double-sided geocomposite
Lower Box	GSE 40 mil LLDPE textured geomembrane
Box Dimensions: 12"x12"x4"	
Interface Conditioning:	Interface soaked and loading applied for a minimum of 15 minutes prior to shear.
Test Condition: Wet	
Shearing Rate: 0.2 inches/minute	

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	9	13	18
Normal Stress (psf)	100	500	1000
Corrected Peak Shear Stress (psf)	127	360	678
Corrected Large Displacement Shear Stress (psf)	72	222	390
Peak Secant Angle (degrees)	51.9	35.7	34.1
Large Displacement Secant Angle (degrees)	35.8	23.9	21.3
Asperity (mils)	29.4	30.0	29.4

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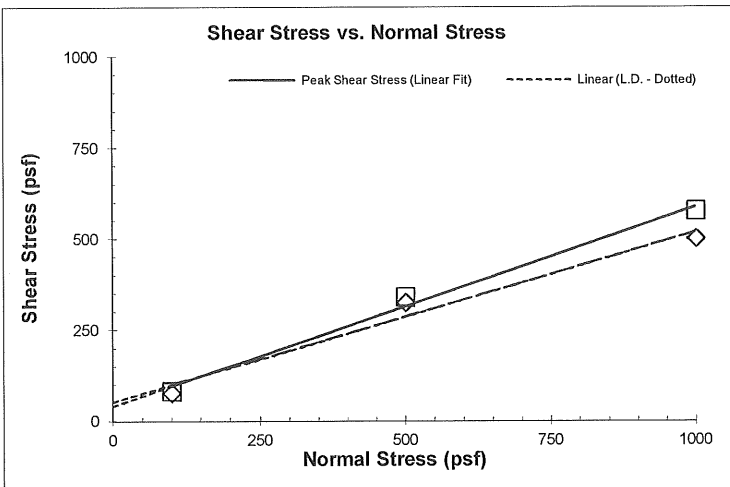
Interface Friction Test Report

Client: **Veolia Environmental Services**
Project: **Zion Landfill, 2012 Cap**
Test Date: 10/30/12-10/31/12

TRI Log#: E2365-93-04
Test Method: ASTM D 5321

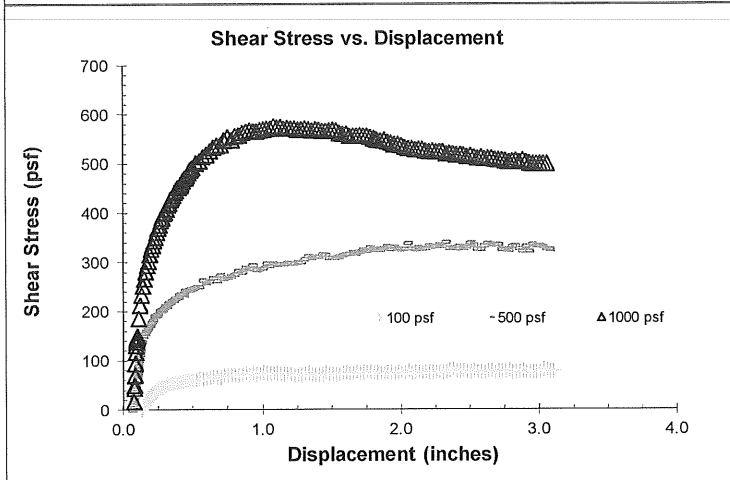
John M. Allen, P.E., 10/31/2012
Quality Review/Date

Tested Interface: Cover Soil vs. GSE Double-sided Geocomposite



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	28.7	24.9
Y-intercept or Adhesion (psf):	42	54

Shearing occurred at the interface.



Test Conditions	
Upper Box &	Cover Soil tamped in place
Lower Box	GSE double-sided geocomposite
Box Dimensions: 12"x12"x4"	
Interface Conditioning:	Interface soaked and loading applied for a minimum of 16 hours prior to shear.
Test Condition: Wet	
Shearing Rate: 0.04 inches/minute	

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	9	13	18
Normal Stress (psf)	100	500	1000
Corrected Peak Shear Stress (psf)	82	341	577
Corrected Large Displacement Shear Stress (psf)	78	326	501
Peak Secant Angle (degrees)	39.5	34.3	30.0
Large Displacement Secant Angle (degrees)	38.0	33.1	26.6

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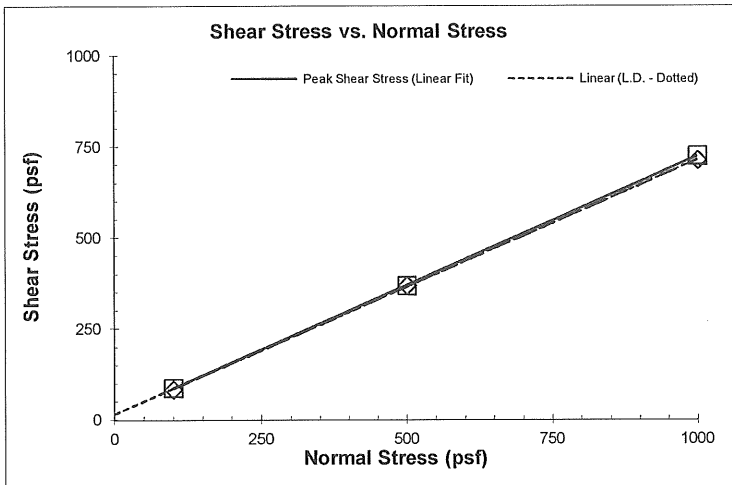
Interface Friction Test Report

Client: **Veolia Environmental Services**
Project: **Zion Landfill, 2012 Cap**
Test Date: 10/31/12-11/01/12

TRI Log#: E2365-93-04
Test Method: ASTM D5321

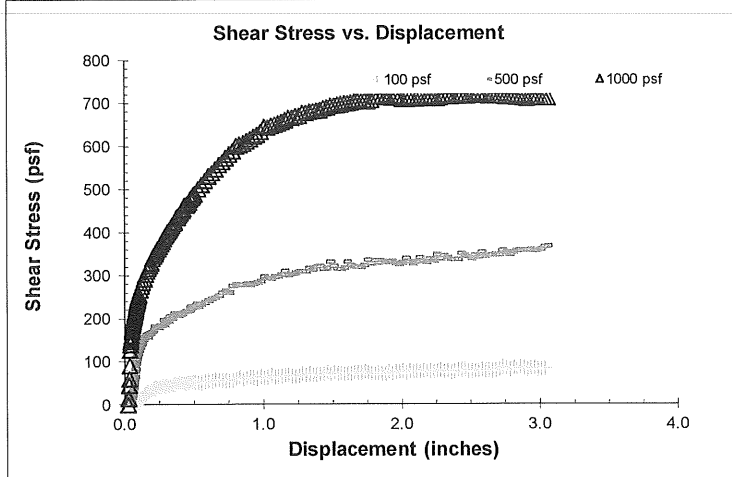
John M. Allen, P.E., 11/01/2012
Quality Review/Date

Tested Interface: Clay Soil vs. GSE 40 mil LLDPE Textured Geomembrane



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	35.3	34.9
Y-intercept or Adhesion (psf):	17	16

Shearing occurred at the interface.



Test Conditions	
Upper Box &	Clay Soil remolded to 131 pcf at 11.5% moisture content
Lower Box	GSE 40 mil LLDPE textured geomembrane
Box Dimensions:	12"x12"x4"
Interface Conditioning:	Interface soaked and loading applied for a minimum of 16 hours prior to shear.
Test Condition:	Wet
Shearing Rate:	0.04 inches/minute

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	9	13	18
Normal Stress (psf)	100	500	1000
Corrected Peak Shear Stress (psf)	88	369	725
Corrected Large Displacement Shear Stress (psf)	84	369	713
Peak Secant Angle (degrees)	41.4	36.4	35.9
Large Displacement Secant Angle (degrees)	40.0	36.4	35.5
Asperity (mils)	23.6	23.6	23.2

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Interface Friction Test Report

Client: **Advanced Disposal Services**

TRI Log#: E2388-71-05

John M. Allen, P.E., 09/17/2015

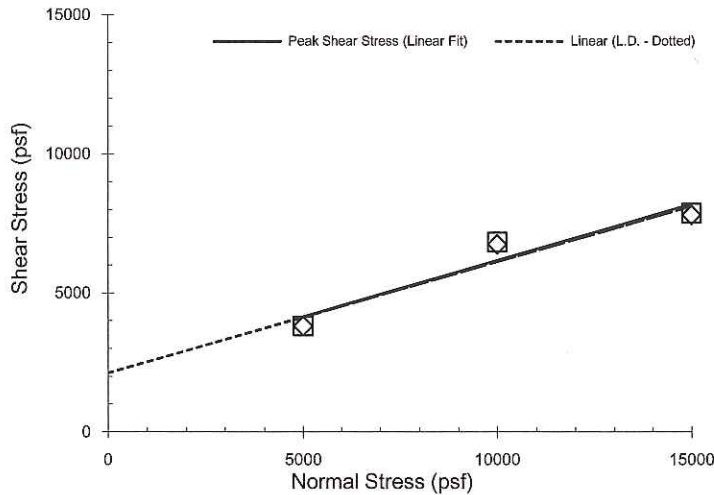
Project: **Zion Landfill, Cell 9**

Test Method: ASTM D5321

Quality Review/Date

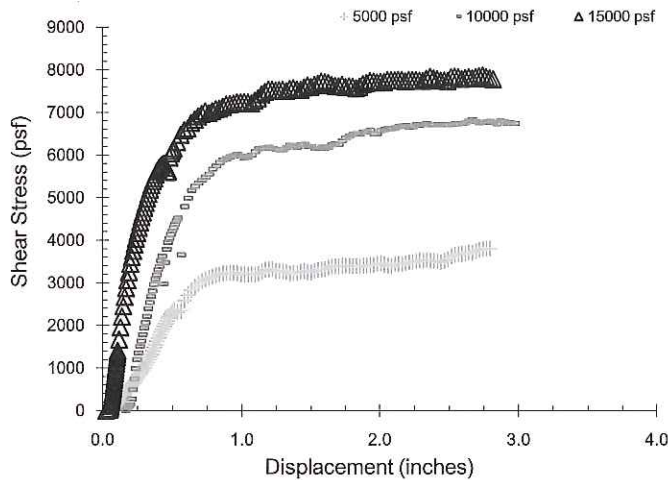
Date: 09-17-2015 to 09-17-2015

Tested Interface: Pea Stone vs. Skaps GE160 Non-woven Geotextile (389841.51)



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	22.1	21.8
Y-intercept or Adhesion (psf):	2112	2125

Shearing occurred at the interface.



Test Conditions	
Upper Box &	Pea Stone tamped in place
Lower Box	Skaps GE160 non-woven geotextile
Box Dimensions:	12"x12"x4"
Interface Conditioning:	Interface soaked and loading applied for a minimum of 1 hours prior to shear.
Test Condition:	Wet
Shearing Rate:	0.04 inches/minute

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	56	103	151
Normal Stress (psf)	5000	10000	15000
Corrected Peak Shear Stress (psf)	3809	6826	7865
Corrected Large Displacement Shear Stress (psf)	3809	6744	7802
Peak Secant Angle (degrees)	37.3	34.3	27.7
Large Displacement Secant Angle (degrees)	37.3	34.0	27.5
Asperity (mils)	--	--	--

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Interface Friction Test Report

Client: **Advanced Disposal Services**

TRI Log#: E2388-71-05

John M. Allen, P.E., 09/16/2015

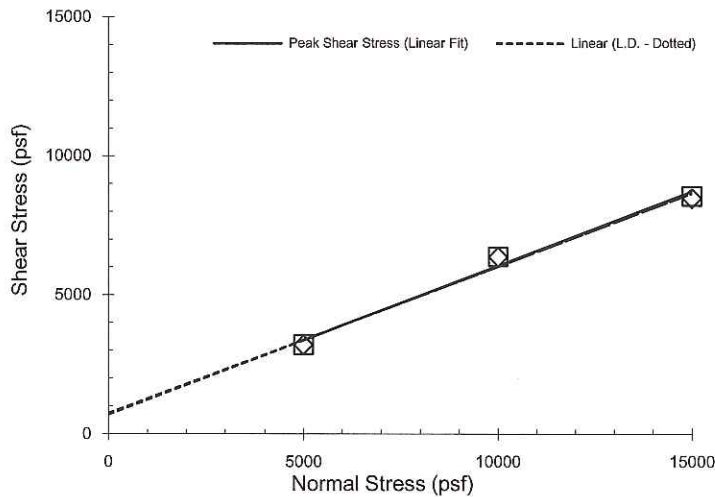
Project: **Zion Landfill, Cell 9**

Test Method: ASTM D5321

Quality Review/Date

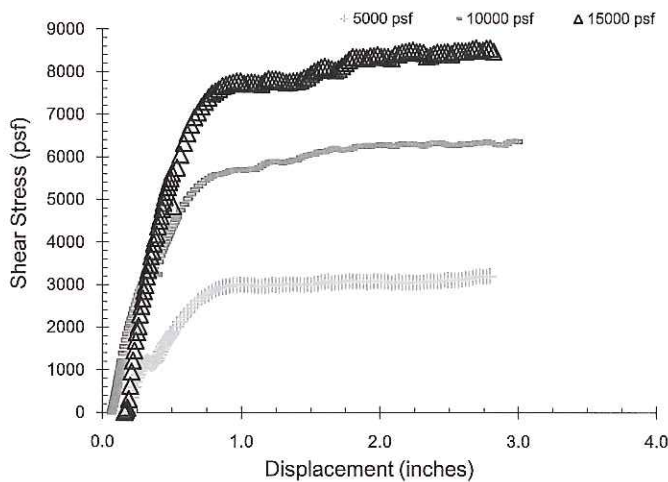
Date: 09-01-2015 to 09-16-2015

Tested Interface: Pea Stone vs. Skaps GE112 Non-woven Geotextile (38984.02)



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	28.1	27.8
Y-intercept or Adhesion (psf):	698	743

Shearing occurred at the interface.



Test Conditions	
Upper Box &	Pea Stone tamped in place
Lower Box	Skaps GE112 non-woven geotextile
Box Dimensions:	12"x12"x4"
Interface Conditioning:	Interface soaked and loading applied for a minimum of 1 hours prior to shear.
Test Condition:	Wet
Shearing Rate:	0.04 inches/minute

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	56	103	151
Normal Stress (psf)	5000	10000	15000
Corrected Peak Shear Stress (psf)	3208	6373	8556
Corrected Large Displacement Shear Stress (psf)	3205	6364	8478
Peak Secant Angle (degrees)	32.7	32.5	29.7
Large Displacement Secant Angle (degrees)	32.7	32.5	29.5
Asperity (mils)	--	--	--

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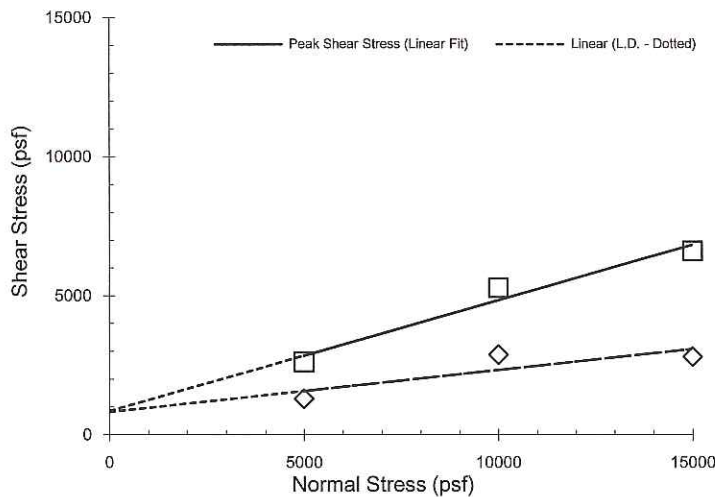
Interface Friction Test Report

Client: **Advanced Disposal Services**
 Project: **Zion Landfill, Cell 9**
 Date: **08-28-2015 to 09-08-2015**

TRI Log#: **E2388-71-05**
 Test Method: **ASTM D5321**

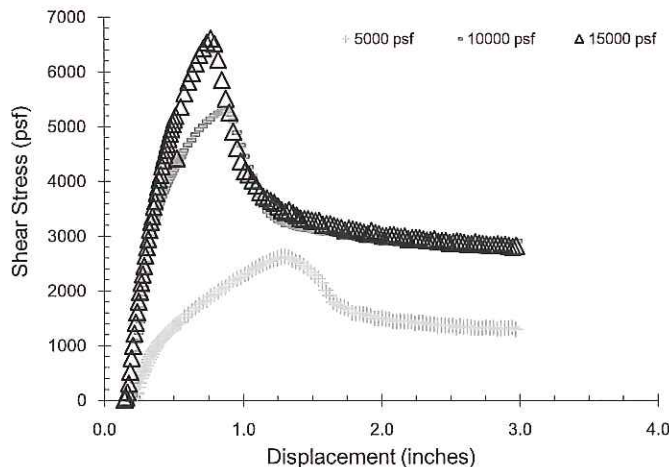
John M. Allen, P.E., 09/08/2015
 Quality Review/Date

Tested Interface: Skaps GE112 Non-woven Geotextile (38984.02) vs. Agru 60 mil HDPE Microspike Geomembrane (G15D304051)



Test Results		
	Peak	Large Displacement (@ 3.0 in.)
Friction Angle (degrees):	21.7	8.6
Y-intercept or Adhesion (psf):	851	815

Shearing occurred at the interface.



Test Conditions	
Upper Box &	Skaps GE112 non-woven geotextile
Lower Box	Agru 60 mil HDPE Microspike geomembrane (dull side)
Box Dimensions:	12"x12"x4"
Interface Conditioning:	Interface soaked and loading applied for a minimum of 15 minutes prior to shear.
Test Condition:	Wet
Shearing Rate:	0.2 inches/minute

Test Data			
Specimen No.	1	2	3
Bearing Slide Resistance (lbs)	56	103	151
Normal Stress (psf)	5000	10000	15000
Corrected Peak Shear Stress (psf)	2621	5284	6607
Corrected Large Displacement Shear Stress (psf)	1294	2879	2805
Peak Secant Angle (degrees)	27.7	27.9	23.8
Large Displacement Secant Angle (degrees)	14.5	16.1	10.6
Asperity (mils)	24.8	24.8	25.0

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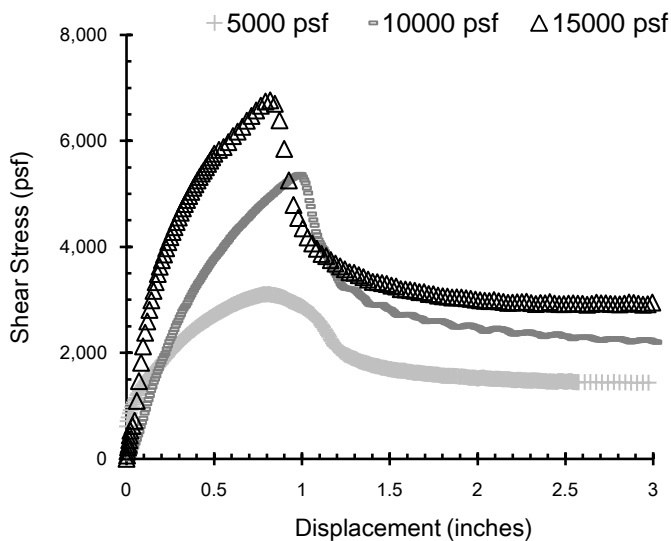
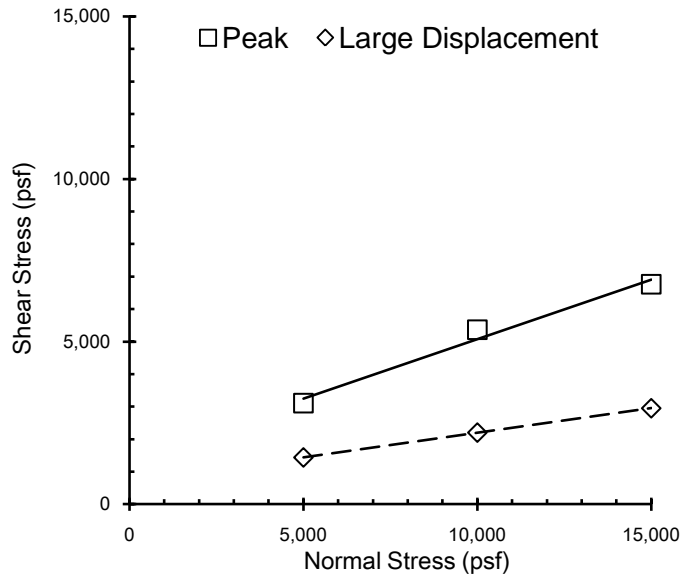
Shear Strength of Geosynthetic-Geosynthetic Interface by Direct Shear (ASTM D5321)

Client: ADS
Project: Zion Landfill

TRI Log #: 45186-4
Richard S. Lacey, P.E. 4/9/2019

Analysis & Quality Review/Date

SKAPS NWGT GE112 (56408.5) vs. Agru 60 mil HDPE MSGM - dull side up



Test Results, Linear Regression			
Mohr-Coulomb Parameters		Peak	Large Displacement
Friction Angle	Degrees	20.1	8.6
Y-intercept or Adhesion	psf	1,421	683
Minimum Secant Angle	Degrees	24.3	11.1

Note - Large Displacement Values Reported for 3.0 inches of Displacement

Test Conditions	
Upper Box	SKAPS NWGT GE112 (56408.5)
Lower Box	Agru 60 mil HDPE MSGM - dull side up
Conditioning	Wet - Loading applied and Interface flooded for a minimum of 15 minutes prior to shear.
Shearing Rate	inches/minute 0.2

Test Notes

Shearing occurred at the interface at all stresses.

Specimen No.	-	1	2	3	
Normal Stress	psf	5,000	10,000	15,000	
Box Edge Dimension	in	12	12	12	
Equivalent Bearing Slide Resist. Correction	psf	56	103	151	
Peak	Shear Stress	psf	3,107	5,365	6,764
	Secant Angle	deg.	31.9	28.2	24.3
Large Displacement	Shear Stress	psf	1,437	2,200	2,949
	Secant Angle	deg.	16.0	12.4	11.1
Asperity Height, Avg. of 5 Meas.	mils	26	27	26	

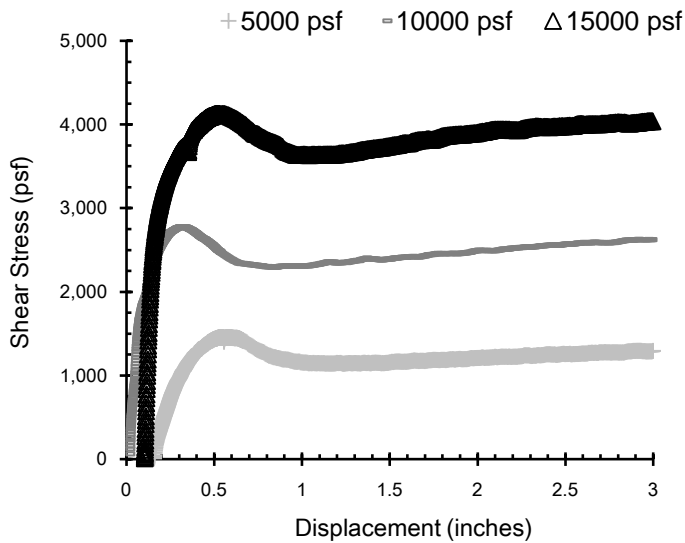
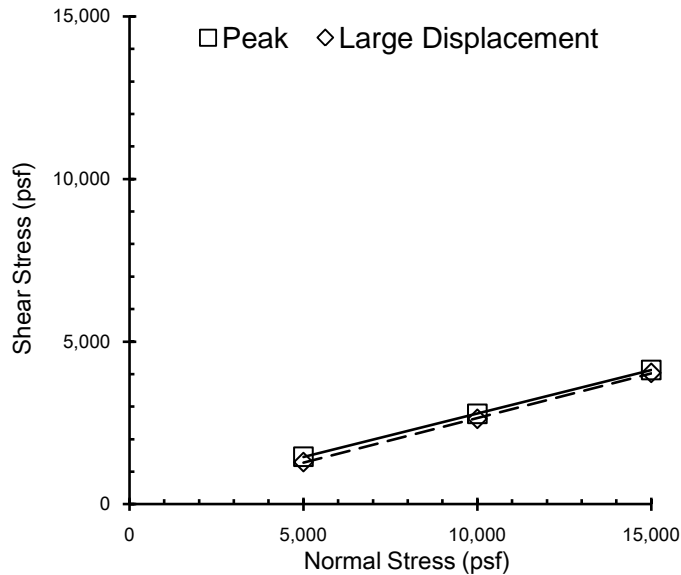


Shear Strength of Soil-Geosynthetic Interface by Direct Shear (ASTM D5321)

Client: ADS
Project: Zion Landfill

TRI Log #: 45186-6
Richard S. Lacey, P.E. 4/15/2019
Analysis & Quality Review/Date

Clay Sample vs.
Agru 60 mil HDPE MSGM - shiny side up



Test Results, Linear Regression			
Mohr-Coulomb Parameters		Peak	Large Displacement
Friction Angle	Degrees	14.9	14.9
Y-intercept or Adhesion	psf	120	0 (Forced)
Minimum Secant Angle	Degrees	15.4	14.5

Note - Large Displacement Values Reported for 3.0 inches of Displacement

Test Conditions	
Upper Box	Clay Sample $\omega = 15.3\% \quad \gamma_d = 114.5 \text{ pcf}$
Lower Box	Agru 60 mil HDPE MSGM - shiny side up
Conditioning	Wet - Loading applied and Interface flooded for a minimum of 24 hours prior to shear.
Shearing Rate	inches/minute 0.04

Test Notes

Shearing occurred at the interface at all stresses.

Specimen No.	-	1	2	3	
Normal Stress	psf	5,000	10,000	15,000	
Box Edge Dimension	in	12	12	12	
Equivalent Bearing Slide Resist. Correction	psf	56	103	151	
Peak	Shear Stress	psf	1,459	2,778	4,127
	Secant Angle	deg.	16.3	15.5	15.4
Large Displacement	Shear Stress	psf	1,294	2,624	4,039
	Secant Angle	deg.	14.5	14.7	15.1
Asperity Height, Avg. of 5 Meas.	mils	31	32	31	



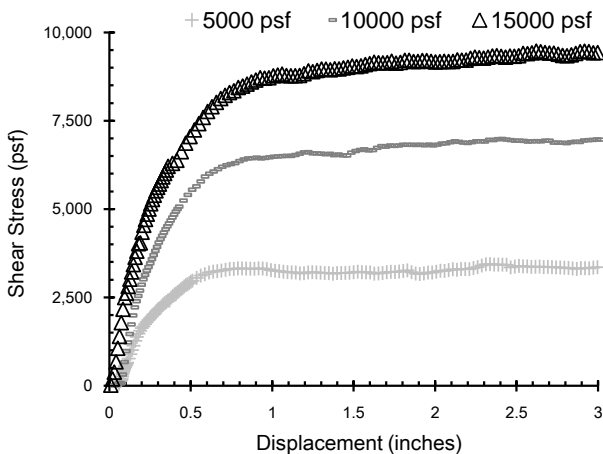
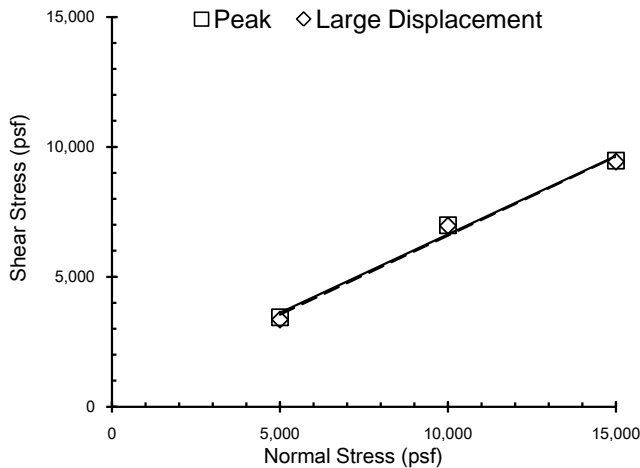
Shear Strength of Soil-Geosynthetic Interface by Direct Shear (ASTM D5321)

Client: ADS
Project: Zion Landfill

TRI Log #: 45186-8
Richard S. Lacey 9/24/2019

Analysis & Quality Review/Date

Drainage Layer vs.
SKAPS NWGT GE112 (56408.5)



Test Results, Linear Regression			
Mohr-Coulomb Parameters		Peak	Large Displacement
Friction Angle	Degrees	31.1	31.3
Y-intercept or Adhesion	psf	603	508
Minimum Secant Angle	Degrees	32.3	32.2

Note - Large Displacement Values Reported for 3.0 inches of Displacement

Test Conditions	
Upper Box	Drainage Layer 60% Midwest Agg.-40% Theilen Tamped in place
Lower Box	SKAPS NWGT GE112 (56408.5)
Conditioning	Wet - Loading applied and Interface flooded for a minimum of 24 hours prior to shear.
Shearing Rate	inches/minute 0.04

Test Notes

Shearing occurred at the interface at all stresses.

Specimen No.		-	1	2	3
Normal Stress	psf		5,000	10,000	15,000
Box Edge Dimension	in		12	12	12
Bearing Slide Resistance	lbs		56	103	151
Peak	Shear Stress	psf	3,442	6,984	9,472
	Secant Angle	deg.	34.5	34.9	32.3
Large Displacement	Shear Stress	psf	3,354	6,966	9,431
	Secant Angle	deg.	33.9	34.9	32.2

Published Technical References

Shear Strength of Municipal Solid Waste

Jonathan D. Bray, F.ASCE¹; Dimitrios Zekkos, M.ASCE²; Edward Kavazanjian Jr., M.ASCE³;
George A. Athanasopoulos, M.ASCE⁴; and Michael F. Riemer, M.ASCE⁵

Abstract: A comprehensive large-scale laboratory testing program using direct shear (DS), triaxial (TX), and simple shear tests was performed on municipal solid waste (MSW) retrieved from a landfill in the San Francisco Bay area to develop insights about and a framework for interpretation of the shear strength of MSW. Stability analyses of MSW landfills require characterization of the shear strength of MSW. Although MSW is variable and a difficult material to test, its shear strength can be evaluated rationally to develop reasonable estimates. The effects of waste composition, fibrous particle orientation, confining stress, rate of loading, stress path, stress-strain compatibility, and unit weight on the shear strength of MSW were evaluated in the testing program described herein. The results of this testing program indicate that the DS test is appropriate to evaluate the shear strength of MSW along its weakest orientation (i.e., on a plane parallel to the preferred orientation of the larger fibrous particles within MSW). These laboratory results and the results of more than 100 large-scale laboratory tests from other studies indicate that the DS static shear strength of MSW is best characterized by a cohesion of 15 kPa and a friction angle of 36° at normal stress of 1 atm with the friction angle decreasing by 5° for every log cycle increase in normal stress. Other shearing modes that engage the fibrous materials within MSW (e.g., TX) produce higher friction angles. The dynamic shear strength of MSW can be estimated conservatively to be 20% greater than its static strength. These recommendations are based on tests of MSW with a moisture content below its field capacity; therefore, cyclic degradation due to pore pressure generation has not been considered in its development.

DOI: 10.1061/(ASCE)GT.1943-5606.0000063

CE Database subject headings: Dynamic properties; Municipal wastes; Solid wastes; Landfills; Shear strength; Stress strain relations; Laboratory tests.

Introduction

Static and seismic stability analyses of municipal solid waste (MSW) landfills require appropriate characterization of the shear strength of MSW. Landfill stability analyses can be no more reliable than the reliability of the engineer's estimate of the shear strength of the waste. Because modern municipal solid waste (MSW) landfills are built with multilayer liner systems that contain materials and interfaces with varied stress-strain responses, including some that may exhibit postpeak drops in shear strength, the stress-strain response of MSW may also need to be considered to provide mobilized shear strength values that are compatible with the level of deformation anticipated along potential failure surfaces.

There is considerable uncertainty associated with the MSW shear strength values currently employed in practice. Obstacles to evaluating the shear strength of MSW include its heterogeneity and the difficulty in recovering and testing representative waste samples due to the large size of some waste constituents. In this paper, relevant studies of MSW shear strength are briefly reviewed, and then the results from a comprehensive testing program on reconstituted specimens of waste sampled at a landfill in northern California are summarized. These data, which include large-scale direct shear (DS), triaxial (TX), and simple shear (SS) test results as well as large-scale testing data of waste from numerous landfills worldwide and back-analyses of failed landfill slopes in the field, are then interpreted to provide both recommendations for assessing the shear strength of MSW on a project-specific basis and a new generic shear strength characterization for MSW for use in design when project-specific data are not available.

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Insights from Previous Studies

A comprehensive discussion of previous studies of the shear strength of MSW is presented in Zekkos (2005). These previous studies of MSW shear strength indicate:

- The Mohr-Coulomb strength criterion is typically used to characterize the shear strength of waste (e.g., Landva and Clark 1990). These characterizations indicate that MSW shear strength is primarily stress dependent (i.e., frictional), particularly at higher confining stresses, but that it also has significant

strength at low confining stresses (i.e., cohesive strength). The shear strength at low confining stress appears to result primarily from the fibrous constituents of the waste.

- The equivalent (secant) friction angle of the MSW Mohr-Coulomb failure envelope appears to decrease as the normal stress increases (Pelkey et al. 2001).
- The shear strength used to characterize MSW may depend on the testing conditions (i.e., stress state, stress path, and strain path), specimen preparation, and the strength criterion used. Typical direct shear test results on both reconstituted specimens and on intact specimens tested in situ at low normal stress suggest a cohesion (c) of between 0 and 50 kPa and a friction angle (ϕ) of between 27 and 41°, with a majority of investigators suggesting $\phi \cong 33^\circ$ (Landva and Clark 1990; Richardson and Reynolds 1991; Houston et al. 1995; Withiam et al. 1995; Kavazanjian et al. 1999; Mazzucato et al. 1999; Pelkey et al. 2001). MSW shear strength interpretation from triaxial tests is complicated by the need to use strain level-based definitions of shear strength due to the lack of a well defined peak strength and a continued increase in mobilized strength at large strains. MSW shear strength in triaxial compression has been defined in the literature as the mobilized shear stress at 5–25% axial strain. Friction angles as high as 45–53° have been reported at high strain levels (Jessberger and Kockel 1995; Grisolia et al. 1995). However, when strength is evaluated at lower strain levels typically considered appropriate for field characterization of shear strength (e.g., 5–10%), triaxial strength values tend to be lower than those from direct shear tests (Vilar and Carvalho 2002) or back-analysis of waste slopes (Kavazanjian et al. 1995; Eid et al. 2000).
- Simple shear tests on MSW are limited. In interpreting this test, an assumption of the orientation of the failure surface or of the value of the horizontal normal stress is required. Assuming the failure plane to be horizontal and interpreting the simple shear test as a direct shear test results in the lowest possible strength estimate, whereas assuming the normal stress on the vertical plane to be the K_0 stress or that the normal stress on the vertical plane is the mean normal stress results in a significantly higher strength estimate. Using $K_0=0.6$, Kavazanjian et al. (1999) estimated a lower bound shear strength envelope of $c=16$ kPa and $\phi=33^\circ$ and an upper bound envelope of $c=30$ kPa and $\phi=59^\circ$ from simple shear tests on reconstituted MSW. However, using the assumption of a horizontal failure plane, Kavazanjian et al. (1999) and Pelkey et al. (2001) found that the shear strength interpreted from large-scale simple shear tests was similar to the value interpreted from large-scale direct shear tests.
- Specimens with higher fiber content appear to be stronger than specimens with lower fiber content. Kavazanjian et al. (1999) observed that large direct shear specimens with lower fiber content were slightly weaker than specimens with more fiber content. Towhata et al. (2004) observed that triaxial specimens that included plastic inclusions sustained higher stresses at large strain than specimens without plastic inclusions.
- Testing to date has not indicated that the strength of MSW varies significantly due to reasonable variations in its unit weight (Kavazanjian et al. 1999; Vilar and Carvalho 2002).
- “Undisturbed” and reconstituted large-scale direct shear tests on MSW performed by Mazzucato et al. (1999) indicate similar shear strengths; however, only the “undisturbed” specimens exhibited a defined peak strength followed by a postpeak strength reduction.
- The shear strength estimated from stable and failed waste

slopes is similar to that estimated from direct shear tests (Kavazanjian et al. 1995; Eid et al. 2000).

Some key observations that may be drawn from these studies on the shear strength of MSW are:

- There is great variability in the reported shear strengths in the literature. Cohesion values from 0 to 80 kPa and friction angles from 0–60° have been reported. In design, the static strength of MSW is often assumed to be that recommended by Kavazanjian et al. (1995) (i.e., $c=24$ kPa and $\phi=0^\circ$ for normal stresses less than 37 kPa, and $c=0$ kPa and $\phi=33^\circ$ for larger normal stresses) or by Eid et al. (2000) (i.e., a mean value of $c=25$ kPa and $\phi=35^\circ$). The dynamic strength of MSW is typically assumed to be at least equal to and occasionally greater than its static strength. Augello et al. (1998) suggest that $\phi \cong 35\text{--}38^\circ$ is a reasonable value for the strength of MSW subject to seismic loading.
 - Specimen preparation and testing procedures are often not reported. Furthermore, there are significant differences among the specimen preparation and testing procedures that are reported.
 - The stress-deformation response of MSW observed in different testing devices is remarkably different. In DS testing, the stress-displacement response is typically observed to be convex shaped (i.e., roughly hyperbolic), may approach an asymptotic value at large deformation, and sometimes includes a postpeak reduction in strength. In TX testing, MSW stress-strain response is often initially convex shaped, then almost linear, and finally becomes concave shaped (i.e. exhibits an increasing upward curvature) without any sign of reaching an asymptotic value, let alone a well-defined peak shear stress.
 - The effects of waste degradation on MSW shear strength have not been addressed to any significant extent. Based on limited testing, it has been suggested that degradation will lower the strength of MSW. For example, triaxial test data presented by Turczynski (1988) indicate that the friction angle of MSW reduced from about 39° for fresh waste to 35°, to 32°, and finally to 26°, for 3 yr, 5 yr, and 15 yr old waste, respectively. The cohesion intercept interpreted from these tests also reduced systematically as the age of the waste increased. However, these trends are not observed in all the laboratory data. The problem is compounded by the fact that there has been no quantification of the level of degradation within waste. Although age is an important parameter, other factors, such as the waste composition, climate, moisture content, and landfill operational procedures are likely to contribute significantly to the rate of waste degradation.
- Based upon these observations, there are still many uncertainties associated with the shear strength of MSW. Key issues associated with the shear strength of MSW include:
- The influence of specimen preparation procedures on the shear strength measured in laboratory tests;
 - The influence of stress state on stress-strain behavior and shear strength (e.g., the discrepancy in the stress-strain-strength response of MSW between the DS and TX testing);
 - The influence of dynamic loading;
 - The influence of degradation; and
 - The relationship of the shear strength measured in laboratory tests to field values of MSW shear strength.
- A comprehensive multi-institution testing program was developed to address some of these issues and to develop revised recommendations for the shear strength of MSW for use in landfill stability analyses.

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Soil friction angle

Geotechdata.info - Updated 14.12.2013

Soil friction angle is a shear strength parameter of soils. Its definition is derived from the Mohr-Coulomb failure criterion and it is used to describe the friction shear resistance of soils together with the normal effective stress.

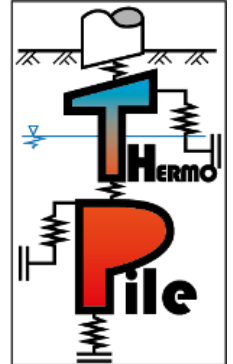
In the stress plane of Shear stress-effective normal stress, the soil friction angle is the angle of inclination with respect to the horizontal axis of the Mohr-Coulomb shear resistance line.

Typical values of soil friction angle for different soils according to USCS

Some typical values of soil friction angle are given below for different USCS soil types at normally consolidated condition unless otherwise stated. These values should be used only as guideline for geotechnical problems; however, specific condition of each engineering problem often needs to be considered for an appropriate choice of geotechnical parameters.

Description	USCS	Soil friction angle [°]			Reference
		min	max	Specific value	
Well graded gravel, sandy gravel, with little or no fines	GW	33	40		[1],[2].
Poorly graded gravel, sandy gravel, with little or no fines	GP	32	44		[1].
Sandy gravels - Loose	(GW, GP)			35	[3 cited in 6]
Sandy gravels - Dense	(GW, GP)			50	[3 cited in 6]
Silty gravels, silty sandy gravels	GM	30	40		[1].
Clayey gravels, clayey sandy gravels	GC	28	35		[1].
Well graded sands, gravelly sands, with little or no fines	SW	33	43		[1].
Well-graded clean sand, gravelly sands - Compacted	SW	-	-	38	[3 cited in 6]
Well-graded sand, angular grains - Loose	(SW)			33	[3 cited in 6]
Well-graded sand, angular grains - Dense	(SW)			45	[3 cited in 6]
Poorly graded sands, gravelly sands, with little or no fines	SP	30	39		[1], [2].
Poorly-graded clean sand - Compacted	SP	-	-	37	[3 cited in 6]
Uniform sand, round grains - Loose	(SP)			27	[3 cited in 6]
Uniform sand, round grains - Dense	(SP)			34	[3 cited in 6]
Sand	SW, SP	37	38		[7].
Loose sand	(SW, SP)	29	30		[5 cited in 6]
Medium sand	(SW, SP)	30	36		[5 cited in 6]
Dense sand	(SW, SP)	36	41		[5 cited in 6]
Silty sands	SM	32	35		[1].
Silty clays, sand-silt mix - Compacted	SM	-	-	34	[3 cited in 6]
Silty sand - Loose	SM	27	33		[3 cited in 6]

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Silty sand - Dense	SM	30	34		[3 cited in 6]
Clayey sands	SC	30	40		[1],
Clayey sands, sandy-clay mix - compacted	SC			31	[3 cited in 6]
Loamy sand, sandy clay Loam	SM, SC	31	34		[7],
Inorganic silts, silty or clayey fine sands, with slight plasticity	ML	27	41		[1],
Inorganic silt - Loose	ML	27	30		[3 cited in 6]
Inorganic silt - Dense	ML	30	35		[3 cited in 6]
Inorganic clays, silty clays, sandy clays of low plasticity	CL	27	35		[1],
Clays of low plasticity - compacted	CL			28	[3 cited in 6]
Organic silts and organic silty clays of low plasticity	OL	22	32		[1],
Inorganic silts of high plasticity	MH	23	33		[1],
Clayey silts - compacted	MH			25	[3 cited in 6]
Silts and clayey silts - compacted	ML			32	[3 cited in 6]
Inorganic clays of high plasticity	CH	17	31		[1],
Clays of high plasticity - compacted	CH			19	[3 cited in 6]
Organic clays of high plasticity	OH	17	35		[1],
Loam	ML, OL, MH, OH	28	32		[7],
Silt Loam	ML, OL, MH, OH	25	32		[7],
Clay Loam, Silty Clay Loam	ML, OL, CL, MH, OH, CH	18	32		[7],
Silty clay	OL, CL, OH, CH	18	32		[7],
Clay	CL, CH, OH, OL	18	28		[7],
Peat and other highly organic soils	Pt	0	10		[2],

Correlation between SPT-N value, friction angle, and relative density

Correlation between SPT-N value and friction angle and Relative density (Meyerhoff 1956)

SPT N3 [Blows/0.3 m - 1 ft]	Soil packing	Relative Density [%]	Friction angle [°]
< 4	Very loose	< 20	< 30
4 - 10	Loose	20 - 40	30 - 35
10 - 30	Compact	40 - 60	35 - 40
30 - 50	Dense	60 - 80	40 - 45
> 50	Very Dense	> 80	> 45

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Soil Cohesion

Geotechdata.info - Updated 31.10.2014

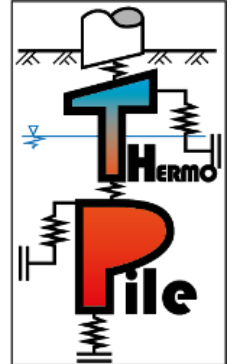
The cohesion is a term used in describing the shear strength soils. Its definition is mainly derived from the Mohr-Coulomb failure criterion and it is used to describe the non-frictional part of the shear resistance which is independent of the normal stress. In the stress plane of Shear stress-effective normal stress, the soil cohesion is the intercept on the shear axis of the Mohr-Coulomb shear resistance line

Typical values of soil cohesion for different soils

Some typical values of soil cohesion are given below for different soil types. The soil cohesion depends strongly on the consistence, packing, and saturation condition. The values given below correspond to normally consolidated condition unless otherwise stated. These values should be used only as guideline for geotechnical problems; however, specific condition of each engineering problem often needs to be considered for an appropriate choice of geotechnical parameters.

Description	USCS	Cohesion [kPa]			Reference
		min	max	Specific value	
Well graded gravel, sandy gravel, with little or no fines	GW	-	-	0	[1],[2],[3],
Poorly graded gravel, sandy gravel, with little or no fines	GP	-	-	0	[1],[2], [3],
Silty gravels, silty sandy gravels	GM	-	-	0	[1],
Clayey gravels, clayey sandy gravels	GC	-	-	20	[1],
Well graded sands, gravelly sands, with little or no fines	SW	-	-	0	[1],[2], [3],
Poorly graded sands, gravelly sands, with little or no fines	SP	-	-	0	[1],[2], [3],
Silty sands	SM	-	-	22	[1],
Silty sands - Saturated compacted	SM	-	-	50	[3],
Silty sands - Compacted	SM	-	-	20	[3],
Clayey sands	SC	-	-	5	[1],
Clayey sands - Compacted	SC	-	-	74	[3],
Clayey sands -Saturated compacted	SC	-	-	11	[3],
Loamy sand, sandy clay Loam - compacted	SM, SC	50	75		[2],
Loamy sand, sandy clay Loam - saturated	SM, SC	10	20		[2],
Sand silt clay with slightly plastic fines - compacted	SM, SC	-	-	50	[3],
Sand silt clay with slightly plastic fines - saturated compacted	SM, SC	-	-	14	[3],
Inorganic silts, silty or clayey fine sands, with slight plasticity	ML	-	-	7	[1],
Inorganic silts and clayey silts - compacted	ML	-	-	67	[3],
Inorganic silts and clayey silts - saturated compacted	ML	-	-	9	[3],
Inorganic clays, silty clays, sandy clays of low plasticity	CL	-	-	4	[1],
Inorganic clays, silty clays, sandy clays of low plasticity - compacted	CL	-	-	86	[3],
Inorganic clays, silty clays, sandy clays	CL	-	-	13	[3],

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of low plasticity - saturated compacted					
Mixture if inorganic silt and clay - compacted	ML-CL	-	-	65	[3],
Mixture if inorganic silt and clay - saturated compacted	ML-CL	-	-	22	[3],
Organic silts and organic silty clays of low plasticity	OL	-	-	5	[1],
Inorganic silts of high plasticity - compactd	MH	-	-	10	[1],
Inorganic silts of high plasticity - saturated compacted	MH	-	-	72	[3],
Inorganic silts of high plasticity	MH	-	-	20	[3],
Inorganic clays of high plasticity	CH	-	-	25	[1],
Inorganic clays of high plasticity - compacted	CH	-	-	103	[3],
Inorganic clays of high plasticity - satrated compacted	CH	-	-	11	[3],
Organic clays of high plasticity	OH	-	-	10	[1],
Loam - Compacted	ML, OL, MH, OH	60	90		[2],
Loam - Saturated	ML, OL, MH, OH	10	20		[2],
Silt Loam - Compacted	ML, OL, MH, OH	60	90		[2],
Silt Loam - Saturated	ML, OL, MH, OH	10	20		[2],
Clay Loam, Silty Clay Loam - Compacted	ML, OL, CL, MH, OH, CH	60	105		[2],
Clay Loam, Silty Clay Loam - Saturated	ML, OL, CL, MH, OH, CH	10	20		[2],
Silty clay, clay - compacted	OL, CL, OH, CH	90	105		[2],
Silty clay, clay - saturated	OL, CL, OH, CH	10	20		[2],
Peat and other highly organic soils	Pt	-	-		

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Citation :

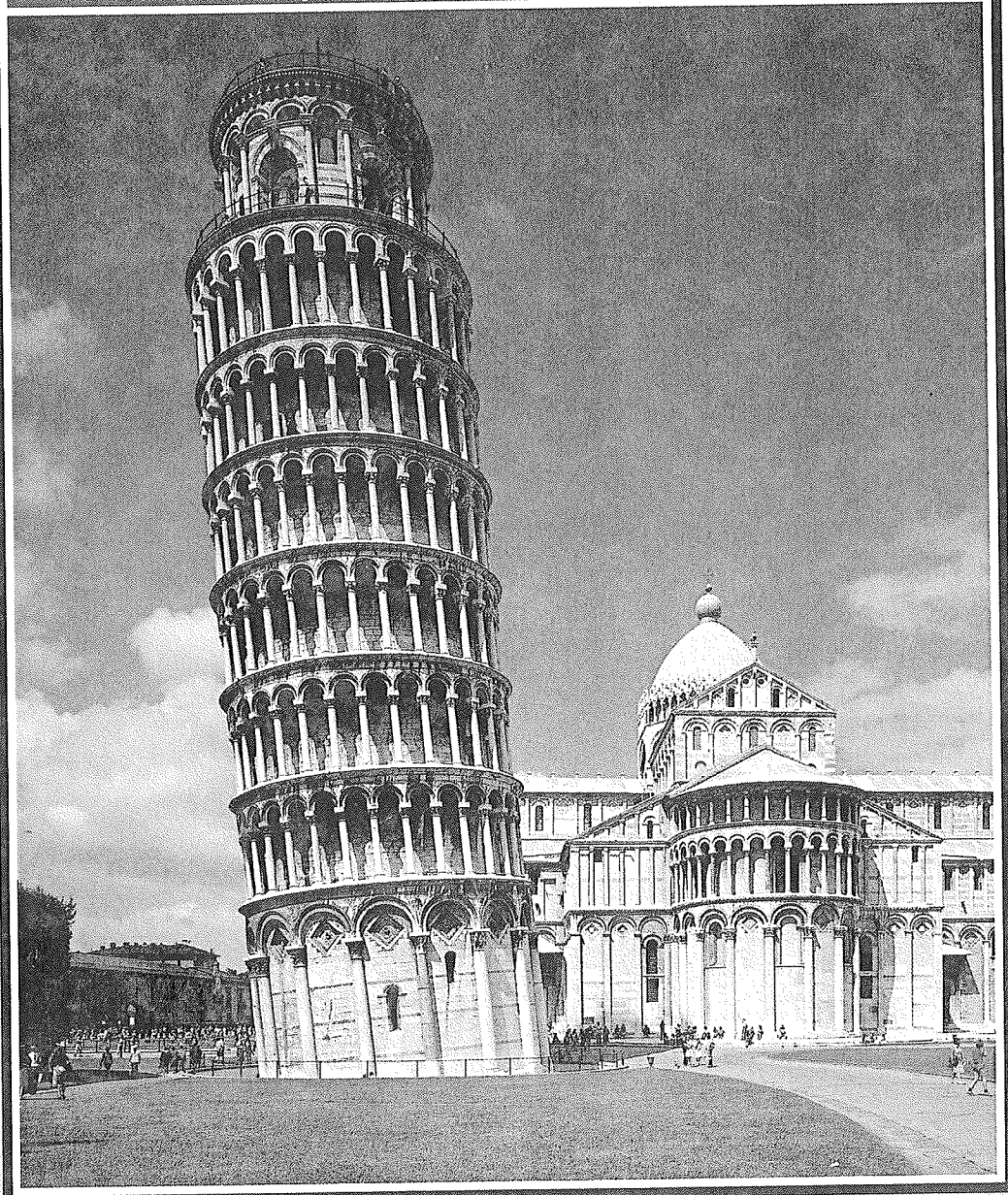
Geotechdata.info, Cohesion, <http://geotechdata.info/parameter/cohesion> (as of December 15, 2013).

Other soil parameters

- » [Angle of friction](#)
- » [Cohesion](#)
- » [Dry unit weight](#)
- » [Young's modulus](#)
- » [Void ratio](#)
- » [Soil Permeability coefficient](#)
- » [Soil porosity](#)

GEOTECHNICAL ENGINEERING

Principles and Practices



DONALD P. CODUTO

$$\gamma = \frac{W}{V} \tag{4.5}$$

The unit weight of undisturbed soil samples can easily be determined in the laboratory by measuring their physical dimensions and weighing them. This method produces reliable assessments of γ for many soils. However, it is affected by sample disturbance, especially in sandy and gravelly soils. Sometimes unit weight measurements are made on supposedly "undisturbed" samples that in reality have significant disturbance. Such measurements are very misleading, so it is best to not even attempt unit weight measurements on poor quality samples. Table 4.1 presents typical ranges of γ for various soils.

TABLE 4.1 TYPICAL UNIT WEIGHTS.

Soil Type and Unified Soil Classification (See Section 5.3)	Typical Unit Weight, γ			
	Above Groundwater Table		Below Groundwater Table	
	(lb/ft ³)	(kN/m ³)	(lb/ft ³)	(kN/m ³)
GP — Poorly graded gravel	110 - 130	17.5 - 20.5	125 - 140	19.5 - 22.0
GW — Well graded gravel	110 - 140	17.5 - 22.0	125 - 150	19.5 - 23.5
GM — Silty gravel	100 - 130	16.0 - 20.5	125 - 140	19.5 - 22.0
GC — Clayey gravel	100 - 130	16.0 - 20.5	125 - 140	19.5 - 22.0
SP — Poorly graded sand	95 - 125	15.0 - 19.5	120 - 135	19.0 - 21.0
SW — Well graded sand	95 - 135	15.0 - 21.0	120 - 145	19.0 - 23.0
SM — Silty sand	80 - 135	12.5 - 21.0	110 - 140	17.5 - 22.0
SC — Clayey sand	85 - 130	13.5 - 20.5	110 - 135	17.5 - 21.0
ML — Low plasticity silt	75 - 110	11.5 - 17.5	80 - 130	12.5 - 20.5
MH — High plasticity silt	75 - 110	11.5 - 17.5	75 - 130	11.5 - 20.5
CL — Low plasticity clay	80 - 110	12.5 - 17.5	75 - 130	11.5 - 20.5
CH — High plasticity clay	80 - 110	12.5 - 17.5	70 - 125	11.0 - 19.5

Two variations of unit weight also are commonly used, the *dry unit weight*, γ_d and the *unit weight of water*, γ_w :

$$\gamma_d = \frac{W_s}{V} \tag{4.6}$$

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where:
 $(N_1)_{60}$ = corrected SPT N -value, as defined in Chapter 3
 C_p = grain size correction factor
 C_A = aging correction factor
 C_{OCR} = overconsolidation correction factor
 D_{50} = grain size at which 50 percent of the soil is finer (mm) as defined in Section 4.4
 t = age of soil (time since deposition in years). If no age information data is available, use $t = 100$ yr.
 OCR = overconsolidation ratio, as defined in Chapter 11. If no information is available to assess the OCR, use a value of 2.
 q_c = cone resistance (kg/cm^2 or ton/ft^2), as defined in Chapter 3
 Q_c = compressibility factor
 = 0.91 for highly compressible sands
 = 1.00 for moderately compressible sands
 = 1.09 for slightly compressible sands
 For purposes of solving this formula, a sand with a high fines content or a high mica content is "highly compressible," whereas a pure quartz sand is "slightly compressible."
 σ_z' = vertical effective stress (lb/ft^2 ; kPa), as defined in Chapter 10

(4.21)

(4.22)

(4.23)

Many people confuse relative density with relative compaction. The latter is defined in Chapter 6. Although the names are similar, and they measure similar properties, these two parameters are numerically different. In addition, some people in other professions use the term "relative density" to describe what we call specific gravity! Geotechnical engineers should never use the term in this way.

Table 4.5 presents typical values of e_{min} and e_{max} for various sandy soils. These are not intended to be used in lieu of laboratory or in-situ tests, but could be used to check test results or for preliminary analyses.

TABLE 4.5 TYPICAL VALUES OF e_{min} AND e_{max} (Hough, 1969; Adapted by permission of John Wiley and Sons, Inc.)

(4.24)

Soil Description	e_{min} (dense)	e_{max} (loose)
Equal spheres (theoretical values)	0.35	0.92
Clean, poorly graded medium sand (Ottawa, Illinois)	0.50	0.80
Clean, fine-to-medium sand	0.40	1.0
Uniform inorganic silt	0.40	1.1
Silty sand	0.30	0.90
Clean fine-to-coarse sand	0.20	0.95
Micaceous sand	0.40	1.2
Silty sand and gravel	0.14	0.85

4.25 English)

(4.25 SI)

TABLE 11.3 TYPICAL CONSOLIDATION PROPERTIES OF SATURATED NORMALLY CONSOLIDATED SANDY SOILS AT VARIOUS RELATIVE DENSITIES (Adapted from Burmister, 1962)

Soil Type	$C_c / (1+e_0)$					$D_r = 100\%$
	$D_r = 0\%$	$D_r = 20\%$	$D_r = 40\%$	$D_r = 60\%$	$D_r = 80\%$	
Medium to coarse sand, some fine gravel (SW)	-	-	0.005	-	-	-
Medium to coarse sand (SW/SP)	0.010	0.008	0.006	0.005	0.003	0.002
Fine to coarse sand (SW)	0.011	0.009	0.007	0.005	0.003	0.002
Fine to medium sand (SW/SP)	0.013	0.010	0.008	0.006	0.004	0.003
Fine sand (SP)	0.015	0.013	0.010	0.008	0.005	0.003
Fine sand with trace fine to coarse silt (SP-SM)	-	-	0.011	-	-	-
Fine sand with little fine to coarse silt (SM)	0.017	0.014	0.012	0.009	0.006	0.003
Fine sand with some fine to coarse silt (SM)	-	-	0.014	-	-	-

For saturated overconsolidated sands, $C_c / (1+e_0)$ is typically about one-third of the values listed in Table 11.3, which makes such soils nearly incompressible. Compacted fills can be considered to be overconsolidated, as can soils that have clear geologic evidence of preloading, such as glacial tills. Therefore, many settlement analyses simply consider the compressibility of such soils to be zero. If it is unclear whether a soil is normally consolidated or overconsolidated, it is conservative to assume it is normally consolidated.

Very few consolidation tests have been performed on gravelly soils, but the compressibility of these soils is probably equal to or less than those for sand, as listed in Table 11.3.

Another characteristic of sands and gravels is their high hydraulic conductivity, which means any excess pore water drains very quickly. Thus, the rate of consolidation is very fast, and typically occurs nearly as fast as the load is applied. Thus, if the load is due to a fill, the consolidation of these soils may have little practical significance.

However, there are at least two cases where consolidation of coarse-grained soils can be very important and needs more careful consideration:

- 1. Loose sandy soils subjected to dynamic loads, such as those from an earthquake.** They can experience very large and irregular settlements that can cause serious damage. Kramer (1996) discusses methods of evaluating this problem.

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secondary compression and occurs under a constant effective stress. We don't fully understand the physical basis for secondary compression, but it appears to be due to particle rearrangement, creep, and the decomposition of organics. Highly plastic clays, organic soils, and sanitary landfills are most likely to have significant secondary compression. However, secondary compression is negligible in sands and gravels.

The *secondary compression index*, C_α , defines the rate of secondary compression. It can be defined either in terms of either void ratio or strain:

$$C_\alpha = - \frac{de}{d \log t} \tag{11.26}$$

$$\frac{C_\alpha}{1 + e_p} = \frac{d\epsilon_z}{d \log t} \tag{11.27}$$

where:

- C_α = secondary compression index
- e = void ratio
- e_p = void ratio at end of consolidation settlement (can use $e_p = e_0$ without introducing much error)
- ϵ_z = vertical strain
- t = time

Design values are normally determined while conducting a laboratory consolidation test. The consolidation settlement occurs very rapidly in the lab (because of the short drainage distance), so it is not difficult to maintain one or more of the load increments beyond the completion of consolidation settlement. The change in void ratio after this point can be plotted against log time to determine C_α .

Another way of developing design values of C_α is to rely on empirical data that relates it to the compression index, C_c . This data is summarized in Table 11.4.

TABLE 11.4 EMPIRICAL CORRELATION BETWEEN C_α AND C_c (Terzaghi, Peck, and Mesri, 1996)

Material	C_α/C_c
Granular soils, including rockfill	0.02 ± 0.01
Shale and mudstone	0.03 ± 0.01
Inorganic clays and silts	0.04 ± 0.01
Organic clays and silts	0.05 ± 0.01
Peat and muskeg	0.06 ± 0.01

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13.4 SHEAR STRENGTH OF SATURATED SANDS AND GRAVELS

Little or no excess pore water pressure occurs in clean sands and gravels under static loading conditions because their hydraulic conductivities are so high. If changes in the normal or shear stresses cause the voids to expand or contract, water easily flows in or out as necessary. Therefore, the pore water pressure, u , is equal to the hydrostatic pore water pressure (Equation 7.7) and shear strength analyses may be based on effective stresses.

Determining c' and ϕ'

If no cementing agents or clay are present, saturated sands and gravels should have $c' = 0$. We determine the friction angle, ϕ' , by conducting field or laboratory tests, as discussed later in this chapter. Figure 13.11 presents typical ϕ' values, which may be used for preliminary estimates or for checking test data.

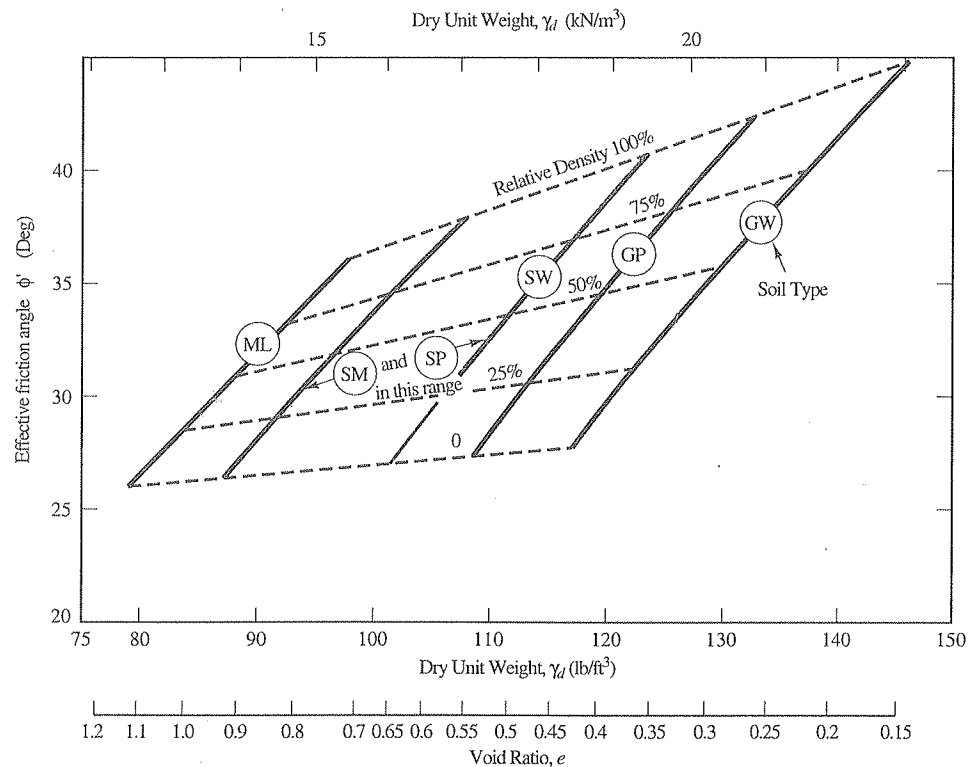


Figure 13.11 Typical ϕ' values for cohesionless soils without clay or cementing agents (Adapted from U.S. Navy, 1982).

Notice how the ϕ' values in Figure 13.11 increase as the unit weight increases. This is one of the reasons for compacting soils. This figure also illustrates that gravels are

(13.14)

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(13.15)

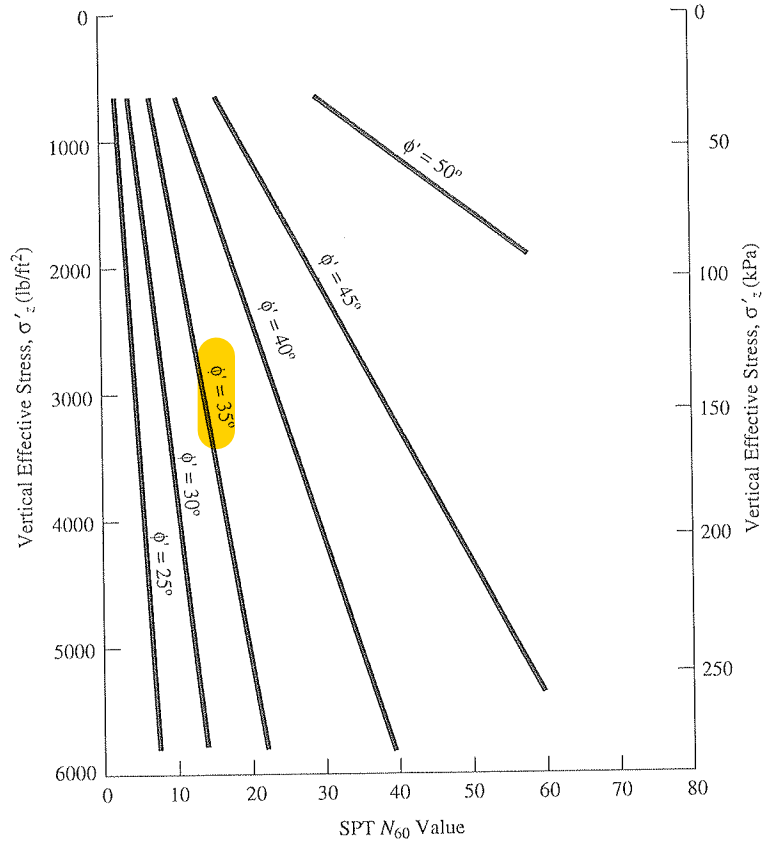


Figure 13.36 Empirical correlation between N_{60} and ϕ' for uncemented sands (Adapted from DeMello, 1971).

Cone Penetration Test

The cone penetration test (CPT), described in Chapter 3, also may be used to determine ϕ' , as shown in Figure 13.37.

13.9 SHEAR STRENGTH AT INTERFACES BETWEEN SOIL AND OTHER MATERIALS

Many geotechnical construction projects use geosynthetic materials embedded into the soil for various purposes (Koerner, 1998). For example, geogrids can be used to reinforce the soil, geotextiles can be used for filtration, and geomembranes can be used to provide impervious barriers. Sometimes the shear strength along the interface between these materials and the adjacent soil becomes important and needs to be evaluated. This strength may be measured in the laboratory using a device similar to a direct shear machine.

13.36 presents an
ion angle, ϕ' , in

Shear strength and consolidation properties of Municipal Solid Waste

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Introduction

In recent years there has been an increasing interest on waste management and disposal. When municipal solid waste (MSW) is concerned, landfills are the most used method of final waste disposal. Apart from environmental aspects, an urban waste landfill must be seen as an engineering structure which must satisfy the usual safety requirements, concerning stability against failure, deformation, stress imposed to foundation soil and the behavior of accessory elements, such as liner and gas and leachate drains. This paper summarizes some aspects of mechanical properties of MSW as measured in laboratory and is a synthesis of the paper by Vilar and Carvalho (2004). These tests were part of a research program performed at the University of São Paulo intended to characterize the behavior of MSW and landfill that encompassed field and laboratory tests, the construction of an experimental landfill and the modeling and analysis of landfill based on test results and results from monitoring.

Characterization of MSW

The tested MSW was recovered from a selected area (Test Area), nearly 450 m², located at Bandeirantes Sanitary Landfill, in the city of São Paulo (Brazil). The waste in this area is about fifteen years old and it was chosen considering the facilities and the existing instruments (gas and leachate piezometers and superficial markers for settlement control). Field penetration tests (SPT and CPT) indicate some large values of strength that were associated to materials such as stone, rubber and wood. So larger values were associated to more resistant materials and were not considered representative of matrix waste. Excluding the values higher than 20 blows, the average SPT is about 7 blows for superficial layers (0-15m depth). It reaches about 12 blows for deeper layers (between 15 and 30m in depth). Tip and lateral resistance of CPT tended also to increase with depth and typical values varied between 2500 - 7500 kPa and 100 - 400 kPa, respectively.

Infiltration tests showed highly variable rates of flow but typical values of refuse coefficient of Hydraulic conductivity vary between 10⁻⁴ cm/s and 10⁻⁶ cm/s.

Refuse sampling was done using a continuous auger 40cm diameter. The gradation curve for the collected waste sample was determined through sieving of dry waste and direct measurement of components larger than 50 mm. In the sieving, plastic materials and textiles, which represent almost 20% of the total sample, were excluded. So, it was found that around 35% of the components present grain size larger than 20mm and approximately 50%, between 20 and 2mm. The waste sample used in strength and compression tests presented the following average physical composition: wood (4%), paper (2%), plastic (17%), textile (3%), metal (5%), glass (2%), rubber (2%), stone (10%) and past organic (55%) from which 12% is organic mater and 43%, ash.

International Workshop « Hydro-Physico-Mechanics of Landfills »

LIRIGM, Grenoble 1 University, France, 21-22 March 2005

Triaxial compression tests

A large triaxial cell was constructed to test samples with 300 or 400mm of height and 150 or 200mm of diameter. Laboratory testing has been conducted using deformed waste samples that were molded to unit weights of 10, 12 and 14 kN/m³. Samples were molded keeping their natural moisture content (in the range between 60 and 68%) and some of them were saturated by back-pressure. Consolidated drained (CD) triaxial tests and undrained tests (CU) were performed at a strain rate of 0.7%/min and effective confining pressures of 100, 200 and 400 kPa. Typical stress- strain curves for CD tests and natural water content waste samples are presented in Figure 1. The results show what can be seen as a typical behavior of urban waste like materials, as reported by many authors. Deviator stress increases with axial strain almost continuously, without any peak in the stress-strain curve or without reaching an ultimate value. The stress path followed in these tests plotted in s',t plane are shown in Fig. 2.

As failure cannot usually be clearly defined when testing MSW and considering the need for an operational shear strength envelope to deal with typical problems such as slope stability, a reliable procedure is to use Mohr-Coulomb criterion related to some value of strain. This is done in Fig. 2, where shear strength envelopes for axial strains of 10, 20 and 30% are included. As can be seen the mobilized shear strength tends to increase with strain. For instance, for the natural moisture specimens and unit weight of 12kN/m³, cohesion departed from 20 kPa for 10% strain and increased to 71 kPa, for 30% strain. For the same strains the friction angle increased from 22° to 33°. However the mobilization of shear strength parameters does not follow the same pattern. The cohesion has relatively initial low values and tends to increase continuously at an increase rate that is much more pronounced than that of friction angle. The friction angle on the other side has relatively initial higher values and their rate of increase suggest that it will tend to reach an ultimate value as strain increases.

The large cohesion intercepts calculated have been credited to the fibre components (specially plastics) that provides a reinforcement mechanism (König and Jessberger 1997) responsible, for instance, for the existence of vertical cuts as high as 4m in the tested waste in the field. Among the factors studied, it was found that the unit weights used in the test program (10, 12 and 14 kN/m³) showed little influence in shear strength, probably because the corresponding dry unit weight varied little between the samples. Similarly, it was noticed that the results from specimens molded at natural moisture content (degree of saturation of about 70%) did not show appreciable differences from the saturated ones. It is believed that the large volume decrease during consolidation and the high initial degree of saturation of natural moisture samples led both samples to similar conditions during shear justifying the closer results.

Confined compression tests

Confined compression tests of waste have been carried out using a oedometer with 385 mm diameter and 365 mm height. The waste sample was placed in the oedometer in 6 layers and lightly pressed to reach the desired initial unit weight. Loading was applied in stages that lasted about 15 days although some stages were prolonged a few days to define adequately the creep characteristics of the waste. Figures 3a shows typical curves of deformation against time. In this figure it can be observed an accentuated secondary compression process (creep).

These data allowed to calculate the secondary compression index C_α ($C_\alpha = \Delta e / \Delta \log t$) and C'_α ($C'_\alpha = C_\alpha / (1 + e_0)$) for each stage of loading. C_α ranged between 0.021 and 0.044, with an average value of 0.032, while C'_α varied in a narrow range, between 0.012 and 0.016. The overburden stress did not influence both coefficients in the range between 100 and 640kPa, although a slight decrease in C_α was observed as the stress increased.

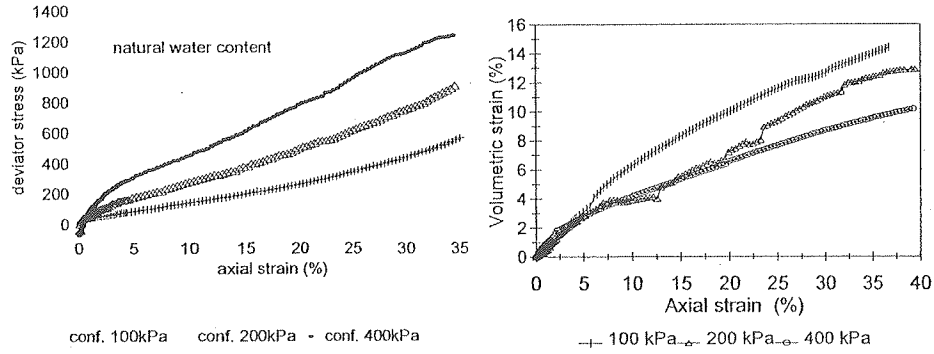


Figure 1. Typical stress - strain curves of waste from triaxial compression tests (CD). Unit weight: 12 kN/m³ and molding water content: 67%.

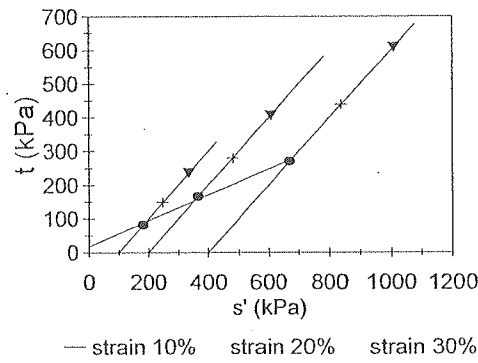


FIG. 2 — Stress paths and shear strength envelopes for different strain of MSW with unit weight of 12kN/m³. Natural moisture content sample.

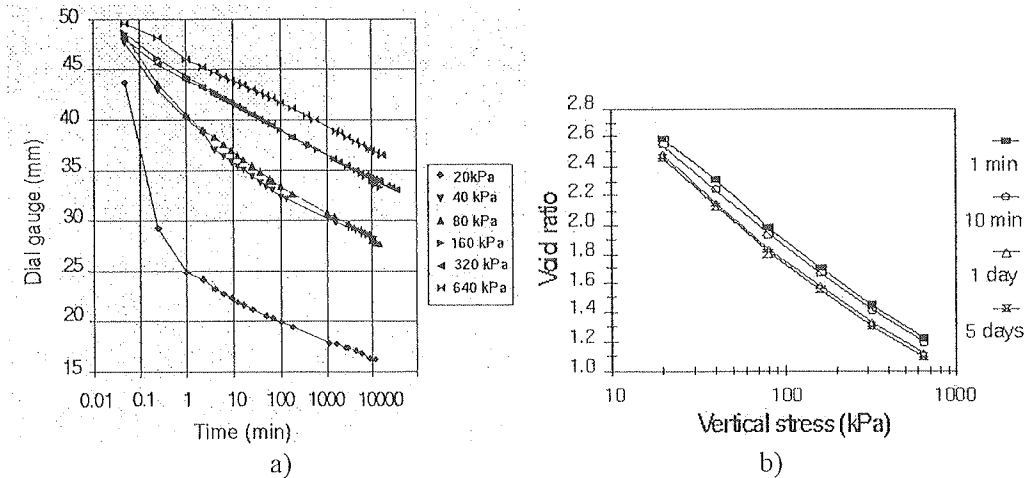


Fig. 3 — Results of consolidation tests of sample T1B molded with unit weight of 10 kN/m³. Specimen prepared at natural moisture content (68%).

As an equilibrium void ratio was not attained in the time spans used in the tests, some curves for different times of loading were plotted. Fig. 3b illustrates some compression curves of specimens molded with initial unit weight of 10 kN/m^3 . The curves were almost parallel but not straight. They were rather slightly concave upwards, however straight curves were fitted to points between 60 and 640 kPa allowing to obtain the primary compression index $[C_c = \Delta e / \Delta \log(\sigma)]$. C_c varied between 0.52 and 0.92, however the normalized compression coefficient $C'_c [C'_c = C_c / (1 + e_0)]$ obtained were very close, averaging 0.21.

Conclusions

Typical results of consolidation and triaxial compression tests on large remolded specimens of a 15 year old sample of Municipal Solid Waste (MSW) were shown. Due to space limitations, many of the analysis regarding the whole set of results could not be presented, but the general behavior of the waste can be summarized as follows:

1. Consolidation tests showed the large compressibility of MSW where secondary compression plays a fundamental role.

2. Secondary compression index, normalized through e_0 , C'_c ($C'_c = C_c / (1 + e_0)$) showed a small range of variation and yielded an average value of 0.013. Primary compression index (C_c) showed to be also dependent on e_0 and normalized $C'_c [C'_c = C_c / (1 + e_0)]$ reached about 0.21.

3. Drained triaxial compression tests showed stress-strain curves that were concave upwards and failure or an ultimate value of deviator stress could not be reached even for large values of strain, above 30%. The shear strength parameters are strain dependent and tended to increase with the deformation.

4. For the same unit weight, the shear strength was little affected by the degree of saturation of the tested specimens. The relatively high initial degree of saturation of the unsaturated specimens (above 70%) and the large volume compression during consolidation probably raised the degree of saturation approaching the saturated condition thus yielding close values of shear strength.

5. The shear strength parameters obtained from specimens with different unit weights presented small variation and an average shear strength envelope could fit the results of the samples in the range of unit weight studied ($10, 12, 14 \text{ kN/m}^3$).

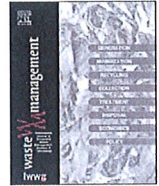
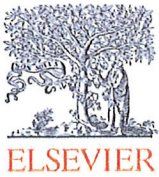
6. Effective stress shear strength parameters from CU tests were misleading and did not agree with the parameters from CD tests. Undrained shear strength was proportional to the effective confining pressure and the relationship between these variables was larger than that usually observed in soils.

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Constitutive model for municipal solid waste incorporating mechanical creep and biodegradation-induced compression

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ABSTRACT

A constitutive model is proposed to describe the stress–strain behavior of municipal solid waste (MSW) under loading using the critical state soil mechanics framework. The modified cam clay model is extended to incorporate the effects of mechanical creep and time dependent biodegradation to calculate total compression under loading. Model parameters are evaluated based on one-dimensional compression and triaxial consolidated undrained test series conducted on three types of MSW: (a) fresh MSW obtained from working phase of a landfill, (b) landfilled waste retrieved from a landfill after 1.5 years of degradation, and (c) synthetic MSW with controlled composition. The model captures the stress–strain and pore water pressure response of these three types of MSW adequately. The model is useful for assessing the deformation and stability of landfills and any post-closure development structures located on landfills.

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1. Introduction

Proper assessment of the stability of municipal solid waste (MSW) landfill slopes has become extremely important in light of the recent failure of several landfills in the United States and worldwide (Koerner and Soong, 2000; Stark et al., 2000; Blight and Fourie, 2005). In addition, the reasonable prediction of MSW settlement has also become important in order to ascertain the stability of landfill appurtenances (e.g., gas collection pipes, leachate collection pipes, etc.) and to plan and design post-closure redevelopment projects. Limit equilibrium methods have been routinely used to analyze landfill slopes and the adopted soil mechanics-based settlement methods have been commonly used to calculate the settlement of MSW (Sharma and Reddy, 2004). These stability and settlement analysis methods require the mechanical properties of MSW such as unit weight, shear strength parameters, and compressibility parameters (compression and recompression ratios, secondary compression ratio). Despite heterogeneous composition, several studies have attempted to quantify the mechanical properties of MSW based on laboratory and field testing programs as well as back-analysis (or inverse analysis) using landfill field performance data (Kavazanjian and Matasovic, 1995; Zekkos, 2005).

The compressibility and shear strength behavior of MSW has drawn the attention of several researchers. For example, Sowers

(1973), Yen and Scanlon (1975), Edil et al. (1990), Landva and Clark (1990), Grisolia et al. (1991), Gabr and Valero (1995), Wall and Zeiss (1995), Park and Lee (1997), Ling et al. (1998), Sivakumar Babu (1999), El-Fadel and Khoury (2000), Park et al. (2002, 2007), Marques et al. (2003), Oweis (2006), Gangathulasi (2008), and Reddy et al. (2009a,b) studied the compression response of MSW and/or proposed different approaches to predict immediate compression and time dependent compression under load. Marques et al. (2003) presented a composite model for compressibility considering three important mechanisms; instantaneous compression in response to applied load, secondary mechanical creep, and time dependent biological decomposition. They illustrated the use of the model consisting of five parameters such as compression index and coefficients and rate constants for both mechanical creep and biodegradation, respectively, to predict the settlement of MSW landfill. Sharma and De (2007) presented methods for estimating settlements of MSW landfills, including bioreactor landfills involving leachate recirculation for enhanced waste degradation, in terms of primary and secondary coefficients and demonstrated their application to post-closure maintenance and development plans of landfills.

Singh and Murphy (1990), Jessberger and Kockel (1993), Kavazanjian and Matasovic (1995), Manassero et al. (1996), Sivakumar Babu (1998), Vilar and Carvalho (2002), Zekkos (2005) and Reddy et al. (2009a,b) studied the shear strength response of MSW. Several investigators evaluated the stability of landfill slopes based on limit equilibrium methods using the drained Mohr–Coulomb shear strength parameters of MSW (e.g., Reddy et al., 1999; Stark et al., 2000).

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Table 3
Properties of the three types of MSW tested in this study.

Type of waste	Average initial moisture content (%)		Average bulk wet density (kg/m ³)	Average specific gravity
	Dry weight basis	Wet weight basis		
Fresh MSW	43.7	30.4	740	0.85
Landfill MSW	43.3	30.2	1020	0.97
Synthetic MSW	134.6	57.3	1150	1.09

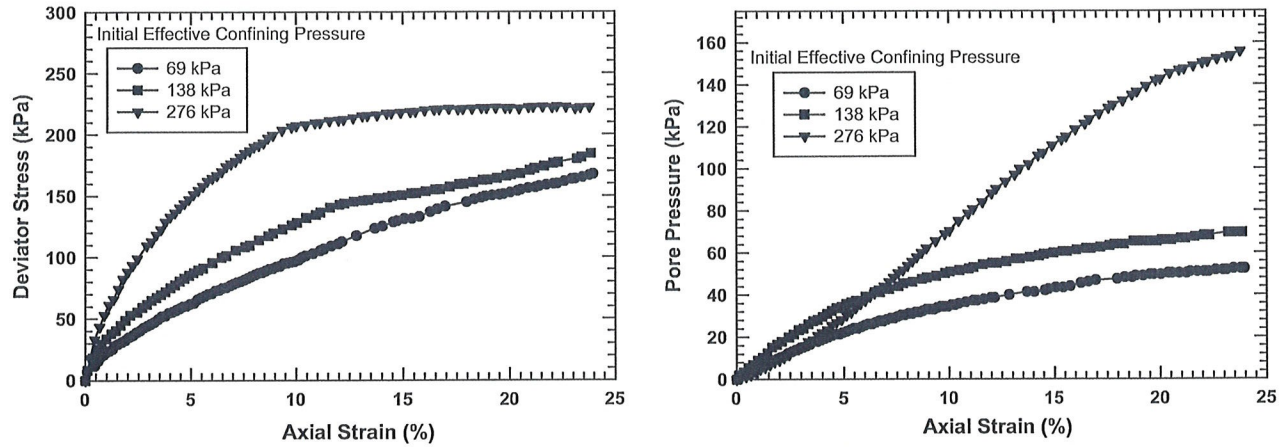


Fig. 5. Typical triaxial consolidated undrained shear test results for landfilled MSW.

ically consolidated under different confining pressures of 69, 138, and 276 kPa and volume change was measured. The MSW samples were finally subjected to shear under undrained condition. Pore water pressures were measured during shearing. To ensure uniform pore pressures throughout the specimen, samples were sheared at a constant strain rate (approximately 1% per minute). Typical results for landfilled MSW are shown in Fig. 5. Reddy et al. (2009a–c) provide the results for fresh MSW and synthetic MSW and also discuss all of the test results.

4. Results and discussion

4.1. Model parameters

The proposed model requires the same parameters as the modified cam clay model. These parameters are: frictional constant (M), compression index (λ), recompression or swelling index (κ), and over-consolidation ratio, which is defined as the ratio of pre-consolidation pressure to mean effective stress ($OCR = p'_0/p'$). In addition to these modified cam clay model parameters, the implementation of the proposed constitutive

Table 5
Additional parameters to account for mechanical creep and biodegradation in the proposed model.

Type of waste	b (m ² /kN)	c (day ⁻¹)	E_{dg}	d (day ⁻¹)
Fresh MSW	0.0081	0.0105	0.160	0.0315
Landfilled MSW	0.0046	0.0127	0.116	0.0243
Synthetic MSW	0.0095	0.0420	0.210	0.0600

model requires four additional parameters. These are: two parameters representing the mechanical creep, namely coefficient of mechanical creep (b) and rate constant (c); and two other parameters representing the biodegradation effect given by total amount of strain (E_{dg}) and rate constant (d), which are functions of biodegradation.

The compression and swelling indices (λ and κ parameters) are obtained from compression test results shown in Fig. 3. The normal pressure vs. axial strain data is replotted as normal pressure vs. void ratio and the slope of the compression line on this plot gives the value of λ . Compression testing did not include an unload and reload cycle; but, the literature shows that recompression index

Table 4
Modified cam clay model parameters.

Type of MSW	Confining pressure (kPa)	ϕ'	Void ratio (e)	λ	κ	G	M	OCR
Fresh MSW	69	16	0.61	0.12	0.012	4274	0.61	1
	138	16	0.65	0.12	0.012	8779	0.61	1
	276	16	0.23	0.12	0.012	13206	0.61	1
Landfilled MSW	69	23	0.46	0.091	0.0091	5092	0.90	1
	138	23	0.42	0.091	0.0091	10303	0.90	1
	276	23	0.50	0.091	0.0091	20997	0.90	1
Synthetic MSW	69	10.3	0.47	0.175	0.0175	2672	0.38	1
	138	10.3	0.51	0.175	0.0175	8287	0.38	1
	276	10.3	0.36	0.175	0.0175	14871	0.38	1

The Impact of Degradation on MSW Shear Strength

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ABSTRACT: Multiple lines of evidence are used to draw conclusions concerning the shear strength of degraded municipal solid waste (MSW). Interpretation of the accumulated evidence leads to the conclusion that for conventional “dry” landfills the shear strength envelopes commonly used in practice provide reasonable strength values for circular and non-horizontal failure surfaces even for old landfills where the waste may be in a relatively advanced state of degradation. However, for horizontal failure surfaces in conventional landfills that may not engage the fibrous waste components, for landfills where mechanical pre-processing has been used to reduce waste constituent size and facilitate degradation, and for bioreactor landfills, a strength envelope characterized by zero cohesion and an effective friction angle of 30 degrees appears to provide a conservative representation of degraded MSW shear strength.

INTRODUCTION

The impact of degradation on the shear strength of municipal solid waste (MSW) is an important consideration in the landfill design. Concerns that degradation reduces the shear strength of MSW must be addressed when considering the long term stability of a conventional landfill. Furthermore, degraded MSW shear strength may govern both short and long term stability of bioreactor landfills. Recent studies suggesting that the degraded shear strength of MSW may be lower than the shear strengths typically employed in landfill design practice give cause for a critical reevaluation of available data on the shear strength of degraded MSW.

MSW SHEAR STRENGTH ENVELOPES USED IN PRACTICE

Two of the most common MSW shear strength envelopes used in practice are the envelopes recommended by Kavazanjian et al. (1995) and Eid et al. (2000). Kavazanjian et al. (1995) recommended a MSW shear strength envelope represented by a cohesion of 24 kPa at low normal stresses and by a friction angle of 33 degrees at high normal stresses. Eid et al. (2000) provided lower bound, median, and upper bound recommendations for MSW shear strength. The lower bound envelope was

represented by a friction angle of 35 degrees, the median envelope was represented by a cohesion of 25 kPa and a friction angle of 35 degrees, and the upper bound envelope was represented by a cohesion of 50 kPa and a friction angle of 35 degrees. The Kavazanjian et al. (1995) bi-linear envelope, based in part on back analysis of stable slopes at existing landfills that are now over 30 year old, may be inferred to include at least some degraded waste and explicitly characterized as a lower bound envelope for all ages of waste. However, Kavazanjian et al. (2001b) suggest that it may not be prudent to include the low normal stress cohesion component of this bi-linear envelope when considering the strength of degraded MSW. As the Eid et al. (2000) back analysis of landfill slopes may also be inferred to include degraded waste, the Eid et al. (2000) lower bound envelope may similarly be considered an indicator of degraded waste shear strength. Thus based on the Kavazanjian et al. (1995) and the Eid et al. (2000) strength envelopes, the shear strength of degraded MSW may be interpreted to be characterized by a friction angle of 33 degrees.

OII LANDFILL SHEAR STRENGTH

The Operating Industries, Inc. (OII) landfill is a predominantly MSW landfill located approximately 20 km east of downtown Los Angeles. Waste was placed in the OII landfill between the late 1940's and 1983. The north slope of the south parcel (NSSP) at OII rises 60 m above grade at a typical inclination of 1.5 horizontal to 1 vertical (1.5H:1V) approximately and were as steep as 1.3H:1V prior to remedial construction in the late 1990s. Waste liquids, including organic solvents, were co-disposed of with MSW at the west end of the OII site from the mid 1970's until 1983. As a result of this co-disposal, much of the waste at the west end of the site is extremely degraded, with samples recovered from depths of 5 m to 30 m described as being composed of greater than 80 percent by weight black, highly organic soil and soil-like material (GeoSyntec, 1996). Based upon the angle of repose alone, the 1.5H:1V slopes on the NSSP suggest a secant friction angle of at least 33 degrees.

The results of direct shear tests on 454 mm-diameter reconstituted specimens reported by Kavazanjian, et al. (1999), conducted as part of the remedial action program at the site, are presented in Figure 1. The numbers next to the data points in Figure 1 represent the percentage of un-degraded waste ("refuse") in the corresponding specimen. The results in Figure 1 suggest that MSW shear strength decreases as the amount of un-degraded waste decreases (i.e. as waste degradation increases). However, the lower bound strength envelope developed using specimens with less than 16 percent and as little as 8 percent refuse, presumed to represent fully degraded MSW, corresponds to a cohesion of 43 kPa and a friction angle of 26 degrees. The secant friction angle (the friction angle calculated assuming zero cohesion) for the point with the highest normal stress on the lower bound envelope (approximately 950 kPa), corresponding to the specimen with 8% refuse and 92% soil and soil-like material, is equal to 28 degrees.

After including the results of direct shear tests and other available data, a shear strength represented by zero cohesion and a friction angle of 30 degrees was adopted for "planes of weakness" in the OII landfill, presumed to represent either continuous layers of daily and intermediate cover soil or soil-like material (e.g. degraded waste)

Compression Indices for Full-Scale Landfills

Date: July 8, 2008

Completed by: Christopher Bareither

Compilation of primary and secondary compression indices from literature

Reference	Origin of Waste	Type of Waste ¹	Comments	Test Surface Area (m ²)	Surcharge Thickness (m)	Waste Thickness (m)	Average C _c '	Average C _α '
Andersen et al. (2004)	Everett Landfill, Washington State	MSW	Averages from extensometer data based on authors interpretation	—	8.64	5.54	0.2	0.027
Bachus et al. (2007)	Tennessee	MSW	Settlement plates on surface monitored for 6 months after construction of surface surcharge	42,100	3	18	0.33	0.010
								0.008
								0.008
								0.008
			0.010					
			0.015					
			0.007					
			0.010					
Settlement plates at various depths in landfill monitored for 6 months after construction of surface surcharge, waste thickness not reported	42,100	3	—					
			—					
			—					
			—					
10,525	6	—						
		—						
		—						
		—						
4,210	9	—						
		—						
		—						
		0.027						
Boutwell & Fiore (1994)	—	Filled early 1970s, left uncovered; MSW (Saturated)	Surcharge placed on unlined MSW/Industrial pit, settlement plates used to monitor surface settlement	100	1	5	0.09	0.007
				100	2	5	0.16	0.009
				100	3	5	0.19	0.012
Bowders and Mitchell (2005)	Columbia, Missouri	MSW - filled from 1986 - 1994	Settlement plate installed 7/23/1999; overburden constant for C _α ' calc	—	1.2192	18	—	0.013
		MSW - filled from 1994 - 1998	Settlement plate installed 1/17/2002; overburden constant for C _α ' calc	—	11	19	—	0.106
Coduto and Huitric (1990)	Spadra Landfill, Pomona, CA	MSW filled from 1976-1978	Monitored settlement at various depths in landfill with Sondex Tube; instruments installed May 1987, monitored through May 1989; report surface C _α ' here	—	—	16	—	0.022
				—	—	22	—	0.021
Deutsch et al. (1994)	Honey Brook, PA (Lanchester)	MSW - newspapers readable to 1970s	—	—	—	9	0.22	—
Dotl et al. (1987)	Collier Road Landfill; Pontiac, MI	MSW and Industrial	Waste filled 1960's to 1985 (capped); 13 acre fill on top of landfill placed 1/1/1985 - 7/1/1986;	—	11	15	—	0.048
Sharma and De (2007)	Northern California	MSW ~ 15 years old	Dump since 1930's; sanitary landfill since 1969; 500,000 m3 fill places on toe of landfill; originally in Sharma et al. (1999)	—	—	16	—	0.020
	San Francisco Bay Area	Commercial	Filled 1956 - 1980s; compression under self weight of waste; originally in Sharma (2000)	—	—	6	—	0.220
	San Francisco Bay Area	Commercial	Filled 1956 - 1980s; compression under self weight of waste; originally in Sharma (2000)	—	—	36	—	0.190

	Southern California	Household and commercial	Filled 1976-1978; used sondex tubes - details in Coduto and Huitric (1990); originally in Sharma (2000)	---	---	16	---	0.280
	Southern California	Household and commercial	Filled 1976-1978; used sondex tubes - details in Coduto and Huitric (1990); originally in Sharma (2000)	---	---	23	---	0.270
Stulgis et al. (1995)	Massachusetts	All Types	Filled 1954 - 1985; ~ 30-50 yrs old in lowest 1/3 and 13-30 yrs old in upper 2/3	32	6.1	21	0.16	0.02
Watts and Charles (1990)	Brogborough (England) - filled 1983 - 1987	Domestic	Monitored settlement at various depths using magnetic extensometer; report active biodegradation during settlement monitoring	---	---	11	---	0.100
	Calvert (England) - filled 1979-1988			---	---	20	---	0.230
Yuen and Styles (2000)	Melbourne, Australia	Composition (dry mass basis) = 47% inert (C&D, soil, etc.), 12% paper, 9% plastic, 23% yard and food + some textile, metal and glass	Control Cell: 150 mm daily cover and 300 mm intermin cover every 2 m of waste (clayey sandy silt), approximately 15% of total volume	36,328	---	7	---	0.019
					---	13	---	0.041
					---	20	---	0.03
					---	43	---	0.028
Zamiskie et al. (1994)	Syosset landfill - Oyster Bay, NY	C&D, minor MSW, ~ 66% soil	Operated from 1936-1976	---	2.75	17	0.03	0.0035

Summary statistics	Average C_c'	Average C_a'
Average	0.172	0.051
Maximum	0.330	0.280
Minimum	0.025	0.004
Coefficient of Variation	0.525	1.529

J.2 – Shear Strength Evaluations

- J.2-A Final Cover Critical Interface
- J.2-B Liner/LCS Prior to Waste Placement
- J.2-C Liner LCS After Waste Placement
(Global Stability Analysis)
- J.2-D Rapid Drawdown of Detention Basin (Global
Stability Analysis)
- J.2-E Summary of Acceptable Shear Strength Values

J.2-A Final Cover Critical Interface



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Problem Statement

Determine the range of acceptable final cover shear peak strength parameters that provide a factor of safety against slope failure of at least 1.5 for static conditions and 1.3 under seismic conditions, in accordance with 35 Ill. Admin. Code Section 811.304 (d).

Background

The material strengths of the protective soils are based on laboratory testing. As geosynthetics are manufactured products, they can be designed and/or selected to meet strength requirements. The stability of the final cover is influenced by the interface shear strengths between geosynthetic components.

This calculation is developed to identify the lowest peak shear strength that produces acceptable factors of safety for any potential interface within the final cover. The most critical failure surface will be located between one of the following interfaces:

- Low permeable soil layer vs. textured geomembrane;
- Textured geomembrane vs. geocomposite; or
- Geocomposite vs. protective soil cover layer.

It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in this calculation.

Given

- Koerner, R.M., *Designing with Geosynthetics*. Prentice Hall, Fifth Edition (refer to attached pages).
- Edil, T.B., "Seepage, Slopes, and Dams." University of Wisconsin-Madison (refer to attached pages).
- Caterpillar Product Brochure: "D8T – Track Type Tractor" (refer to attached pages).
- Landfill design specifications for layer types, thicknesses, and material properties (refer to **Appendix J.1**).
- USGS National Seismic Hazard Mapping Project found on the internet / online at <http://earthquake.usgs.gov/research/hazmaps/design/> (A value of 0.0461g for horizontal acceleration — please refer to **Appendix J.1**).
- Cross-sectional sketches of the final cover system and force diagrams of analyses (refer to attached pages).



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Assumptions

- The final cover system includes the following components, from top to bottom:
 - 3-foot vegetative cover (0.5-foot vegetative support layer and 2.5-foot protective cover layer)
 - Double sided geocomposite drainage layer
 - 40-mil LLDPE geomembrane
 - 2-foot compacted low permeable soil layer ($k \leq 1 \times 10^{-5}$ cm/sec).
- The maximum slope of the final landform is 4H:1V, therefore $\beta = 14.04$ degrees (refer to sketch).
- The final cover is conservatively assumed to be saturated.
- The protective cover soil layer and the final cover barrier soil layer of the final cover system are assumed to have a saturated density, $\gamma_{sat} = 130.3$ pcf.
- Maximum horizontal acceleration = 0.0461g (see **Appendix J.1**).
- $\alpha = 2.64$ degrees (refer to sketch).
- Pore pressure remains unchanged during earthquake conditions.
- The stability of the final cover system will depend on interface peak shear strengths between final cover system components. The most critical failure surface will be located between one of the following interfaces:
 - Low permeable soil layer vs. textured geomembrane;
 - Textured geomembrane vs. geocomposite; or
 - Geocomposite vs. protective soil cover layer.

It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in this calculation.

- Internal friction angle of the final cover soil, $\phi = 11.8$ degrees.
- Wedges analyzed are shown on attached sketch in the attached pages. Weight per liner foot (W) of each wedge is the cross-sectional area multiplied by the buoyant unit weight. A unit width of 1 was used to determine the cross-sectional area.
- Minimum required factor of safety (FS) = 1.5 (static), 1.3 (seismic), per 35 Ill. Admin. Code Section 811.304 (d).
- A tractor load of 568 pounds per square foot (psf) over a track length of 15.25 feet was included in the calculation to determine the factor of safety under static conditions. It is noted that this is based on a Caterpillar D8T tractor load.



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- ❑ The maximum change in elevation along the steepest slope of the final landform is 139-feet along the 4H:1V portion of the slope. This occurs in the northeast and northwest sections of the horizontal expansion. The final landform slopes vary from 4H:1V along the final landform side slopes to 10H:1V on the final landform plateau.
- ❑ The equations on the following page were used to determine the factors of safety against slope failure (refer to attached sketches).

Static Conditions:

$$E_A = \left[W_A \frac{\sin(\beta - \delta_M)}{\sin(90^\circ - \delta_M)} \right] - (C_a)(L)$$

$$E_{NB} = \left[W_{NB} \frac{\sin(\phi_M)}{\sin(90^\circ - \beta - \phi_M)} \right]$$

$$\Delta E = E_A - E_{NB}$$

Seismic Conditions:

$$E_A = \left[1.0011 W_A \frac{\sin(\beta - \delta_m + \alpha)}{\sin(90^\circ - \delta_m)} \right] - (C_a)(L)$$

$$E_{NB} = \left[1.0011 W_{NB} \frac{\sin(\phi_m - \alpha)}{\sin(90^\circ - \beta - \phi_m)} \right]$$

$$\Delta E = E_A - E_{NB}$$

Where,

E_A	=	Force of Active Block (lb/ft)
E_{NB}	=	Force of Neutral Block (lb/ft)
$W_{A/NB}$	=	Weight of soil blocks (active and neutral) (lb/ft)
β	=	Angle of slope
δ_m	=	Mobilized interface friction angle
ϕ_m	=	Internal soil friction angle
α	=	Resultant angle of seismic force
C_a, C	=	Adhesion, Mobilized Adhesion (psf)
L	=	Length of sliding block (ft)

The factor of safety is adjusted for the next trial based on the difference of the forces acting on the wedges. As shown in the attached reference (Edil), if ΔE is negative, then the assumed factor of safety is too low; if ΔE is positive, then the assumed factor of safety is too high.

Multiple friction angles and adhesion values for the critical interface are evaluated in spreadsheet calculators to determine a window of acceptable values. For demonstrative purposes of this calculation, a friction angle of 21.9 degrees is selected with an associated adhesion of 0 psf.



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TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Saturated Conditions Assumed for Cover Soil
Calculate loading due to CAT D8T tractor (see attached Caterpillar Product Information)

$$\begin{aligned}
 \text{Tractor Weight per Foot} &= \text{Contact Pressure (P)} \times (\text{Length of Tractor}) \\
 &= 568 \text{ psf} \times \left(183.0 \text{ in} \times \frac{1 \text{ ft}}{12 \text{ in}}\right) \\
 &= 568 \text{ psf} \times (15.25 \text{ ft}) \\
 &= 8,662 \text{ lb/ft}
 \end{aligned}$$

Estimate the weight of the active and neutral blocks:

$$\begin{aligned}
 W_A (\text{Seismic}) &= (\text{Area}) \gamma \\
 &= (560.36 \text{ ft})(3 \text{ ft})(130.3 \text{ pcf}) = 219,045 \text{ lb/ft}
 \end{aligned}$$

$$\begin{aligned}
 W_A (\text{Static}) &= (\text{Area}) \gamma + (\text{tractor loading})(\text{tractor length}) \\
 &= (560.36 \text{ ft})(3 \text{ ft})(130.3 \text{ pcf}) + (568 \text{ psf})(15.25 \text{ ft}) = 227,707 \text{ lb/ft}
 \end{aligned}$$

$$\begin{aligned}
 W_{NB} &= (\text{Area}) \gamma \\
 &= (0.5)(12.37 \text{ ft})(3.09 \text{ ft})(130.3 \text{ pcf}) = 2,491 \text{ lb/ft}
 \end{aligned}$$

Calculate δ_M and ϕ_M assuming FS = 1.5:

$$\tan \delta_M = \frac{\tan(\delta)}{FS} \rightarrow \delta_M = \tan^{-1} \left(\frac{\tan(21.9^\circ)}{1.5} \right) = 15.00^\circ$$

$$\tan \phi_M = \frac{\tan(\phi)}{FS} \rightarrow \phi_M = \tan^{-1} \left(\frac{\tan(11.8^\circ)}{1.5} \right) = 7.93^\circ$$

$$C_a = \frac{c}{1.5} = \frac{0 \text{ psf}}{1.5} = 0 \text{ psf}$$

For Static Conditions (with tractor loading), calculate ΔE for a factor of safety of 1.5:

$$E_A = \left[W_A \frac{\sin(\beta - \delta_M)}{\sin(90^\circ - \delta_M)} \right] - (C)(L) = \left[(227,707 \text{ lb/ft}) \left(\frac{\sin(14.04^\circ - 15.00^\circ)}{\sin(90^\circ - 15.00^\circ)} \right) \right] - (0 \text{ psf})(560.36 \text{ ft}) = -3,950 \text{ lb/ft}$$

$$E_{NB} = \left[W_{NB} \frac{\sin(\phi_M)}{\sin(90^\circ - \beta - \phi_M)} \right] = \left[(2,491 \text{ lb/ft}) \left(\frac{\sin(7.93^\circ)}{\sin(90^\circ - 14.04^\circ - 7.93^\circ)} \right) \right] = 371 \text{ lb/ft}$$

$$\Delta E = E_A - E_{NB} = -3,950 \text{ lb/ft} - 371 \text{ lb/ft} = -4,320 \text{ lb/ft}$$

For Seismic Conditions (no tractor loading), calculate ΔE for a factor of safety of 1.5:

$$\begin{aligned}
 E_A &= \left[(1.0011)W_A \frac{\sin(\beta - \delta_M + \alpha)}{\sin(90^\circ - \delta_M)} \right] - (C)(L) \\
 &= \left[(1.0011)(219,045 \text{ lb/ft}) \left(\frac{\sin(14.04^\circ - 15.00^\circ + 2.64^\circ)}{\sin(90^\circ - 15.00^\circ)} \right) \right] - (0 \text{ psf})(560.36 \text{ ft}) = 6,656 \text{ lb/ft}
 \end{aligned}$$



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$$E_{NB} = \left[(1.0011)W_{NB} \frac{\sin(\phi_M - \alpha)}{\sin(90^\circ - \beta - \phi_M)} \right] = \left[(1.0011)(2,491 \text{ lb/ft}) \left(\frac{\sin(7.93^\circ - 2.64^\circ)}{\sin(90^\circ - 14.04^\circ - 7.93^\circ)} \right) \right] = 248 \text{ lb/ft}$$

$$\Delta E = E_A - E_{NB} = 6,656 \text{ lb/ft} - 248 \text{ lb/ft} = 6,408 \text{ lb/ft}$$

Iterate the calculation until the correct factor of safety is achieved, as shown in the following table. The correct factor of safety will be achieved when ΔE equals zero.

STATIC CONDITIONS						
FS	δ_M	ϕ_M	C_a	E_A	E_{NB}	ΔE
1.8	12.59	6.62	0.0	5,904	307	5,597
1.7	13.30	7.01	0.0	3,022	326	2,696
1.65	13.69	7.22	0.0	1,432	336	1,096
1.60	14.10	7.44	0.0	-246	347	-592
1.5	15.00	7.93	0.0	-3,950	371	-4,320
1.4	16.02	8.49	0.0	-8,185	398	-8,583

SEISMIC CONDITIONS						
FS	δ_M	ϕ_M	C_a	E_A	E_{NB}	ΔE
1.5	15.00	7.83	0.0	6,656	248	6,408
1.4	16.02	8.49	0.0	2,628	275	2,353
1.35	16.58	8.80	0.0	399	290	109
1.30	17.18	9.13	0.0	-2,003	307	-2,310
1.2	18.52	9.88	0.0	-7,425	344	-7,769
1.1	20.07	10.75	0.0	-13,805	388	-14,193



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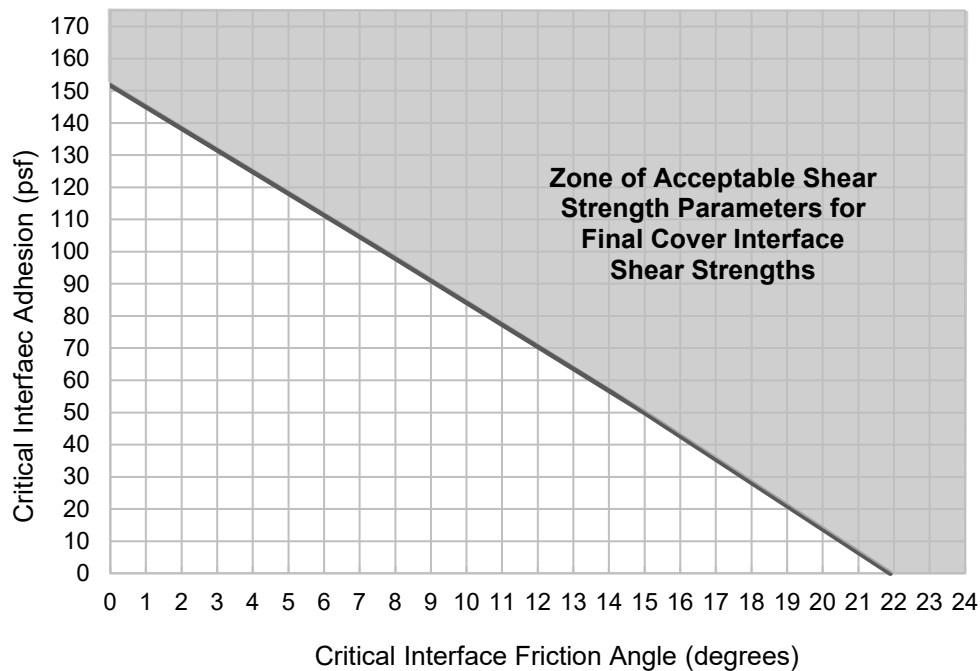
Date: 05/2022

TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Results

Multiple combinations of friction angles and adhesions were evaluated to determine the minimum acceptable interface peak shear strength envelope to achieve stability of the final cover using the calculation methodology described in the previous text. Spreadsheets of values used to generate this curve are attached.

The acceptable combinations of peak shear strength values are shown in the graph below. It is noted that the grey line identifies the zone of acceptable peak shear strength values for the final cover system and was obtained using this analysis.





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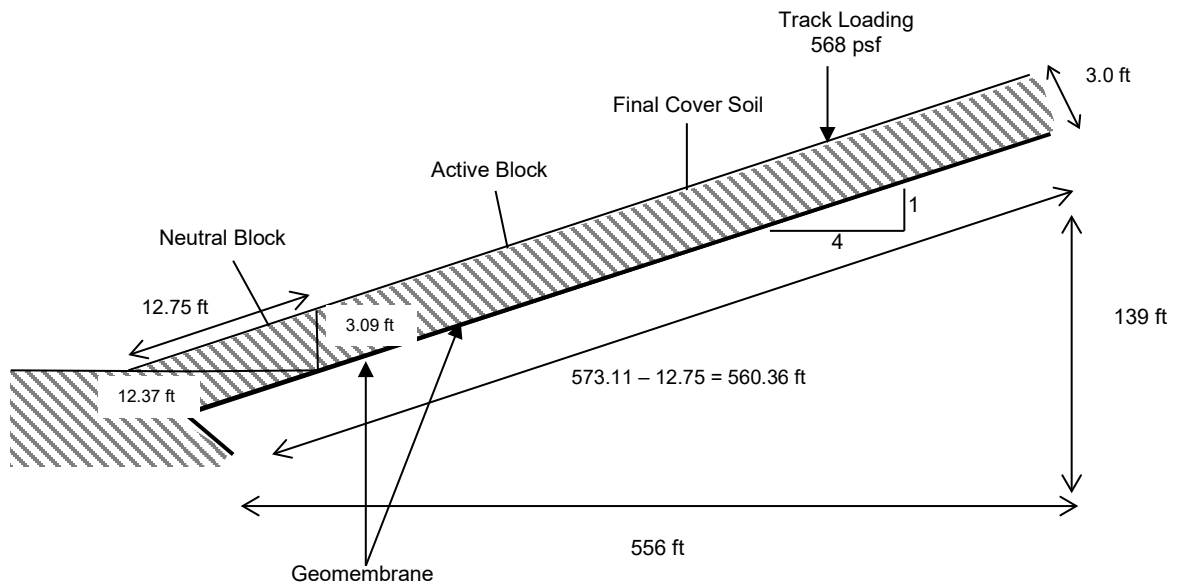
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TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Cross Section of the Final Cover System (not to scale)





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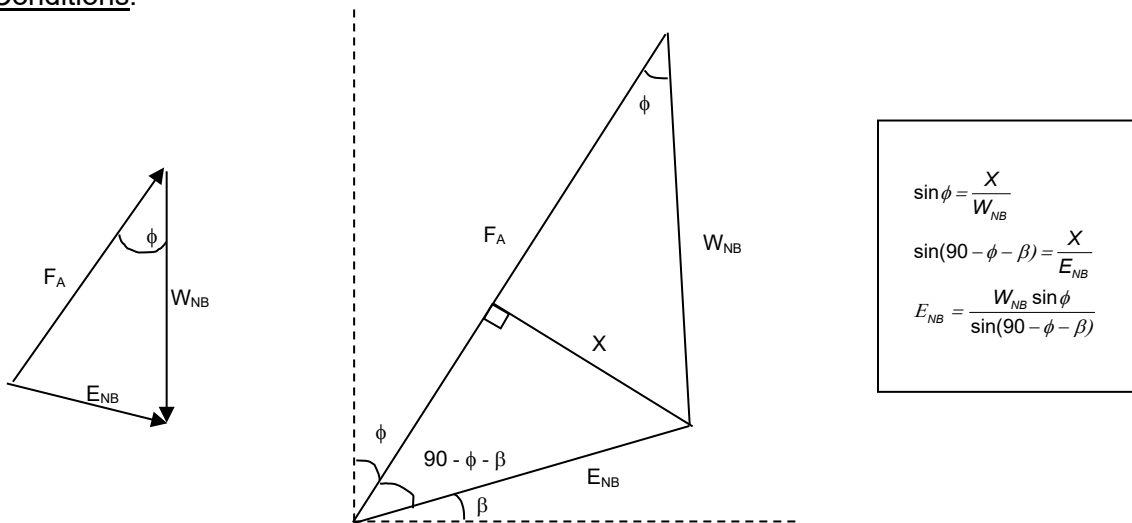
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Date: 05/2022

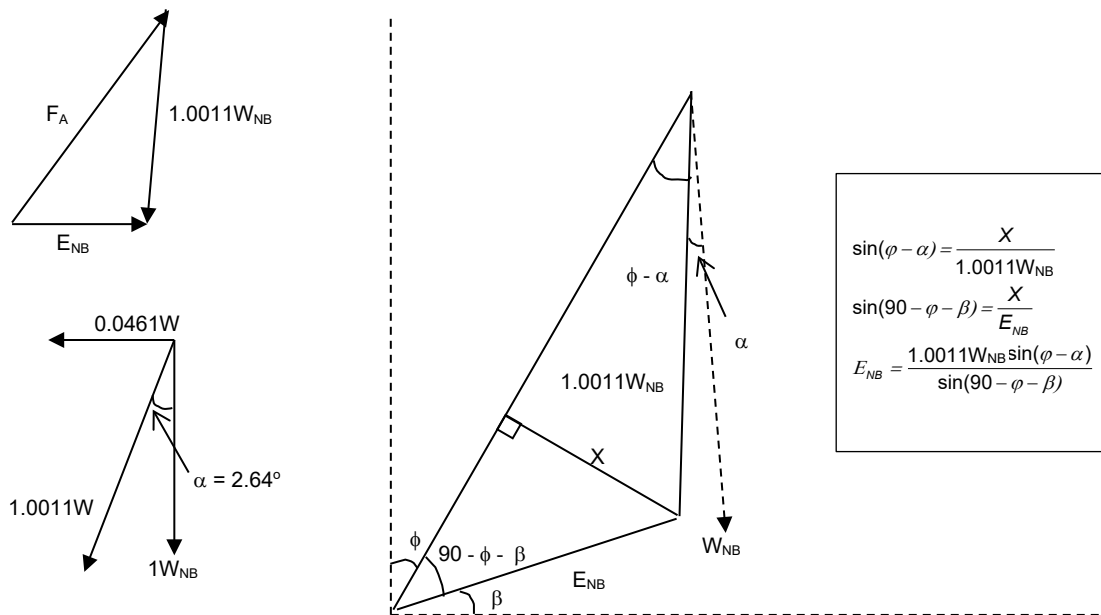
TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Derive Force of Neutral Block

Static Conditions:



Seismic Conditions:





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Calculated By: ORC

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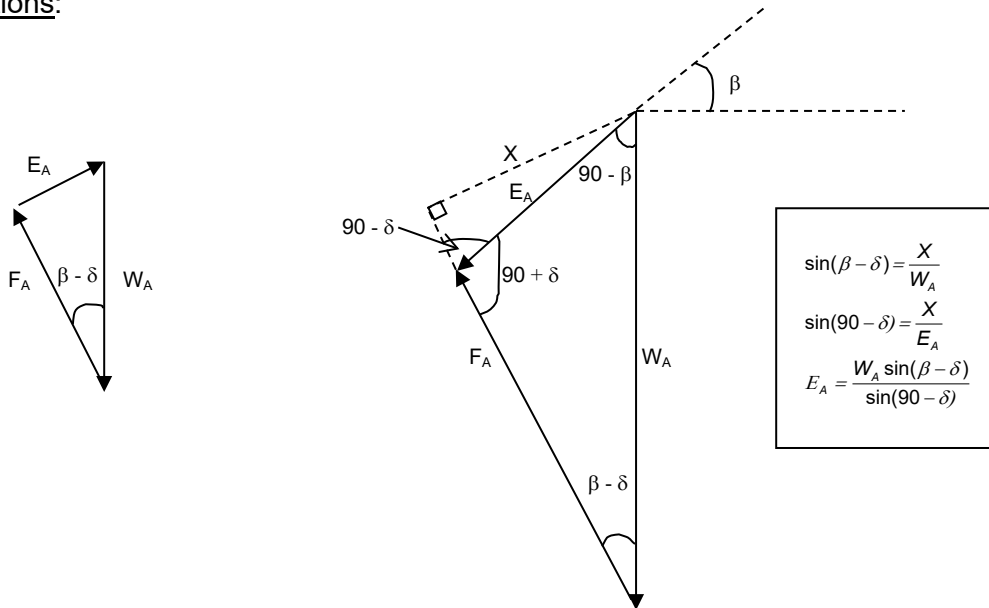
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Date: 05/2022

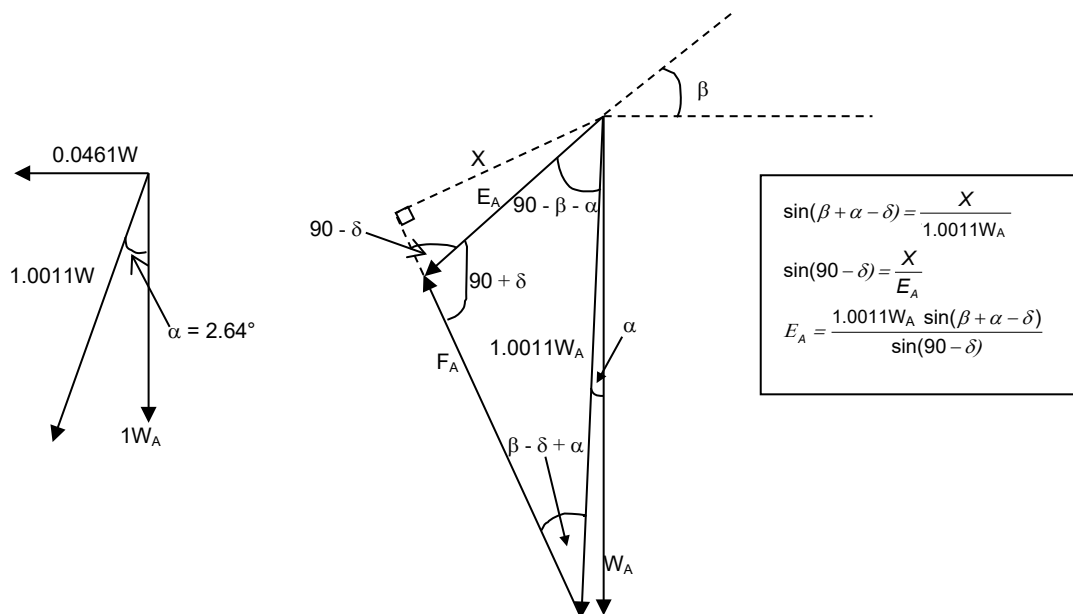
TITLE: SHEAR STRENGTH EVALUATION OF FINAL COVER CRITICAL INTERFACE

Derive Force of Active Block

Static Conditions:



Seismic Conditions:



Cover Soil <u>saturated</u> unit weight =	130.3	pcf
Cover Soil layer thickness =	3	feet
Tractor weight =	8,662	pcf
Height of Slope =	139	feet
Slope (H:V) =	4	Horizontal : <input type="text" value="1"/> Vertical

Neutral Block Dimensions =	<input type="text" value="12.37"/> <input type="text" value="12.75"/>	Base (ft)	<input type="text" value="3.09"/>	Height (ft)
		Length along Slope (ft)		
Cover Soil Friction Angle (ϕ) =	<input type="text" value="11.8"/>	degrees		
Critical Interface Friction Angle (δ) =	<input type="text" value="21.9"/>	degrees		
Critical Interface Adhesion (C) =	<input type="text" value="0"/>	psf		

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Length of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.8	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	6.62	12.59	5,904	307	5,597
1.7	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	7.01	13.30	3,022	326	2,696
1.65	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	7.22	13.69	1,432	336	1,096
1.60	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	7.44	14.10	-246	347	-592
1.5	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	7.93	15.00	-3,950	371	-4,320
1.4	14.04	573.11	560.36	227,707	2,491	12	22	0	0.00	8.49	16.02	-8,185	398	-8,583

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Length of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	7.93	15.00	6,656	248	6,408
1.4	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	8.49	16.02	2,628	275	2,353
1.35	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	8.80	16.58	399	290	109
1.30	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	9.13	17.18	-2,003	307	-2,310
1.2	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	9.88	18.52	-7,425	344	-7,769
1.1	14.04	573.11	560.36	219,045	2,491	12	22	0	0.00	10.75	20.07	-13,805	388	-14,193

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011

Cover Soil <u>saturated</u> unit weight =	130.3	pcf
Cover Soil layer thickness =	3	feet
Tractor weight =	8,662	pcf
Height of Slope =	139	feet
Slope (H:V) =	4	Horizontal : 1 Vertical

Neutral Block Dimensions =	12.37	Base (ft)	3.09	Height (ft)
	12.75	Length along Slope (ft)		
Cover Soil Friction Angle (ϕ) =	11.8	degrees		
Critical Interface Friction Angle (δ) =	15.0	degrees		
Critical Interface Adhesion (C) =	50	psf		

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.8	14.04	573.11	560.36	227,707	2,491	12	15	50	27.78	6.62	8.47	6,779	307	6,472
1.7	14.04	573.11	560.36	227,707	2,491	12	15	50	29.41	7.01	8.96	3,932	326	3,606
1.60	14.04	573.11	560.36	227,707	2,491	12	15	50	31.25	7.44	9.51	724	347	377
1.55	14.04	573.11	560.36	227,707	2,491	12	15	50	32.26	7.68	9.81	-1,032	358	-1,390
1.5	14.04	573.11	560.36	227,707	2,491	12	15	50	33.33	7.93	10.13	-2,904	371	-3,274
1.4	14.04	573.11	560.36	227,707	2,491	12	15	50	35.71	8.49	10.83	-7,029	398	-7,427

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	14.04	573.11	560.36	219,045	2,491	12	15	50	33.33	7.93	10.13	6,733	248	6,485
1.4	14.04	573.11	560.36	219,045	2,491	12	15	50	35.71	8.49	10.83	2,745	275	2,470
1.35	14.04	573.11	560.36	219,045	2,491	12	15	50	37.04	8.80	11.23	478	290	188
1.30	14.04	573.11	560.36	219,045	2,491	12	15	50	38.46	9.13	11.65	-1,921	307	-2,227
1.2	14.04	573.11	560.36	219,045	2,491	12	15	50	41.67	9.88	12.59	-7,325	344	-7,668
1.1	14.04	573.11	560.36	219,045	2,491	12	15	50	45.45	10.75	13.69	-13,696	388	-14,083

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011

Cover Soil <u>saturated</u> unit weight =	130.3	pcf
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Height of Slope =	139	feet
Slope (H:V) =	4	Horizontal : 1 Vertical

Neutral Block Dimensions =	12.37	Base (ft)	3.09	Height (ft)
	12.75	Length along Slope (ft)		
Cover Soil Friction Angle (ϕ) =	11.8	degrees		
Critical Interface Friction Angle (δ) =	7.7	degrees		
Critical Interface Adhesion (C) =	100	psf		

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.8	14.04	573.11	560.36	227,707	2,491	12	8	100	55.56	6.62	4.30	7,498	307	7,191
1.7	14.04	573.11	560.36	227,707	2,491	12	8	100	58.82	7.01	4.55	4,702	326	4,376
1.60	14.04	573.11	560.36	227,707	2,491	12	8	100	62.50	7.44	4.83	1,553	347	1,206
1.55	14.04	573.11	560.36	227,707	2,491	12	8	100	64.52	7.68	4.99	-201	358	-559
1.5	14.04	573.11	560.36	227,707	2,491	12	8	100	66.67	7.93	5.15	-2,027	371	-2,398
1.4	14.04	573.11	560.36	227,707	2,491	12	8	100	71.43	8.49	5.52	-6,133	398	-6,532

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	14.04	573.11	560.36	219,045	2,491	12	8	100	66.67	7.93	5.15	6,650	248	6,402
1.4	14.04	573.11	560.36	219,045	2,491	12	8	100	71.43	8.49	5.52	2,614	275	2,339
1.35	14.04	573.11	560.36	219,045	2,491	12	8	100	74.07	8.80	5.72	394	290	104
1.30	14.04	573.11	560.36	219,045	2,491	12	8	100	76.92	9.13	5.94	-2,018	307	-2,324
1.2	14.04	573.11	560.36	219,045	2,491	12	8	100	83.33	9.88	6.43	-7,427	344	-7,771
1.1	14.04	573.11	560.36	219,045	2,491	12	8	100	90.91	10.75	7.01	-13,831	388	-14,218

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



Cover Soil <u>saturated</u> unit weight =	130.3	pcf
Cover Soil layer thickness =	3	feet
Tractor weight =	8,662	pcf
Height of Slope =	139	feet
Slope (H:V) =	4	Horizontal : 1 Vertical

Neutral Block Dimensions =	12.37	Base (ft)	3.09	Height (ft)
	12.75	Length along Slope (ft)		
Cover Soil Friction Angle (ϕ) =	11.8	degrees		
Critical Interface Friction Angle (δ) =	0.0	degrees		
Critical Interface Adhesion (C) =	151.8	psf		

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.7	14.04	573.11	560.36	227,707	2,491	12	0	152	89.29	7.01	0.00	5,207	326	4,881
1.6	14.04	573.11	560.36	227,707	2,491	12	0	152	94.88	7.44	0.00	2,075	347	1,728
1.55	14.04	573.11	560.36	227,707	2,491	12	0	152	97.94	7.68	0.00	360	358	2
1.50	14.04	573.11	560.36	227,707	2,491	12	0	152	101.20	7.93	0.00	-1,467	371	-1,837
1.4	14.04	573.11	560.36	227,707	2,491	12	0	152	108.43	8.49	0.00	-5,518	398	-5,916
1.3	14.04	573.11	560.36	227,707	2,491	12	0	152	116.77	9.13	0.00	-10,192	430	-10,622

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Cover Soil Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (cover soil friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.6	14.04	573.11	560.36	219,045	2,491	12	0	152	94.88	7.44	0.00	9,774	224	9,550
1.5	14.04	573.11	560.36	219,045	2,491	12	0	152	101.20	7.93	0.00	6,232	248	5,985
1.40	14.04	573.11	560.36	219,045	2,491	12	0	152	108.43	8.49	0.00	2,181	275	1,906
1.35	14.04	573.11	560.36	219,045	2,491	12	0	152	112.44	8.80	0.00	-66	290	-356
1.3	14.04	573.11	560.36	219,045	2,491	12	0	152	116.77	9.13	0.00	-2,492	307	-2,799
1.2	14.04	573.11	560.36	219,045	2,491	12	0	152	126.50	9.88	0.00	-7,945	344	-8,288

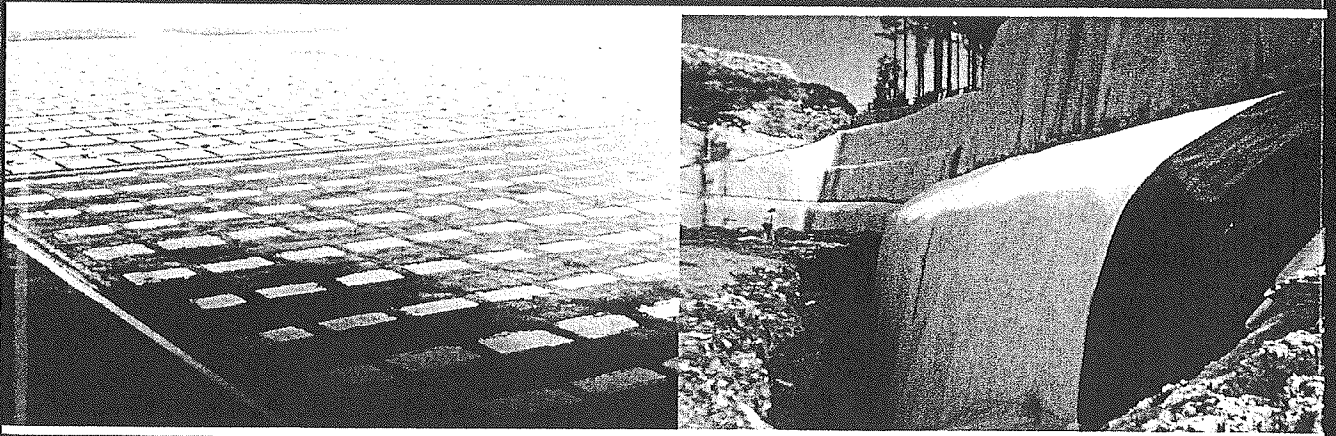
PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



Published Technical References

DESIGNING WITH GEOSYNTHETICS

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the same as that described in Section 2.7.4 using geotextiles. High strength geotextiles and geogrids are competitive in this particular application. The technique is considered very appropriate when stone columns are used as the ground modification technique. Sometimes the stone columns are actually contained in a geogrid enclosure, which appears to be a growing application [56].

3.2.7 Veneer Cover Soils

Whenever a lined slope (geomembrane, GCL, or compacted clay) is covered with soil, a stability calculation should be made to assess the potential for sliding failure of the soil on the barrier layer. Three situations come to mind: (1) landfill liners with leachate collection sand or gravel above them until such time that the solid waste acts as a passive resistance restraint; (2) surface impoundment liners where the cover soil is placed over the geomembrane to shield it from ultraviolet light, heat degradation, and equipment damage; and (3) landfill covers that have topsoil and protection soil placed over the geomembrane. In all cases the soil layer is relatively thin (0.3 to 1.0 m), hence the sliding stability of such a veneer of cover soil is the issue.

Due to the typically low shear strength of the covering soil to the liner material, numerous stability problems have arisen. The driving forces creating the instability are gravitational forces, equipment loads, surcharge loads, seepage forces, and/or seismic forces. Each must be carefully considered in the context of the site-specific conditions.

Koerner and Soong [57] have analyzed the general situation through the use of limit equilibrium and a finite slope model, as shown in Figure 3.22. Consider a cover soil placed directly on a geomembrane (or other barrier layer) at a slope angle β . Two discrete zones can be visualized, as shown in Figure 3.22a. There is a small passive wedge near the toe of the slope resisting a long thin active wedge extending the length of the slope. It is assumed that the cover soil is of uniform thickness and constant unit weight. At the top of the slope or at an intermediate berm, we anticipate that a tension crack in the cover soil will occur, thereby breaking continuity with the remaining cover soil at the crest.

Resisting the tendency for the cover soil to slide is the interface friction and/or adhesion of the cover soil to the specific type of underlying geomembrane. The shear strength values of δ and c_a must be obtained from a laboratory direct-shear test, as described earlier. Note that the passive wedge is assumed to move on the underlying cover soil so that the shear strength parameters ϕ and c , which come from soil-to-soil friction tests, will also be required.

By taking free bodies of the passive and active wedges with the appropriate forces being applied, the formulation for the factor of safety results. The resulting equation is not an explicit solution for the FS, and it must be solved using the quadratic equation. The complete development of the equation is given in [57]. Other approaches are found in Giroud and Beech [58], Koerner and Hwu [59], and Thiel and Stewart [60].

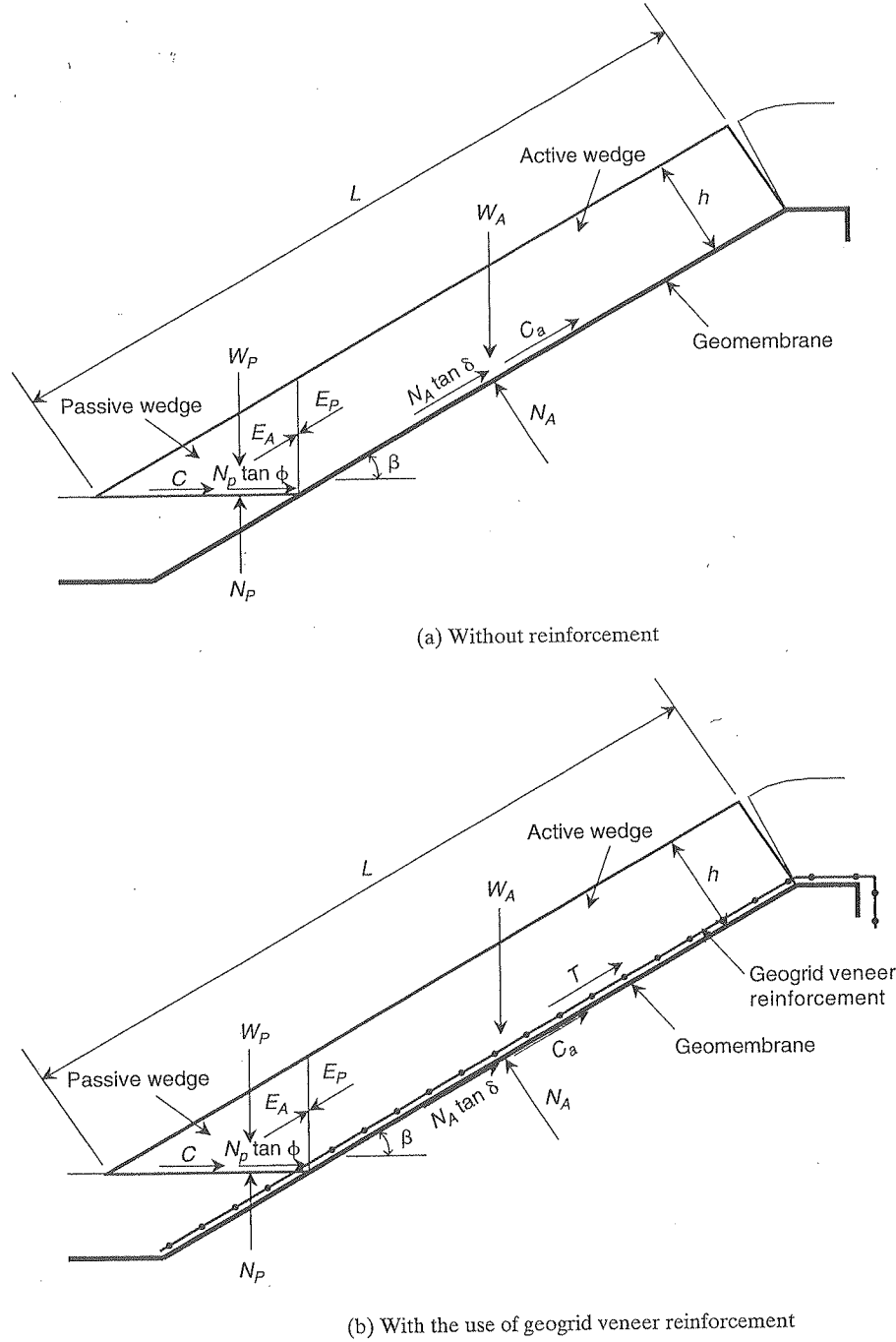


Figure 3.22 Limit equilibrium forces involved in a finite length slope analysis for a uniformly thick cover soil. (After Koerner and Soong [57])

The expression for determining the factor of safety, considering the active wedge, can be derived as follows:

$$W_A = \gamma h^2 \left(\frac{L}{h} - \frac{1}{\sin \beta} - \frac{\tan \beta}{2} \right) \quad (3.15)$$

$$N_A = W_A \cos \beta \quad (3.16)$$

$$C_a = c_a \left(L - \frac{h}{\sin \beta} \right) \quad (3.17)$$

By balancing the forces in the vertical direction, the following formulation results:

$$E_A \sin \beta = W_A - N_A \cos \beta - \frac{N_A \tan \delta + C_a}{\text{FS}} \sin \beta$$

Hence the interwedge force acting on the active wedge is

$$E_A = \frac{(\text{FS})(W_A - N_A \cos \beta) - (N_A \tan \delta + C_a) \sin \beta}{\sin \beta (\text{FS})}$$

The passive wedge can be considered in a similar manner:

$$W_p = \frac{\gamma h^2}{\sin 2\beta} \quad (3.18)$$

$$N_p = W_p + E_p \sin \beta \quad (3.19)$$

$$C = \frac{(c)(h)}{\sin \beta} \quad (3.20)$$

By balancing the forces in the horizontal direction, the following formulation results:

$$E_p \cos \beta = \frac{C + N_p \tan \phi}{\text{FS}}$$

Hence the interwedge force acting on the passive wedge is

$$E_p = \frac{C + W_p \tan \phi}{\cos \beta (\text{FS}) - \sin \beta \tan \phi}$$

By setting $E_A = E_p$, the following equation can be arranged in the form of $ax^2 + bx + c = 0$, which in our case, using FS values, is

$$a(\text{FS})^2 + b(\text{FS}) + c = 0 \quad (3.21)$$

where

$$a = (W_A - N_A \cos \beta) \cos \beta, \quad (3.22)$$

$$b = -[(W_A - N_A \cos \beta) \sin \beta \tan \phi + (N_A \tan \delta + C_a) \sin \beta \cos \beta + \sin \beta (C + W_p \tan \phi)], \text{ and} \quad (3.23)$$

$$c = (N_A \tan \delta + C_a) \sin^2 \beta \tan \phi \quad (3.24)$$

The resulting FS value is then obtained from the following equation:

$$FS = \frac{-b + \sqrt{b^2 - 4ac}}{2a} \quad (3.25)$$

where (in Figure 3.22a and in the above analysis)

- W_A = total weight of the active wedge,
- W_P = total weight of the passive wedge,
- N_A = effective force normal to the failure plane of the active wedge,
- N_P = effective force normal to the failure plane of the passive wedge,
- γ = unit weight of the cover soil,
- h = thickness of the cover soil,
- L = length of slope measured along the geomembrane,
- β = soil slope angle beneath the geomembrane,
- ϕ = friction angle of the cover soil,
- δ = interface friction angle between cover soil and geomembrane,
- C_a = adhesive force between cover soil of the active wedge and the geomembrane,
- c_a = adhesion between cover soil of the active wedge and the geomembrane,
- C = cohesive force along the failure plane of the passive wedge,
- c = cohesion of the cover soil,
- E_A = interwedge force acting on the active wedge from the passive wedge,
- E_P = interwedge force acting on the passive wedge from the active wedge, and
- FS = factor of safety against cover soil sliding on the geomembrane

When the calculated FS value falls below 1.0, a stability failure of the cover soil sliding on the geomembrane is to be anticipated. Thus a value greater than 1.0 must be targeted as being the minimum factor of safety. How much greater than 1.0 the FS value should be is a design and/or regulatory issue. Example 3.12 illustrates the procedure.

Example 3.12

Given a cover soil slope of $\beta = 18.4^\circ$ (i.e. 3H-to-1V), $L = 30$ m, $h = 900$ mm, $\gamma = 18$ kN/m³, $c = 0$, $\phi = 30^\circ$, $c_a = 0$, $\delta = 18^\circ$, determine the resulting factor of safety.

Solution:

$$\begin{aligned}
 W_A &= \gamma h^2 \left(\frac{L}{h} - \frac{1}{\sin \beta} - \frac{\tan \beta}{2} \right) \\
 &= (18.0)(0.90)^2 \left(\frac{30}{0.90} - \frac{1}{\sin 18.4} - \frac{\tan 18.4}{2} \right) \\
 &= 14.58(33.3 - 3.17 - 0.17) \\
 &= 437 \text{ kN/m}
 \end{aligned}$$

If the cut-slope is made in a cohesive material (containing clay and silt), the failure is usually deeper and like rotational slips as shown in Fig. 2. The long-term (prior to waste filling and after waste filling) and the end-of-excavation stability have to be analyzed separately. The former requires an effective stress analysis using drained (effective) strength parameters (ϕ' , c') and the later a total stress analysis using undrained (total) strength parameters (c_u , usually $\phi_u = 0$) determined on undisturbed samples of the materials. Typically, a finite slope analysis using a method of slices such as the modified Bishop method is used for the stability analysis. The total stress analysis can be performed also with the simpler circular arc method. The position of the failure surface is unknown and determined by a trial and error method. There are computer programs available for slope stability analysis (Bosscher, 1990). Chart solutions for simple slopes using the finite slope analysis are provided in Appendix A.

Berm Slopes

Since berms are constructed by compacting selected earthen materials, the characterization of the materials is much simpler than the cuts. The end-of-construction and the long-term analyses can be made in a manner similar to the ones described for cut slopes. The end-of-construction stability is likely to be more critical than the long-term stability. The strength parameters are determined on compacted specimens of the borrow materials, i.e., ϕ_u , c_u and ϕ' , c' .

COVER SOIL STABILITY

The usual cover soil is a relatively thin layer of soil of either uniform thickness or tapered thickness. When clay covers are placed on side slopes, they sometimes fail by slumping. The accumulated soil gathers at the toe of the slope. This same situation is even more apt to occur for cover soils placed on geosynthetics, which are invariably lower in frictional resistance than the substrate soils from which the slope itself is formed.

The failure of cover soils with uniform depth is basically a surface raveling type of failure along the interface as shown in Fig. 3 and can be analyzed by an infinite slope analysis using the following expression for a safety factor:

$$F = (\tan \delta / \tan \beta) [1 - (\gamma_w h_p / \gamma d) \sec^2 \beta] \quad (2)$$

where δ is the interface friction (between the geosynthetic and cover soil or the in situ soil); the other terms are as defined before in Fig. 1. The critical parameters are the interface friction angle or the shear strength between the soil and the geosynthetic, the inclination of slope, and the seepage forces (due to infiltrating water or rapid draw down of reservoir levels).

To alleviate the slumping of cover soils down slope, it is quite common to construct them with a taper, i.e., thicker at the bottom and gradually thinner going toward the top as shown in Fig. 4. The stability can be analyzed using a wedge stability method as described in Appendix B.

SLOPE LINER STABILITY

Principal sliding surfaces in landfills often coincide within the multi-layer liner system underlying the wastefill. The Kettleman Hills waste landfill slide in California occurred in this fashion in 1988. The foundation soil including the cut slopes is usually

much stronger than the critical liner-system interfaces because the settlement considerations usually dictate selection of sites with competent foundation soils. There is some evidence that relatively steep slopes of high refuse fills also do not fail in deep rotational slides if they are founded on firm foundations. Therefore, the liner-system interfaces become the most critical slip zone. The interfaces may be: (1) between geomembrane liner and geotextile or geonet layer, (2) between geomembrane liner and compacted clay liner, and (3) between geotextile or geomembrane and granular layer. The interfaces between these materials are characterized by low frictional resistance. The frictional resistance is affected by various properties including degree of polishing (of geomembrane by geotextile with increasing shear displacement) and whether the interfaces are wet or dry. The interface strength properties can be evaluated by use of laboratory direct shear or pullout tests (Mitchell et al, 1990).

Failure by Sliding on Liner Interfaces

The stability analysis for sliding along planar liner interface surfaces as shown in Fig. 5 can be performed using a wedge stability method as given in Appendix B. The analysis of Kettleman Hills waste landfill slide by this approach provided a reasonable estimate of the failure based on the interface shear strengths measured in the laboratory. Three-dimensional effects were found to lower safety factors by 10 to 15% (Seed et al., 1990).

Failure with Liner Pullout or Break

In the geomembrane lined reservoirs or landfills, the liner comes up from the bottom of the pit, covering the side slopes, then running over the top a short distance. It often terminates vertically down in an anchor trench as shown in Fig. 6.

The stress generated in the geomembrane, σ , for a given length of runout, L_{TO} , and trench depth, d_t , can be obtained from the following expression:

$$\sigma = \frac{\gamma d_c \tan \delta + 2K_o \gamma d_t (d_c + 0.5 d_t) \tan \delta + 2\gamma (d_c + d_t) b \tan \delta}{t (\cos \beta - \sin \beta \tan \delta)} \quad (3)$$

where

t = the geomembrane thickness

β = the slope angle

δ = the interface friction angle

d_c = the depth of cover soil

b = the width of trench

γ = the unit weight of cover and trench soil

K_o = the lateral coefficient of earth in trench (it can be taken as 0.5)

The safety factor, F , against pullout or break can be computed from

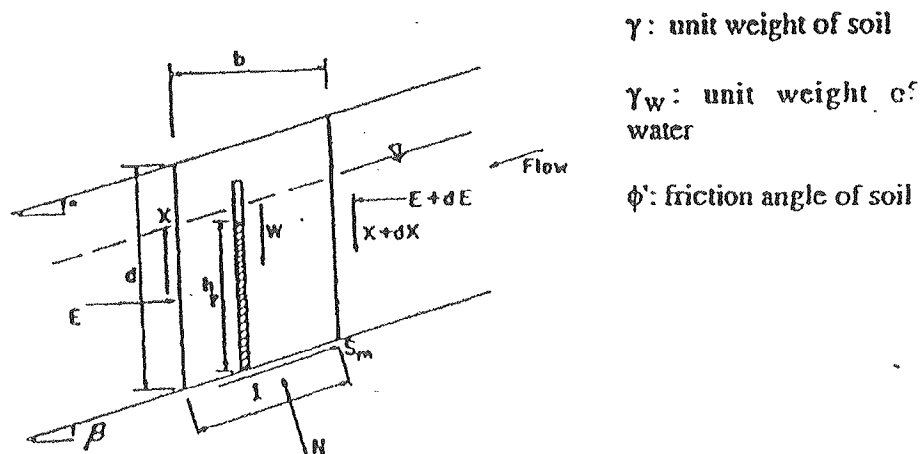


Fig. 1. Idealized Infinite Slope Analysis

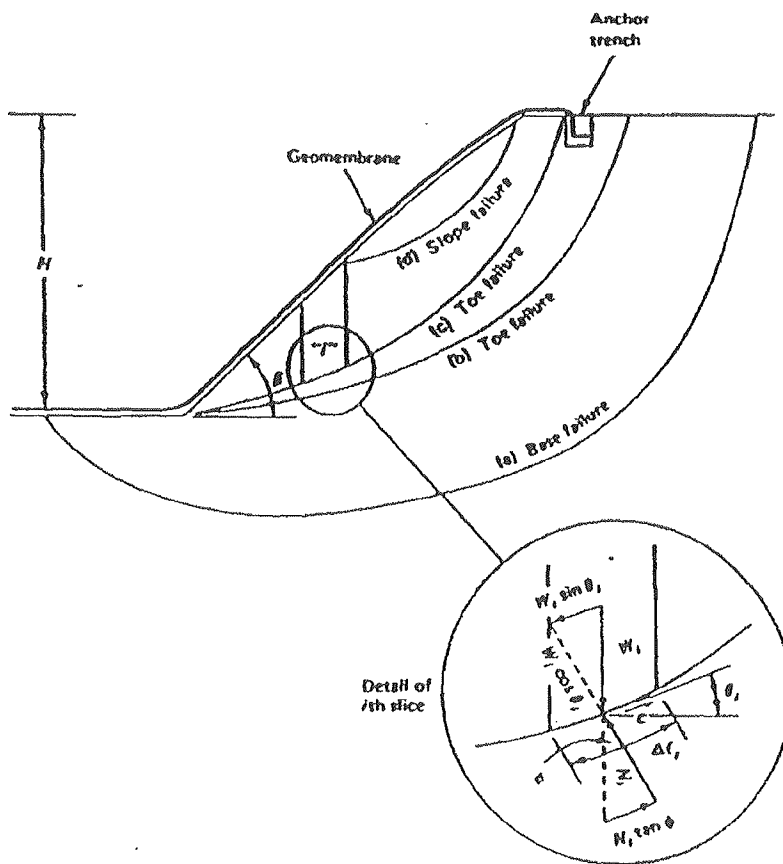


Fig. 2. Various Types of Finite Slope Failures

APPENDIX B

WEDGE METHOD OF SLOPE STABILITY ANALYSIS IN WASTE DISPOSAL

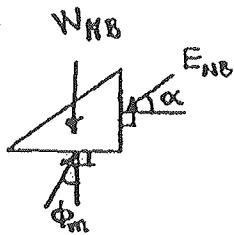
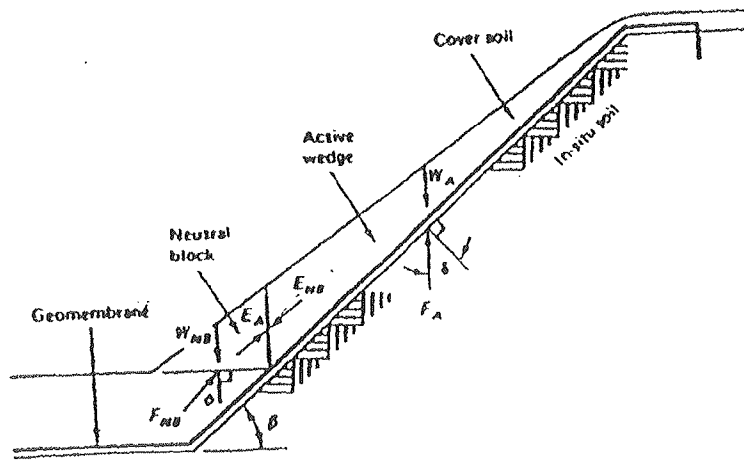
In this method the potential sliding mass is separated into a series of wedges and the vertical and horizontal force equilibrium of each wedge is considered in turn. This method is most appropriate for conditions where the failure surface can be approximated by a series of planar surfaces. For analysis by the wedge method, the mass above the trial slip surface is divided by vertical lines into a number of wedges. The side force transmitted across the interface between any two wedges is not known but an assumption regarding its inclination must be made. The assumption that the side forces between wedges are horizontal is conservative with an error for most cases no more than 15%. Experience indicates that assuming an inclination of 10 to 15 degrees from the horizontal usually gives reasonable results. The factor of safety, F (defined as the ratio between the shear strength and the shear stress required for equilibrium), is calculated by trial-and-error. A value of F is assumed, and then checked to determine if the shear resistance along the failure surface reduced by this F satisfies equilibrium. If not, a new trial is initiated with another assumed safety factor. The analysis can be performed either graphically or numerically.

Two common cases encountered in waste disposal which can be analyzed by the wedge method are presented by use of the numerical method. These cases involve the stability along a liner and tapered depth soil cover as shown in Figure B1 and B2. The steps are as follows:

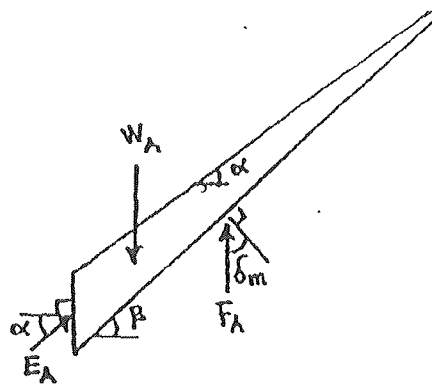
1. Define a trial slip surface. In these two cases it is primarily along the soil or waste and liner interface.
2. Divide the mass above the slip surface into wedges. In these two cases, the sliding mass is divided into an active wedge and a neutral block.
3. Calculate the weight of each wedge. This analysis is a two-dimensional analysis and the stability of a unit length of material along the slope is considered. Therefore, the weight of each wedge is its cross-sectional area (times unit length) times the unit weight of the sliding mass.
4. Assume a value of factor of safety, F and calculate the trial values of mobilized friction angle along the slip surface, δ_m (soil/waste-liner friction angle) or ϕ_m (soil-soil friction angle) using the following definitions:

$$\tan \delta_m = \frac{\tan \delta}{F} \qquad \tan \phi_m = \frac{\tan \phi}{F}$$

5. Assume the inclination of the force acting between the active wedge and the neutral block, α . For the tapered depth cover soil, α may be assumed parallel to cover soil slope. Otherwise, assume a value between 0 - 20°.
6. Using the equations given in Figures B1 and B2, calculate the force from neutral block acting on active wedge, E_A and the force from active wedge acting on neutral block, E_{NB} from the force equilibrium of each block. These forces are equal in magnitude opposite in direction at equilibrium. Note that the friction is mobilized in proportion to the assumed F along the slip surface; therefore, there may be a difference, ΔE in the calculated magnitudes of E_A and E_{NB} .
7. Plot ΔE versus assumed F.
8. Assume a new F and repeat steps 6 and 7. Try additional values of F until ΔE is negligibly small. This value of F is the correct safety factor.



NEUTRAL BLOCK



ACTIVE BLOCK

$$E_A = W_A \frac{\sin(\beta - \delta_m)}{\sin(90^\circ + \alpha - \beta + \delta_m)}$$

$$E_{NB} = W_{NB} \frac{\sin \phi_m}{\sin(90^\circ - \alpha - \phi_m)}$$

$$\Delta E = E_A - E_{NB}$$

$$\tan \delta_m = \frac{f_{cu} \delta}{F}$$

$\alpha = \beta$ in leachate collection system (18.4°)

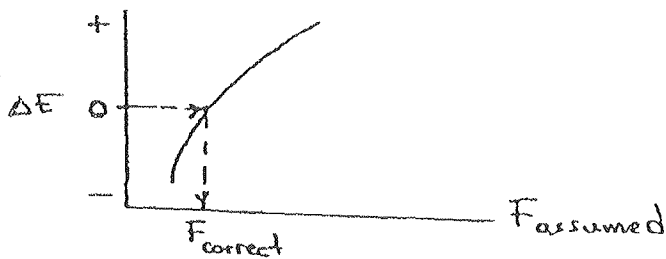


Figure B1. Wedge analysis as applied to tapered depth soil cover.

D8T

Track-Type Tractor



Engine

Engine Model	Cat® C15 ACERT™	
Emissions	U.S. EPA Tier 4 Final/EU Stage IV/ Japan 2014 (Tier 4 Final)/ Korea Tier 4 Final	
Engine Power (Maximum)		
SAE J1995	271 kW	363 hp
ISO 14396	268 kW	359 hp
ISO 14396 (DIN)		364 hp

Engine (continued)

Net Power (Rated)		
ISO 9249/SAE J1349	233 kW	312 hp
ISO 9249/SAE J1349 (DIN)		317 hp
Net Power (Maximum)		
ISO 9249/SAE J1349	252 kW	335 hp
ISO 9249/SAE J1349 DIN		343 hp
Operating Weights		
Standard	39 420 kg	86,900 lb
LGP	37 420 kg	82,496 lb

D8T Track-Type Tractor Specifications

Engine

Engine Model	C15 ACERT	
Engine Power (Maximum*)		
SAE J1995	271 kW	363 hp
ISO 14396	268 kW	359 hp
ISO 14396 (DIN)	364 hp	
Net Power (Rated**)		
ISO 9249/SAE J1349	233 kW	312 hp
ISO 9249/SAE J1349 (DIN)	317 hp	
Net Power (Maximum*)		
ISO 9249/SAE J1349	252 kW	335 hp
ISO 9249/SAE J1349 DIN	343 hp	
Bore	137 mm	5.4 in
Stroke	172 mm	6.75 in
Displacement	15.2 L	928 in ³

*Engine speed 1,700 rpm

**Rated speed 1,900 rpm

- Net power advertised is the power available at the flywheel when the engine is equipped with fan, air cleaner, muffler, and alternator.
- No derating required up to 3566 m (11,700 ft) altitude, beyond 3566 m (11,700 ft) automatic derating occurs.
- All non road Tier 4 Interim and Final, Stage IIIB and IV, Japan 2011 and 2014 (Tier 4 Interim and Tier 4 Final) and Korea Tier 4 Final diesel engines are required to use only Ultra Low Sulfur Diesel (ULSD) fuels containing 15 ppm (mg/kg) sulfur or less. Biodiesel blends up to B20 (20% blend by volume) are acceptable when blended with 15 ppm (mg/kg) sulfur or less ULSD. B20 should meet ASTM D7467 specification (biodiesel blend stock should meet Cat biodiesel spec, ASTM D6751 or EN 14214). Cat DEO-ULS™ or oils that meet the Cat ECF-3, API CJ-4, and ACEA E9 specification are required. Consult your OMM for further machine specific fuel recommendations.
- Diesel Exhaust Fluid (DEF) used in Cat Selective Catalytic Reduction (SCR) systems must meet the requirements outlined in the International Organization for Standardization (ISO) standard 22241.

Service Refill Capacities

Fuel Tank	627 L	165 gal
DEF Tank	24 L	6.3 gal
Cooling System	86 L	22.7 gal
Engine Crankcase*	38 L	10 gal
Power Train	155 L	41 gal
Final Drives (each)	12.5 L	3.3 gal
Roller Frames (each)	65 L	17.2 gal
Pivot Shaft Compartment	40 L	10.6 gal
Hydraulic Tank	75 L	19.8 gal

*With oil filter

Weights

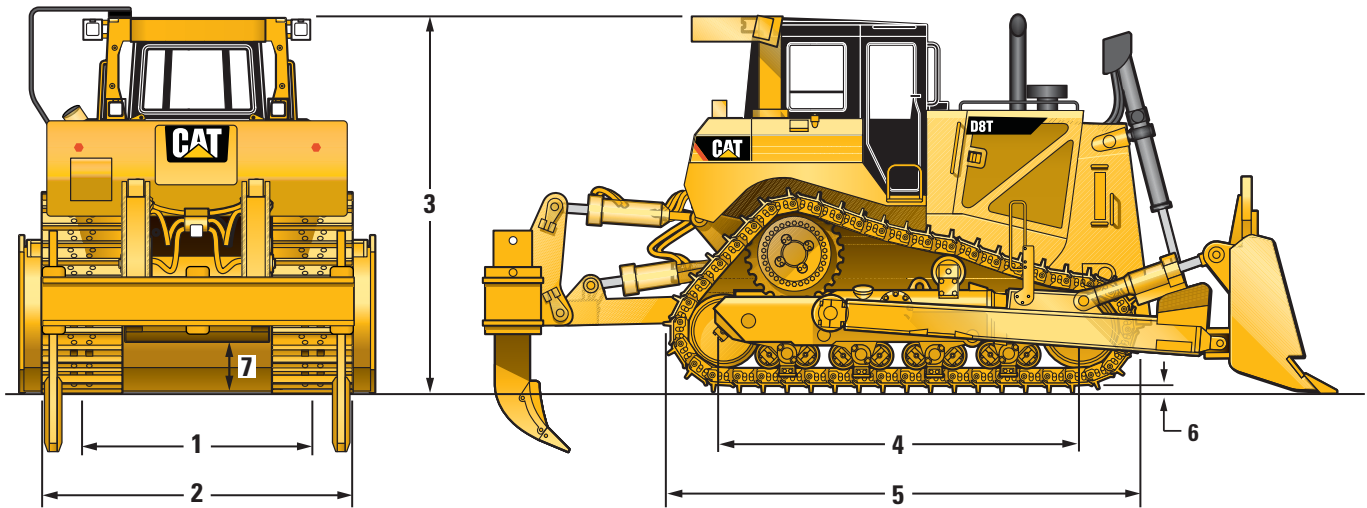
Operating Weight		
Standard	39 420 kg	86,900 lb
LGP	37 420 kg	82,496 lb
Shipping Weight		
Standard SU Blade	30 070 kg	66,300 lb
LGP	31 160 kg	68,700 lb

- Operating Weight – Standard: Includes hydraulic controls, blade, tilt cylinder, coolant, lubricants, 100% fuel, ROPS, FOPS cab, SU-Blade, Single-Shank Ripper, 610 mm (24 in) MS shoes, and operator.
- Operating Weight – LGP: Includes hydraulic controls, blade, tilt cylinder, drawbar, coolant, lubricants, 100% fuel, ROPS, FOPS cab, SU-Blade, 965 mm (38 in) MS shoes, and operator.
- Shipping Weight – Standard: Includes coolant, lubricants, 20% fuel, ROPS, FOPS cab, and 610 mm (24 in) MS shoes.
- Shipping Weight – LGP: Includes coolant, lubricants, 20% fuel, ROPS, FOPS cab, and 965 mm (38 in) MS shoes.

D8T Track-Type Tractor Specifications

Dimensions

All dimensions are approximate.



	Standard		Non-Suspended		LGP*	
1 Track Gauge	2083 mm	82.0 in	2083 mm	82.0 in	2337 mm	92.0 in
2 Width of Tractor						
Over Trunnions	3057 mm	120.4 in	3057 mm	120.4 in	3311 mm	130.4 in
Without Trunnions (Standard shoe width)	2693 mm	106.0 in	2693 mm	106.0 in	3302 mm	130.0 in
3 Machine Height**, from Tip of Grouser						
Exhaust Stack	3472 mm	136.7 in	3463 mm	136.3 in	3295 mm	129.7 in
EROPS (to top of railing)	3566 mm	140.4 in	3575 mm	140.7 in	3566 mm	140.4 in
4 Length of Track on Ground	3206 mm	126.2 in	3258 mm	128.3 in	3206 mm	126.2 in
5 Length of Basic Tractor (tag link trunnion to tip of rear grouser)	4647 mm	183.0 in	4647 mm	183.0 in	4647 mm	183.0 in
With the following attachments add:						
Ripper – Single Shank (with tip at ground line)	1519 mm	59.8 in	1519 mm	59.8 in		N/A
Ripper – Multi Shank (with tip at ground line)	1613 mm	63.5 in	1613 mm	63.5 in		N/A
SU Blade	1844 mm	72.6 in	1844 mm	72.6 in	1844 mm	72.6 in
U Blade	2241 mm	88.2 in	2241 mm	88.2 in		N/A
A Blade (not angled)	2027 mm	79.8 in	2027 mm	79.8 in		N/A
A Blade (angled 25 degrees)	3068 mm	120.8 in	3068 mm	120.8 in		N/A
Drawbar	406 mm	16.0 in	406 mm	16.0 in	406 mm	16.0 in
6 Height of Grouser	78 mm	3.1 in	78 mm	3.1 in	78 mm	3.1 in
7 Ground Clearance	613 mm	24.1 in	606 mm	23.8 in	613 mm	24.1 in

*Standard shoe width of D8T LGP with non-suspended undercarriage is 965 mm (38 in).

**When Cat Grade Control 3D antennas are installed, overall machine height increases by approximately 82 mm/3.2 in.

J.2-B Liner/LCS Prior to Waste Placement



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

Problem Statement

Determine the range of acceptable liner and leachate collection system peak shear strength parameters that provide a factor of safety against slope failure prior to waste placement, in accordance with 35 Ill. Admin. Code Section 811.306 (b). This regulation requires factors of safety of at least 1.3 for static conditions and 1.0 under seismic conditions.

It is noted that this range may not be acceptable for global stability analyses after waste placement. The range for global stability is evaluated in a subsequent calculation. Please refer to **Appendix J.2-E** for a shear strength window that meets all landfill design requirements.

Background

The material strengths of earthen layers (recompacted clay liner, granular drainage layer) are based on laboratory testing. However, as geosynthetics are manufactured products, they can be designed and/or selected to meet strength requirements. The stability of the liner and leachate collection system is influenced by the interface shear strengths between geosynthetic liner/leachate collection system components.

This calculation is developed to identify the lowest peak shear strength that produces acceptable factors of safety for any potential interface within the liner and leachate collection system prior to waste placement. The interfaces that are considered include the following:

- Geotextile to granular drainage layer;
- Geotextile to geomembrane; or
- Geomembrane to low permeable earth liner.

It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in this calculation.

Given

- Landfill design specifications for layer types, thicknesses, and material properties (refer to **Appendix J.1**).
- Koerner, R.M., Designing with Geosynthetics. Prentice Hall, Fifth Edition (refer to attached pages).
- Edil, T.B., "Seepage, Slopes, and Dams." University of Wisconsin-Madison (refer to attached pages).
- Caterpillar Product Information, 836K Landfill Compactor (refer to attached pages).
- Landfill design specifications for layer types and thicknesses.



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

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TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

- Cross-sectional sketches of liner and leachate collection system and force diagrams of analyses (refer to attached pages).

Assumptions

- Liner and leachate collection system in the vertical and horizontal expansion sideslopes includes (assumed to be worst case scenario), from top to bottom:
 - 8-oz/yd² non-woven geotextile filter
 - 1-foot granular drainage layer ($k \geq 1 \times 10^{-3}$ cm/sec)
 - 12-oz/yd² non-woven cushion geotextile
 - 60-mil HDPE textured geomembrane
 - 5-foot low permeability earth liner ($k \leq 1 \times 10^{-7}$ cm/sec)
- Slope of the proposed liner sideslopes is 3H:1V, therefore $\beta = 18.43$ degrees.
- The granular drainage layer is conservatively assumed to be saturated.
- Saturated unit weight of granular material, $\gamma_{\text{sat}} = 130$ pcf.
- Buoyant unit weight of granular material, $\gamma_b = \gamma_{\text{sat}} - \gamma_w = 130 - 62.4 = 67.6$ pcf.
- Maximum horizontal acceleration = 0.0461g (reference **Appendix J.1**).
- Pore pressure remains unchanged during earthquake conditions.
- Internal friction angle of the low permeable earth liner, $\Phi = 11.8$ degrees.
- The material strengths of earthen layers (recompacted clay liner, granular drainage layer) are based on laboratory testing. As geosynthetics are manufactured products, they can be designed to meet strength requirements.
- Wedges analyzed are shown on attached sketch. Weight per liner foot (W) of each wedge is the cross-sectional area multiplied by the buoyant unit weight. A unit width of one foot (1') was used to determine the cross-sectional area.
- A compactor load of 3,397 pounds per square foot (psf) was included in the calculation to determine the factor of safety under static conditions. It is noted that this is based on a Caterpillar 836K compactor load. Although a compactor will not operate directly over the liner and leachate collection system, it provides significantly more bearing pressure than a dozer and therefore is conservatively used in this analysis.



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TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

- The maximum depth of excavation is assumed to be approximately 63 feet – vertical distance measured from the crest of the liner system slope (edge of waste boundary) to the toe of the liner slope. The maximum depth of excavation is located along the southwest edge of the horizontal landfill expansion.
- The following equations were used to determine the factors of safety against slope failure (refer to attached sketches).

Static Conditions:

$$E_A = \left[W_A \frac{\sin(\beta - \delta_m)}{\sin(90^\circ - \delta_m)} \right] - (C_a)(L)$$

$$E_{NB} = \left[W_{NB} \frac{\sin(\phi_m)}{\sin(90^\circ - \beta - \phi_m)} \right]$$

$$\Delta E = E_A - E_{NB}$$

Seismic Conditions:

$$E_A = \left[1.0011 W_A \frac{\sin(\beta - \delta_m + \alpha)}{\sin(90^\circ - \delta_m)} \right] - (C_a)(L)$$

$$E_{NB} = \left[1.0011 W_{NB} \frac{\sin(\phi_m - \alpha)}{\sin(90^\circ - \beta - \phi_m)} \right]$$

$$\Delta E = E_A - E_{NB}$$

Where,

E_A	=	Force of Active Block (lb/ft)
E_{NB}	=	Force of Neutral Block (lb/ft)
$W_{A / NB}$	=	Weight of soil blocks (active and neutral) (lb/ft)
β	=	Angle of slope
δ_m	=	Mobilized interface friction angle
ϕ_m	=	Internal soil friction angle (drainage layer)
α	=	Resultant angle of seismic force
C_a	=	Mobilized Adhesion (psf)
L	=	Length of sliding block (ft)

The factor of safety is adjusted for the next trial based on the difference of the forces acting on the wedges. As shown in the attached reference (Edil), if ΔE is negative, then the assumed factor of safety is too low; if ΔE is positive, then the assumed factor of safety is too high.

Multiple friction angle and adhesion values for the interfaces are evaluated in spreadsheet calculators to determine a window of acceptable values. For demonstrative purposes of this calculation, a friction angle of 24.1 degrees is selected with an associated adhesion of 0 psf.



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TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

Saturated Conditions Assumed for Random Fill / Select Waste Layer

Calculate loading due to CAT 836K compactor (see attached Caterpillar Product Information)

$$\text{Weight of the Vehicle } (W_{eq}) = 123,319 \text{ lb}$$

$$\text{Contact Pressure } (P) \quad P = \frac{123,319 \text{ lb}}{(4 \text{ drums} \times \text{Area}_{\text{contact}})}$$

$$P = \frac{123,319 \text{ lb}}{(4 \text{ drums} \times (4.67 \text{ ft} \times \frac{1}{3} \times 5.83 \text{ ft}))}$$

$$P = 3,397 \text{ psf}$$

$$\begin{aligned} \text{Weight per Wheel} &= P \times (\text{Wheelbase of Vehicle} / 2) \\ &= 3,397 \text{ psf} \times (14.92 \text{ ft} / 2) \\ &= 3,397 \text{ psf} \times (7.46 \text{ ft}) \\ &= 25,342 \text{ lb/ft} \end{aligned}$$

Estimate the weight of the active and neutral blocks:

$$\begin{aligned} W_A (\text{Seismic}) &= (\text{Area}) \gamma \\ &= (195.89 \text{ ft})(1.0 \text{ ft})(130 \text{ pcf}) = 25,466 \text{ lb/ft} \end{aligned}$$

$$\begin{aligned} W_A (\text{Static}) &= (\text{Area}) \gamma + (\text{compactor loading})(\text{compactor length}) \\ &= (195.89 \text{ ft})(1.0 \text{ ft})(130 \text{ pcf}) + (3,397 \text{ psf})(7.46 \text{ ft}) = 50,807 \text{ lb/ft} \end{aligned}$$

$$\begin{aligned} W_{NB} &= (\text{Area}) \gamma \\ &= (0.5)(3.16 \text{ ft})(1.05 \text{ ft})(130 \text{ pcf}) = 217 \text{ lb/ft} \end{aligned}$$

Calculate δ_M and ϕ_M assuming FS = 1.3:

$$\tan \delta_M = \frac{\tan(\delta)}{FS} \rightarrow \delta_M = \tan^{-1} \left(\frac{\tan(24.1^\circ)}{1.3} \right) = 18.99^\circ$$

$$\tan \phi_M = \frac{\tan(\phi)}{FS} \rightarrow \phi_M = \tan^{-1} \left(\frac{\tan(11.8^\circ)}{1.3} \right) = 9.13^\circ$$

$$C_a = \frac{c}{1.3} = \frac{0 \text{ psf}}{1.3} = 0 \text{ psf}$$

For Static Conditions (with compactor loading), calculate ΔE for a factor of safety of 1.3:

$$E_A = \left[W_A \frac{\sin(\beta - \delta_M)}{\sin(90^\circ - \delta_M)} \right] - (C_a)(L) = \left[(50,807 \text{ lb/ft}) \left(\frac{\sin(18.43^\circ - 18.99^\circ)}{\sin(90^\circ - 18.99^\circ)} \right) \right] - (0 \text{ psf})(195.89 \text{ ft}) = -525 \text{ lb/ft}$$



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TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

$$E_{NB} = \left[W_{NB} \frac{\sin(\phi_M)}{\sin(90^\circ - \beta - \phi_M)} \right] = \left[(217 \text{ lb/ft}) \left(\frac{\sin(9.13^\circ)}{\sin(90^\circ - 18.43^\circ - 9.13^\circ)} \right) \right] = 39 \text{ lb/ft}$$

$$\Delta E = E_A - E_{NB} = -525 \text{ lb/ft} - 39 \text{ lb/ft} = -564 \text{ lb/ft}$$

For Seismic Conditions (no compactor loading), calculate ΔE for a factor of safety of 1.3:

$$E_A = \left[(1.0011) W_A \frac{\sin(\beta - \delta_M + \alpha)}{\sin(90^\circ - \delta_M)} \right] - (C_a)(L) =$$

$$= \left[(1.0011)(25,466 \text{ lb/ft}) \left(\frac{\sin(18.43^\circ - 18.99^\circ + 2.64^\circ)}{\sin(90^\circ - 18.99^\circ)} \right) \right] - (0 \text{ psf})(195.89 \text{ ft}) = 979 \text{ lb/ft}$$

$$E_{NB} = \left[(1.0011) W_{NB} \frac{\sin(\phi_M - \alpha)}{\sin(90^\circ - \beta - \phi_M)} \right] = \left[(1.0011)(217 \text{ lb/ft}) \left(\frac{\sin(9.13^\circ - 2.64^\circ)}{\sin(90^\circ - 18.43^\circ - 9.13^\circ)} \right) \right] = 28 \text{ lb/ft}$$

$$\Delta E = E_A - E_{NB} = 979 \text{ lb/ft} - 28 \text{ lb/ft} = 952 \text{ lb/ft}$$

Iterate the calculation until the correct factor of safety is achieved, as shown in the following table. The correct factor of safety will be achieved when ΔE equals zero.

STATIC CONDITIONS						
FS	δ_M	ϕ_M	C_a	E_A	E_{NB}	ΔE
1.5	16.61	7.93	0.0	1,684	33	1,651
1.4	17.72	8.49	0.0	661	36	625
1.35	18.33	8.80	0.0	93	37	56
1.30	18.99	9.13	0.0	-525	39	-564
1.2	20.44	9.88	0.0	-1,902	42	-1,944
1.1	22.13	10.75	0.0	-3,539	46	-3,586
1.0	24.10	11.80	0.0	-5,499	51	-5,550

SEISMIC CONDITIONS						
FS	δ_M	ϕ_M	C_a	E_A	E_{NB}	ΔE
1.4	17.72	8.49	0.0	1,564	25	1,539
1.3	18.99	9.13	0.0	979	28	952
1.20	20.44	9.88	0.0	297	31	266
1.15	21.25	10.30	0.0	-88	33	-121
1.1	22.13	10.75	0.0	-509	35	-544
1.0	24.10	11.38	0.0	-1,476	40	-1,516
0.9	26.43	13.07	0.0	-2,659	46	-2,705



Client: Zion Landfill, Inc.

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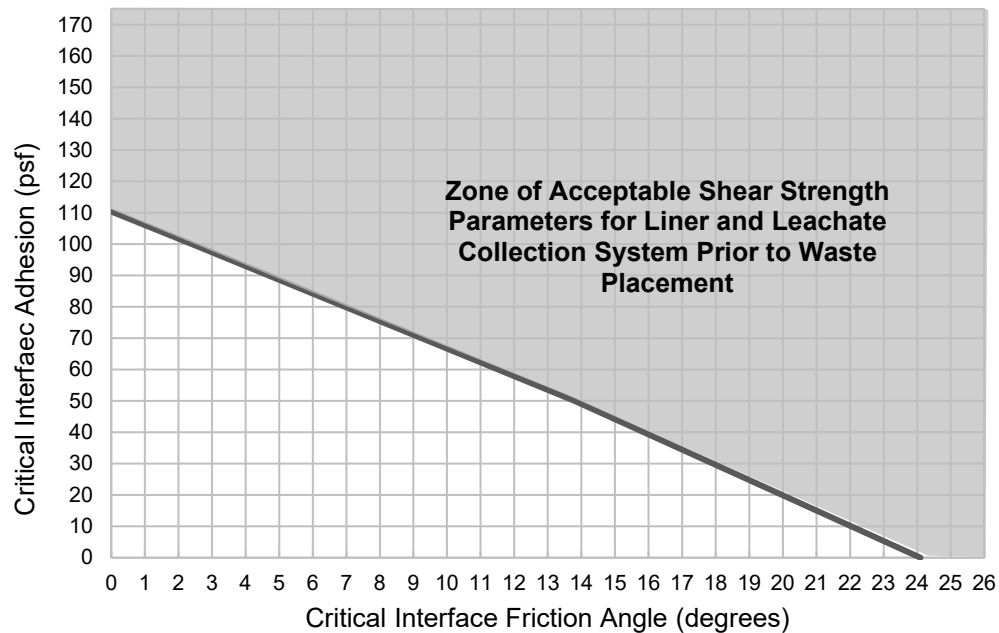
Date: 05/2022

TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

Results

Multiple combinations of friction angles and adhesions were evaluated to determine the minimum acceptable interface peak shear strength envelope to achieve stability of the liner and leachate collection system prior to waste placement using the calculation methodology described in the previous text. Spreadsheets of values used to generate this curve are attached.

The acceptable combinations of peak shear strength values are shown in the shaded region in the graph below. It is noted that the grey line identifies the zone of acceptable shear strength values for the liner and leachate collection system prior to waste placement and the results were obtained using this analysis. It is noted that this range may not be acceptable for global stability analyses after waste placement. Please refer to **Appendix J.2-E** for a shear strength window that meets all landfill design requirements.





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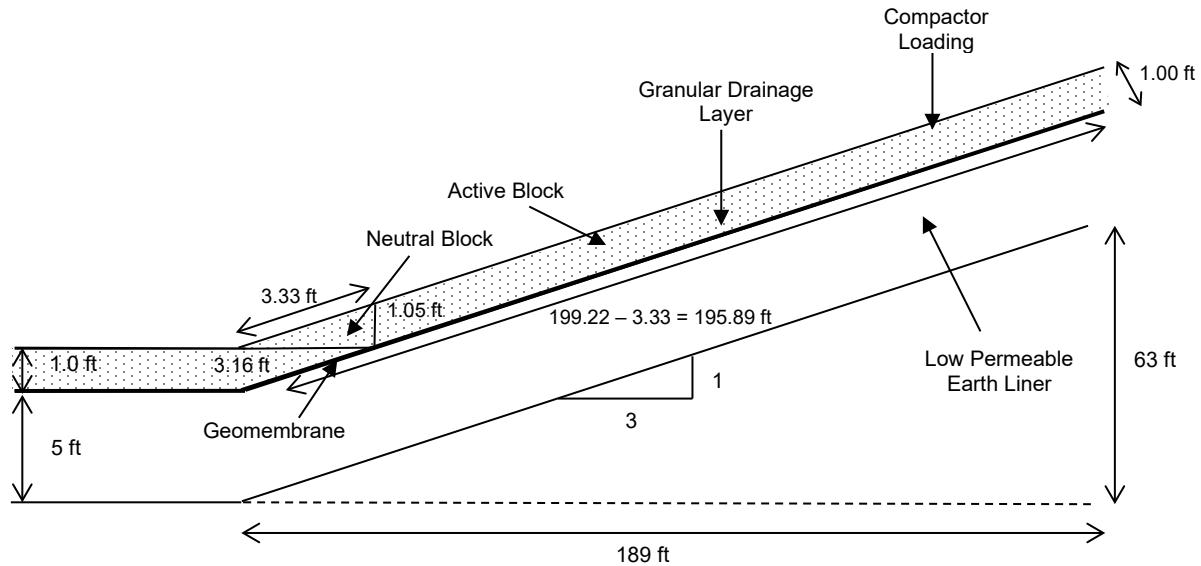
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Cross Section of Liner and Leachate Collection System (not to scale)





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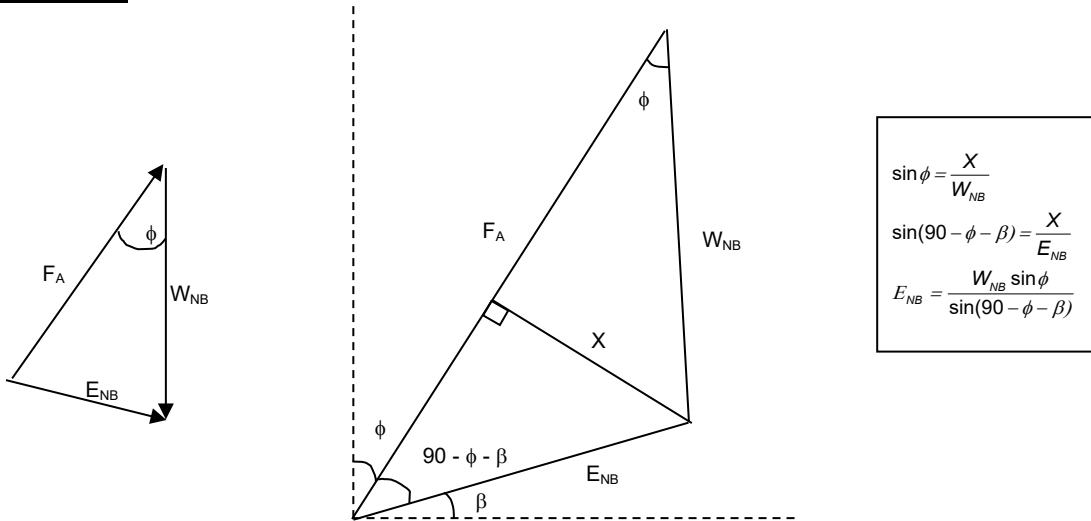
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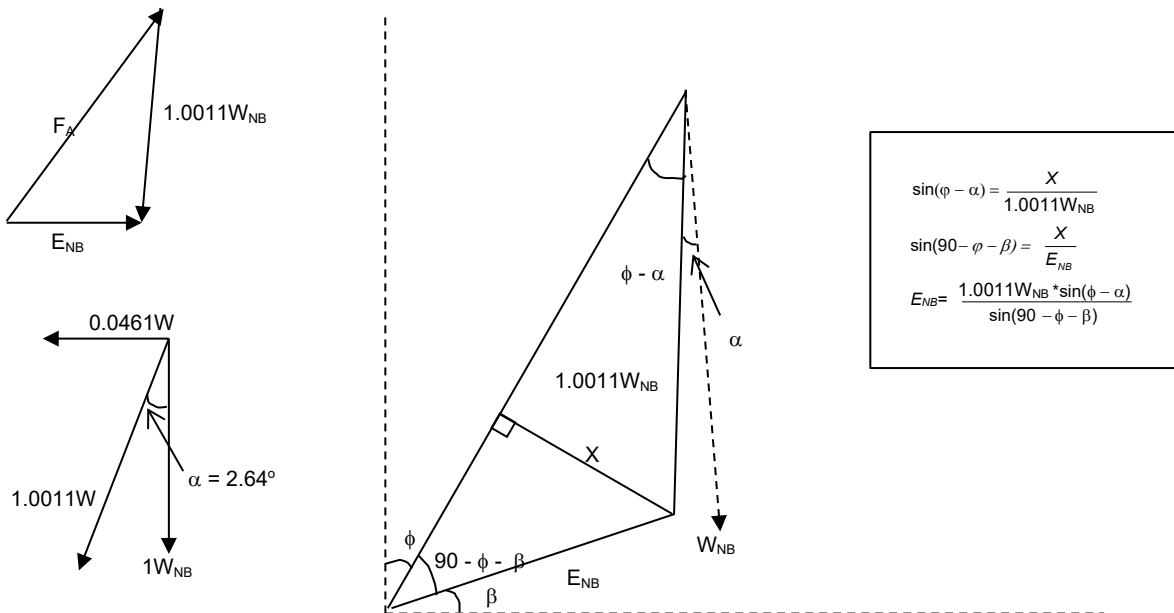
TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

Derive Force of Neutral Block

Static Conditions:



Seismic Conditions





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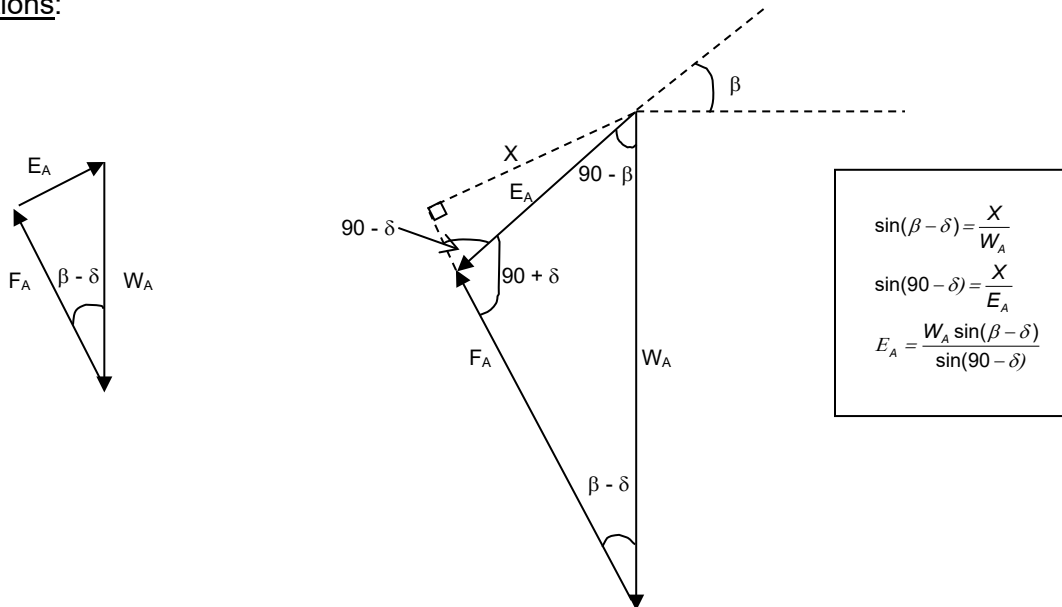
Checked by: DAM

Date: 05/2022

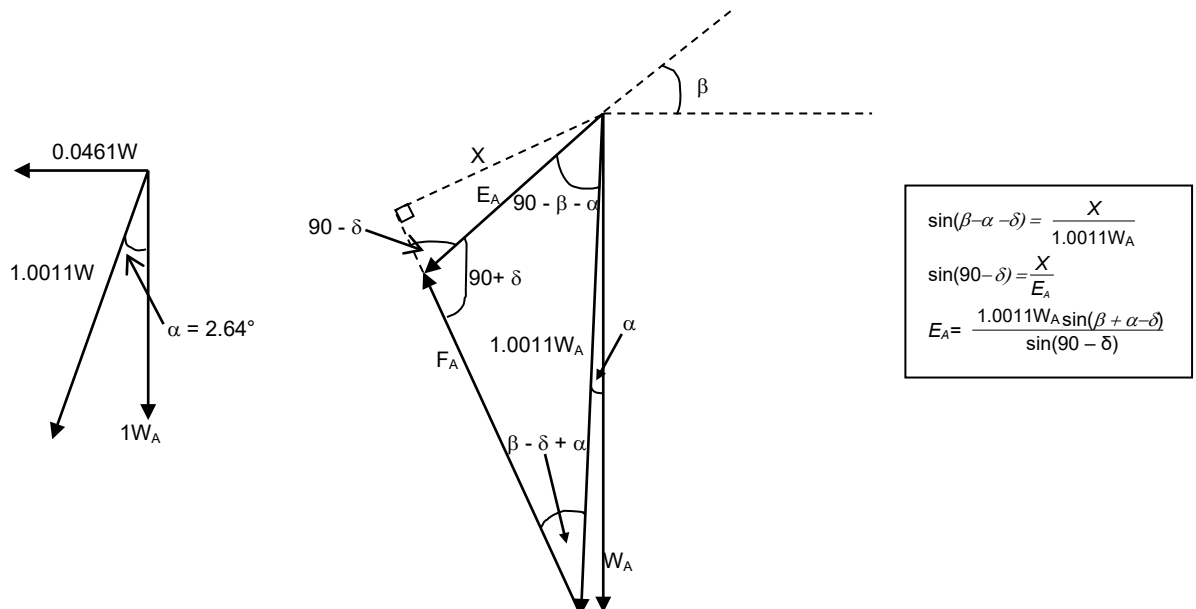
TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM PRIOR TO WASTE PLACEMENT

Derive Force of Active Block

Static Conditions:



Seismic Conditions:



LCS saturated unit weight = 130 pcf LCS drainage layer thickness = 1 feet Tractor weight = 25,342 lb/ft Height of Slope = 63 feet Slope (H:V) = 3 Horizontal : 1 Vertical	Neutral Block Dimensions = 3.16 Base (ft) 1.05 Height (ft) 3.33 Length along Slope (ft)
Cover Soil Friction Angle (ϕ) = 11.8 degrees Critical Interface Friction Angle (δ) = 24.1 degrees Critical Interface Adhesion (C) = 0 psf	

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	7.93	16.61	1,684	33	1,651
1.4	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	8.49	17.72	661	36	625
1.35	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	8.80	18.33	93	37	56
1.30	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	9.13	18.99	-525	39	-564
1.2	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	9.88	20.44	-1,902	42	-1,944
1.1	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	10.75	22.13	-3,539	46	-3,586
1.0	18.43	199.22	195.89	50,807	217	11.8	24	0	0.00	11.80	24.10	-5,499	51	-5,550

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.4	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	8.49	17.72	1,564	25	1,539
1.3	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	9.13	18.99	979	28	952
1.20	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	9.88	20.44	297	31	266
1.15	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	10.30	21.25	-88	33	-121
1.1	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	10.75	22.13	-509	35	-544
1.0	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	11.80	24.10	-1,476	40	-1,516
0.9	18.43	199.22	195.89	25,466	217	11.8	24	0	0.00	13.07	26.43	-2,659	46	-2,705

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



LCS saturated unit weight = 130 pcf LCS drainage layer thickness = 1 feet Tractor weight = 25,342 lb/ft Height of Slope = 63 feet Slope (H:V) = 3 Horizontal : 1 Vertical	Neutral Block Dimensions = 3.16 Base (ft) 1.05 Height (ft) 3.33 Length along Slope (ft)
Cover Soil Friction Angle (ϕ) = 11.8 degrees Critical Interface Friction Angle (δ) = 13.8 degrees Critical Interface Adhesion (C) = 50 psf	

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	18.43	199.22	195.89	50,807	217	11.8	14	50	33.30	7.93	9.30	1,646	33	1,613
1.4	18.43	199.22	195.89	50,807	217	11.8	14	50	35.70	8.49	9.95	613	36	578
1.35	18.43	199.22	195.89	50,807	217	11.8	14	50	37.00	8.80	10.31	46	37	9
1.30	18.43	199.22	195.89	50,807	217	11.8	14	50	38.50	9.13	10.70	-587	39	-626
1.2	18.43	199.22	195.89	50,807	217	11.8	14	50	41.70	9.88	11.57	-1,974	42	-2,016
1.1	18.43	199.22	195.89	50,807	217	11.8	14	50	45.50	10.75	12.59	-3,616	46	-3,662
1.0	18.43	199.22	195.89	50,807	217	11.8	14	50	50.00	11.80	13.80	-5,571	51	-5,623

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.9	18.43	199.22	195.89	25,466	217	11.8	14	50	26.30	6.27	7.37	937	15	922
1.8	18.43	199.22	195.89	25,466	217	11.8	14	50	27.80	6.62	7.77	473	17	456
1.75	18.43	199.22	195.89	25,466	217	11.8	14	50	28.60	6.81	7.99	223	17	206
1.70	18.43	199.22	195.89	25,466	217	11.8	14	50	29.40	7.01	8.22	-32	18	-50
1.6	18.43	199.22	195.89	25,466	217	11.8	14	50	31.30	7.44	8.73	-619	20	-639
1.5	18.43	199.22	195.89	25,466	217	11.8	14	50	33.30	7.93	9.30	-1,254	22	-1,276
1.4	18.43	199.22	195.89	25,466	217	11.8	14	50	35.70	8.49	9.95	-2,002	25	-2,027

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



LCS saturated unit weight = 130 pcf LCS drainage layer thickness = 1 feet Tractor weight = 25,342 lb/ft Height of Slope = 63 feet Slope (H:V) = 3 Horizontal : 1 Vertical	Neutral Block Dimensions = 3.16 Base (ft) 1.05 Height (ft) 3.33 Length along Slope (ft)
Cover Soil Friction Angle (ϕ) = 11.8 degrees Critical Interface Friction Angle (δ) = 2.4 degrees Critical Interface Adhesion (C) = 100 psf	

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	18.43	199.22	195.89	50,807	217	11.8	2	100	66.70	7.93	1.60	1,650	33	1,617
1.4	18.43	199.22	195.89	50,807	217	11.8	2	100	71.40	8.49	1.71	637	36	601
1.35	18.43	199.22	195.89	50,807	217	11.8	2	100	74.10	8.80	1.78	49	37	12
1.30	18.43	199.22	195.89	50,807	217	11.8	2	100	76.90	9.13	1.85	-558	39	-597
1.2	18.43	199.22	195.89	50,807	217	11.8	2	100	83.30	9.88	2.00	-1,938	42	-1,981
1.1	18.43	199.22	195.89	50,807	217	11.8	2	100	90.90	10.75	2.18	-3,579	46	-3,625
1.0	18.43	199.22	195.89	50,807	217	11.8	2	100	100.00	11.80	2.40	-5,547	51	-5,598

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
2.4	18.43	199.22	195.89	25,466	217	11.8	2	100	41.70	4.97	1.00	581	10	571
2.3	18.43	199.22	195.89	25,466	217	11.8	2	100	43.50	5.19	1.04	210	11	199
2.25	18.43	199.22	195.89	25,466	217	11.8	2	100	44.40	5.30	1.07	24	11	13
2.20	18.43	199.22	195.89	25,466	217	11.8	2	100	45.50	5.42	1.09	-202	12	-213
2.1	18.43	199.22	195.89	25,466	217	11.8	2	100	47.60	5.68	1.14	-635	13	-647
2.0	18.43	199.22	195.89	25,466	217	11.8	2	100	50.00	5.96	1.20	-1,128	14	-1,142
1.9	18.43	199.22	195.89	25,466	217	11.8	2	100	52.60	6.27	1.26	-1,664	15	-1,679

PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



LCS saturated unit weight = 130 pcf LCS drainage layer thickness = 1 feet Tractor weight = 25,342 lb/ft Height of Slope = 63 feet Slope (H:V) = 3 Horizontal : 1 Vertical	Neutral Block Dimensions = 3.16 Base (ft) 1.05 Height (ft) 3.33 Length along Slope (ft)
Cover Soil Friction Angle (ϕ) = 11.8 degrees Critical Interface Friction Angle (δ) = 0.0 degrees Critical Interface Adhesion (C) = 110.4 psf	

Static Conditions - Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
1.5	18.43	199.22	195.89	50,807	217	11.8	0	110	73.60	7.93	0.00	1,645	33	1,612
1.4	18.43	199.22	195.89	50,807	217	11.8	0	110	78.90	8.49	0.00	607	36	571
1.35	18.43	199.22	195.89	50,807	217	11.8	0	110	81.80	8.80	0.00	39	37	1
1.30	18.43	199.22	195.89	50,807	217	11.8	0	110	84.90	9.13	0.00	-569	39	-607
1.2	18.43	199.22	195.89	50,807	217	11.8	0	110	92.00	9.88	0.00	-1,959	42	-2,002
1.1	18.43	199.22	195.89	50,807	217	11.8	0	110	100.40	10.75	0.00	-3,605	46	-3,651
1.0	18.43	199.22	195.89	50,807	217	11.8	0	110	110.40	11.80	0.00	-5,564	51	-5,615

Seismic Conditions - NO Tractor Loading

Safety Factor	Slope (degrees)	Length of Slope (ft)	Lenth of Sliding Block (ft)	Weight of Active Block (Wa)	Weight of Neutral Block (Wnb)	Internal Friction Angle (degrees)	Critical Interface Friction Angle (degrees)	Critical Interface Adhesion (psf)	Mobilized Adhesion (psf)	S.F. (drainage friction angle)	S.F. (critical friction angle)	Active Block (Ea)	Neutral Block (Enb)	Ea - Enb
2.6	18.43	199.22	195.89	25,466	217	11.8	0	110	42.50	4.59	0.00	839	8	831
2.5	18.43	199.22	195.89	25,466	217	11.8	0	110	44.20	4.78	0.00	506	9	497
2.40	18.43	199.22	195.89	25,466	217	11.8	0	110	46.00	4.97	0.00	154	10	144
2.35	18.43	199.22	195.89	25,466	217	11.8	0	110	47.00	5.08	0.00	-42	10	-52
2.3	18.43	199.22	195.89	25,466	217	11.8	0	110	48.00	5.19	0.00	-238	11	-249
2.2	18.43	199.22	195.89	25,466	217	11.8	0	110	50.20	5.42	0.00	-669	12	-681
2.1	18.43	199.22	195.89	25,466	217	11.8	0	110	52.60	5.68	0.00	-1,139	13	-1,152

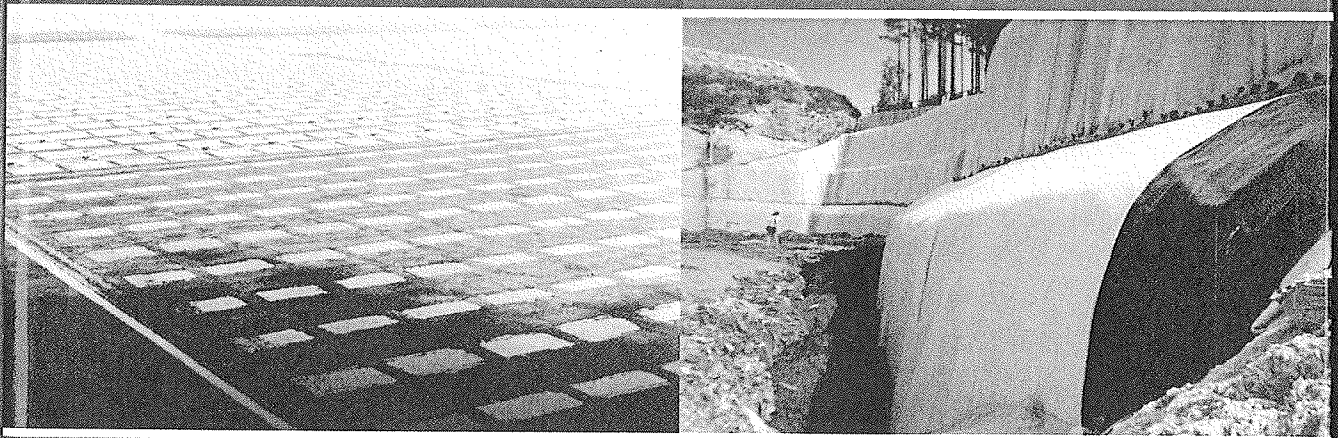
PGA 2%,50yr (%g)	α (degrees)	Result. Force
4.61	2.64	1.0011



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DESIGNING WITH GEOSYNTHETICS

FIFTH EDITION



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the same as that described in Section 2.7.4 using geotextiles. High strength geotextiles and geogrids are competitive in this particular application. The technique is considered very appropriate when stone columns are used as the ground modification technique. Sometimes the stone columns are actually contained in a geogrid enclosure, which appears to be a growing application [56].

3.2.7 Veneer Cover Soils

Whenever a lined slope (geomembrane, GCL, or compacted clay) is covered with soil, a stability calculation should be made to assess the potential for sliding failure of the soil on the barrier layer. Three situations come to mind: (1) landfill liners with leachate collection sand or gravel above them until such time that the solid waste acts as a passive resistance restraint; (2) surface impoundment liners where the cover soil is placed over the geomembrane to shield it from ultraviolet light, heat degradation, and equipment damage; and (3) landfill covers that have topsoil and protection soil placed over the geomembrane. In all cases the soil layer is relatively thin (0.3 to 1.0 m), hence the sliding stability of such a veneer of cover soil is the issue.

Due to the typically low shear strength of the covering soil to the liner material, numerous stability problems have arisen. The driving forces creating the instability are gravitational forces, equipment loads, surcharge loads, seepage forces, and/or seismic forces. Each must be carefully considered in the context of the site-specific conditions.

Koerner and Soong [57] have analyzed the general situation through the use of limit equilibrium and a finite slope model, as shown in Figure 3.22. Consider a cover soil placed directly on a geomembrane (or other barrier layer) at a slope angle β . Two discrete zones can be visualized, as shown in Figure 3.22a. There is a small passive wedge near the toe of the slope resisting a long thin active wedge extending the length of the slope. It is assumed that the cover soil is of uniform thickness and constant unit weight. At the top of the slope or at an intermediate berm, we anticipate that a tension crack in the cover soil will occur, thereby breaking continuity with the remaining cover soil at the crest.

Resisting the tendency for the cover soil to slide is the interface friction and/or adhesion of the cover soil to the specific type of underlying geomembrane. The shear strength values of δ and c_a must be obtained from a laboratory direct-shear test, as described earlier. Note that the passive wedge is assumed to move on the underlying cover soil so that the shear strength parameters ϕ and c , which come from soil-to-soil friction tests, will also be required.

By taking free bodies of the passive and active wedges with the appropriate forces being applied, the formulation for the factor of safety results. The resulting equation is not an explicit solution for the FS, and it must be solved using the quadratic equation. The complete development of the equation is given in [57]. Other approaches are found in Giroud and Beech [58], Koerner and Hwu [59], and Thiel and Stewart [60].

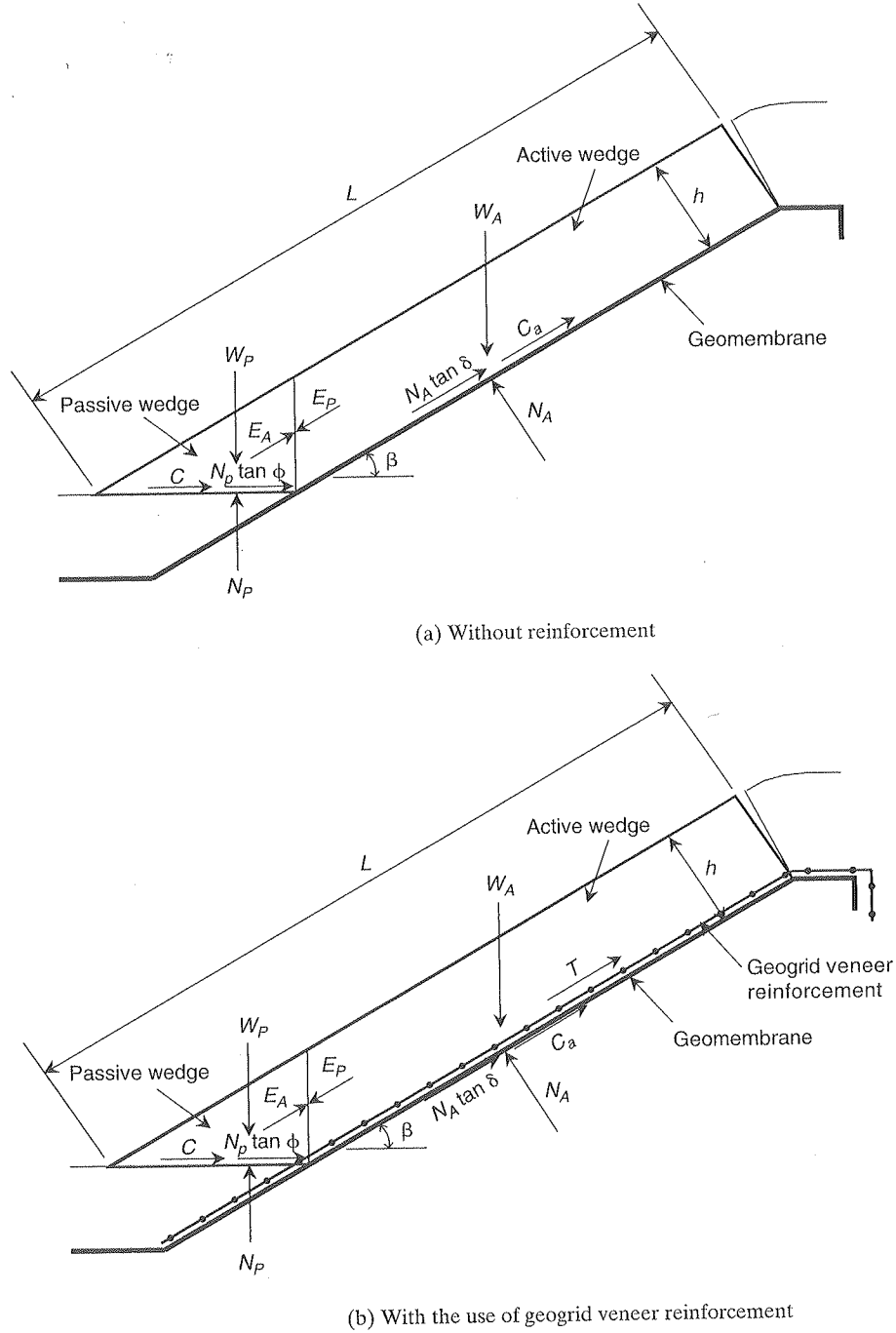


Figure 3.22 Limit equilibrium forces involved in a finite length slope analysis for a uniformly thick cover soil. (After Koerner and Soong [57])

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The expression for determining the factor of safety, considering the active wedge, can be derived as follows:

$$W_A = \gamma h^2 \left(\frac{L}{h} - \frac{1}{\sin \beta} - \frac{\tan \beta}{2} \right) \quad (3.15)$$

$$N_A = W_A \cos \beta \quad (3.16)$$

$$C_a = c_a \left(L - \frac{h}{\sin \beta} \right) \quad (3.17)$$

By balancing the forces in the vertical direction, the following formulation results:

$$E_A \sin \beta = W_A - N_A \cos \beta - \frac{N_A \tan \delta + C_a}{\text{FS}} \sin \beta$$

Hence the interwedge force acting on the active wedge is

$$E_A = \frac{(\text{FS})(W_A - N_A \cos \beta) - (N_A \tan \delta + C_a) \sin \beta}{\sin \beta (\text{FS})}$$

The passive wedge can be considered in a similar manner:

$$W_p = \frac{\gamma h^2}{\sin 2\beta} \quad (3.18)$$

$$N_p = W_p + E_p \sin \beta \quad (3.19)$$

$$C = \frac{(c)(h)}{\sin \beta} \quad (3.20)$$

By balancing the forces in the horizontal direction, the following formulation results:

$$E_p \cos \beta = \frac{C + N_p \tan \phi}{\text{FS}}$$

Hence the interwedge force acting on the passive wedge is

$$E_p = \frac{C + W_p \tan \phi}{\cos \beta (\text{FS}) - \sin \beta \tan \phi}$$

By setting $E_A = E_p$, the following equation can be arranged in the form of $ax^2 + bx + c = 0$, which in our case, using FS values, is

$$a(\text{FS})^2 + b(\text{FS}) + c = 0 \quad (3.21)$$

where

$$a = (W_A - N_A \cos \beta) \cos \beta, \quad (3.22)$$

$$b = -[(W_A - N_A \cos \beta) \sin \beta \tan \phi + (N_A \tan \delta + C_a) \sin \beta \cos \beta + \sin \beta (C + W_p \tan \phi)], \text{ and} \quad (3.23)$$

$$c = (N_A \tan \delta + C_a) \sin^2 \beta \tan \phi \quad (3.24)$$

wedge,

The resulting FS value is then obtained from the following equation:

(3.15)

$$FS = \frac{-b + \sqrt{b^2 - 4ac}}{2a} \quad (3.25)$$

(3.16)

where (in Figure 3.22a and in the above analysis)

(3.17)

W_A = total weight of the active wedge,

W_P = total weight of the passive wedge,

N_A = effective force normal to the failure plane of the active wedge,

N_P = effective force normal to the failure plane of the passive wedge,

γ = unit weight of the cover soil,

h = thickness of the cover soil,

L = length of slope measured along the geomembrane,

β = soil slope angle beneath the geomembrane,

ϕ = friction angle of the cover soil,

δ = interface friction angle between cover soil and geomembrane,

(3.18)

C_a = adhesive force between cover soil of the active wedge and the geomembrane,

(3.19)

c_a = adhesion between cover soil of the active wedge and the geomembrane,

(3.20)

C = cohesive force along the failure plane of the passive wedge,

c = cohesion of the cover soil,

E_A = interwedge force acting on the active wedge from the passive wedge,

E_P = interwedge force acting on the passive wedge from the active wedge, and

FS = factor of safety against cover soil sliding on the geomembrane

results:

When the calculated FS value falls below 1.0, a stability failure of the cover soil sliding on the geomembrane is to be anticipated. Thus a value greater than 1.0 must be targeted as being the minimum factor of safety. How much greater than 1.0 the FS value should be is a design and/or regulatory issue. Example 3.12 illustrates the procedure.

Example 3.12

form of

Given a cover soil slope of $\beta = 18.4^\circ$ (i.e. 3H-to-1V), $L = 30$ m, $h = 900$ mm, $\gamma = 18$ kN/m³, $c = 0$, $\phi = 30^\circ$, $c_a = 0$, $\delta = 18^\circ$, determine the resulting factor of safety.

Solution:

(3.21)

$$W_A = \gamma h^2 \left(\frac{L}{h} - \frac{1}{\sin \beta} - \frac{\tan \beta}{2} \right)$$

(3.22)

$$= (18.0)(0.90)^2 \left(\frac{30}{0.90} - \frac{1}{\sin 18.4} - \frac{\tan 18.4}{2} \right)$$

(3.23)

$$= 14.58(33.3 - 3.17 - 0.17)$$

(3.24)

$$= 437 \text{ kN/m}$$

For termination of double liner systems, the designer is faced with a number of possible choices. Major considerations are to protect the integrity of both geomembranes and to keep surface water out of the leak detection system. In this regard, the two geomembranes can enter separate anchor trenches or come together in a common anchor trench. The primary geomembrane can also be cut short of the anchor trench and welded to the secondary geomembrane along the horizontal runout distance. In seismically active areas, consideration should be given to this latter approach with no vertical anchor trench at all; the logic being that geomembrane pullout is more desirable than geomembrane tensile failure somewhere along the side slope.

The terminus of the liner of a completed internal cell within a zoned landfill, with its eventual extension into an adjacent cell, is usually done by overlapping and seaming along the horizontal runout length of an intermediate berm. When waste fills the second cell, the berm is entombed and the process is then continued from cell to cell. Shear stresses on the geomembranes in both cells over this berm have been evaluated by large-scale laboratory models and found to be generally small and geomembrane-dependent (see Koerner and Wayne [79]). In high berms where higher stresses are generated, an auxiliary (or sacrificial) geomembrane rub-sheet over the crest of the berm should effectively dissipate the stresses before they propagate down to the underlying primary geomembrane.

5.6.9 Side Slope Subgrade Soil Stability

The design of the stability of the soil mass beneath the liner system of a solid-waste landfill is carried out in exactly the same manner as was discussed for liquid containment (reservoir) slopes and berms (recall Section 5.3.5). The process can include the strength of the covering liner materials, but if they are not included in the analysis, the error is on the conservative side. Interior berms, with or without geosynthetic inclusions, are also handled in the same manner as previously described.

5.6.10 Multilined Side Slope Cover Soil Stability

The situation of a liner and its leachate collection cover soil stability, or slumping, becomes quite complicated for multilined geomembrane and geonet collection systems of the type shown in Figure 5.40. Consider such a system, as shown in Figure 5.40e. The leachate collection system soil gravitationally induces shear stress through the system, thereby challenging each of the interface layers that are in the cross section. If all of the interface shear strengths are greater than the slope angle, stability is achieved and the only deformation involved is a small amount to achieve elastic equilibrium (Wilson-Fahmy and Koerner [80]). However, if any interface shear strengths are lower than the slope angle, wide-width tensile stresses are induced into the overlying geosynthetics. This can cause the failure of the geosynthetics or pullout from the anchor trench, or it can result in quasistability via tensile reinforcement. If the last is the case, we can refer to the overlying geosynthetics as acting as *nonintentional* veneer reinforcement.

If the situation consists of the double liner system shown in Figure 5.45, all of the interface surfaces can be made quite stable by proper selection of the geosynthetics.

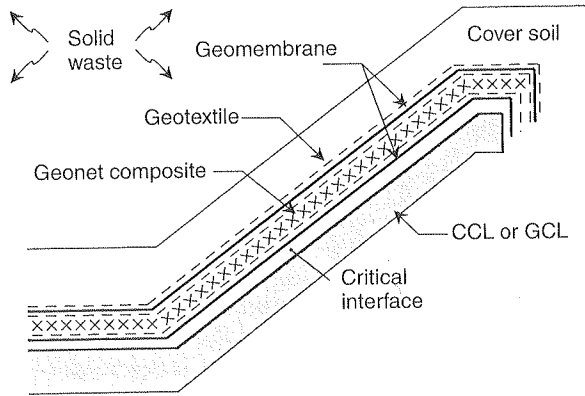


Figure 5.45 Geotextile/geomembrane/geonet composite/geomembrane above a CCL or GCL.

For example, textured geomembranes could be selected, and these together with nonwoven needle-punched geotextiles will usually result in peak friction angles in excess of 25°. Furthermore, by thermally bonding the geotextiles in the leak detection system to the geonet, these surfaces are also stable at relatively high slope angles. Thus, the critical interfaces are at the upper (leachate collection sand or gravel) and the lower (CCL or GCL) surfaces. The upper surface is analyzed exactly as described in Section 3.2.7 for the case without geogrid reinforcement. The proper selection of cover soil against a nonwoven needle-punched geotextile (acting as a protection material, recall Section 5.6.7) should also result in a peak friction angle in excess of 25°. This leaves the lower surface of the secondary geomembrane against the clay liner as being the potentially low-interface surface. If the clay liner is a CCL, the concern is with the expelled consolidation water lubricating the interface. This surface has been involved in a major failure of a hazardous waste liner system, as reported by Byrne et al. [81] with an interface friction angle of 10°. If the liner is a GCL, the concern is the hydrated bentonite being extruded out of the upper geotextile and lubricating the interface with an interface friction angle of 5 to 10°. This surface was involved in two slides of full-scale field tests both involving woven geotextiles on the GCL, by Daniel et al. [82].

The analysis of multilined slopes of the type being discussed is a direct extension of the veneer reinforcement model presented in Section 3.2.7 on geogrids. Recalling Figure 3.22b, the analysis results in equation (3.21):

$$a(\text{FS})^2 + b(\text{FS}) + c = 0$$

where

$$a = (W_A - N_A \cos \beta - T \sin \beta) \cos \beta,$$

$$b = -[(W_A - N_A \cos \beta - T \sin \beta) \sin \beta \tan \phi + (N_A \tan \delta + C_a) \sin \beta \cos \beta + \sin \beta (C + W_P \tan \phi)], \text{ and}$$

$$c = (N_A \tan \delta + C_a) \sin^2 \beta \tan \phi.$$

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The resulting FS value is then obtained from equation (3.22):

$$FS = \frac{-b + \sqrt{b^2 - 4ac}}{2a}$$

The variables and values of W_A , N_A , T , and W_P were defined in Sections 3.2.7 and 5.3.5. The critical parameter in the above equation is T , the allowable wide-width tension strength of the geosynthetic layers above the potential failure surface. For the cross section shown in Figure 5.45, T represents the allowable strength of all of the geosynthetic materials above the critical interface. Not only is the issue of reduction factors difficult to assess for the liner materials per se, but the issue of strain compatibility is also unwieldy. In this latter regard, the wide-width tensile strength of each geosynthetic material must be determined, plotted on the same axes, and assessed at a specific value of strain. That is, the liner system components cannot act individually and must act as an equally strained unit. Example 5.20 illustrates the situation.

Example 5.20

For a 30 m long slope at 3(H) to 1(V)—i.e., $\beta = 18.4^\circ$ —lined with a double liner system consisting of GT/GM/GC/GM/CCL or GCL (as in Figure 5.45), the lowest friction angle is assumed to be the secondary geomembrane to the underlying clay interface, which is 10° . All other interface friction angles are in excess of 18.4° . The wide-width tensile behavior of the various candidate geosynthetics is given in the following graph. The leachate collection cover soil is 450 mm thick with a unit weight of 18.0 kN/m^3 and a friction angle of 30° . What is the factor of safety of the slope based on a cumulative reduction factor of 2.0?

Solution:

$$\begin{aligned} W_A &= \gamma h^2 \left[\frac{L}{h} - \frac{1}{\sin \beta} - \frac{\tan \beta}{2} \right] \\ &= (18.0)(0.45)^2 \left[\frac{30}{0.45} - \frac{1}{\sin 18.4} - \frac{\tan 18.4}{2} \right] \\ &= 3.65[63.3] \\ &= 231 \text{ kN/m} \\ N_A &= W_A \cos \beta \\ &= 231 \cos 18.4 \\ &= 219 \text{ kN/m} \\ W_P &= \frac{\gamma h^2}{\sin 2\beta} \\ &= \frac{(18.0)(0.45)^2}{\sin 36.8} \\ &= 6.08 \text{ kN/m} \end{aligned}$$

T_{ult} taken at the first geosynthetic failure, which is the nonwoven needle-punched geotextile at 25 kN/m, is

$$\begin{aligned} T_{\text{ult}} &= 25 + 2(22) + 36 \\ &= 105 \text{ kN/m} \end{aligned}$$

If the cut-slope is made in a cohesive material (containing clay and silt), the failure is usually deeper and like rotational slips as shown in Fig. 2. The long-term (prior to waste filling and after waste filling) and the end-of-excavation stability have to be analyzed separately. The former requires an effective stress analysis using drained (effective) strength parameters (ϕ' , c') and the later a total stress analysis using undrained (total) strength parameters (c_u , usually $\phi_u = 0$) determined on undisturbed samples of the materials. Typically, a finite slope analysis using a method of slices such as the modified Bishop method is used for the stability analysis. The total stress analysis can be performed also with the simpler circular arc method. The position of the failure surface is unknown and determined by a trial and error method. There are computer programs available for slope stability analysis (Bosscher, 1990). Chart solutions for simple slopes using the finite slope analysis are provided in Appendix A.

Berm Slopes

Since berms are constructed by compacting selected earthen materials, the characterization of the materials is much simpler than the cuts. The end-of-construction and the long-term analyses can be made in a manner similar to the ones described for cut slopes. The end-of-construction stability is likely to be more critical than the long-term stability. The strength parameters are determined on compacted specimens of the borrow materials, i.e., ϕ_u , c_u and ϕ' , c' .

COVER SOIL STABILITY

The usual cover soil is a relatively thin layer of soil of either uniform thickness or tapered thickness. When clay covers are placed on side slopes, they sometimes fail by slumping. The accumulated soil gathers at the toe of the slope. This same situation is even more apt to occur for cover soils placed on geosynthetics, which are invariably lower in frictional resistance than the substrate soils from which the slope itself is formed.

The failure of cover soils with uniform depth is basically a surface raveling type of failure along the interface as shown in Fig. 3 and can be analyzed by an infinite slope analysis using the following expression for a safety factor:

$$F = (\tan \delta / \tan \beta) [1 - (\gamma_w h_p / \gamma d) \sec^2 \beta] \quad (2)$$

where δ is the interface friction (between the geosynthetic and cover soil or the in situ soil); the other terms are as defined before in Fig. 1. The critical parameters are the interface friction angle or the shear strength between the soil and the geosynthetic, the inclination of slope, and the seepage forces (due to infiltrating water or rapid draw down of reservoir levels).

To alleviate the slumping of cover soils down slope, it is quite common to construct them with a taper, i.e., thicker at the bottom and gradually thinner going toward the top as shown in Fig. 4. The stability can be analyzed using a wedge stability method as described in Appendix B.

SLOPE LINER STABILITY

Principal sliding surfaces in landfills often coincide within the multi-layer liner system underlying the wastefill. The Kettleman Hills waste landfill slide in California occurred in this fashion in 1988. The foundation soil including the cut slopes is usually

much stronger than the critical liner-system interfaces because the settlement considerations usually dictate selection of sites with competent foundation soils. There is some evidence that relatively steep slopes of high refuse fills also do not fail in deep rotational slides if they are founded on firm foundations. Therefore, the liner-system interfaces become the most critical slip zone. The interfaces may be: (1) between geomembrane liner and geotextile or geonet layer, (2) between geomembrane liner and compacted clay liner, and (3) between geotextile or geomembrane and granular layer. The interfaces between these materials are characterized by low frictional resistance. The frictional resistance is affected by various properties including degree of polishing (of geomembrane by geotextile with increasing shear displacement) and whether the interfaces are wet or dry. The interface strength properties can be evaluated by use of laboratory direct shear or pullout tests (Mitchell et al, 1990).

Failure by Sliding on Liner Interfaces

The stability analysis for sliding along planar liner interface surfaces as shown in Fig. 5 can be performed using a wedge stability method as given in Appendix B. The analysis of Kettleman Hills waste landfill slide by this approach provided a reasonable estimate of the failure based on the interface shear strengths measured in the laboratory. Three-dimensional effects were found to lower safety factors by 10 to 15% (Seed et al., 1990).

Failure with Liner Pullout or Break

In the geomembrane lined reservoirs or landfills, the liner comes up from the bottom of the pit, covering the side slopes, then running over the top a short distance. It often terminates vertically down in an anchor trench as shown in Fig. 6.

The stress generated in the geomembrane, σ , for a given length of runout, L_{TO} , and trench depth, d_t , can be obtained from the following expression:

$$\sigma = \frac{\gamma d_c \tan \delta + 2K_o \gamma d_t (d_c + 0.5 d_t) \tan \delta + 2\gamma (d_c + d_t) b \tan \delta}{t (\cos \beta - \sin \beta \tan \delta)} \quad (3)$$

where

t = the geomembrane thickness

β = the slope angle

δ = the interface friction angle

d_c = the depth of cover soil

b = the width of trench

γ = the unit weight of cover and trench soil

K_o = the lateral coefficient of earth in trench (it can be taken as 0.5)

The safety factor, F , against pullout or break can be computed from

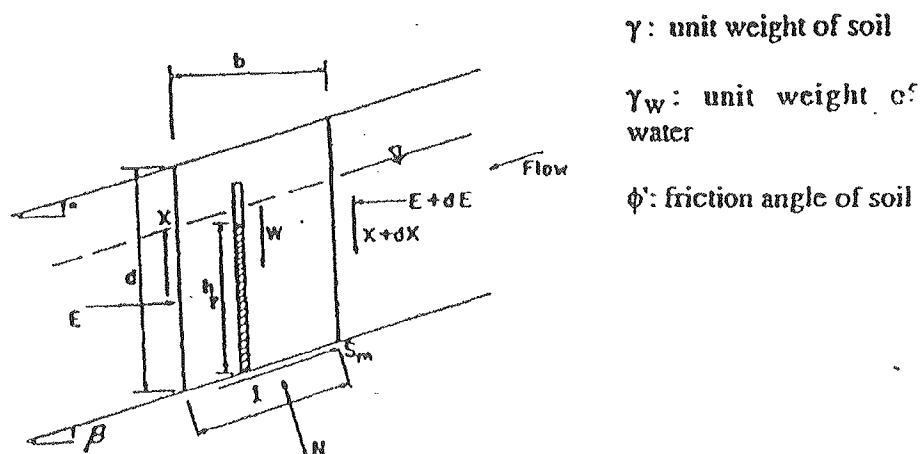


Fig. 1. Idealized Infinite Slope Analysis

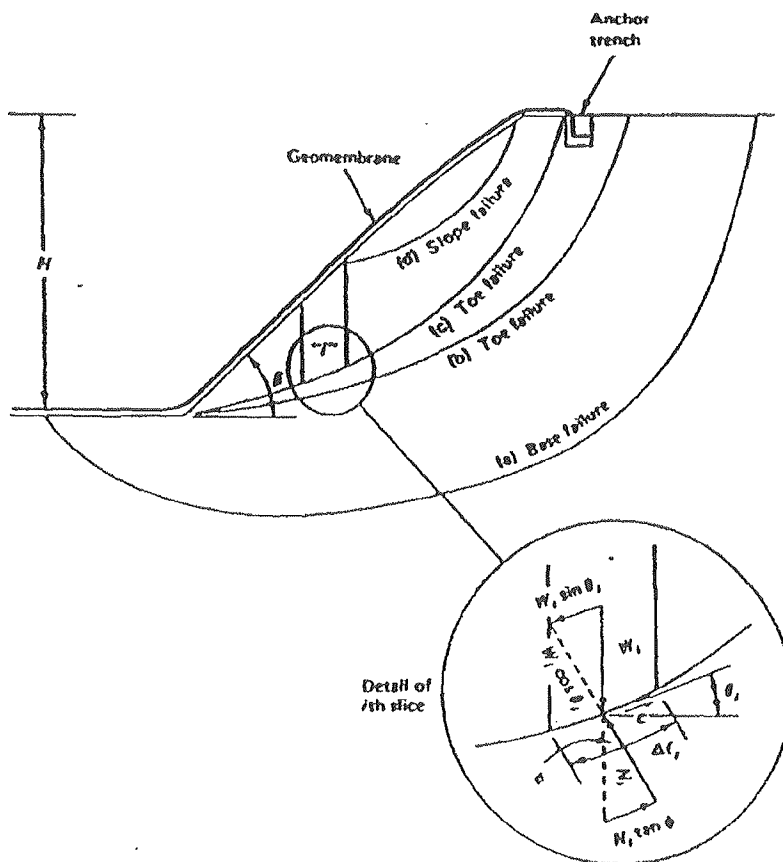


Fig. 2. Various Types of Finite Slope Failures

APPENDIX B

WEDGE METHOD OF SLOPE STABILITY ANALYSIS IN WASTE DISPOSAL

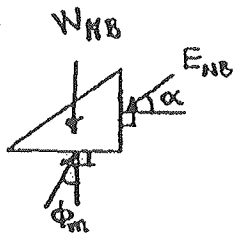
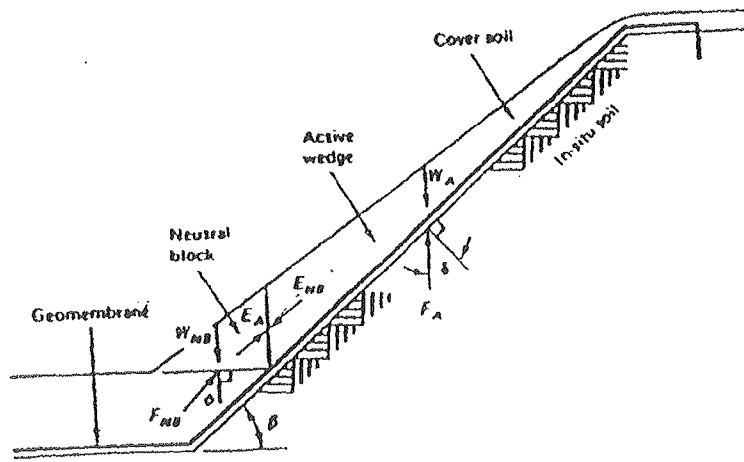
In this method the potential sliding mass is separated into a series of wedges and the vertical and horizontal force equilibrium of each wedge is considered in turn. This method is most appropriate for conditions where the failure surface can be approximated by a series of planar surfaces. For analysis by the wedge method, the mass above the trial slip surface is divided by vertical lines into a number of wedges. The side force transmitted across the interface between any two wedges is not known but an assumption regarding its inclination must be made. The assumption that the side forces between wedges are horizontal is conservative with an error for most cases no more than 15%. Experience indicates that assuming an inclination of 10 to 15 degrees from the horizontal usually gives reasonable results. The factor of safety, F (defined as the ratio between the shear strength and the shear stress required for equilibrium), is calculated by trial-and-error. A value of F is assumed, and then checked to determine if the shear resistance along the failure surface reduced by this F satisfies equilibrium. If not, a new trial is initiated with another assumed safety factor. The analysis can be performed either graphically or numerically.

Two common cases encountered in waste disposal which can be analyzed by the wedge method are presented by use of the numerical method. These cases involve the stability along a liner and tapered depth soil cover as shown in Figure B1 and B2. The steps are as follows:

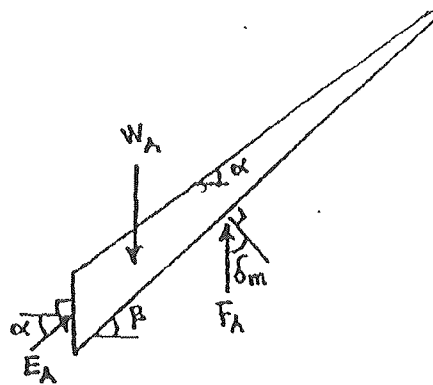
1. Define a trial slip surface. In these two cases it is primarily along the soil or waste and liner interface.
2. Divide the mass above the slip surface into wedges. In these two cases, the sliding mass is divided into an active wedge and a neutral block.
3. Calculate the weight of each wedge. This analysis is a two-dimensional analysis and the stability of a unit length of material along the slope is considered. Therefore, the weight of each wedge is its cross-sectional area (times unit length) times the unit weight of the sliding mass.
4. Assume a value of factor of safety, F and calculate the trial values of mobilized friction angle along the slip surface, δ_m (soil/waste-liner friction angle) or ϕ_m (soil-soil friction angle) using the following definitions:

$$\tan \delta_m = \frac{\tan \delta}{F} \qquad \tan \phi_m = \frac{\tan \phi}{F}$$

5. Assume the inclination of the force acting between the active wedge and the neutral block, α . For the tapered depth cover soil, α may be assumed parallel to cover soil slope. Otherwise, assume a value between 0 - 20°.
6. Using the equations given in Figures B1 and B2, calculate the force from neutral block acting on active wedge, E_A and the force from active wedge acting on neutral block, E_{NB} from the force equilibrium of each block. These forces are equal in magnitude opposite in direction at equilibrium. Note that the friction is mobilized in proportion to the assumed F along the slip surface; therefore, there may be a difference, ΔE in the calculated magnitudes of E_A and E_{NB} .
7. Plot ΔE versus assumed F.
8. Assume a new F and repeat steps 6 and 7. Try additional values of F until ΔE is negligibly small. This value of F is the correct safety factor.



NEUTRAL BLOCK



ACTIVE BLOCK

$$E_A = W_A \frac{\sin(\beta - \delta_m)}{\sin(90^\circ + \alpha - \beta + \delta_m)}$$

$$E_{NB} = W_{NB} \frac{\sin \phi_m}{\sin(90^\circ - \alpha - \phi_m)}$$

$$\Delta E = E_A - E_{NB}$$

$$\tan \delta_m = \frac{f_{cu} \delta}{F}$$

$\alpha = \beta$ in leachate collection system (18.4°)

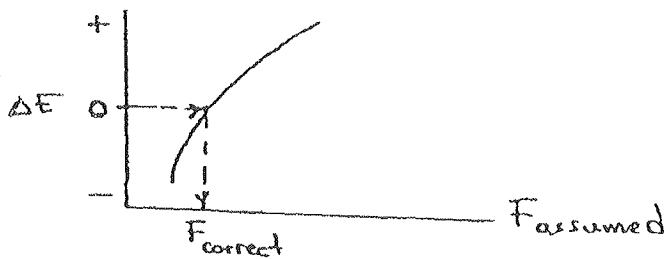


Figure B1. Wedge analysis as applied to tapered depth soil cover.

836K

Landfill Compactor



Engine

Engine Model	Cat® C18 ACERT™	
Emissions	Tier 4 Final/Stage IV	
Gross (SAE J1349)	419 kW	562 hp

Operating Specifications

Maximum Operating Weight –	55 927 kg	123,319 lb
Multiple Blade and Wheel Offerings		

836K Landfill Compactor Specifications

Engine

Engine Model	C18 ACERT	
Emissions	U.S. EPA Tier 4 Final and EU Stage IV	
Rated Power (Lab)	414 kW	555 hp
Rated Power (Net ISO 14396)	412 kW	553 hp
Gross (SAE J1349)	419 kW	562 hp
Net Power – SAE J1349		
Direct Drive – Gross Power	370 kW	496 hp
Direct Drive – Torque Rise	52%	
Converter Drive – Gross Power	370 kW	496 hp
Converter Drive – Torque Rise	52%	
Maximum Gross Torque @ 1,300 rpm	3085 N·m	2,275 lbf-ft
Maximum Altitude without Derating	2286 m	7,500 ft
Bore	145 mm	5.71 in
Stroke	183 mm	7.2 in
Displacement	18.1 L	1,104.5 in ³
High Idle Speed	2,120 rpm	
Low Idle Speed	750 rpm	

Operating Specifications

Operating Weight with Full Tank Capacities and U-blade	55 927 kg	123,319 lb
--	-----------	------------

Transmission

Transmission Type	Planetary – Powershift – ECPC	
Travel Speeds		
Forward – Converter 1st	6.2 km/h	3.9 mph
Forward – Lockup 1st	6.5 km/h	4 mph
Forward – Converter 2nd	10.9 km/h	6.8 mph
Forward – Lockup 2nd	11.7 km/h	7.3 mph
Reverse – Converter 1st	6.5 km/h	4 mph
Reverse – Lockup 1st	6.9 km/h	4.3 mph
Reverse – Converter 2nd	10.4 km/h	6.5 mph
Reverse – Lockup 2nd	12.3 km/h	7.6 mph

Hydraulic System

Hydraulic System	Flow Sharing Implement	
Maximum Supply Pressure	32 000 kPa	4,640 psi
Main Relief Pressure	24 100 kPa	3,495 psi
Pump Flow at 2,006 rpm	250 L/min	66 gal/min
Steering System	Double Acting – End Mounted	
Bore	127 mm	5 in
Stroke	740 mm	29.1 in
Vehicle Articulation Angle	86°	
Lift System	Double Acting Cylinder	
Bore	137.9 mm	5.5 in
Stroke	1021 mm	40.2 in

Service Refill Capacities

Fuel Tank	793 L	209 gal
Cooling System	107 L	28 gal
Crankcase	60 L	16 gal
Diesel Engine Fluid Tank	32.8 L	9 gal
Transmission	120 L	32 gal
Differentials and Final Drives – Front	186 L	49 gal
Differentials and Final Drives – Rear	190 L	50 gal
Hydraulic System (tank only)	240 L	63 gal

- All non-road Tier 4 Final/Stage IV, and Japan (MLIT) Step 4 diesel engines are required to use:
 - Ultra Low Sulfur Diesel (ULSD) fuels containing 15 ppm (mg/kg) sulfur or less. Biodiesel blends up to B20 are acceptable when blended with 15 ppm (mg/kg) sulfur or less ULSD and when the biodiesel feedstock meets ASTM D7467 specifications.
 - Cat DEO-ULS™ or oils that meet the Cat ECF-3, API CJ-4, and ACEA E9 specifications are required.

Axles

Front	Planetary – Fixed
Rear	Planetary – Oscillating
Oscillation Angle	13°

Brakes

Control System	Full Hydraulic Split Circuit
Parking Brake	Spring Applied, Hydraulic Released

Cab

	Standard	Suppression
Interior Sound Level	72 dB(A)	71 dB(A)
Exterior Sound Level	111 dB(A)	109 dB(A)

Hydraulic System – Steering

Steering System – Circuit	Steering Double Acting – End Mounted	
Steering System – Pump	Piston – Variable Displacement	
Maximum Flow @ × rpm	52 L/min @ 2,006 rpm	
Steering Pressure Limited	24 100 kPa	3,495 psi
Total Steering Angle	86 degrees	

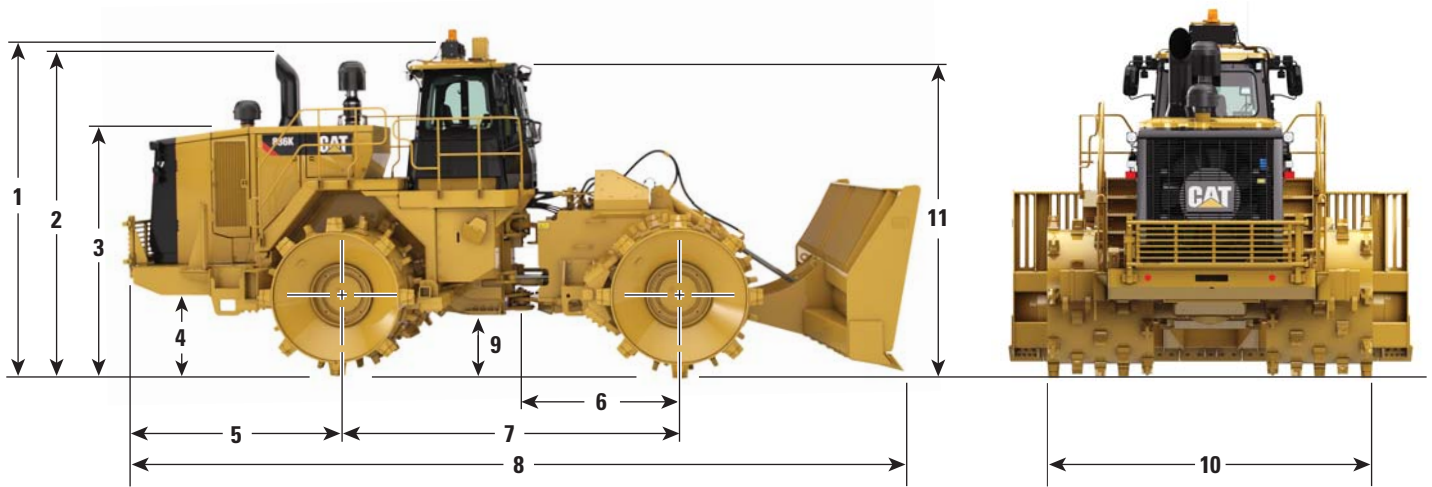
Wheels and Tips

Drum Width	1400 mm	4 ft 8 in
Drum Diameter	1770 mm	5 ft 10 in
Diameter with Tips	2125 mm	7 ft 0 in
Tips per Wheel	40	

836K Landfill Compactor Specifications

Dimensions

All dimensions are approximate.



1	Height to Top of Cab with A/C	4655 mm	15 ft 3 in
2	Height to Top of Exhaust Pipe	4608 mm	15 ft 1 in
3	Height to Top of Hood	3421 mm	11 ft 3 in
4	Ground Clearance to Bumper	1029 mm	3 ft 5 in
5	Center Line of Rear Axle to Edge of Counterweight	3187 mm	10 ft 5 in
6	Hitch to Center Line of Front Axle	2275 mm	7 ft 6 in
7	Wheelbase	4550 mm	14 ft 11 in
8	Length with Blade on Ground (straight blade)	10 182 mm	33 ft 5 in
9	Ground Clearance	632 mm	2 ft 1 in
10	Width over Wheels	4280 mm	14 ft 1 in
11	Height to ROPS/Canopy	4284 mm	14 ft 1 in
	Height to Top of Cab with Strobe	4845 mm	15 ft 11 in
	Turning Radius – Inside of Wheels	3635 mm	11 ft 11 in

Blade Selection

	Straight Blade		Semi U-blade		U-blade	
Width – Moldboard Length	4990 mm	16 ft 4 in	5238 mm	17 ft 2 in	5172 mm	17 ft
Width Over End Bits	5193 mm	17 ft	5311 mm	17 ft 5 in	5258 mm	17 ft 3 in
Height with Cutting Edge and Screen	2236 mm	7 ft 4 in	2215 mm	7 ft 3 in	2210 mm	7 ft 3 in
Height with Cutting Edge, No Screen	1217 mm	4 ft	1253 mm	4 ft 1 in	1255 mm	4 ft 1 in
Maximum Depth of Cut	364 mm	1 ft 2 in	362 mm	1 ft 2 in	934 mm	3 ft 1 in
Maximum Lift above Ground	1730 mm	5 ft 8 in	1735 mm	5 ft 8 in	1198 mm	3 ft 11 in
Cutting Edges, Reversible						
Length, Each End Section (3 edges)	1408.2 mm	4 ft 7 in	816.6 mm	2 ft 8 in	2 @ 779.1 mm and 1 @ 856 mm	2 @ 2 ft 7 in and 1 @ 2 ft 10 in
Length, Each End Section (2 edges)	NA		988 mm	3 ft 3 in	1094.4 mm	3 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
End Bits (2), Self-sharpening						
Length, Each	472 mm	1 ft 7 in	472 mm	1 ft 7 in	472 mm	1 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
Capacity, Rated	19.3 m ³	25.9 yd ³	22.4 m ³	29.3 yd ³	9.74 m ³	13 yd ³
Turning Diameter, Outside Corner of Blade at 43° ART	8737 mm	28 ft 8 in	8823 mm	28 ft 11 in	8795 mm	28 ft 10 in
Overall Machine Length	10 182 mm	33 ft 5 in	10 379 mm	34 ft 1 in	10 272 mm	33 ft 8 in

J.2-C Liner/LCS After Waste Placement (Global Stability Analysis)



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2021

Checked by: DAM

Date: 05/2021

TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM AFTER WASTE PLACEMENT (GLOBAL STABILITY ANALYSIS)

Problem Statement

Determine the range of acceptable liner and leachate collection system peak shear strength parameters that provide a factor of safety against slope failure during the construction/operation and closure periods of the landfill under static and seismic loading conditions.

Title 35 Ill. Admin. Code Section 811.304 (d) requires that “the waste disposal unit shall be designed to achieve a factor of safety against slope failure of at least: 1.5 for static conditions and 1.3 under seismic conditions.”

Background

The material strengths of earthen layers (recompacted clay liner, granular drainage layer) are based on laboratory testing. However, as geosynthetics are manufactured products, they can be designed and/or selected to meet strength requirements. The stability of the liner and leachate collection system is influenced by the interface shear strengths between geosynthetic liner/leachate collection system components.

This calculation is developed to identify the lowest peak shear strength that produces acceptable factors of safety for any potential interface within the liner and leachate collection system during operations and after complete build-out of the landfill. The interfaces that are considered include the following:

- Geotextile to granular drainage layer;
- Geotextile to geomembrane; or
- Geomembrane to low permeable earth liner.

It is noted that all interfaces will be required to meet the minimum interface peak shear strength values determined in this calculation.

Given

- Hydrogeologic and Design Drawings contained in this Application.
- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this Application.
- Peak horizontal ground acceleration (PGHA) determined from USGS National Seismic Hazard Mapping Project website (value of 0.0461g for horizontal acceleration — please refer to **Appendix J.1**).
- Computer model SLIDE - An Interactive Slope Stability Program, version 9.006, developed by Rocscience, Inc. was used for the stability analyses.
- Figure Nos. 1 and 2** shows the locations of the critical cross section (please refer to the attached pages).



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Assumptions

Selected Critical Cross Section

Critical Cross Section A-A' - Cross Section A-A' as shown on **Figure Nos. 1 and 2** (see attached pages) is a critical cross section oriented overall from south to north, starting in the existing landfill and moving north through the vertical and then horizontal expansions, and then through the detention basin. A critical cross section is used to be conservative, as it captures the steepest possible slopes and peak elevations. The cross section is characterized by the following features:

- Peak final landform elevation of 896 feet MSL for the horizontal and vertical expansions;
- Maximum waste column thickness of 198 feet located at peak final landform elevation of approximately 896 feet MSL in the horizontal expansion area (Cell 11);
- Maximum waste column thickness of 206 feet located at final landform elevation of approximately 896 feet MSL in the vertical expansion area (Cell 7);
- Final cover side slopes of 4H:1V with a slope of 10H:1V across the plateau;
- Intermediate waste slopes of 3H:1V; and
- Cell excavation side slopes of 3H:1V in the horizontal and vertical expansion areas.

Critical Cross Section B-B' - Cross Section B-B' as shown on **Figure Nos. 1 and 2** (see attached pages) is a critical cross section oriented overall from west to east in Cell 11 of the horizontal expansion. The cross section moves along the LCS pipe through Cell 11 and through the facility boundary to the east. A critical cross section is used to be conservative, as it captures the steepest possible slopes and peak elevations. The cross section is characterized by the following features:

- Peak final landform elevation of 896 feet MSL for the horizontal expansion;
- Maximum waste column thickness of 198 feet located at peak final landform elevation of approximately 896 feet MSL in the horizontal expansion area (Cell 11);
- Final cover side slopes of 4H:1V with a slope of 10H:1V across the plateau;
- Intermediate waste slopes of 3H:1V; and
- Cell excavation side slopes of 3H:1V in the horizontal expansion area.

Landfill Stages Analyzed and Modes of Failure

Stability of the landfill was analyzed for two different landfill stages:

- Complete landfill build-out / final landform; and
- Intermediate/operational buildout from the liner to the maximum waste height in the horizontal expansion, with complete buildout of the vertical expansion area.



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The landfill stages were analyzed using two modes of failure within the SLIDE model — translational (non-circular / block) failure and rotational (circular) failure. The translational failure mode was used to analyze the stability of the liner system along critical (weak) interfaces; and the rotational failure mode was used to analyze the stability of the waste mass and the foundation.

Stability of Liner System by Translational Failure Mode. The stability of the liner system was evaluated by constraining the failure surfaces (generated by the SLIDE model) to occur within the liner system at the most critical interface. The SLIDE model was used to perform a block search for translational failure surfaces (i.e., non-circular failure surfaces that follow along a weak plane or interface). A constraining boundary was applied through the upper portion of the base liner and side slope liner within the SLIDE model. Failure surfaces were then generated through the liner layer, and the most critical failure surface was determined (i.e., lowest factor of safety).

Stability of Waste Mass and Foundation by Rotational Failure Mode. The stability of the waste mass and foundation was evaluated within the SLIDE model using a grid search to find the most critical circular failure surfaces within the waste mass and foundation. The grid search was performed in an iterative manner by the SLIDE model user. Each time the user adjusted / fine-tuned the grid to the point where the model generated the absolute lowest factor of safety.

Limit Equilibrium Analysis Methods

The limit equilibrium analysis methods used in the SLIDE model analyses included the Bishop Simplified Method, the Janbu Corrected Method, the Spencer Method, and the GLE (Generalized Limit Equilibrium) / Morgenstern-Price Method. The lowest factor of safety from the four methods used is reported on the SLIDE plot for each modeled scenario (attached pages) and on the summary table on the following page. All of the modeled scenarios are graphically presented on the SLIDE plots provided in the attached pages.

Failure Conditions

The stability analyses were performed for both short-term and long-term shear strength under static and seismic loading conditions. Long-term shear strength conditions will most likely occur following the complete buildout of the landfill. Short term shear strength conditions will most likely occur in the intermediate/operation scenario.

Global Stability Analysis of Intermediate Build-Out (Operational Period)

For the operational period, an interim waste slope was developed that rises from the base grades to the peak landfill height (206' along Cross Section A-A' and 196' along Cross Section B-B'). The waste slope was assumed to have a slope of 3H:1V.

Global stability of the intermediate landfill buildout (operational landform) condition was analyzed to identify liner and leachate collection system shear strength parameters that provide adequate factors



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of safety for the constructed landfill. This scenario was modeled to ensure that the expanded landfill will be stable after closure.

The stability of the landform was evaluated using the unit weights of the geologic units and earthen constructed landfill components as described in **Appendix J.1**.

The water table was assumed to be located in contact with the bottom the low permeable earth liner or 5-feet below the ground surface (beyond the limits of the landfill). This is conservative and represents the worst-case groundwater scenario across the site. A liquid level representing a 1-foot leachate level was modeled as a piezometric surface, at the top of the LCS drainage layer.

The final cover critical interface was modeled as a thin layer, sandwiched within the final cover soil. The final cover critical interface shear strength value was selected based on minimum acceptable geosynthetic interface peak shear strength evaluations provided in **Appendix J.2-A**. The final cover critical interface was modeled using a friction angle of 21.9 degrees and no cohesion. It is noted that this is conservative because the friction angle used in the model is shown to barely be within the zone of acceptable peak shear strength values (see **Appendix J.2-A**). The shear strength of the final cover soils was modeled using the reported values in **Appendix J.1**.

The shear strength values of the critical interface in the leachate and liner system was conservatively assumed to rely only on interface friction angle. The model was iteratively run to identify the minimum interface friction angle that provided adequate factors of safety for the peak shear strength values of the critical interface for the textured geomembrane (used in vertical and horizontal expansion and on side slopes underlying vertical expansion).

The north side of Cross Section A-A' (i.e. horizontal expansion) and the east side of Cross Section (B-B') were analyzed for the following conditions:

- Stability of Liner System* - evaluated stability of the liner system under short-term shear strength / static and seismic conditions.
- Stability of Waste and Foundation* - evaluated stability of the waste mass and foundation under short-term strength / static and seismic conditions.

Generally speaking, short term analyses evaluate the stability shortly after complete buildout. These analyses conservatively assume that the waste is placed before the foundation soils are allowed to consolidate under the weight of the overlying landfill and before excess pore-water pressures induced by the loading are allowed to dissipate. Long-term analyses typically represent the final conditions after the foundation soils consolidate and excess pore-water pressure dissipates. Therefore, long-term conditions were not modeled for the intermediate scenario as operational conditions are not anticipated to occur for extended periods of time.



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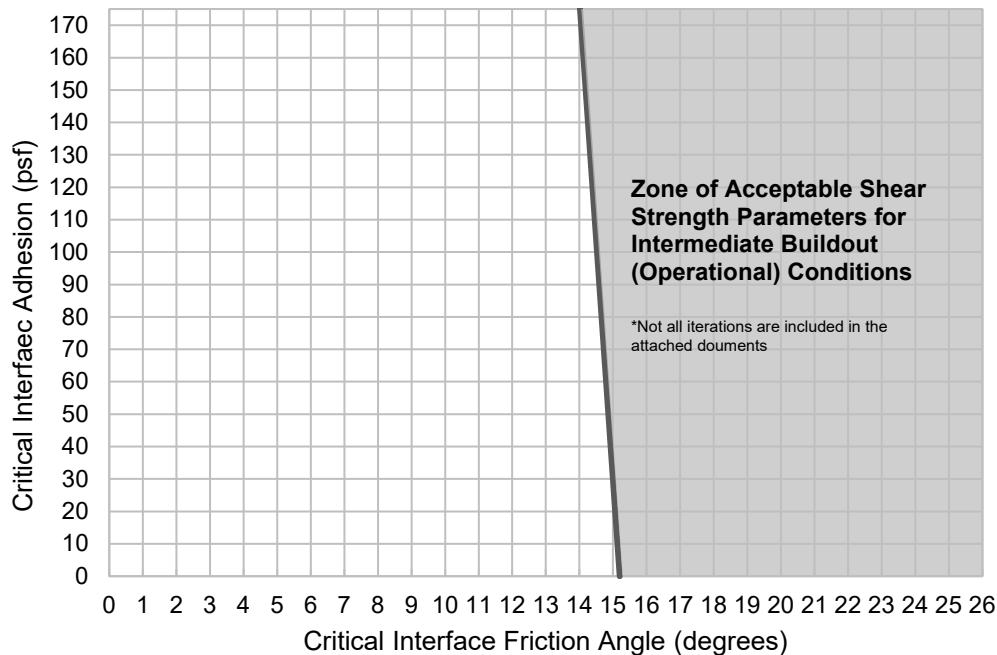
Date: 05/2021

TITLE: SHEAR STRENGTH EVALUATION OF LINER / LEACHATE COLLECTION SYSTEM AFTER WASTE PLACEMENT (GLOBAL STABILITY ANALYSIS)

Intermediate conditions of the vertical expansion were not analyzed. The existing landfill was designed and permitted with a larger waste thickness and elevation than what is proposed for the vertical expansion. Therefore, it is not anticipated that the vertical expansion will cause failure during intermediate/operational conditions.

Intermediate Conditions (Operational) Results

A critical landfill stability model (with 3H:1V operational slopes) was completed to determine the range of interface shear strength friction angles of the LCS/liner system in the horizontal expansion and adhesion parameters that meet the minimum safety factors of 1.5 for static conditions and 1.3 for seismic conditions, as shown below.

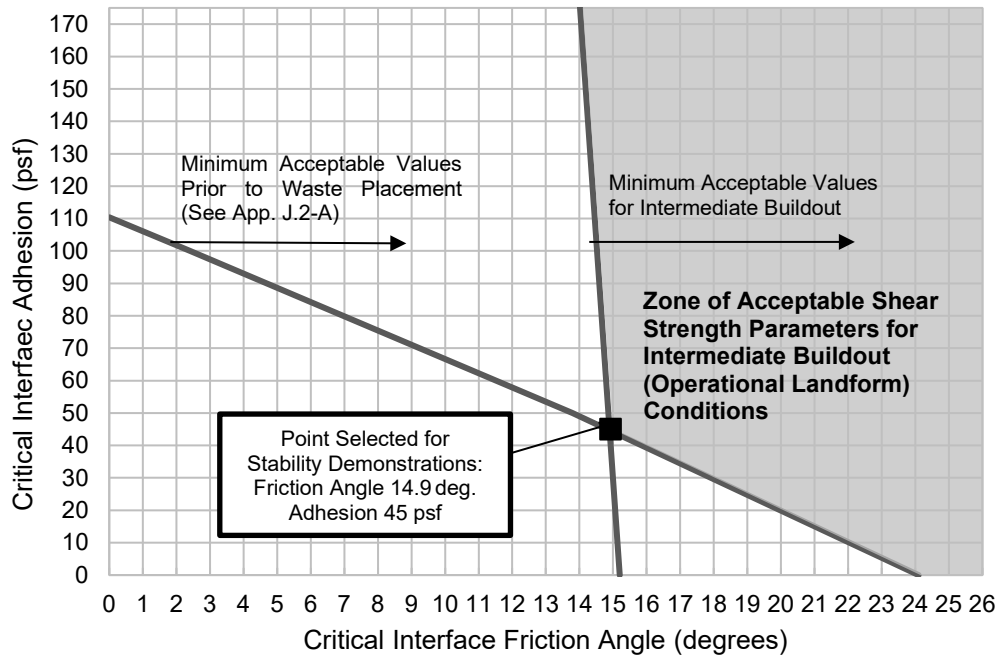




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Based on these modeling results and the analyses conducted in **Appendix J.2-B**, a friction angle of 14.9 degrees with an adhesion of 45 psf for the base liner and leachate collection system critical interface was chosen based on the figure below.



Using this critical interface friction angle for the bottom liner system for the horizontal expansion, the required factors of safety for the complete buildout were achieved, as shown in **Table J.2C-1**.

Table J.2C-1 Zion Landfill – Site 2 North Expansion Intermediate Waste Height / Partial Buildout Liner/LCS Interface: Friction Angle 14.9 degrees, 45 psf adhesion		
Analysis	Factors of Safety	
	Short-Term Shear Strength	
	Static	Seismic
Stability Cross Section A-A' – Horizontal Expansion (northern slope): Intermediate Waste Height / Partial Buildout		
NonCircular / Liner Block Search	1.523	1.300
Circular / Grid Search	1.709	1.332
Stability Cross Section B-B' – Horizontal Expansion (eastern slope): Intermediate Waste Height / Partial Buildout		
NonCircular / Liner Block Search	1.549	1.320
Circular / Grid Search	1.850	1.532



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Global Stability Analysis of Complete Build-Out (Final Landform) Condition

Global stability of the complete landfill buildout (final landform) condition was analyzed to ensure that the expanded landfill will be stable after closure. The material properties for these conditions were the same as in the intermediate buildout scenario.

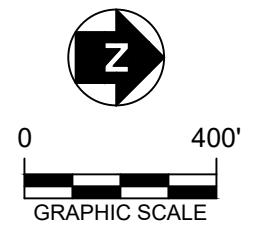
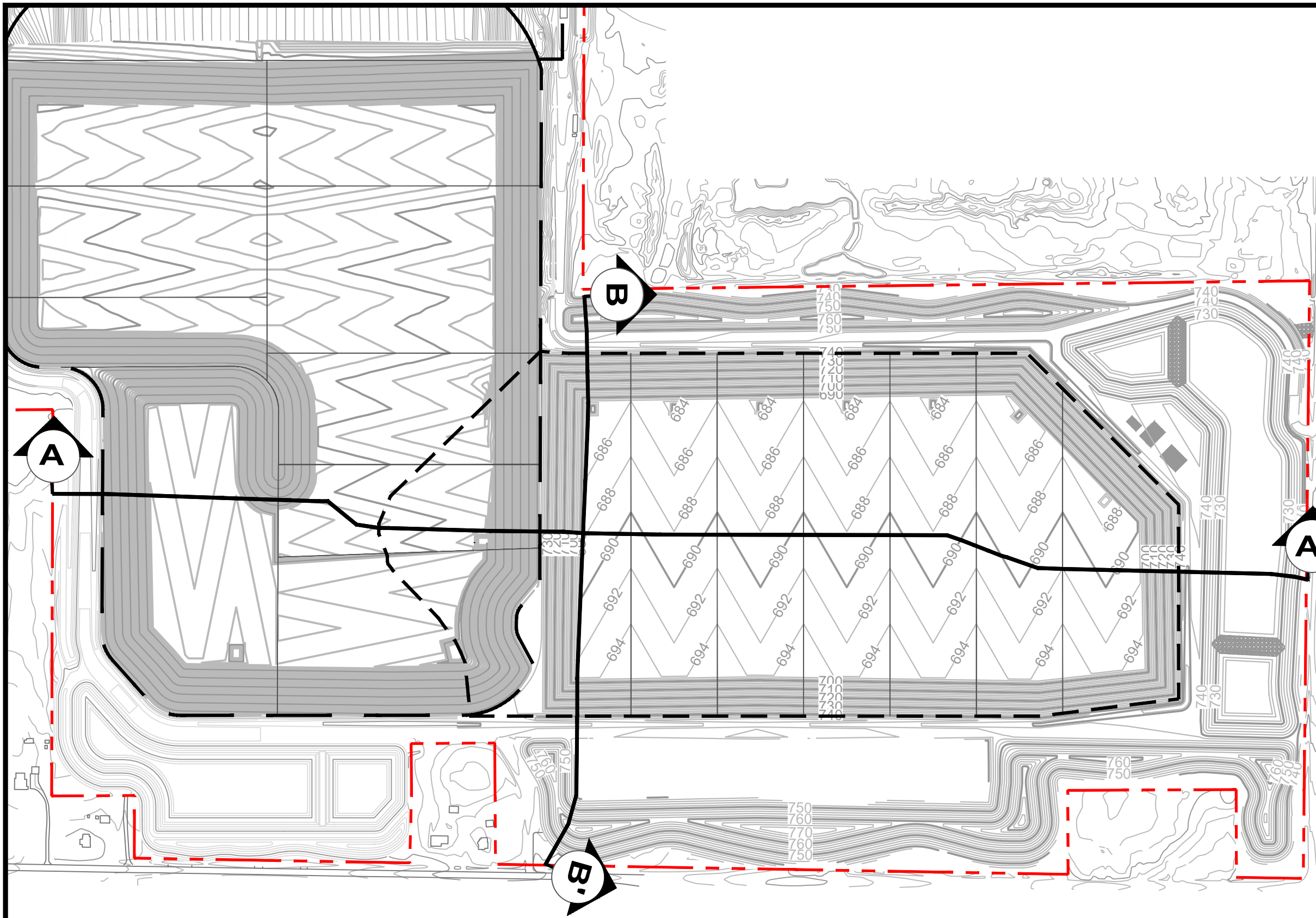
The north side of Cross Section A-A' and the west and east sides of Cross Section B-B' were analyzed to ensure that the landfill would be stable under both of the following conditions:

- Stability of Liner System* - evaluated stability of the liner system under short-term and long-term shear strength / static and seismic conditions.
- Stability of Waste and Foundation* - evaluated stability of the waste mass and foundation under short-term and long-term shear strength / static and seismic conditions.

Complete Build-Out (Final Landform) Results

A landfill stability model was completed to ensure that the final buildout would achieve the required factors of safety of 1.5 and 1.3 for static and seismic conditions, respectively, based on liner and leachate collection system interfaces of 14.9 degrees and 45 psf adhesion. See **Table J.2C-2** below for the achieved factors of safety.

Table J.2C-2 Zion Landfill – Site 2 North Expansion Complete Buildout / Final Conditions Liner/LCS Interface: Friction Angle 14.9 degrees, 45 psf adhesion				
Analysis	Factors of Safety			
	Short-Term Shear Strength		Long-Term Shear Strength	
	Static	Seismic	Static	Seismic
Stability Cross Section A-A' – Horizontal Expansion (northern slope) : Complete Buildout				
NonCircular / Liner Block Search	2.127	1.738	1.984	1.628
Circular / Grid Search	2.658	1.914	2.337	1.949
Stability Cross Section B-B' – Horizontal Expansion (western slope) : Complete Buildout				
NonCircular / Liner Block Search	2.188	1.790	2.040	1.676
Circular / Grid Search	2.711	2.117	2.339	1.951
Stability Cross Section B-B' – Horizontal Expansion (eastern slope) : Complete Buildout				
NonCircular / Liner Block Search	2.128	1.742	1.982	1.629
Circular / Grid Search	2.623	2.042	2.340	1.953



LEGEND

- - - - - APPROXIMATE FACILITY BOUNDARY
- - - - - APPROXIMATE EXISTING WASTE BOUNDARY
- — — — — APPROXIMATE PROPOSED EXPANSION WASTE BOUNDARY

NOTES

1. EXISTING CONTOURS DEVELOPED FROM SITE AERIAL TOPOGRAPHIC SURVEY BY CQA, INC. ON 10/22/2018.
2. FOR CLARITY, NOT ALL SITE FEATURES MAY BE SHOWN.
3. CURRENT TOPOGRAPHY MAY DIFFER FROM THAT SHOWN.

REV. NO.	DATE	DESCRIPTION

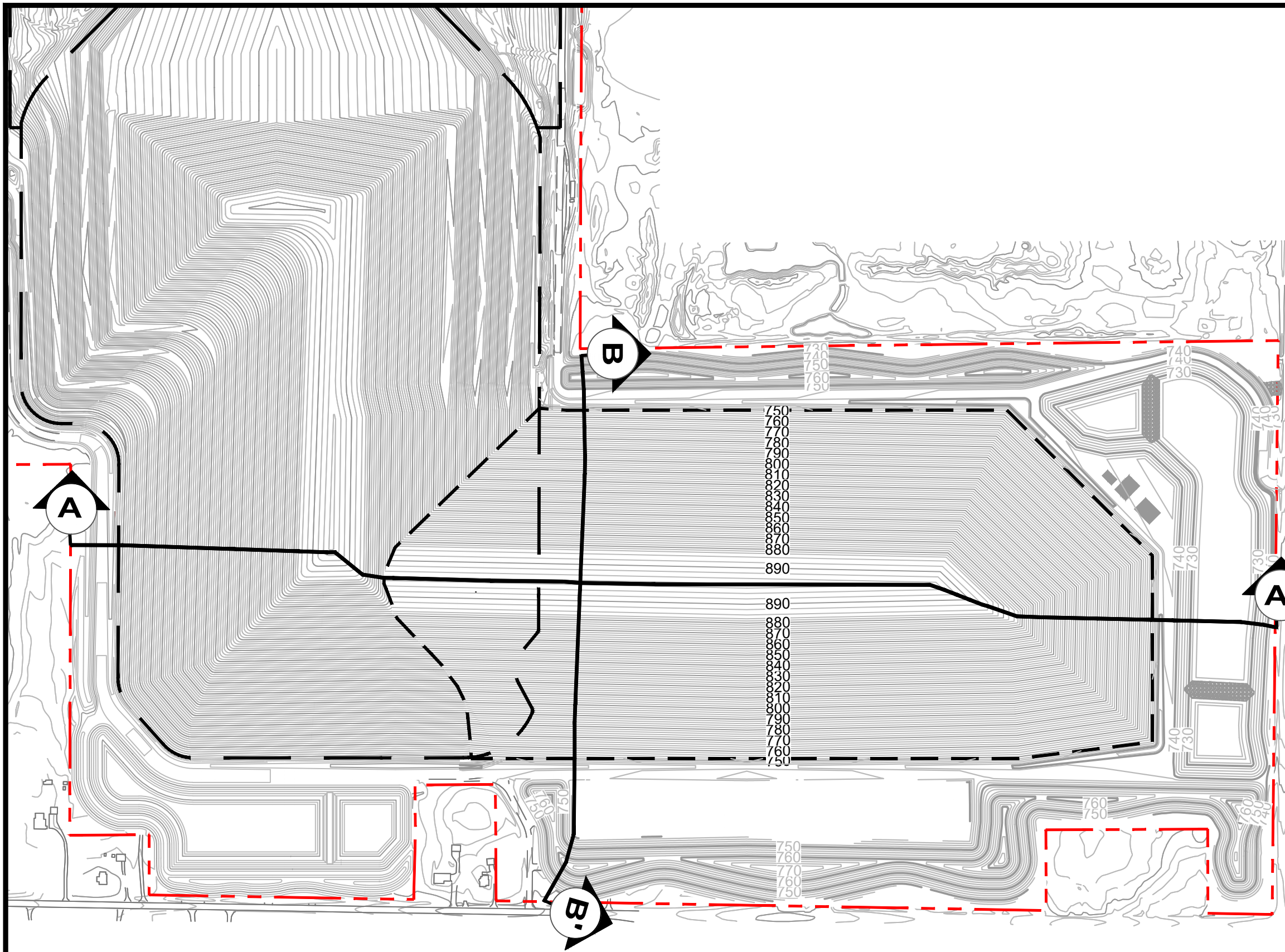


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**GFL EVERGLADES HOLDINGS, LLC
 ZION, ILLINOIS**

**FIGURE 1
 SLOPE STABILITY CROSS SECTION LOCATION
 EXCAVATION GRADES**

DRAWN BY: ORC APPROVED BY: RDS PROJ. NO.: 003211 DATE: FEBRUARY 2020



LEGEND

- APPROXIMATE FACILITY BOUNDARY
- APPROXIMATE EXISTING WASTE BOUNDARY
- APPROXIMATE PROPOSED EXPANSION WASTE BOUNDARY

NOTES

1. EXISTING CONTOURS DEVELOPED FROM SITE AERIAL TOPOGRAPHIC SURVEY BY CQA, INC. ON 10/22/2018.
2. FOR CLARITY, NOT ALL SITE FEATURES MAY BE SHOWN.
3. CURRENT TOPOGRAPHY MAY DIFFER FROM THAT SHOWN.

REV. NO.	DATE	DESCRIPTION



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**GFL EVERGLADES HOLDINGS, LLC
 ZION, ILLINOIS**

**FIGURE 2
 SLOPE STABILITY CROSS SECTION LOCATION
 FINAL COVER GRADES**

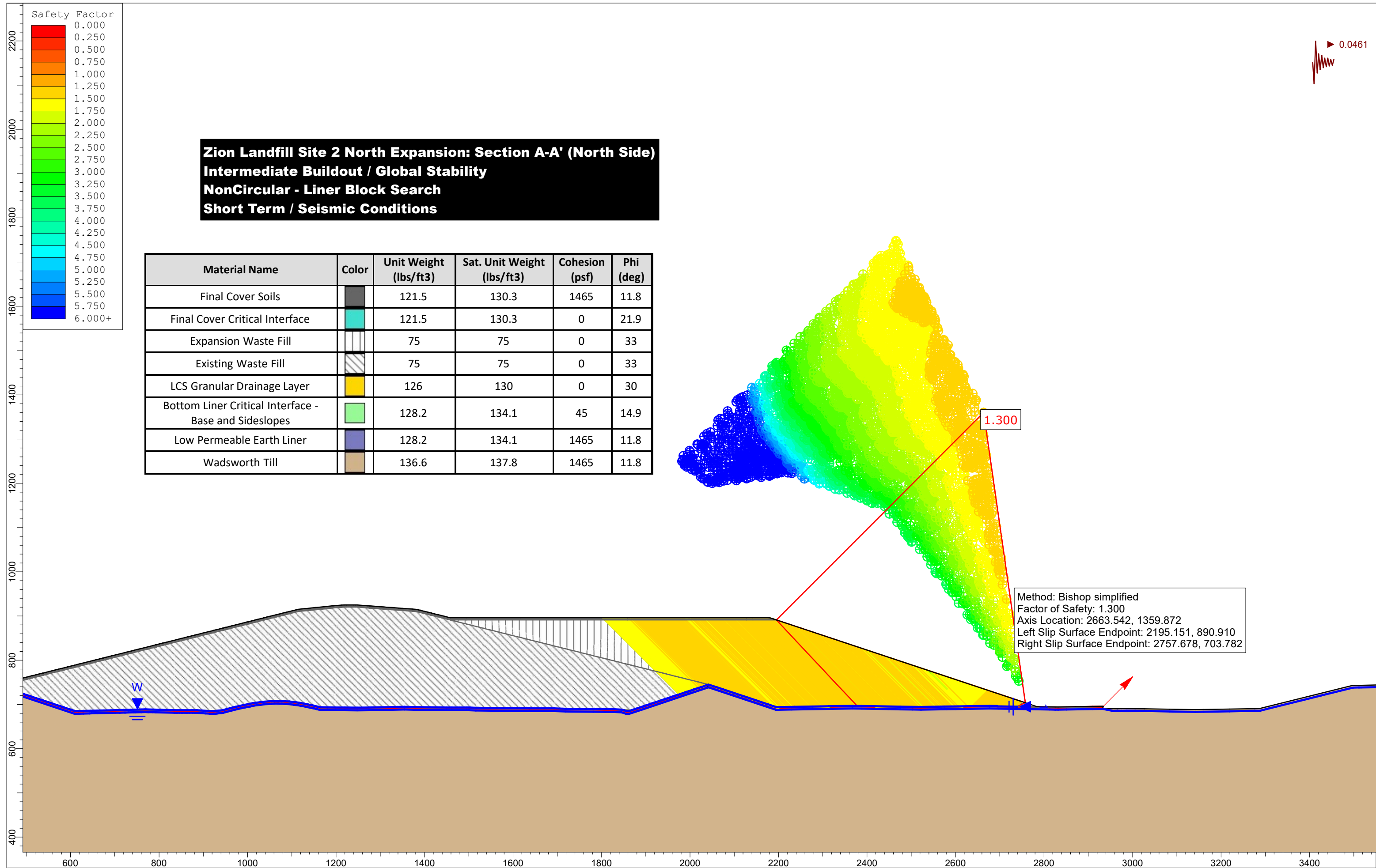
DRAWN BY: ORC APPROVED BY: RDS PROJ. NO.: 003211 DATE: MAY 2020

**SLIDE OUTPUT:
PLOTS & MODEL RUNS**

SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION (**NORTH SLOPE**)

GLOBAL LINER STABILITY ANALYSIS

**INTERIM WASTE HEIGHT / PARTIAL BUILDOUT LANDFORM
BLOCK ANALYSIS OF LINER SYSTEM
(TRANSLATIONAL SLOPE FAILURE)**



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Intermediate Buildout / Global Stability
NonCircular - Liner Block Search
Short Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8

Method: Bishop simplified
 Factor of Safety: 1.300
 Axis Location: 2663.542, 1359.872
 Left Slip Surface Endpoint: 2195.151, 890.910
 Right Slip Surface Endpoint: 2757.678, 703.782

Slide Analysis Information

A-A_inter_north_ST

Project Summary

File Name:	A-A_inter_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:11m:56.708s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1

Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1

Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.299580
Axis Location:		2663.542, 1359.872
Left Slip Surface Endpoint:		2195.151, 890.910
Right Slip Surface Endpoint:		2757.678, 703.782
Resisting Moment:		7.57371e+08 lb-ft
Driving Moment:		5.8278e+08 lb-ft
Total Slice Area:		39674.6 ft2
Surface Horizontal Width:		562.526 ft
Surface Average Height:		70.5294 ft

Method: janbu corrected

	FS	1.323270
Axis Location:		2663.542, 1359.872
Left Slip Surface Endpoint:		2195.151, 890.910
Right Slip Surface Endpoint:		2757.678, 703.782
Resisting Horizontal Force:		1.02225e+06 lb
Driving Horizontal Force:		772519 lb
Total Slice Area:		39674.6 ft2
Surface Horizontal Width:		562.526 ft
Surface Average Height:		70.5294 ft

Method: spencer

FS	1.362860
Axis Location:	2663.542, 1359.872
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2757.678, 703.782
Resisting Moment:	7.34814e+08 lb-ft
Driving Moment:	5.39171e+08 lb-ft
Resisting Horizontal Force:	947266 lb
Driving Horizontal Force:	695058 lb
Total Slice Area:	39674.6 ft ²
Surface Horizontal Width:	562.526 ft
Surface Average Height:	70.5294 ft

Method: gle/morgenstern-price

FS	1.356640
Axis Location:	2663.542, 1359.872
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2757.678, 703.782
Resisting Moment:	7.29318e+08 lb-ft
Driving Moment:	5.37591e+08 lb-ft
Resisting Horizontal Force:	941002 lb
Driving Horizontal Force:	693626 lb
Total Slice Area:	39674.6 ft ²
Surface Horizontal Width:	562.526 ft
Surface Average Height:	70.5294 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
2195.15	890.91
2206.6	878.702
2218.06	866.493
2229.51	854.285
2240.96	842.077
2252.41	829.869
2263.86	817.661
2275.32	805.453
2286.77	793.244
2298.22	781.036
2309.67	768.828
2321.12	756.62
2332.58	744.412
2344.03	732.204
2355.48	719.995
2366.93	707.787
2378.38	695.579
2396.46	695.232
2414.54	694.884
2432.62	694.537
2452.62	694.178
2472.61	693.818
2487.31	693.561
2500.41	693.334
2521.91	692.963
2537.26	693.229
2552.62	693.495
2567.98	693.76
2583.34	694.026
2602.44	694.358
2621.55	694.689
2635.72	694.935
2649.89	695.181
2664.06	695.428
2678.23	695.674
2699.34	695.308
2715.89	695.025
2738.11	694.742
2747.9	699.262
2757.68	703.782

Method: janbu corrected

X	Y
2195.15	890.91
2206.6	878.702
2218.06	866.493
2229.51	854.285
2240.96	842.077
2252.41	829.869
2263.86	817.661
2275.32	805.453
2286.77	793.244
2298.22	781.036
2309.67	768.828
2321.12	756.62
2332.58	744.412
2344.03	732.204
2355.48	719.995
2366.93	707.787
2378.38	695.579
2396.46	695.232
2414.54	694.884
2432.62	694.537
2452.62	694.178
2472.61	693.818
2487.31	693.561
2500.41	693.334
2521.91	692.963
2537.26	693.229
2552.62	693.495
2567.98	693.76
2583.34	694.026
2602.44	694.358
2621.55	694.689
2635.72	694.935
2649.89	695.181
2664.06	695.428
2678.23	695.674
2699.34	695.308
2715.89	695.025
2738.11	694.742
2747.9	699.262
2757.68	703.782

Method: spencer

X	Y
2195.15	890.91
2206.6	878.702
2218.06	866.493
2229.51	854.285
2240.96	842.077
2252.41	829.869
2263.86	817.661
2275.32	805.453
2286.77	793.244
2298.22	781.036
2309.67	768.828
2321.12	756.62
2332.58	744.412
2344.03	732.204
2355.48	719.995
2366.93	707.787
2378.38	695.579
2396.46	695.232
2414.54	694.884
2432.62	694.537
2452.62	694.178
2472.61	693.818
2487.31	693.561
2500.41	693.334
2521.91	692.963
2537.26	693.229
2552.62	693.495
2567.98	693.76
2583.34	694.026
2602.44	694.358
2621.55	694.689
2635.72	694.935
2649.89	695.181
2664.06	695.428
2678.23	695.674
2699.34	695.308
2715.89	695.025
2738.11	694.742
2747.9	699.262
2757.68	703.782

Method: gle/morgenstern-price

X	Y
2195.15	890.91
2206.6	878.702
2218.06	866.493
2229.51	854.285
2240.96	842.077
2252.41	829.869
2263.86	817.661
2275.32	805.453
2286.77	793.244
2298.22	781.036
2309.67	768.828
2321.12	756.62
2332.58	744.412
2344.03	732.204
2355.48	719.995
2366.93	707.787
2378.38	695.579
2396.46	695.232
2414.54	694.884
2432.62	694.537
2452.62	694.178
2472.61	693.818
2487.31	693.561
2500.41	693.334
2521.91	692.963
2537.26	693.229
2552.62	693.495
2567.98	693.76
2583.34	694.026
2602.44	694.358
2621.55	694.689
2635.72	694.935
2649.89	695.181
2664.06	695.428
2678.23	695.674
2699.34	695.308
2715.89	695.025
2738.11	694.742
2747.9	699.262
2757.68	703.782

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4874
Number of Invalid Surfaces: 130

Error Codes

Error Code -108 reported for 4 surfaces
Error Code -110 reported for 126 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4872
 Number of Invalid Surfaces: 132

Error Codes

Error Code -108 reported for 6 surfaces
 Error Code -110 reported for 126 surfaces

Method: spencer

Number of Valid Surfaces: 4318
 Number of Invalid Surfaces: 686

Error Codes

Error Code -108 reported for 44 surfaces
 Error Code -110 reported for 126 surfaces
 Error Code -111 reported for 516 surfaces

Method: gl/morgenstern-price

Number of Valid Surfaces: 4164
 Number of Invalid Surfaces: 840

Error Codes

Error Code -108 reported for 50 surfaces
 Error Code -110 reported for 126 surfaces
 Error Code -111 reported for 664 surfaces

Error Code Descriptions

The following errors were encountered during the computation:
 -108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
 -110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
 -111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	2179.66, 896.002
	2177.34, 896
	2141.81, 896
	2127.47, 896
	1858.87, 896
	1616.33, 896
	1495.01, 896
	1458.28, 896
	1456.86, 896.353
	1456.85, 896.354
	1454.84, 896.853
	1447.37, 898.703

1446.76, 898.853
1439.22, 900.721
1438.69, 900.853
1431.08, 902.74
1430.62, 902.853
1429.15, 903.217
1429.15, 903.218
1422.55, 904.853
1415.47, 906.607
1415.47, 906.607
1414.48, 906.853
1406.62, 908.801
1406.4, 908.853
1398.46, 910.822
1398.33, 910.853
1390.3, 912.844
1390.26, 912.853
1390.12, 912.889
1390.12, 912.889
1386.77, 913.717
1380.94, 914.966
1373.16, 915.464
1354.04, 916.986
1349.5, 917.317
1340.81, 917.854
1326.59, 918.986
1314.16, 919.891
1308.46, 920.243
1299.13, 920.986
1278.82, 922.465
1276.11, 922.633
1271.72, 922.983
1264.11, 923.65
1247.79, 925
1244.03, 925
1239.2, 925
1223.74, 925
1219.79, 925
1211.96, 924.561
1200.98, 923.615
1197.39, 923.302
1194.12, 922.975
1192.25, 922.788
1174.12, 920.975
1161.84, 919.746
1154.12, 918.975
1141.84, 917.746
1134.12, 916.975
1131.52, 916.714
1113.76, 914.939
1110.03, 914.006
1110.01, 914.002
1105.41, 912.851
1104.42, 912.604
1097.41, 910.851
1096.34, 910.584
1089.41, 908.851
1088.27, 908.565
1081.41, 906.851

1076.31, 905.577
1076.3, 905.573
1073.41, 904.851
1070.76, 904.188
1070.74, 904.184
1065.41, 902.851
1064.04, 902.51
1057.41, 900.851
1050.91, 899.226
1049.41, 898.851
1042.98, 897.243
1041.41, 896.851
1035.75, 895.435
1035.73, 895.432
1033.41, 894.851
1031.24, 894.309
1031.23, 894.306
1025.41, 892.851
1023.7, 892.424
1017.41, 890.851
1015.64, 890.407
1009.41, 888.851
1002.24, 887.059
1001.87, 886.967
1001.85, 886.96
1001.53, 886.882
1001.41, 886.851
1000.31, 886.576
998.452, 886.111
998.336, 886.082
993.409, 884.851
989.728, 883.93
988.483, 883.619
988.367, 883.59
986.509, 883.126
985.409, 882.851
982.082, 882.019
977.409, 880.851
973.251, 879.811
969.409, 878.851
964.222, 877.554
961.409, 876.851
959.545, 876.385
957.953, 875.986
953.409, 874.851
950.878, 874.218
950.2, 874.048
949.069, 873.766
947.594, 873.397
945.409, 872.851
945.25, 872.811
944.49, 872.621
943.871, 872.466
937.409, 870.851
935.469, 870.366
929.409, 868.851
922.837, 867.208
921.409, 866.851
914.83, 865.206

913.409, 864.851
912.054, 864.512
905.409, 862.851
904.157, 862.538
897.409, 860.851
895.171, 860.291
889.409, 858.851
883.455, 857.362
882.565, 857.14
881.409, 856.851
880.304, 856.574
873.409, 854.851
866.47, 853.116
865.409, 852.851
858.425, 851.105
857.409, 850.851
850.382, 849.094
849.409, 848.851
848.476, 848.617
841.409, 846.851
834.511, 845.126
833.409, 844.851
832.349, 844.586
825.409, 842.851
819.152, 841.286
817.409, 840.851
811.119, 839.278
809.409, 838.851
807.747, 838.435
801.409, 836.851
794.398, 835.098
793.409, 834.851
792.434, 834.607
792.426, 834.605
785.409, 832.851
778.07, 831.016
777.409, 830.851
777.357, 830.838
777.354, 830.838
769.409, 828.851
766.195, 828.047
761.409, 826.851
754.352, 825.086
754.344, 825.084
753.409, 824.851
752.484, 824.619
752.476, 824.617
745.409, 822.851
742.15, 822.036
737.409, 820.851
734.118, 820.028
729.409, 818.851
726.082, 818.019
721.42, 816.851
714.316, 815.075
714.308, 815.073
713.42, 814.851
712.543, 814.631
712.535, 814.629

External Boundary

705.42, 812.851
701.994, 811.994
697.42, 810.851
692.873, 809.714
689.42, 808.851
684.906, 807.722
681.42, 806.851
674.27, 805.063
674.262, 805.061
673.422, 804.851
672.586, 804.643
665.409, 802.851
661.711, 801.926
657.419, 800.851
653.685, 799.917
649.422, 798.851
645.167, 797.79
641.409, 796.851
634.21, 795.051
633.409, 794.851
632.619, 794.653
625.409, 792.851
621.53, 791.881
617.419, 790.851
613.503, 789.872
609.419, 788.851
605.466, 787.862
601.421, 786.851
594.16, 785.039
593.415, 784.852
592.654, 784.664
585.393, 782.851
581.342, 781.838
577.393, 780.851
573.314, 779.831
569.392, 778.851
565.288, 777.824
561.39, 776.85
554.142, 775.035
553.404, 774.85
552.668, 774.665
545.409, 772.851
538.007, 771
538.003, 771
537.388, 770.846
529.393, 768.851
525.016, 767.756
521.392, 766.85
514.117, 765.028
513.413, 764.851
512.696, 764.673
505.386, 762.851
500.914, 761.732
497.408, 760.85
492.912, 759.725
489.386, 758.851
485.922, 757.977
481.42, 756.851
474.122, 755.025

473.423, 754.851
472.728, 754.677
465.411, 752.851
460.744, 751.683
457.419, 750.85
453.425, 749.85
449.755, 748.932
449.43, 748.851
449.065, 748.76
441.43, 746.851
411.625, 739.4
408.752, 739.647
407.707, 739.78
399.328, 740
391.535, 740.576
375.004, 740.361
356.331, 740.013
351.947, 740
351.399, 739.824
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594.889, 782.13
602.147, 783.941
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634.937, 792.14
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645.894, 794.88
650.148, 795.941
654.413, 797.007
658.147, 797.94
662.44, 799.016
666.137, 799.94
673.312, 801.732
674.148, 801.941
674.99, 802.151
674.997, 802.153
682.148, 803.94
685.634, 804.812
690.148, 805.94
693.601, 806.804
698.148, 807.94
702.721, 809.084
706.148, 809.94
713.263, 811.719
713.271, 811.721
714.148, 811.94
715.036, 812.162
715.044, 812.164
722.148, 813.94
726.812, 815.109
730.137, 815.94
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738.137, 817.94
742.878, 819.125
746.137, 819.94
753.204, 821.707
753.211, 821.709
754.137, 821.94
755.071, 822.174
755.079, 822.176
762.137, 823.94
766.923, 825.137
770.137, 825.94
777.722, 827.837
777.725, 827.837

Material Boundary

778.137, 827.94
778.797, 828.105
786.137, 829.94
793.153, 831.694
793.161, 831.696
794.137, 831.94
795.126, 832.187
802.137, 833.94
808.474, 835.525
810.137, 835.94
811.847, 836.368
818.137, 837.94
819.879, 838.376
826.137, 839.94
833.077, 841.675
834.137, 841.94
835.239, 842.216
842.137, 843.94
849.204, 845.707
850.137, 845.94
851.109, 846.183
858.137, 847.94
859.153, 848.194
866.137, 849.94
867.198, 850.205
874.137, 851.94
881.031, 853.664
882.137, 853.94
883.293, 854.229
884.182, 854.452
890.137, 855.94
895.898, 857.381
898.137, 857.94
904.884, 859.627
906.137, 859.94
912.782, 861.602
914.137, 861.94
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923.565, 864.297
930.137, 865.94
936.197, 867.455
938.137, 867.94
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951.605, 871.307
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958.68, 873.076
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	1389.58, 909.932
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	1405.89, 905.889
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	1414.75, 903.696
	1414.75, 903.695
	1421.83, 901.941
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	1428.43, 900.305
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	1430.35, 899.828
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	1457.91, 893
	1495.01, 893
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	2141.81, 893
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	2188.78, 893
	426.201, 740.106
	442.121, 744.086
	449.756, 745.995
	450.121, 746.086
	450.446, 746.167
	454.117, 747.086
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	461.436, 748.919
	466.102, 750.086
	473.419, 751.912

474.113,	752.086
474.811,	752.26
482.112,	754.086
486.617,	755.213
490.077,	756.086
493.601,	756.959
498.102,	758.085
501.607,	758.968
506.077,	760.086
513.383,	761.907
514.104,	762.086
514.812,	762.264
522.084,	764.086
525.707,	764.991
530.085,	766.086
538.079,	768.081
538.353,	768.15
538.357,	768.15
546.101,	770.086
553.36,	771.901
554.096,	772.086
554.833,	772.27
562.083,	774.086
565.979,	775.06
570.084,	776.086
574.006,	777.066
578.084,	778.086
582.033,	779.073
586.085,	780.086
593.341,	781.898
594.104,	782.086
594.853,	782.275
602.111,	784.086
606.157,	785.097
610.11,	786.086
614.194,	787.107
618.11,	788.086
622.223,	789.116
626.101,	790.086
633.31,	791.888
634.101,	792.086
634.901,	792.286
642.101,	794.086
645.858,	795.025
650.112,	796.086
654.377,	797.152
658.111,	798.086
662.404,	799.162
666.101,	800.086
673.276,	801.878
674.112,	802.086
674.953,	802.296
674.961,	802.298
682.111,	804.086
685.598,	804.957
690.111,	806.086
693.564,	806.949
698.111,	808.086
702.685,	809.229

706.111, 810.086
713.227, 811.865
713.234, 811.867
714.111, 812.086
714.999, 812.308
715.007, 812.31
722.112, 814.086
726.775, 815.254
730.101, 816.086
734.809, 817.263
738.101, 818.086
742.842, 819.271
746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
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770.101, 826.086
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777.707, 827.987
778.101, 828.086
778.761, 828.251
786.101, 830.086
793.117, 831.84
793.125, 831.842
794.101, 832.086
795.089, 832.333
802.101, 834.086
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811.81, 836.513
818.101, 838.086
819.843, 838.521
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842.101, 844.086
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882.101, 854.086
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884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773
906.101, 860.086
912.746, 861.747


Material Boundary

914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
964.913, 874.789
970.101, 876.086
973.942, 877.046
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982.774, 879.254
986.101, 880.086
987.201, 880.361
989.058, 880.825
989.174, 880.854
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994.101, 882.086
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999.144, 883.347
1001, 883.811
1002.1, 884.086
1002.22, 884.117
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1002.57, 884.202
1002.93, 884.294
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1016.33, 887.642
1018.1, 888.086
1024.39, 889.659
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1031.94, 891.544
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1036.43, 892.667
1036.44, 892.671
1042.1, 894.086
1043.67, 894.478
1050.1, 896.086
1051.6, 896.461
1058.1, 898.086
1064.74, 899.745
1066.1, 900.086
1071.43, 901.419
1071.45, 901.423
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1076.99, 902.808

1077.01, 902.812
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1098.1, 908.086
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1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
1114.25, 912.124
1131.8, 913.879
1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718
1219.87, 922.15
1223.74, 922.15
1239.2, 922.15
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1247.67, 922.15
1263.87, 920.811
1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
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1349.31, 914.473
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1372.95, 912.621
1380.54, 912.136
1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
1397.65, 908.087
1397.77, 908.056
1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
1414.78, 903.841
1414.79, 903.84
1421.86, 902.087
1428.46, 900.451
1428.47, 900.45
1429.93, 900.087
1430.39, 899.974
1438.01, 898.087
1438.54, 897.955

1446.08, 896.087
1446.69, 895.936
1454.15, 894.087
1456.17, 893.588
1456.17, 893.587
1457.93, 893.15
1495.01, 893.15
1616.33, 893.15
1858.87, 893.15
2127.47, 893.15
2141.81, 893.15
2181.65, 893.15
2188.32, 893.15

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	




412.029, 734.347
439.628, 735.289
488.131, 719.138
522.779, 707.602
608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
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836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092

948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262
1026.16, 698.595
1029.95, 699.071
1037.13, 700.174
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1053.89, 701.352
1060.67, 701.77
1068.84, 701.86
1069.64, 701.862
1072.74, 701.68
1077.4, 701.408
1085.39, 700.951
1093.76, 700.535
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1105.21, 699.039
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1461.71, 687
1495.01, 687
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1692.76, 685
1718.93, 684.138
1781.09, 683.397
1807.08, 683

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1816.05, 683
1826.62, 683
1845.4, 682.209
1854.88, 679
1862.97, 679
1893.13, 688.871
1989.22, 720.9
2041.9, 738.472
2194.18, 687.752
2229.99, 688.38
2366.91, 690.776
2388.53, 690.405
2447.68, 689.392
2475.13, 688.921
2503.25, 688.435
2521.93, 688.112
2580.04, 689.12
2600.94, 689.482
2620.01, 689.813
2676.93, 690.8
2698.09, 690.434
2754.87, 689.452
2812.87, 688.448
2824.98, 688.239
2831.93, 688.119
2850.97, 688.449
2908.97, 689.455
2928.03, 689.785
2933.03, 689.872
2955.52, 685.26
2966.15, 685.445
2974.76, 685.594
2986.93, 685.805
3005.67, 685.481
3026.51, 685.12
3083.88, 684.128
3141.93, 683.124
3161.05, 683.455
3219.11, 684.462
3276.35, 685.455
3287.99, 685.657
3307.91, 690.624
3403.42, 714.438
3425.9, 720.041
3498.4, 738.118
3511.05, 738.153
3513.27, 738.159
3516.47, 738.166
3555.99, 738.915
3560.24, 738.996
3560.38, 738.996
3562.81, 738.996
3565.35, 738.996
3572.15, 738.997
3572.42, 738.997
3572.75, 738.997
3574.89, 738.997
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3660.68, 738.616
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3731.88, 738.243
3847.86, 737.635
3874.25, 737.524
3878.92, 737.484
3880.4, 737.484
3891.32, 737.481
3896.18, 737.436
3923.28, 737.159
3936.98, 737.008
3939.65, 737.003
3940.24, 737.002
3940.54, 737.001
3941.23, 737
3943.19, 736.963
3984.29, 736.189
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4083.35, 733.007
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4134.44, 731.066
4136.21, 731.004
4147.03, 730.471
4149.86, 730.331
4152.27, 730.207
4159.68, 729.828
4161.98, 729.722
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4170.28, 729.323
4176.46, 729.027
4183.66, 729
4184.9, 728.957
4186.51, 728.967
4190.68, 728.712
4192, 728.641
4193.48, 728.515
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4212.1, 727.044
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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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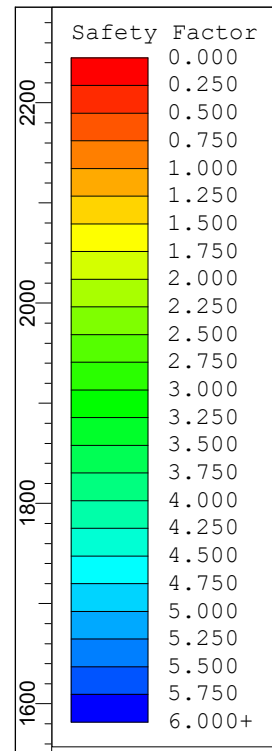
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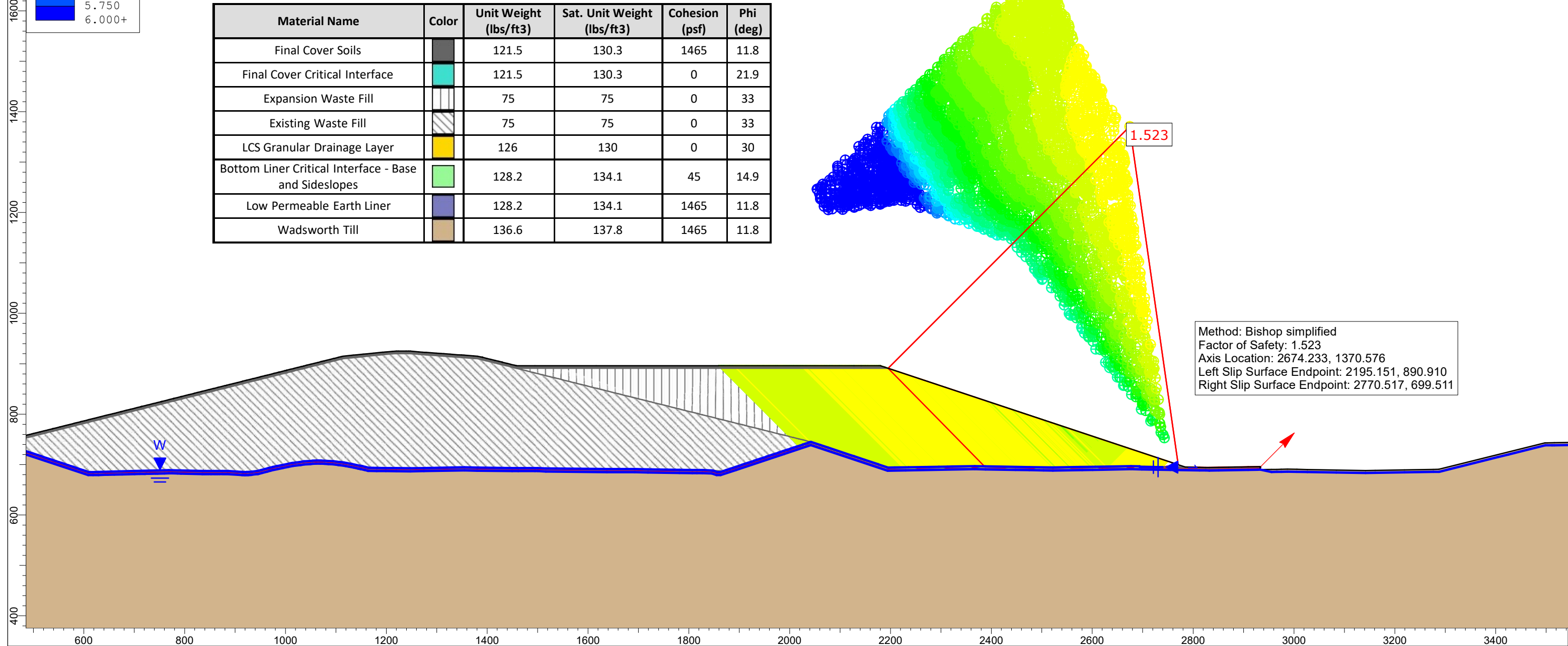
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Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Intermediate Buildout / Global Stability
NonCircular - Liner Block Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Bishop simplified
 Factor of Safety: 1.523
 Axis Location: 2674.233, 1370.576
 Left Slip Surface Endpoint: 2195.151, 890.910
 Right Slip Surface Endpoint: 2770.517, 699.511

Slide Analysis Information

A-A_inter_north_ST

Project Summary

File Name:	A-A_inter_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:03m:05.813s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1

Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465

Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.522900
Axis Location:	2674.233, 1370.576
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2770.517, 699.511
Resisting Moment:	7.86325e+08 lb-ft
Driving Moment:	5.16332e+08 lb-ft
Total Slice Area:	38938.6 ft2
Surface Horizontal Width:	575.365 ft
Surface Average Height:	67.6762 ft

Method: janbu corrected

FS	1.555300
Axis Location:	2674.233, 1370.576
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2770.517, 699.511
Resisting Horizontal Force:	1.0411e+06 lb
Driving Horizontal Force:	669383 lb
Total Slice Area:	38938.6 ft2
Surface Horizontal Width:	575.365 ft
Surface Average Height:	67.6762 ft

Method: spencer

FS	1.586940
Axis Location:	2674.233, 1370.576
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2770.517, 699.511
Resisting Moment:	7.62896e+08 lb-ft
Driving Moment:	4.80735e+08 lb-ft
Resisting Horizontal Force:	961819 lb
Driving Horizontal Force:	606085 lb
Total Slice Area:	38938.6 ft2
Surface Horizontal Width:	575.365 ft
Surface Average Height:	67.6762 ft

Method: gle/morgenstern-price

FS	1.591750
Axis Location:	2674.233, 1370.576
Left Slip Surface Endpoint:	2195.151, 890.910
Right Slip Surface Endpoint:	2770.517, 699.511
Resisting Moment:	7.60825e+08 lb-ft
Driving Moment:	4.7798e+08 lb-ft
Resisting Horizontal Force:	959700 lb
Driving Horizontal Force:	602922 lb
Total Slice Area:	38938.6 ft ²
Surface Horizontal Width:	575.365 ft
Surface Average Height:	67.6762 ft

Global Minimum Coordinates**Method: bishop simplified**

X	Y
2195.15	890.91
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2473.92	693.791
2502.36	693.301
2521.95	692.962
2586.23	694.081
2601.71	694.346
2621.26	694.685
2677.31	695.656
2699.65	695.274
2735.4	694.71
2770.52	699.511

Method: janbu corrected

X	Y
2195.15	890.91
2291.11	793.17
2387.07	695.43
2446.3	694.266
2473.92	693.791
2502.36	693.301
2521.95	692.962
2586.23	694.081
2601.71	694.346
2621.26	694.685
2677.31	695.656
2699.65	695.274
2735.4	694.71
2770.52	699.511

Method: spencer

X	Y
2195.15	890.91
2291.11	793.17
2387.07	695.43
2446.3	694.266
2473.92	693.791
2502.36	693.301
2521.95	692.962
2586.23	694.081
2601.71	694.346
2621.26	694.685
2677.31	695.656
2699.65	695.274
2735.4	694.71
2770.52	699.511

Method: gle/morgenstern-price

X	Y
2195.15	890.91
2291.11	793.17
2387.07	695.43
2446.3	694.266
2473.92	693.791
2502.36	693.301
2521.95	692.962
2586.23	694.081
2601.71	694.346
2621.26	694.685
2677.31	695.656
2699.65	695.274
2735.4	694.71
2770.52	699.511

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces: 4446
Number of Invalid Surfaces: 558

Error Codes

Error Code -106 reported for 2 surfaces
Error Code -108 reported for 5 surfaces
Error Code -110 reported for 527 surfaces
Error Code -111 reported for 24 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4448

Number of Invalid Surfaces: 556

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 4 surfaces

Error Code -110 reported for 527 surfaces

Error Code -111 reported for 23 surfaces

Method: spencer

Number of Valid Surfaces: 4006

Number of Invalid Surfaces: 998

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 42 surfaces

Error Code -110 reported for 527 surfaces

Error Code -111 reported for 427 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 3884

Number of Invalid Surfaces: 1120

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 29 surfaces

Error Code -110 reported for 527 surfaces

Error Code -111 reported for 562 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	2179.66, 896.002
	2177.34, 896
	2141.81, 896
	2127.47, 896
	1858.87, 896
	1616.33, 896
	1495.01, 896

1458.28, 896
1456.86, 896.353
1456.85, 896.354
1454.84, 896.853
1447.37, 898.703
1446.76, 898.853
1439.22, 900.721
1438.69, 900.853
1431.08, 902.74
1430.62, 902.853
1429.15, 903.217
1429.15, 903.218
1422.55, 904.853
1415.47, 906.607
1415.47, 906.607
1414.48, 906.853
1406.62, 908.801
1406.4, 908.853
1398.46, 910.822
1398.33, 910.853
1390.3, 912.844
1390.26, 912.853
1390.12, 912.889
1390.12, 912.889
1386.77, 913.717
1380.94, 914.966
1373.16, 915.464
1354.04, 916.986
1349.5, 917.317
1340.81, 917.854
1326.59, 918.986
1314.16, 919.891
1308.46, 920.243
1299.13, 920.986
1278.82, 922.465
1276.11, 922.633
1271.72, 922.983
1264.11, 923.65
1247.79, 925
1244.03, 925
1239.2, 925
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1219.79, 925
1211.96, 924.561
1200.98, 923.615
1197.39, 923.302
1194.12, 922.975
1192.25, 922.788
1174.12, 920.975
1161.84, 919.746
1154.12, 918.975
1141.84, 917.746
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1113.76, 914.939
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1064.04, 902.51
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1035.73, 895.432
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1031.23, 894.306
1025.41, 892.851
1023.7, 892.424
1017.41, 890.851
1015.64, 890.407
1009.41, 888.851
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998.336, 886.082
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792.426, 834.605
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	569.392, 778.851
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	538.003, 771
	537.388, 770.846
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Material Boundary	1458.25, 890.855 1495.01, 890.845 1549.83, 890.87 1858.87, 891 2127.47, 890.865 2142.12, 890.909 2195.15, 890.91
	2932.94, 694.87 2908.97, 694.455 2851.01, 693.449 2831.93, 693.119 2824.98, 693.239 2812.84, 693.449 2754.87, 694.452 2698.12, 695.433 2676.93, 695.8 2620.04, 694.813 2600.94, 694.482 2580.01, 694.119 2521.93, 693.112 2503.22, 693.435 2475.13, 693.921 2447.68, 694.392 2388.56, 695.405

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1941.76, 710.423
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1719.06, 689.137
1692.86, 690
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1049.43, 706
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1029.03, 704
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1013.62, 702
1011.96, 701.672
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990.827, 697.433
984.141, 696
982.543, 695.691
974.842, 694
974.456, 693.923
971.617, 693.286
967.06, 692.116
966.644, 692
965.137, 691.608
951.886, 688
947.065, 686.849
944.266, 686
933.943, 684.923
931.35, 684.433
922.861, 684.058
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918.713, 683.952
916.783, 684
914.512, 684.056
914.501, 684.056
885.837, 686
885.02, 686
874.622, 686
835.534, 686
768.454, 687.654
700.514, 686
628.175, 684.242
628.127, 684.241
618.233, 684

Material Boundary

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608.129, 684.513
603.662, 686
599.571, 687.362
597.655, 688
594.551, 689.033
585.639, 692
583.706, 692.643
583.706, 692.643
579.631, 694
577.692, 694.645
573.623, 696
570.071, 697.182
567.615, 698
560.534, 700.359
555.606, 702
551.706, 703.298
549.599, 704
545.694, 705.3
543.591, 706
539.827, 707.301
537.749, 708
529.162, 710.807
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521.35, 713.401
519.551, 714
515.347, 715.4
513.543, 716
509.344, 717.398
507.535, 718
499.16, 720.79
495.524, 722
493.701, 722.607
489.513, 724
486.035, 725.158
483.505, 726
480.031, 727.156
477.496, 728
470.568, 730.307
465.482, 732
465.481, 732
462.023, 733.151
459.474, 734
456.913, 734.853
454.639, 735.61
453.47, 736
450.033, 737.146
447.47, 738
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435.797, 740.289
426.87, 740.119
426.201, 740.106
425.234, 740.088
425.659, 739.946
439.628, 735.289
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1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
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1131.8, 913.879
1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718
1219.87, 922.15
1223.74, 922.15
1239.2, 922.15
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1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
1340.6, 915.011
1349.31, 914.473
1353.82, 914.144
1372.95, 912.621
1380.54, 912.136
1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
1397.65, 908.087
1397.77, 908.056
1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
1414.78, 903.841
1414.79, 903.84
1421.86, 902.087
1428.46, 900.451

1428.47, 900.45
1429.93, 900.087
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1438.54, 897.955
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1446.69, 895.936
1454.15, 894.087
1456.17, 893.588
1456.17, 893.587
1457.93, 893.15
1495.01, 893.15
1616.33, 893.15
1858.87, 893.15
2127.47, 893.15
2141.81, 893.15
2181.65, 893.15
2188.32, 893.15

Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	



372.719, 735.318
379.181, 735.415
391.383, 735.573
399.078, 735.005
407.323, 734.789
412.029, 734.347
439.628, 735.289
488.131, 719.138
522.779, 707.602
608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332

930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262
1026.16, 698.595
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1068.84, 701.86
1069.64, 701.862
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1077.4, 701.408
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1105.21, 699.039
1108.51, 698.705
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1144.75, 691.993
1145.42, 691.857
1146.43, 691.649
1147.98, 691.316
1148.3, 691.248
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1156.36, 689.409
1157.27, 689.184
1163.63, 687.532
1180.77, 687.159
1199.14, 687
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1274.47, 687.001
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1495.01, 687
1500.34, 687
1516.14, 686.278


Water Table

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1692.76, 685
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1781.09, 683.397
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1816.05, 683
1826.62, 683
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1854.88, 679
1862.97, 679
1893.13, 688.871
1989.22, 720.9
2041.9, 738.472
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2229.99, 688.38
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2388.53, 690.405
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2475.13, 688.921
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2521.93, 688.112
2580.04, 689.12
2600.94, 689.482
2620.01, 689.813
2676.93, 690.8
2698.09, 690.434
2754.87, 689.452
2812.87, 688.448
2824.98, 688.239
2831.93, 688.119
2850.97, 688.449
2908.97, 689.455
2928.03, 689.785
2933.03, 689.872
2955.52, 685.26
2966.15, 685.445
2974.76, 685.594
2986.93, 685.805
3005.67, 685.481
3026.51, 685.12
3083.88, 684.128
3141.93, 683.124
3161.05, 683.455
3219.11, 684.462
3276.35, 685.455
3287.99, 685.657
3307.91, 690.624
3403.42, 714.438
3425.9, 720.041
3498.4, 738.118
3511.05, 738.153
3513.27, 738.159
3516.47, 738.166
3555.99, 738.915
3560.24, 738.996
3560.38, 738.996
3562.81, 738.996
3565.35, 738.996
3572.15, 738.997

3572.42, 738.997
3572.75, 738.997
3574.89, 738.997
3575.23, 738.997
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3577.15, 738.997
3579.94, 738.997
3582.64, 738.997
3589.33, 738.997
3589.49, 738.996
3608.27, 738.896
3660.68, 738.616
3711.23, 738.347
3731.88, 738.243
3847.86, 737.635
3874.25, 737.524
3878.92, 737.484
3880.4, 737.484
3891.32, 737.481
3896.18, 737.436
3923.28, 737.159
3936.98, 737.008
3939.65, 737.003
3940.24, 737.002
3940.54, 737.001
3941.23, 737
3943.19, 736.963
3984.29, 736.189
4046.31, 735.003
4069.83, 733.736
4083.35, 733.007
4091.4, 732.6
4099.87, 732.313
4107.4, 732.001
4111.22, 731.848
4120.86, 731.486
4126.77, 731.314
4134.44, 731.066
4136.21, 731.004
4147.03, 730.471
4149.86, 730.331
4152.27, 730.207
4159.68, 729.828
4161.98, 729.722
4168.35, 729.381
4170.28, 729.323
4176.46, 729.027
4183.66, 729
4184.9, 728.957
4186.51, 728.967
4190.68, 728.712
4192, 728.641
4193.48, 728.515
4210.43, 727.177
4212.1, 727.044
4212.72, 727
4212.98, 727

	<p>4213.12, 727 4217.08, 727 4218.09, 727 4220.71, 727 4222.66, 727 4227.99, 727 4240.16, 727 4251.63, 727 4253.3, 727 4258.25, 727 4278.23, 727 4287.06, 727 4288.26, 727 4302.4, 727 4314.27, 727 4336.91, 727 4362.15, 735.41 4369.76, 737.944 4370.23, 737.927 4374.32, 736.567 4379.34, 734.898 4386.69, 734.881 4390.98, 734.901 4397.07, 736.947 4400, 737.917 4410.58, 738.104 4428.73, 738.546 4434.16, 742.558 4734.38, 742.558 4750.1, 742.558 4763.34, 747 4763.91, 747 4769.69, 747 4774.89, 745.413 4775.26, 745.277 4780.91, 743.405 4781.36, 743.255 4786.89, 741.421 4787.39, 741.255 4792.88, 739.436 4793.56, 739.207 4794.3, 739.011 4794.4, 738.986 4794.51, 738.956 4794.64, 738.918 4795.45, 738.671 4799.4, 737.389 4800.33, 737.082 4801.58, 737.091 4806.03, 737.155 5009.05, 738.418</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735</p>	<p>Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and</p>

459.474, 735
462.023, 734.151
465.481, 733
465.482, 733
470.568, 731.307
477.496, 729
480.031, 728.156
483.505, 727
486.035, 726.158
489.513, 725
493.701, 723.607
495.524, 723
499.16, 721.79
507.535, 719
509.344, 718.398
513.543, 717
515.347, 716.4
519.551, 715
521.35, 714.401
525.559, 713
529.162, 711.807
537.749, 709
539.827, 708.301
543.591, 707
545.694, 706.3
549.599, 705
551.706, 704.298
555.606, 703
560.534, 701.359
567.615, 699
570.071, 698.182
573.623, 697
577.692, 695.645
579.631, 695
583.706, 693.643
583.706, 693.643
585.639, 693
594.551, 690.033
597.655, 689
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603.662, 687
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609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
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835.534, 687
874.622, 687
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885.837, 687
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914.512, 685.056
916.783, 685
918.713, 684.952
920.67, 685
922.861, 685.058
931.35, 685.433
933.943, 685.923

Sideslopes
 Low Permeable
Earth Liner

Piezoline

944.266, 687
947.065, 687.849
951.886, 689
965.137, 692.608
966.644, 693
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1119.86, 703.126
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1152.34, 696.587
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2025.75, 739.402
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	440.59, 740.214	
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	450.009, 737.075	
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	465.47, 731.925	
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	521.326, 713.33	
	525.536, 711.929	
	529.138, 710.736	
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	539.803, 707.23	
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	545.671, 705.229	
	549.575, 703.929	
	551.683, 703.227	
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	618.234, 683.925	
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	628.177, 684.167	
	700.515, 685.925	
	768.454, 687.578	
	835.533, 685.925	

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Block Search Polyline

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1867.46, 685.702
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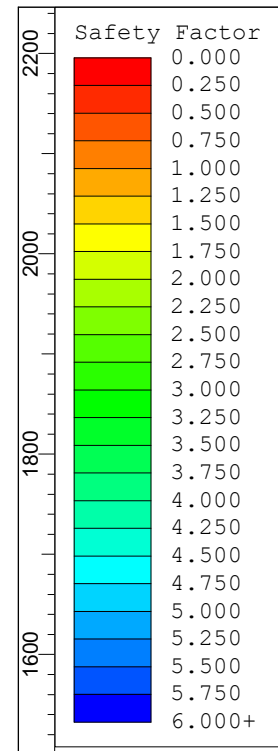
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SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION (**NORTH SLOPE**)

GLOBAL LINER STABILITY ANALYSIS

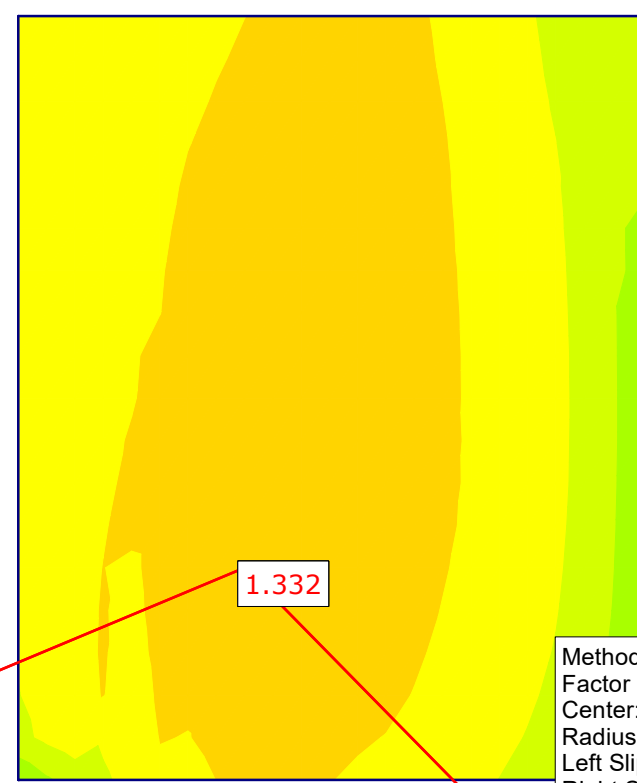
**INTERIM WASTE HEIGHT / PARTIAL BUILDOUT LANDFORM
CIRCULAR ANALYSIS OF WASTE AND FOUNDATION
(ROTATIONAL SLOPE FAILURE)**



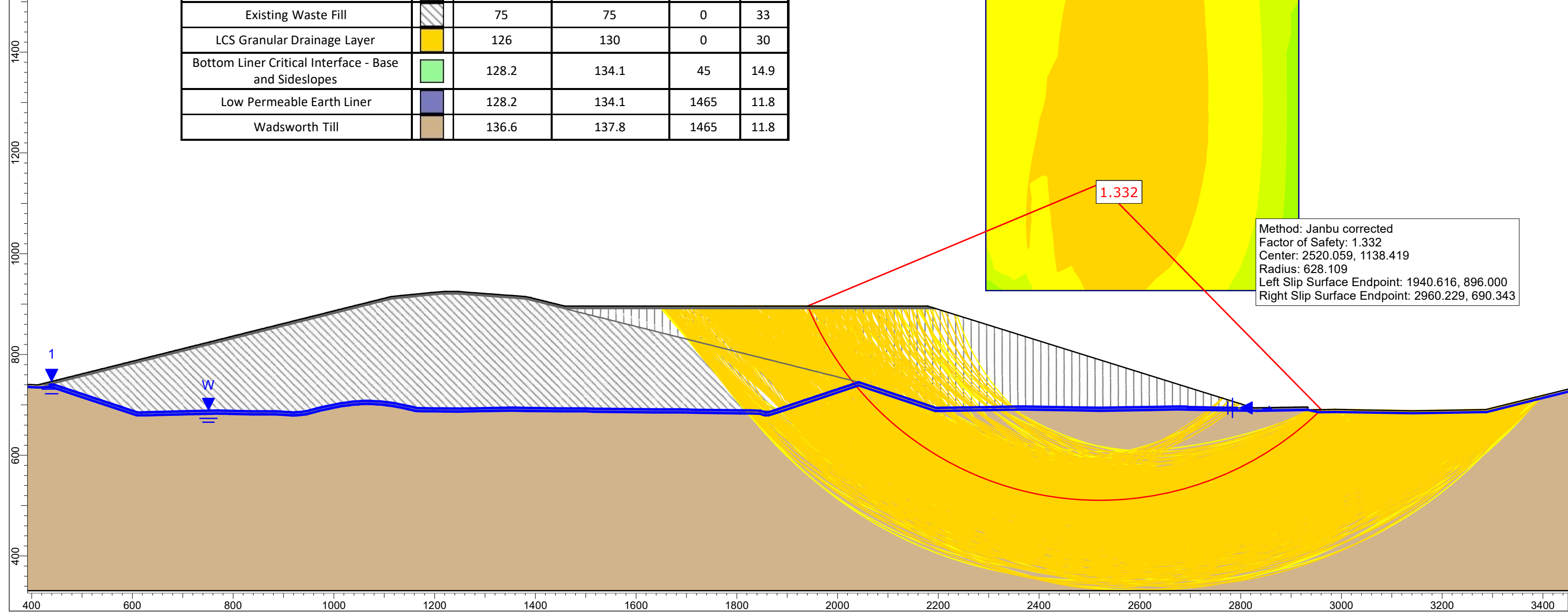
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Intermediate Buildout / Global Stability
Circular - Grid Search
Short Term / Seismic Conditions

0.0461

Material Name	Color	Unit Weight (lbs/ft ³)	Sat. Unit Weight (lbs/ft ³)	Cohesion (psf)	Phi (deg)
Final Cover Soils	[Grey]	121.5	130.3	1465	11.8
Final Cover Critical Interface	[Cyan]	121.5	130.3	0	21.9
Expansion Waste Fill	[Hatched]	75	75	0	33
Existing Waste Fill	[Hatched]	75	75	0	33
LCS Granular Drainage Layer	[Yellow]	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	[Light Green]	128.2	134.1	45	14.9
Low Permeable Earth Liner	[Dark Blue]	128.2	134.1	1465	11.8
Wadsworth Till	[Brown]	136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.332
 Center: 2520.059, 1138.419
 Radius: 628.109
 Left Slip Surface Endpoint: 1940.616, 896.000
 Right Slip Surface Endpoint: 2960.229, 690.343



Slide Analysis Information

A-A_inter_north_ST

Project Summary

File Name:	A-A_inter_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:13.171s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft3]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.402300
Center:	2520.059, 1265.503
Radius:	725.194
Left Slip Surface Endpoint:	1896.061, 896.000
Right Slip Surface Endpoint:	2961.793, 690.370
Resisting Moment:	3.79593e+09 lb-ft
Driving Moment:	2.70693e+09 lb-ft
Total Slice Area:	200273 ft ²
Surface Horizontal Width:	1065.73 ft
Surface Average Height:	187.921 ft

Method: janbu corrected

FS	1.331790
Center:	2520.059, 1138.419
Radius:	628.109
Left Slip Surface Endpoint:	1940.616, 896.000
Right Slip Surface Endpoint:	2960.229, 690.343
Resisting Horizontal Force:	5.19977e+06 lb
Driving Horizontal Force:	3.90434e+06 lb
Total Slice Area:	213900 ft ²
Surface Horizontal Width:	1019.61 ft
Surface Average Height:	209.786 ft

Method: spencer

FS	1.394670
Center:	2520.059, 1307.864
Radius:	758.315
Left Slip Surface Endpoint:	1883.342, 896.000
Right Slip Surface Endpoint:	2960.182, 690.342
Resisting Moment:	3.87945e+09 lb-ft
Driving Moment:	2.78162e+09 lb-ft
Resisting Horizontal Force:	4.66009e+06 lb
Driving Horizontal Force:	3.34135e+06 lb
Total Slice Area:	195494 ft ²
Surface Horizontal Width:	1076.84 ft
Surface Average Height:	181.544 ft

Method: gle/morgenstern-price

FS	1.398200
Center:	2548.305, 1307.864
Radius:	755.890
Left Slip Surface Endpoint:	1914.477, 896.000
Right Slip Surface Endpoint:	2984.842, 690.770
Resisting Moment:	3.74508e+09 lb-ft
Driving Moment:	2.6785e+09 lb-ft
Resisting Horizontal Force:	4.50888e+06 lb
Driving Horizontal Force:	3.22477e+06 lb
Total Slice Area:	186725 ft ²
Surface Horizontal Width:	1070.37 ft
Surface Average Height:	174.45 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	3040
Number of Invalid Surfaces:	1767

Error Codes

Error Code -103 reported for 1355 surfaces
 Error Code -110 reported for 384 surfaces
 Error Code -112 reported for 28 surfaces

Method: janbu corrected

Number of Valid Surfaces:	3056
Number of Invalid Surfaces:	1751

Error Codes

Error Code -103 reported for 1355 surfaces
 Error Code -110 reported for 384 surfaces
 Error Code -112 reported for 12 surfaces

Method: spencer

Number of Valid Surfaces:	3031
Number of Invalid Surfaces:	1776

Error Codes

Error Code -103 reported for 1355 surfaces
 Error Code -110 reported for 384 surfaces
 Error Code -111 reported for 3 surfaces
 Error Code -112 reported for 34 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	3036
Number of Invalid Surfaces:	1771

Error Codes

Error Code -103 reported for 1355 surfaces
 Error Code -110 reported for 384 surfaces
 Error Code -112 reported for 32 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

Entity Information

◆ Circular - thin layers

Shared Entities

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	2141.81, 896
	2127.47, 896
	1858.87, 896
	1616.33, 896
	1495.01, 896
	1458.28, 896
	1456.86, 896.353
	1456.85, 896.354
	1454.84, 896.853
	1447.37, 898.703
	1446.76, 898.853
	1439.22, 900.721
	1438.69, 900.853
	1431.08, 902.74
	1430.62, 902.853
	1429.15, 903.217
	1429.15, 903.218
	1422.55, 904.853
	1415.47, 906.607
	1415.47, 906.607
	1414.48, 906.853
	1406.62, 908.801
	1406.4, 908.853
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755.043, 822.321
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Material Boundary

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778.761, 828.251
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938.101, 868.086
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1162.12, 916.911
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2187.87, 893.15

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
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	1.45658, 723.917	
	1.93345, 723.924	

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267.74, 727.882
267.83, 727.883
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269.061, 727.877
272.624, 727.847
281.529, 727.773
284.508, 727.748
284.959, 727.743
286.557, 727.708
289.734, 727.637
295.936, 727.498
301.316, 727.378
309.904, 727.186
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317.794, 727
322.428, 727
327.892, 727
334.438, 727
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356.385, 735.013
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372.719, 735.318
379.181, 735.415
391.383, 735.573
399.078, 735.005
407.323, 734.789
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439.628, 735.289
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746.542, 682.118
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925.393, 679.164
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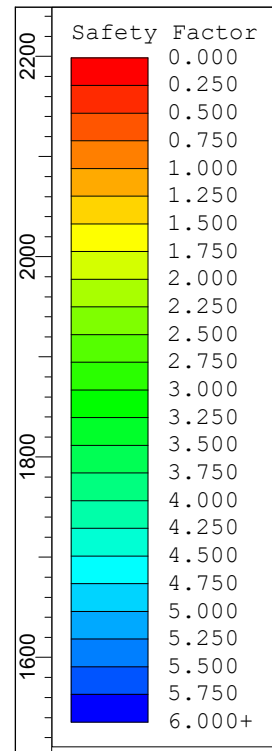
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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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560.534, 701.359
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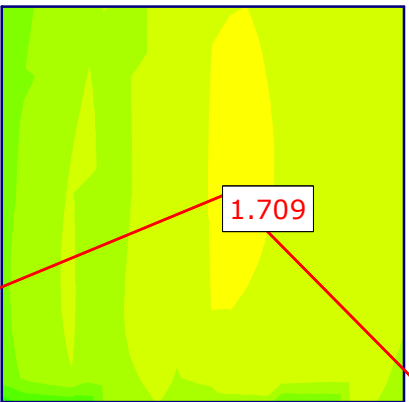
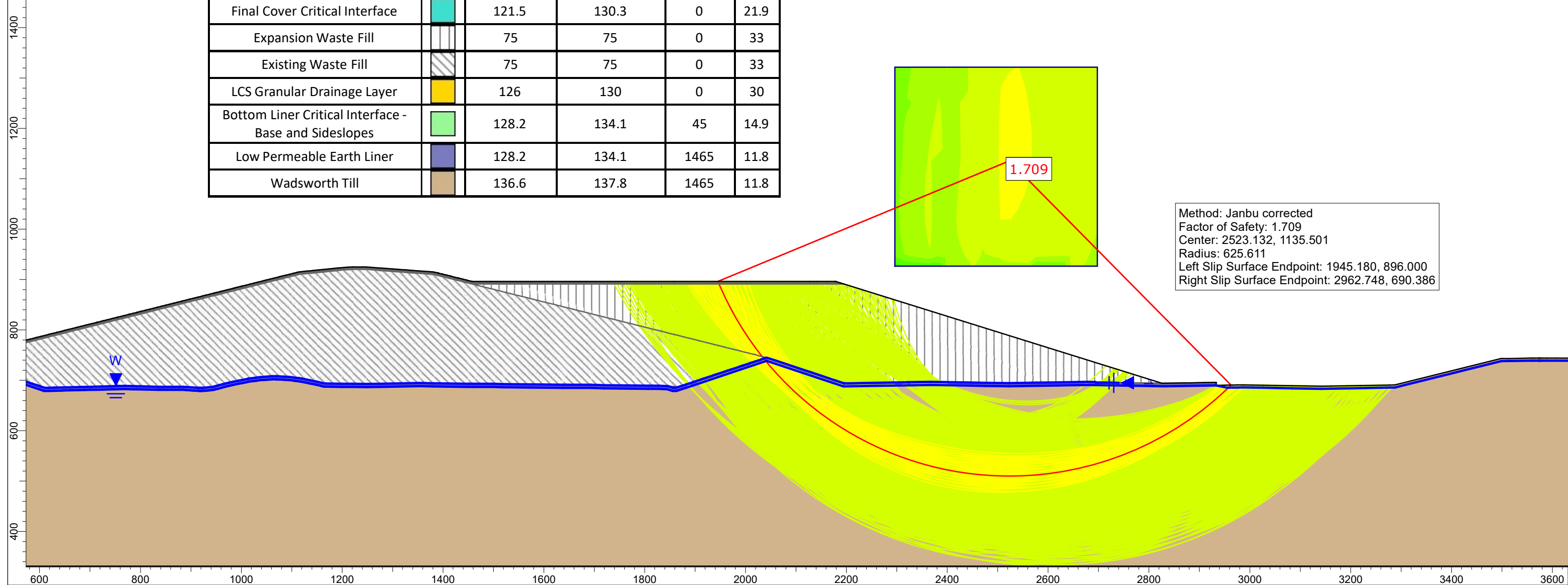
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Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Intermediate Buildout / Global Stability
Circular - Grid Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.709
 Center: 2523.132, 1135.501
 Radius: 625.611
 Left Slip Surface Endpoint: 1945.180, 896.000
 Right Slip Surface Endpoint: 2962.748, 690.386

Slide Analysis Information

A-A_inter_north_ST

Project Summary

File Name:	A-A_inter_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:08.443s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


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Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.786670
Center:	2540.592, 1274.851	
Radius:	730.642	
Left Slip Surface Endpoint:	1915.845, 896.000	
Right Slip Surface Endpoint:	2979.428, 690.676	
Resisting Moment:	3.75664e+09 lb-ft	
Driving Moment:	2.1026e+09 lb-ft	
Total Slice Area:	192763 ft ²	
Surface Horizontal Width:	1063.58 ft	
Surface Average Height:	181.239 ft	

Method: janbu corrected

	FS	1.708600
Center:	2523.132, 1135.501	
Radius:	625.611	
Left Slip Surface Endpoint:	1945.180, 896.000	
Right Slip Surface Endpoint:	2962.748, 690.386	
Resisting Horizontal Force:	5.22396e+06 lb	
Driving Horizontal Force:	3.05745e+06 lb	
Total Slice Area:	213179 ft ²	
Surface Horizontal Width:	1017.57 ft	
Surface Average Height:	209.498 ft	

Method: spencer

	FS	1.777800
Center:	2540.592, 1274.851	
Radius:	730.642	
Left Slip Surface Endpoint:	1915.845, 896.000	
Right Slip Surface Endpoint:	2979.428, 690.676	
Resisting Moment:	3.738e+09 lb-ft	
Driving Moment:	2.1026e+09 lb-ft	
Resisting Horizontal Force:	4.61498e+06 lb	
Driving Horizontal Force:	2.5959e+06 lb	
Total Slice Area:	192763 ft ²	
Surface Horizontal Width:	1063.58 ft	
Surface Average Height:	181.239 ft	

Method: gle/morgenstern-price

	FS	1.779740
Center:		2540.592, 1321.301
Radius:		767.156
Left Slip Surface Endpoint:		1902.119, 896.000
Right Slip Surface Endpoint:		2977.394, 690.640
Resisting Moment:		3.85454e+09 lb-ft
Driving Moment:		2.16579e+09 lb-ft
Resisting Horizontal Force:		4.57226e+06 lb
Driving Horizontal Force:		2.56907e+06 lb
Total Slice Area:		187618 ft ²
Surface Horizontal Width:		1075.27 ft
Surface Average Height:		174.484 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	1994
Number of Invalid Surfaces:	2758

Error Codes

Error Code -103 reported for 2055 surfaces
 Error Code -110 reported for 639 surfaces
 Error Code -112 reported for 64 surfaces

Method: janbu corrected

Number of Valid Surfaces:	2022
Number of Invalid Surfaces:	2730

Error Codes

Error Code -103 reported for 2055 surfaces
 Error Code -110 reported for 639 surfaces
 Error Code -112 reported for 36 surfaces

Method: spencer

Number of Valid Surfaces:	1991
Number of Invalid Surfaces:	2761

Error Codes

Error Code -103 reported for 2055 surfaces
 Error Code -110 reported for 639 surfaces
 Error Code -112 reported for 67 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	1992
Number of Invalid Surfaces:	2760

Error Codes

Error Code -103 reported for 2055 surfaces
 Error Code -110 reported for 639 surfaces
 Error Code -112 reported for 66 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

Entity Information

Circular - thin layers

Shared Entities

Type	Coordinates (x,y)
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	2177.34, 896
	2141.81, 896
	2127.47, 896
	1858.87, 896
	1616.33, 896
	1495.01, 896
	1458.28, 896
	1456.86, 896.353
	1456.85, 896.354
	1454.84, 896.853
	1447.37, 898.703
	1446.76, 898.853
	1439.22, 900.721
	1438.69, 900.853
	1431.08, 902.74
	1430.62, 902.853
	1429.15, 903.217
	1429.15, 903.218
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883.293, 854.229
884.182, 854.452
890.137, 855.94
895.898, 857.381
898.137, 857.94
904.884, 859.627
906.137, 859.94
912.782, 861.602
914.137, 861.94
915.557, 862.295
922.137, 863.94
923.565, 864.297
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938.137, 867.94
944.598, 869.556
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948.322, 870.486
949.797, 870.855
950.928, 871.138
951.605, 871.307
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964.95, 874.643
970.137, 875.94
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986.137, 879.94
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989.211, 880.709
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994.137, 881.94
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1077.04, 902.666
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1098.14, 907.94
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1106.14, 909.94
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	1446.04, 895.941
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	1454.11, 893.941
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	1457.91, 893
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	426.201, 740.106
	442.121, 744.086
	449.756, 745.995
	450.121, 746.086
	450.446, 746.167
	454.117, 747.086
	458.111, 748.086
	461.436, 748.919
	466.102, 750.086
	473.419, 751.912
	474.113, 752.086
	474.811, 752.26
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	486.617, 755.213
	490.077, 756.086
	493.601, 756.959
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	530.085, 766.086
	538.079, 768.081
	538.353, 768.15
	538.357, 768.15
	546.101, 770.086
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	554.096, 772.086
	554.833, 772.27

562.083, 774.086
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594.853, 782.275
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626.101, 790.086
633.31, 791.888
634.101, 792.086
634.901, 792.286
642.101, 794.086
645.858, 795.025
650.112, 796.086
654.377, 797.152
658.111, 798.086
662.404, 799.162
666.101, 800.086
673.276, 801.878
674.112, 802.086
674.953, 802.296
674.961, 802.298
682.111, 804.086
685.598, 804.957
690.111, 806.086
693.564, 806.949
698.111, 808.086
702.685, 809.229
706.111, 810.086
713.227, 811.865
713.234, 811.867
714.111, 812.086
714.999, 812.308
715.007, 812.31
722.112, 814.086
726.775, 815.254
730.101, 816.086
734.809, 817.263
738.101, 818.086
742.842, 819.271
746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
766.886, 825.282
770.101, 826.086
777.704, 827.987


Material Boundary

777.707, 827.987
778.101, 828.086
778.761, 828.251
786.101, 830.086
793.117, 831.84
793.125, 831.842
794.101, 832.086
795.089, 832.333
802.101, 834.086
808.438, 835.67
810.101, 836.086
811.81, 836.513
818.101, 838.086
819.843, 838.521
826.101, 840.086
833.041, 841.821
834.101, 842.086
835.202, 842.361
842.101, 844.086
849.167, 845.852
850.101, 846.086
851.073, 846.329
858.101, 848.086
859.116, 848.34
866.101, 850.086
867.161, 850.351
874.101, 852.086
880.995, 853.809
882.101, 854.086
883.256, 854.375
884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773
906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
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970.101, 876.086
973.942, 877.046

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989.174, 880.854
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999.144, 883.347
1001, 883.811
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1002.22, 884.117
1002.54, 884.195
1002.57, 884.202
1002.93, 884.294
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1036.44, 892.671
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1077.01, 902.812
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1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
1114.25, 912.124
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1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718

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1247.67, 922.15
1263.87, 920.811
1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
1340.6, 915.011
1349.31, 914.473
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1372.95, 912.621
1380.54, 912.136
1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
1397.65, 908.087
1397.77, 908.056
1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
1414.78, 903.841
1414.79, 903.84
1421.86, 902.087
1428.46, 900.451
1428.47, 900.45
1429.93, 900.087
1430.39, 899.974
1438.01, 898.087
1438.54, 897.955
1446.08, 896.087
1446.69, 895.936
1454.15, 894.087
1456.17, 893.588
1456.17, 893.587
1457.93, 893.15
1495.01, 893.15
1616.33, 893.15
1858.87, 893.15
2127.47, 893.15
2141.81, 893.15
2181.65, 893.15
2187.87, 893.15

Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	

4.30816, 723.96
12.6281, 724.084
24.2881, 724.257
113.381, 725.584
150.125, 726.131
251.754, 727.644
262.684, 727.807
264.857, 727.839
267.412, 727.877
267.74, 727.882
267.83, 727.883
268.093, 727.885
268.368, 727.882
269.061, 727.877
272.624, 727.847
281.529, 727.773
284.508, 727.748
284.959, 727.743
286.557, 727.708
289.734, 727.637
295.936, 727.498
301.316, 727.378
309.904, 727.186
313.768, 727.1
314.73, 727
317.794, 727
322.428, 727
327.892, 727
334.438, 727
352.838, 735.003
356.385, 735.013
366.425, 735.2
372.719, 735.318
379.181, 735.415
391.383, 735.573
399.078, 735.005
407.323, 734.789
412.029, 734.347
439.628, 735.289
488.131, 719.138
522.779, 707.602
608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481




785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
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983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262
1026.16, 698.595
1029.95, 699.071
1037.13, 700.174
1049.53, 700.993
1053.89, 701.352
1060.67, 701.77
1068.84, 701.86
1069.64, 701.862
1072.74, 701.68
1077.4, 701.408
1085.39, 700.951

Water Table

1093.76, 700.535
1103.93, 699.206
1105.21, 699.039
1108.51, 698.705
1108.62, 698.694
1115.88, 697.749
1119.27, 697.117
1126.35, 695.778
1134.85, 694.035
1143.12, 692.32
1144.75, 691.993
1145.42, 691.857
1146.43, 691.649
1147.98, 691.316
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1149.18, 691.068
1151.08, 690.719
1156.36, 689.409
1157.27, 689.184
1163.63, 687.532
1180.77, 687.159
1199.14, 687
1219.87, 686.821
1247.6, 686.53
1274.47, 687.001
1354.82, 688.409
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1692.76, 685
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1781.09, 683.397
1807.08, 683
1816.05, 683
1826.62, 683
1845.4, 682.209
1854.88, 679
1862.97, 679
1893.13, 688.871
1989.22, 720.9
2041.9, 738.472
2194.18, 687.752
2229.99, 688.38
2366.91, 690.776
2388.53, 690.405
2447.68, 689.392
2475.13, 688.921
2503.25, 688.435
2521.93, 688.112
2580.04, 689.12
2600.94, 689.482
2620.01, 689.813
2676.93, 690.8
2698.09, 690.434
2754.87, 689.452

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2831.93, 688.119
2850.97, 688.449
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2928.03, 689.785
2933.03, 689.872
2955.52, 685.26
2966.15, 685.445
2974.76, 685.594
2986.93, 685.805
3005.67, 685.481
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3083.88, 684.128
3141.93, 683.124
3161.05, 683.455
3219.11, 684.462
3276.35, 685.455
3287.99, 685.657
3307.91, 690.624
3403.42, 714.438
3425.9, 720.041
3498.4, 738.118
3511.05, 738.153
3513.27, 738.159
3516.47, 738.166
3555.99, 738.915
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3560.38, 738.996
3562.81, 738.996
3565.35, 738.996
3572.15, 738.997
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3572.75, 738.997
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3608.27, 738.896
3660.68, 738.616
3711.23, 738.347
3731.88, 738.243
3847.86, 737.635
3874.25, 737.524
3878.92, 737.484
3880.4, 737.484
3891.32, 737.481
3896.18, 737.436
3923.28, 737.159
3936.98, 737.008
3939.65, 737.003
3940.24, 737.002
3940.54, 737.001

3941.23, 737
3943.19, 736.963
3984.29, 736.189
4046.31, 735.003
4069.83, 733.736
4083.35, 733.007
4091.4, 732.6
4099.87, 732.313
4107.4, 732.001
4111.22, 731.848
4120.86, 731.486
4126.77, 731.314
4134.44, 731.066
4136.21, 731.004
4147.03, 730.471
4149.86, 730.331
4152.27, 730.207
4159.68, 729.828
4161.98, 729.722
4168.35, 729.381
4170.28, 729.323
4176.46, 729.027
4183.66, 729
4184.9, 728.957
4186.51, 728.967
4190.68, 728.712
4192, 728.641
4193.48, 728.515
4210.43, 727.177
4212.1, 727.044
4212.72, 727
4212.98, 727
4213.12, 727
4217.08, 727
4218.09, 727
4220.71, 727
4222.66, 727
4227.99, 727
4240.16, 727
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4253.3, 727
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4287.06, 727
4288.26, 727
4302.4, 727
4314.27, 727
4336.91, 727
4362.15, 735.41
4369.76, 737.944
4370.23, 737.927
4374.32, 736.567
4379.34, 734.898
4386.69, 734.881
4390.98, 734.901
4397.07, 736.947
4400, 737.917
4410.58, 738.104
4428.73, 738.546

	<p>4434.16, 742.558 4734.38, 742.558 4750.1, 742.558 4763.34, 747 4763.91, 747 4769.69, 747 4774.89, 745.413 4775.26, 745.277 4780.91, 743.405 4781.36, 743.255 4786.89, 741.421 4787.39, 741.255 4792.88, 739.436 4793.56, 739.207 4794.3, 739.011 4794.4, 738.986 4794.51, 738.956 4794.64, 738.918 4795.45, 738.671 4799.4, 737.389 4800.33, 737.082 4801.58, 737.091 4806.03, 737.155 5009.05, 738.418</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

555.606, 703
560.534, 701.359
567.615, 699
570.071, 698.182
573.623, 697
577.692, 695.645
579.631, 695
583.706, 693.643
583.706, 693.643
585.639, 693
594.551, 690.033
597.655, 689
599.571, 688.362
603.662, 687
608.129, 685.513
609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
700.514, 687
768.454, 688.654
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874.622, 687
885.02, 687
885.837, 687
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914.512, 685.056
916.783, 685
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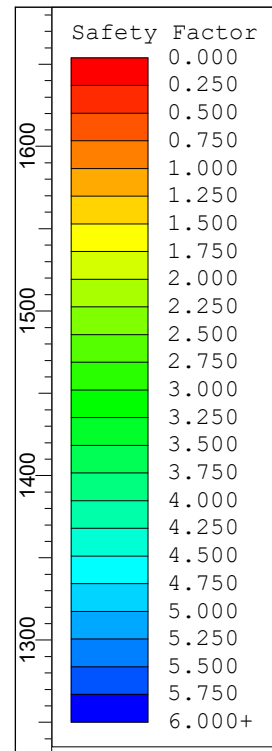
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SLOPE STABILITY
SECTION **B-B'** – HORIZONTAL EXPANSION (**EAST SLOPE**)

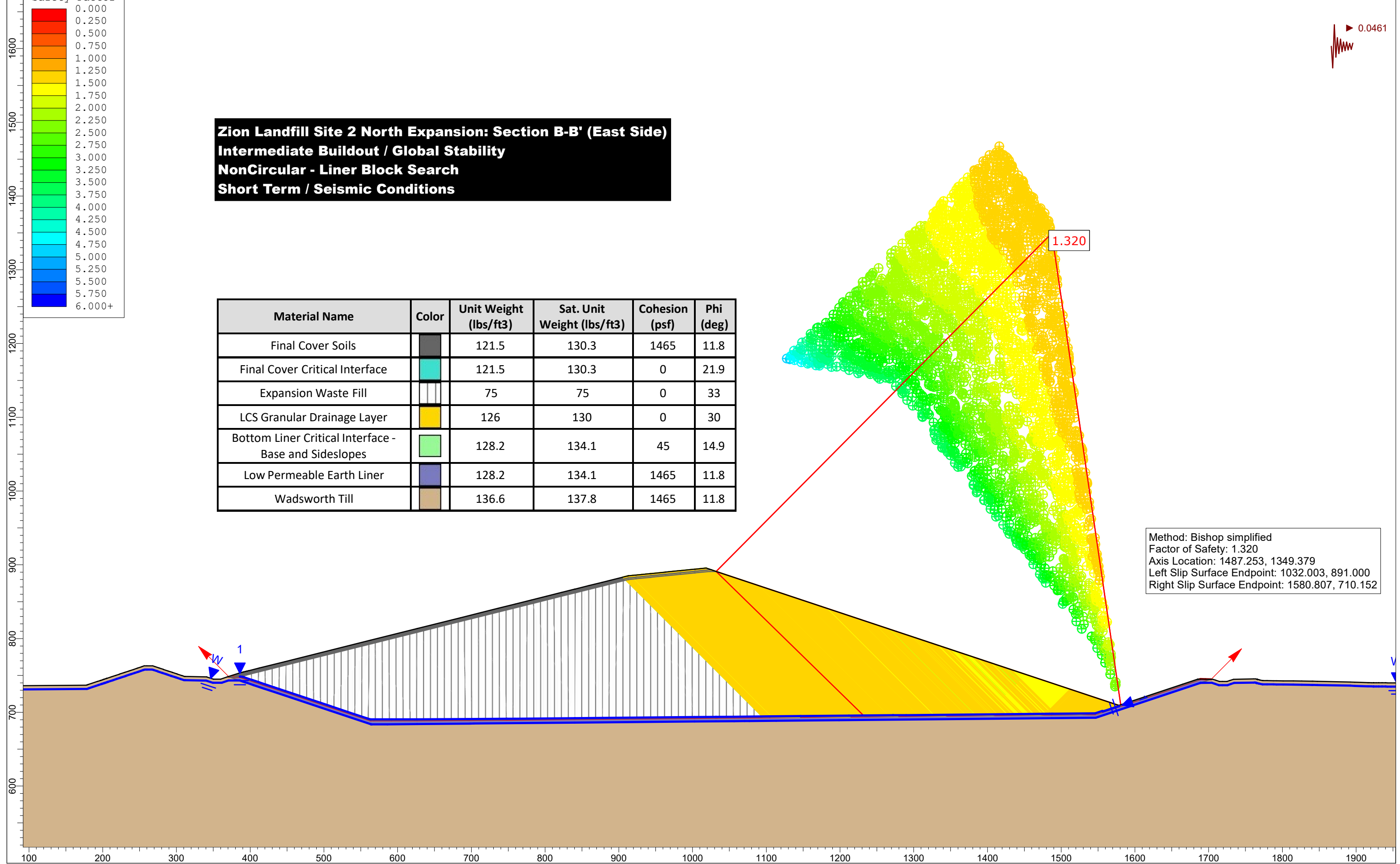
GLOBAL LINER STABILITY ANALYSIS

**INTERIM WASTE HEIGHT / PARTIAL BUILDOUT LANDFORM
BLOCK ANALYSIS OF LINER SYSTEM
(TRANSLATIONAL SLOPE FAILURE)**



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Intermediate Buildout / Global Stability
NonCircular - Liner Block Search
Short Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft ³)	Sat. Unit Weight (lbs/ft ³)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Slide Analysis Information

B-B_inter_east_ST

Project Summary

File Name:	B-B_inter_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:10m:53.588s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8

Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.319690
Axis Location:	1487.253, 1349.379
Left Slip Surface Endpoint:	1032.003, 891.000
Right Slip Surface Endpoint:	1580.807, 710.152
Resisting Moment:	7.12336e+08 lb-ft
Driving Moment:	5.39776e+08 lb-ft
Total Slice Area:	37219.3 ft ²
Surface Horizontal Width:	548.803 ft
Surface Average Height:	67.819 ft

Method: janbu corrected

FS	1.343590
Axis Location:	1486.824, 1346.376
Left Slip Surface Endpoint:	1032.003, 891.000
Right Slip Surface Endpoint:	1578.232, 709.294
Resisting Horizontal Force:	993871 lb
Driving Horizontal Force:	739712 lb
Total Slice Area:	37742.5 ft ²
Surface Horizontal Width:	546.229 ft
Surface Average Height:	69.0965 ft

Method: spencer

FS	1.367370
Axis Location:	1477.562, 1376.895
Left Slip Surface Endpoint:	1006.007, 894.328
Right Slip Surface Endpoint:	1580.683, 710.111
Resisting Moment:	7.65887e+08 lb-ft
Driving Moment:	5.60118e+08 lb-ft
Resisting Horizontal Force:	987892 lb
Driving Horizontal Force:	722477 lb
Total Slice Area:	37629.2 ft ²
Surface Horizontal Width:	574.676 ft
Surface Average Height:	65.4789 ft

Method: gle/morgenstern-price

FS	1.377360
Axis Location:	1477.562, 1376.895
Left Slip Surface Endpoint:	1006.007, 894.328
Right Slip Surface Endpoint:	1580.683, 710.111
Resisting Moment:	7.63127e+08 lb-ft
Driving Moment:	5.54051e+08 lb-ft
Resisting Horizontal Force:	985702 lb
Driving Horizontal Force:	715647 lb
Total Slice Area:	37629.2 ft ²
Surface Horizontal Width:	574.676 ft
Surface Average Height:	65.4789 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1032	891
1233.03	694.54
1509.97	697.113
1540.14	697.393
1546.14	697.53
1556.93	700.982
1575.74	707.284
1580.81	710.152

Method: janbu corrected

X	Y
1032	891
1227.73	694.499
1510.3	697.116
1540.15	697.394
1543.74	697.429
1546.19	697.582
1547.71	697.908
1575.79	707.268
1578.23	709.294

Method: spencer

X	Y
1006.01	894.328
1250.94	694.857
1508.27	697.097
1540.13	697.394
1546.04	697.53
1549.18	698.396
1575.98	707.387
1580.68	710.111

Method: gle/morgenstern-price

X	Y
1006.01	894.328
1250.94	694.857
1508.27	697.097
1540.13	697.394
1546.04	697.53
1549.18	698.396
1575.98	707.387
1580.68	710.111

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 3086
 Number of Invalid Surfaces: 1919

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 1896 surfaces

Method: janbu corrected

Number of Valid Surfaces: 3085
 Number of Invalid Surfaces: 1919

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 1896 surfaces

Method: spencer

Number of Valid Surfaces: 2814
 Number of Invalid Surfaces: 2190

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 17 surfaces
 Error Code -110 reported for 1896 surfaces
 Error Code -111 reported for 256 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 2657
 Number of Invalid Surfaces: 2348

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 23 surfaces
 Error Code -110 reported for 1896 surfaces
 Error Code -111 reported for 408 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	1688.75, 745.08
	1685.18, 744.943
	1685.18, 744.943
	1685.16, 744.936
	1685.16, 744.935
	1685.15, 744.933
	1685.14, 744.929
	1685.08, 744.911
	1664.79, 738.147
	1578.23, 709.294
	1032, 891
	1025.99, 893
	1025.54, 893.15
	1018.3, 895.558
	1003.75, 894.103
	1002.72, 894
	1002.59, 893.986
	983.533, 892.081
	982.719, 892
	963.529, 890.082
	962.713, 890
	943.526, 888.082
	942.708, 888
	923.523, 886.082
	922.702, 886
	913.109, 885.041
	912.699, 885
	912.114, 884.941
	912.089, 884.939
	877.133, 876.202
	859.589, 871.816
	841.214, 867.222
	840.146, 866.956
	822.141, 862.454
	802.222, 857.474
	781.387, 852.265
	759.573, 846.811
	736.709, 841.095

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447.096, 768.701
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397.597, 756.33
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1709.37, 743.539
1705.77, 744.739
1703.67, 744.781
1695.56, 744.943

Material Boundary

371.139, 747.994
371.563, 747.853
384.979, 743.38
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684
698.374, 684.674
703.436, 684.721
840.146, 686
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1031.85, 687.778
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1070.79, 688.133
1076.9, 688.19
1113.49, 688.53
1198.73, 689.322
1487.03, 692
1546.99, 692.557
1550.33, 693.668
1688.75, 739.809
1696.74, 742.47
1703.25, 744.639
1703.64, 744.772
1703.65, 744.774
1703.66, 744.776
1703.67, 744.781

Material Boundary

371.563, 747.853
386.035, 748.141
390.055, 746.801
398.858, 743.867
399.741, 743.572
406.921, 741.178
409.057, 740.466
417.336, 737.706
429.767, 733.562
437.517, 730.978
447.048, 727.801
513.014, 705.81
558.684, 690.525
562.742, 689.207
564.035, 689.194
567.392, 689.194
591.838, 689.21
626.364, 689.21
695.699, 689.663
697.255, 689.673
736.378, 689.929
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840.147, 690.9
877.73, 691.247
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1033.47, 692.692
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1695.55, 744.793
1703.25, 744.639

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	379.447, 748.16
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	1271.77, 696
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	1488.81, 698.064
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	1555.18, 701.611
	1578.23, 709.294

Material Boundary

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399.789, 743.714
406.969, 741.321
409.105, 740.609
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429.814, 733.705
437.565, 731.121
447.096, 727.943
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567.392, 689.344
591.838, 689.36
626.363, 689.36
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697.254, 689.823
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1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary


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391.025, 751.751
391.998, 751.994
392.991, 752.242
394.005, 752.495
395.042, 752.754
396.101, 753.019
397.183, 753.289
398.289, 753.566
399.521, 753.875
412.966, 757.235
447.787, 765.936
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627.055, 810.743
685.799, 825.43
713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
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841.905, 864.458
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962.997, 887.164
963.813, 887.246
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Material Boundary

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392.034, 751.848
393.027, 752.096
394.042, 752.35
395.078, 752.609
396.137, 752.873
397.22, 753.144
398.326, 753.42
399.558, 753.729
413.002, 757.089
447.823, 765.791
564.764, 795.013
627.091, 810.598
685.835, 825.284
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	391.546, 749.665
	392.519, 749.908
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	713.934, 830.247
	737.922, 836.244
	760.786, 841.96
	782.6, 847.414
Material Boundary	803.434, 852.623
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	842.427, 862.372
	860.802, 866.965
	878.345, 871.351
	912.948, 880
	913.197, 880.025
	913.606, 880.066
	923.199, 881.025
	924.02, 881.107
	943.205, 883.025
	944.023, 883.107
	963.211, 885.025
	964.027, 885.106
	983.216, 887.025
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	1003.22, 889.025
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Material Boundary	371.139, 747.994
	371.196, 747.996

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
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	198.448, 738.364	

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201.708, 739.45
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202.96, 739.868
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205.695, 740.779
206.113, 740.918
206.403, 741.015
206.704, 741.116
206.992, 741.211
207.162, 741.268
207.356, 741.333
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223.854, 746.832
225.127, 747.257
229.854, 748.832
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237.127, 751.257
238.4, 751.681
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249.127, 755.257
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255.127, 757.257
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259.897, 757.906
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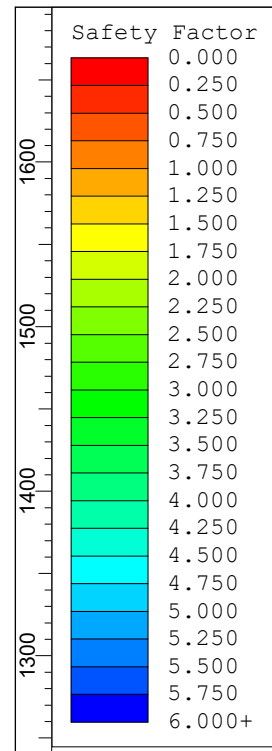
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Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1555.88, 701.845 1556.2, 700.896 1569.82, 705.436 1578.49, 708.327 1578.54, 708.185	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

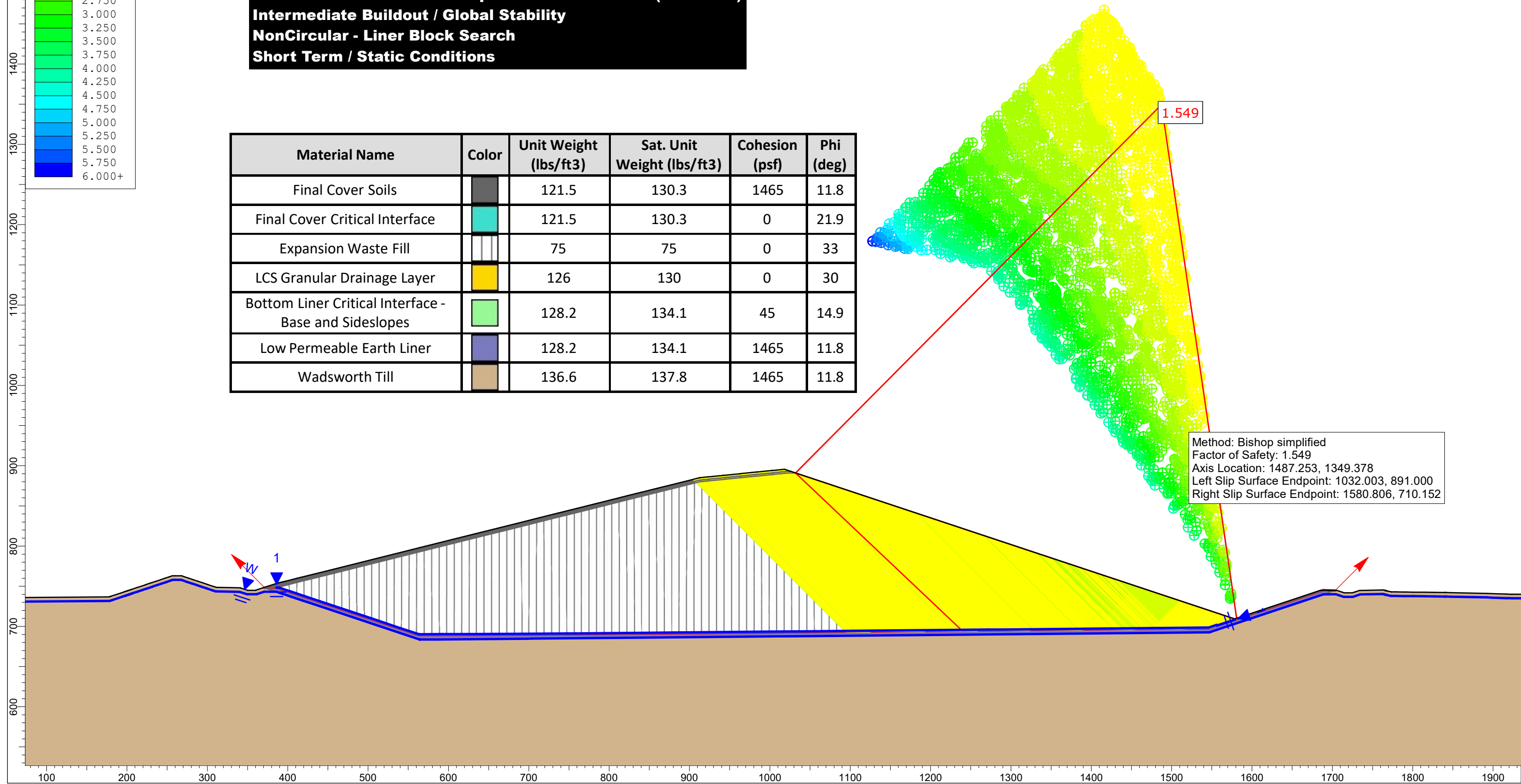
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Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Intermediate Buildout / Global Stability
NonCircular - Liner Block Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Slide Analysis Information

B-B_inter_east_ST

Project Summary

File Name:	B-B_inter_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:12m:50.768s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading







Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465

Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.549040
Axis Location:	1487.253, 1349.378
Left Slip Surface Endpoint:	1032.003, 891.000
Right Slip Surface Endpoint:	1580.806, 710.152
Resisting Moment:	7.23986e+08 lb-ft
Driving Moment:	4.67377e+08 lb-ft
Total Slice Area:	36591.7 ft ²
Surface Horizontal Width:	548.802 ft
Surface Average Height:	66.6755 ft

Method: janbu corrected

FS	1.581870
Axis Location:	1486.824, 1346.376
Left Slip Surface Endpoint:	1032.003, 891.000
Right Slip Surface Endpoint:	1578.232, 709.294
Resisting Horizontal Force:	1.00947e+06 lb
Driving Horizontal Force:	638151 lb
Total Slice Area:	36995.2 ft ²
Surface Horizontal Width:	546.229 ft
Surface Average Height:	67.7284 ft

Method: spencer

FS	1.605820
Axis Location:	1478.681, 1374.088
Left Slip Surface Endpoint:	1008.072, 894.535
Right Slip Surface Endpoint:	1579.958, 709.869
Resisting Moment:	7.65599e+08 lb-ft
Driving Moment:	4.76764e+08 lb-ft
Resisting Horizontal Force:	992212 lb
Driving Horizontal Force:	617883 lb
Total Slice Area:	36332.6 ft ²
Surface Horizontal Width:	571.886 ft
Surface Average Height:	63.5312 ft

Method: gle/morgenstern-price

FS	1.618270
Axis Location:	1478.681, 1374.088
Left Slip Surface Endpoint:	1008.072, 894.535
Right Slip Surface Endpoint:	1579.958, 709.869
Resisting Moment:	7.63198e+08 lb-ft
Driving Moment:	4.71614e+08 lb-ft
Resisting Horizontal Force:	990193 lb
Driving Horizontal Force:	611884 lb
Total Slice Area:	36332.6 ft ²
Surface Horizontal Width:	571.886 ft
Surface Average Height:	63.5312 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1032	891
1239.34	694.599
1510.25	697.115
1540.22	697.394
1544.71	697.502
1546.33	697.609
1564.66	703.557
1576.17	707.526
1580.81	710.152

Method: janbu corrected

X	Y
1032	891
1235.25	694.568
1510.3	697.116
1540.15	697.394
1543.63	697.428
1546.15	697.575
1547.72	697.91
1576.02	707.447
1578.23	709.294

Method: spencer

X	Y
1008.07	894.535
1261.49	694.955
1508.28	697.097
1540.13	697.394
1546.04	697.53
1549.12	698.377
1576.18	707.399
1579.96	709.869

Method: gle/morgenstern-price

X	Y
1008.07	894.535
1261.49	694.955
1508.28	697.097
1540.13	697.394
1546.04	697.53
1549.12	698.377
1576.18	707.399
1579.96	709.869

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces: 3088
 Number of Invalid Surfaces: 1916

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 1 surface
 Error Code -110 reported for 1894 surfaces

Method: janbu corrected

Number of Valid Surfaces: 3087
 Number of Invalid Surfaces: 1917

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 1894 surfaces

Method: spencer

Number of Valid Surfaces: 2865
 Number of Invalid Surfaces: 2139

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 21 surfaces
 Error Code -110 reported for 1894 surfaces
 Error Code -111 reported for 203 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 2729
 Number of Invalid Surfaces: 2276

Error Codes

Error Code -106 reported for 21 surfaces
 Error Code -108 reported for 15 surfaces
 Error Code -110 reported for 1894 surfaces
 Error Code -111 reported for 346 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	1688.75, 745.08
	1685.18, 744.943
	1685.18, 744.943
	1685.16, 744.936
	1685.16, 744.935
	1685.15, 744.933
	1685.14, 744.929
	1685.08, 744.911
	1664.79, 738.147
	1578.23, 709.294
	1032, 891
	1025.99, 893
	1025.54, 893.15
	1018.3, 895.558
	1003.75, 894.103
	1002.72, 894
	1002.59, 893.986
	983.533, 892.081
	982.719, 892
	963.529, 890.082
	962.713, 890
	943.526, 888.082
	942.708, 888
	923.523, 886.082
	922.702, 886
	913.109, 885.041
	912.699, 885
	912.114, 884.941
	912.089, 884.939
	877.133, 876.202
	859.589, 871.816
	841.214, 867.222
	840.146, 866.956
	822.141, 862.454
	802.222, 857.474
	781.387, 852.265
	759.573, 846.811
	736.709, 841.095

712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076

External Boundary

205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729

2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
1960.65, 739.995
1960.64, 739.995
1958.84, 739.961
1958.8, 739.962

1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
1940.44, 740
1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001
1923.26, 740
1923.21, 740
1920.59, 740.087
1917.66, 740.181
1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824

1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539
1705.77, 744.739
1703.67, 744.781
1695.56, 744.943

Material Boundary

371.139, 747.994
371.563, 747.853
384.979, 743.38
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684
698.374, 684.674
703.436, 684.721
840.146, 686
1002.59, 687.5
1031.85, 687.778
1056.47, 688
1070.79, 688.133
1076.9, 688.19
1113.49, 688.53
1198.73, 689.322
1487.03, 692
1546.99, 692.557
1550.33, 693.668
1688.75, 739.809
1696.74, 742.47
1703.25, 744.639
1703.64, 744.772
1703.65, 744.774
1703.66, 744.776
1703.67, 744.781

Material Boundary

371.563, 747.853
386.035, 748.141
390.055, 746.801
398.858, 743.867
399.741, 743.572
406.921, 741.178
409.057, 740.466
417.336, 737.706
429.767, 733.562
437.517, 730.978
447.048, 727.801
513.014, 705.81
558.684, 690.525
562.742, 689.207
564.035, 689.194
567.392, 689.194
591.838, 689.21
626.364, 689.21
695.699, 689.663
697.255, 689.673
736.378, 689.929
748.11, 690.039
840.147, 690.9
877.73, 691.247
1002.59, 692.4
1030.76, 692.667
1031.85, 692.678
1033.44, 692.692
1033.47, 692.692
1056.47, 692.9
1104.66, 693.348
1147.07, 693.742
1212.44, 694.349
1487.04, 696.9
1532.11, 697.319
1546.04, 697.45
1546.34, 697.453
1550.38, 698.797
1653.06, 733.024
1688.78, 744.929
1695.55, 744.793
1703.25, 744.639

	369.041, 747.954
	369.564, 747.963
	371.196, 747.996
	378.927, 748.15
	379.447, 748.16
	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625
	420.368, 737.91
	432.54, 733.852
	440.132, 731.322
	447.096, 729
	503.929, 710.053
	539.503, 697.951
	558.611, 691.762
	558.865, 691.679
	562.929, 690.36
	564.036, 690.348
	567.84, 690.349
	591.835, 690.364
Material Boundary	623.213, 690.364
	626.363, 690.364
	694.207, 690.807
	696.794, 690.824
	748.23, 691.161
	758.969, 691.241
	840.146, 692
	840.75, 692.006
	878.605, 692.355
	1002.59, 693.5
	1031.66, 693.776
	1031.69, 693.776
	1031.85, 693.778
	1056.47, 694
	1104.1, 694.443
	1146.52, 694.837
	1211.99, 695.445
	1271.75, 696
	1271.77, 696
	1289.59, 696.166
	1484.09, 698.02
	1487.03, 698.047
	1488.81, 698.064
	1546.16, 698.605
	1550.33, 699.993
	1555.18, 701.611
	1578.23, 709.294

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary


378.927, 748.15
386.841, 750.704
390.794, 751.693
391.025, 751.751
391.998, 751.994
392.991, 752.242
394.005, 752.495
395.042, 752.754
396.101, 753.019
397.183, 753.289
398.289, 753.566
399.521, 753.875
412.966, 757.235
447.787, 765.936
564.727, 795.159
627.055, 810.743
685.799, 825.43
713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1025.54, 893.15

Material Boundary

379.447, 748.16
386.882, 750.56
390.83, 751.547
391.061, 751.605
392.034, 751.848
393.027, 752.096
394.042, 752.35
395.078, 752.609
396.137, 752.873
397.22, 753.144
398.326, 753.42
399.558, 753.729
413.002, 757.089
447.823, 765.791
564.764, 795.013
627.091, 810.598
685.835, 825.284
713.449, 832.187
737.436, 838.184
760.301, 843.901
782.115, 849.354
802.949, 854.563
822.869, 859.544
840.874, 864.045
841.942, 864.312
860.317, 868.905
877.86, 873.291
912.605, 881.976
912.998, 882.015
913.408, 882.056
923, 883.015
923.821, 883.097
943.006, 885.015
943.825, 885.097
963.012, 887.015
963.828, 887.096
983.017, 889.015
983.831, 889.096
1002.88, 891.001
1003.02, 891.015
1004.05, 891.118
1022.87, 893
1024.07, 893
1025.99, 893

	386.057, 748.292
	387.433, 748.636
	391.315, 749.607
	391.546, 749.665
	392.519, 749.908
	393.512, 750.156
	394.527, 750.409
	395.563, 750.668
	396.622, 750.933
	397.705, 751.204
	398.812, 751.48
	400.043, 751.789
	413.487, 755.149
	448.308, 763.851
	565.249, 793.073
	627.576, 808.658
	686.32, 823.344
	713.934, 830.247
	737.922, 836.244
	760.786, 841.96
	782.6, 847.414
Material Boundary	803.434, 852.623
	823.354, 857.603
	841.359, 862.105
	842.427, 862.372
	860.802, 866.965
	878.345, 871.351
	912.948, 880
	913.197, 880.025
	913.606, 880.066
	923.199, 881.025
	924.02, 881.107
	943.205, 883.025
	944.023, 883.107
	963.211, 885.025
	964.027, 885.106
	983.216, 887.025
	984.03, 887.106
	1003.08, 889.011
	1003.22, 889.025
	1004.25, 889.128
	1022.97, 891
	1024.07, 891
	1032, 891
Material Boundary	371.139, 747.994
	371.196, 747.996

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:  Wadsworth Till
	178.514, 731.719	
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	

198.554, 738.399
198.861, 738.501
199.339, 738.661
200.776, 739.14
200.997, 739.213
201.651, 739.431
201.708, 739.45
202.375, 739.673
202.96, 739.868
203.066, 739.903
203.951, 740.198
204.163, 740.269
205.695, 740.779
206.113, 740.918
206.403, 741.015
206.704, 741.116
206.992, 741.211
207.162, 741.268
207.356, 741.333
213.598, 743.414
219.127, 745.257
223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203

	385.911, 743.07	
	390.102, 741.672	
	392.178, 740.98	
	393.625, 740.498	
	395.889, 739.743	
	404.687, 736.81	
	418.13, 732.329	
	426.507, 729.536	
	447.096, 722.672	
	507.505, 702.534	
	563.515, 683.862	
	563.605, 683.831	
	563.976, 683.705	
	564.036, 683.685	
	564.179, 683.637	
	564.311, 683.592	
	564.361, 683.575	
	564.491, 683.575	
	564.857, 683.576	
	586.189, 683.61	
	626.363, 684	
	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.33, 740.212	
	1764.53, 739.813	
	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
Water Table	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	

1773.36, 737.944
1779.28, 737.93
1787.81, 737.786
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1826.26, 737.371
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1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383

1998.94, 739.947
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2017.72, 744.335
2019.79, 744.824
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2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
2054.59, 753.317
2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493

	2243.14, 734.24 2246.4, 734.069 2300, 733.229	
Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1555.88, 701.845 1556.2, 700.896 1569.82, 705.436 1578.49, 708.327 1578.54, 708.185	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

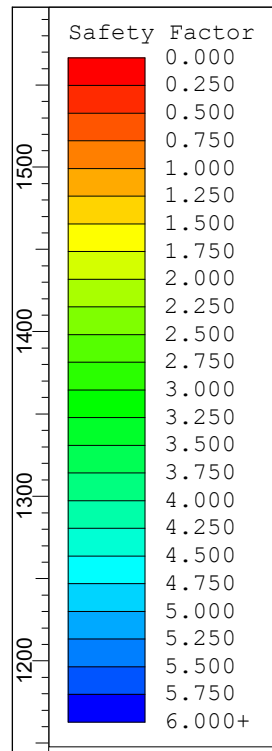
Block Search Polyline

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409.081, 740.537
417.36, 737.777
429.79, 733.633
437.541, 731.05
447.072, 727.872
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1002.59, 692.475
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1031.85, 692.753
1033.44, 692.767
1033.47, 692.767
1056.47, 692.975
1104.66, 693.423
1147.07, 693.817
1212.44, 694.424
1487.03, 696.975
1532.11, 697.394
1546.04, 697.525
1546.33, 697.528
1550.35, 698.868
1653.04, 733.095
1688.76, 745.005
1695.56, 744.868
1703.46, 744.71

SLOPE STABILITY
SECTION **B-B'** – HORIZONTAL EXPANSION (**EAST SLOPE**)

GLOBAL LINER STABILITY ANALYSIS

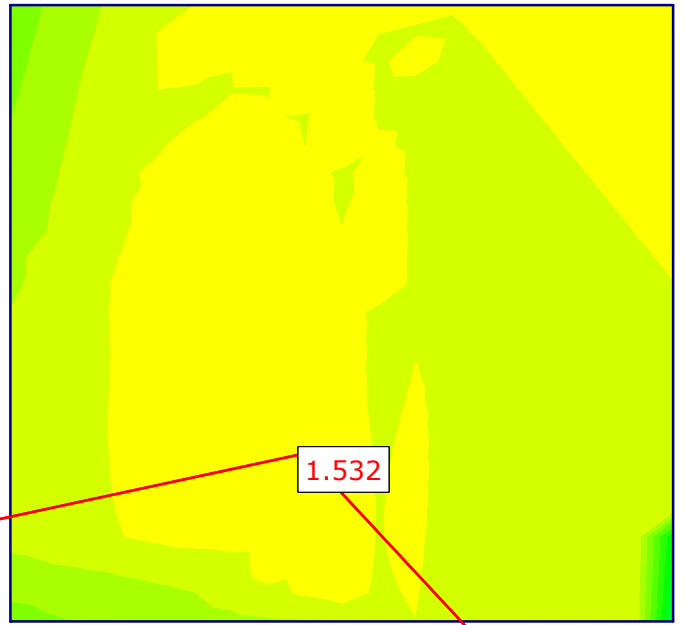
**INTERIM WASTE HEIGHT / PARTIAL BUILDOUT LANDFORM
CIRCULAR ANALYSIS OF WASTE AND FOUNDATION
(ROTATIONAL SLOPE FAILURE)**



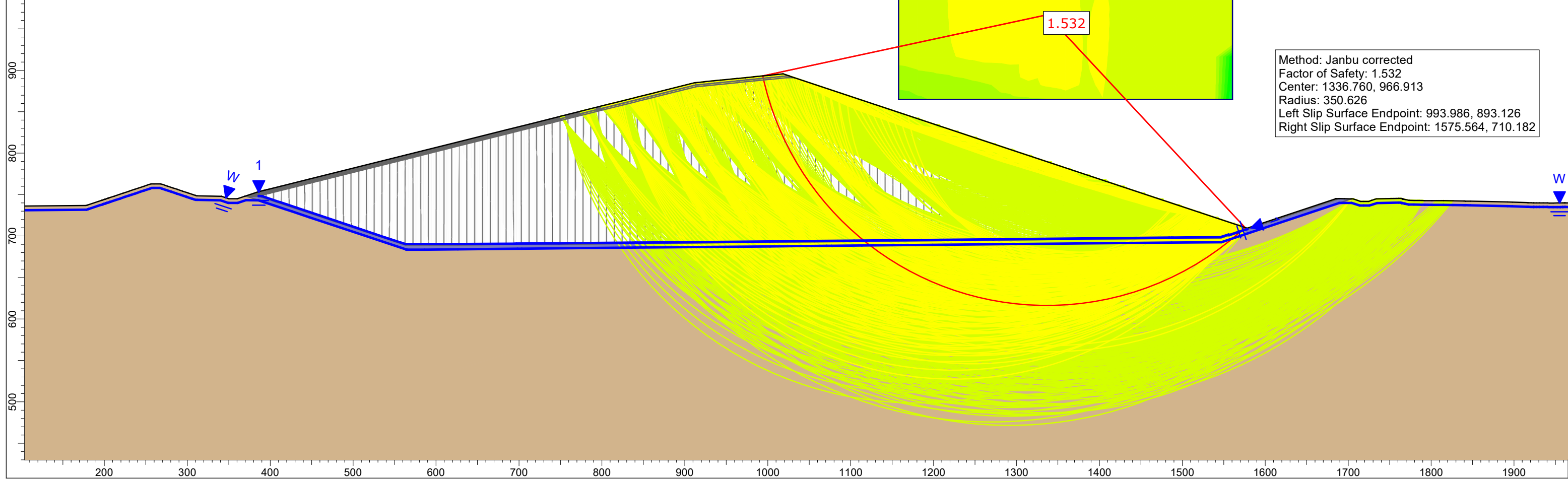
Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Intermediate Buildout / Global Stability
Circular - Grid Search
Short Term / Seismic Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	1465	11.8
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	33
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	1465	11.8
Wadsworth Till	Brown	136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.532
 Center: 1336.760, 966.913
 Radius: 350.626
 Left Slip Surface Endpoint: 993.986, 893.126
 Right Slip Surface Endpoint: 1575.564, 710.182



Slide Analysis Information

B-B_inter_east_ST

Project Summary

File Name:	B-B_inter_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:06.205s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.663310
Center:	1314.382, 1052.026
Radius:	428.452
Left Slip Surface Endpoint:	919.538, 885.684
Right Slip Surface Endpoint:	1573.555, 710.850
Resisting Moment:	1.25102e+09 lb-ft
Driving Moment:	7.52127e+08 lb-ft
Total Slice Area:	90101.7 ft ²
Surface Horizontal Width:	654.016 ft
Surface Average Height:	137.767 ft

Method: janbu corrected

FS	1.532290
Center:	1336.760, 966.913
Radius:	350.626
Left Slip Surface Endpoint:	993.986, 893.126
Right Slip Surface Endpoint:	1575.564, 710.182
Resisting Horizontal Force:	2.33608e+06 lb
Driving Horizontal Force:	1.52457e+06 lb
Total Slice Area:	79621.5 ft ²
Surface Horizontal Width:	581.578 ft
Surface Average Height:	136.906 ft

Method: spencer

FS	1.627470
Center:	1336.760, 1017.981
Radius:	384.332
Left Slip Surface Endpoint:	973.967, 891.125
Right Slip Surface Endpoint:	1569.560, 712.179
Resisting Moment:	9.41997e+08 lb-ft
Driving Moment:	5.7881e+08 lb-ft
Resisting Horizontal Force:	2.12121e+06 lb
Driving Horizontal Force:	1.30338e+06 lb
Total Slice Area:	74221.2 ft ²
Surface Horizontal Width:	595.593 ft
Surface Average Height:	124.617 ft

Method: gle/morgenstern-price

	FS	1.626360
Center:		1336.760, 1035.003
Radius:		395.922
Left Slip Surface Endpoint:		968.134, 890.542
Right Slip Surface Endpoint:		1567.114, 712.992
Resisting Moment:		9.58708e+08 lb-ft
Driving Moment:		5.8948e+08 lb-ft
Resisting Horizontal Force:		2.10439e+06 lb
Driving Horizontal Force:		1.29392e+06 lb
Total Slice Area:		72428.5 ft ²
Surface Horizontal Width:		598.98 ft
Surface Average Height:		120.92 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	3952
Number of Invalid Surfaces:	855

Error Codes

Error Code -103 reported for 62 surfaces
 Error Code -110 reported for 435 surfaces
 Error Code -112 reported for 192 surfaces
 Error Code -114 reported for 166 surfaces

Method: janbu corrected

Number of Valid Surfaces:	3980
Number of Invalid Surfaces:	827

Error Codes

Error Code -103 reported for 62 surfaces
 Error Code -108 reported for 1 surface
 Error Code -110 reported for 435 surfaces
 Error Code -112 reported for 163 surfaces
 Error Code -114 reported for 166 surfaces

Method: spencer

Number of Valid Surfaces:	3920
Number of Invalid Surfaces:	887

Error Codes

Error Code -103 reported for 62 surfaces
 Error Code -108 reported for 1 surface
 Error Code -110 reported for 435 surfaces
 Error Code -111 reported for 7 surfaces
 Error Code -112 reported for 216 surfaces
 Error Code -114 reported for 166 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 3931

Number of Invalid Surfaces: 876

Error Codes

Error Code -103 reported for 62 surfaces

Error Code -108 reported for 1 surface

Error Code -110 reported for 435 surfaces

Error Code -112 reported for 212 surfaces

Error Code -114 reported for 166 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-114 = Surface with Reverse Curvature.

Entity Information

◆ Circ - thin layers

Shared Entities

Type	Coordinates (x,y)
	1688.75, 745.08
	1685.18, 744.943
	1685.18, 744.943
	1685.16, 744.936
	1685.16, 744.935
	1685.15, 744.933
	1685.14, 744.929
	1685.08, 744.911
	1664.79, 738.147
	1578.23, 709.294
	1032, 891
	1026.16, 893
	1025.72, 893.15
	1018.58, 895.585
	1003.75, 894.103
	1002.72, 894
	1002.59, 893.986
	983.533, 892.081
	982.719, 892
	963.529, 890.082

962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762

267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
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External Boundary

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Material Boundary

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393.625, 740.498
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Material Boundary

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	395.042, 752.754
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	399.521, 753.875
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	782.078, 849.5
Material Boundary	802.913, 854.709
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	841.905, 864.458
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	877.824, 873.437
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	912.983, 882.164
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	923.806, 883.246
	942.991, 885.164
	943.81, 885.246
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	963.813, 887.246
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	983.816, 889.246
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	1025.72, 893.15

Material Boundary

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392.034, 751.848
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394.042, 752.35
395.078, 752.609
396.137, 752.873
397.22, 753.144
398.326, 753.42
399.558, 753.729
413.002, 757.089
447.823, 765.791
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685.835, 825.284
713.449, 832.187
737.436, 838.184
760.301, 843.901
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	944.023, 883.107
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	964.027, 885.106
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	984.03, 887.106
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	1003.22, 889.025
	1004.25, 889.128
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	1024.07, 891
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Material Boundary	371.139, 747.994
	371.196, 747.996

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
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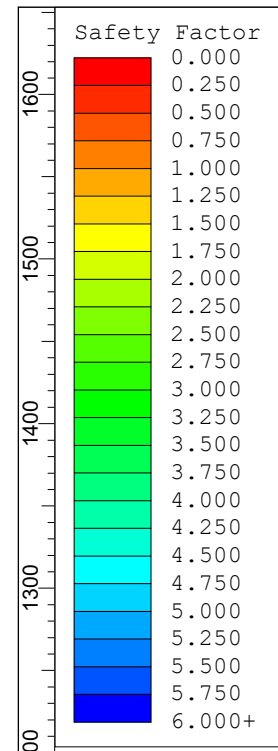
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	564.491, 683.575	
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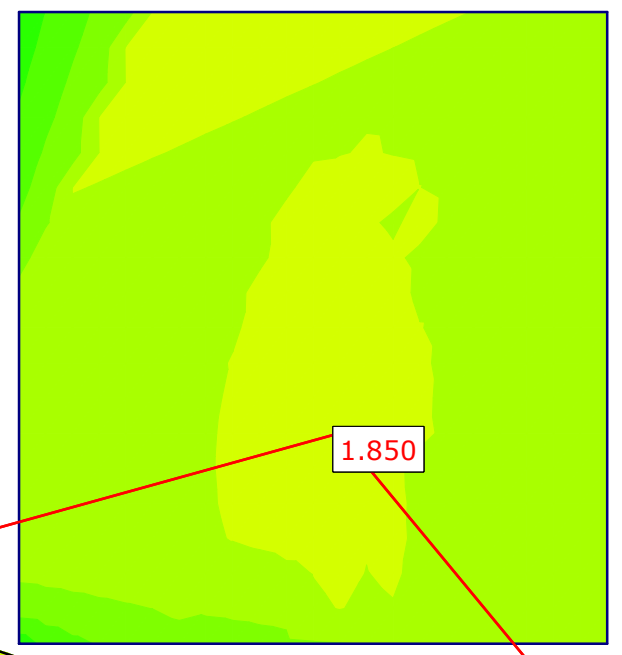
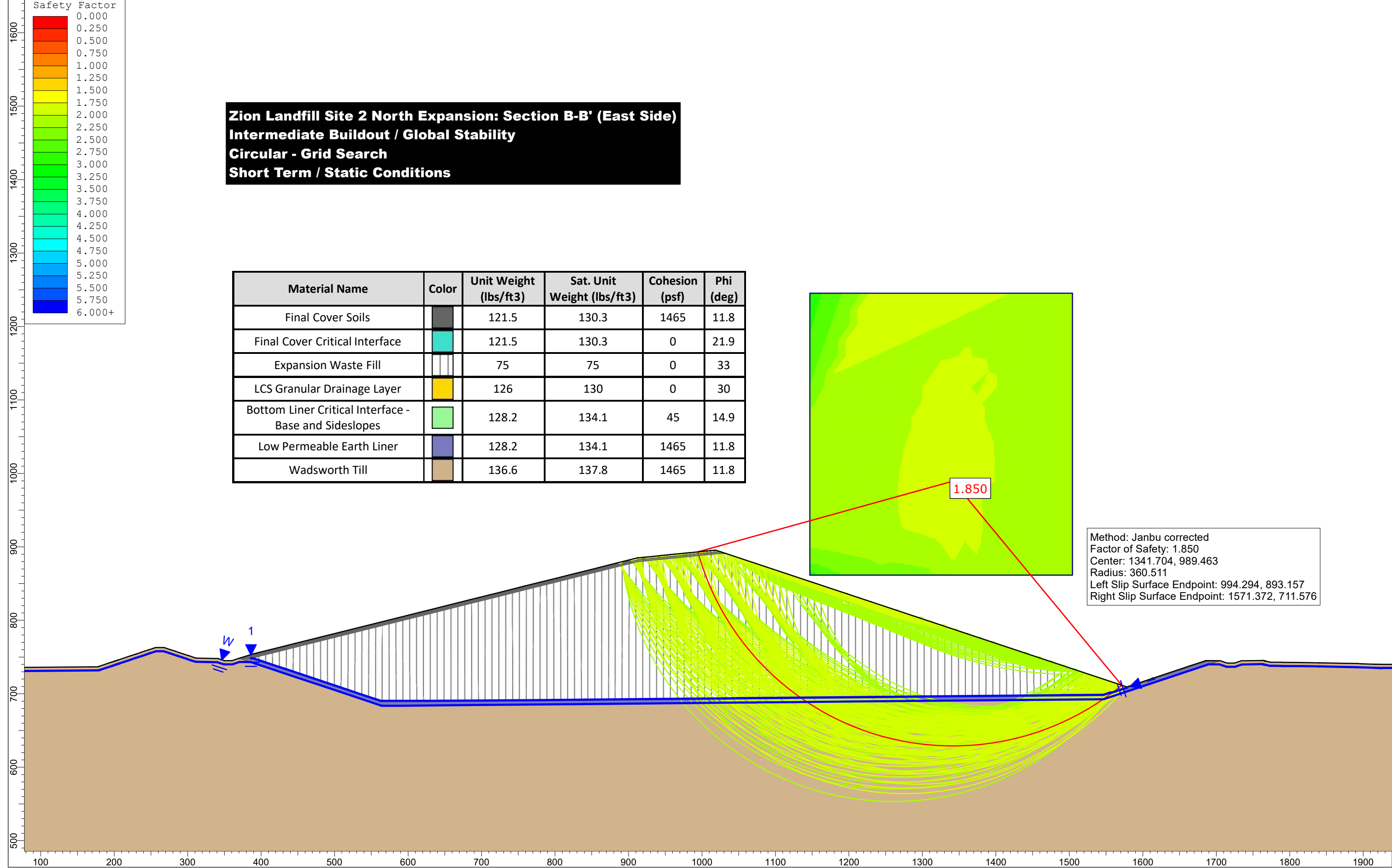
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2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493

	2243.14, 734.24 2246.4, 734.069 2300, 733.229	
Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1555.88, 701.845 1556.2, 700.896 1569.82, 705.436 1578.49, 708.327 1578.54, 708.185	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Intermediate Buildout / Global Stability
Circular - Grid Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.850
 Center: 1341.704, 989.463
 Radius: 360.511
 Left Slip Surface Endpoint: 994.294, 893.157
 Right Slip Surface Endpoint: 1571.372, 711.576

Slide Analysis Information

B-B_inter_east_ST

Project Summary

File Name:	B-B_inter_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:05.414s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.976480
Center:		1227.976, 1245.266
Radius:		403.573
Left Slip Surface Endpoint:		1038.996, 888.674
Right Slip Surface Endpoint:		1165.690, 846.528
Resisting Moment:		9.27754e+06 lb-ft
Driving Moment:		4.69397e+06 lb-ft
Total Slice Area:		495.404 ft ²
Surface Horizontal Width:		126.694 ft
Surface Average Height:		3.91026 ft

Method: janbu corrected

	FS	1.850490
Center:		1341.704, 989.463
Radius:		360.511
Left Slip Surface Endpoint:		994.294, 893.157
Right Slip Surface Endpoint:		1571.372, 711.576
Resisting Horizontal Force:		2.253e+06 lb
Driving Horizontal Force:		1.21752e+06 lb
Total Slice Area:		72867.6 ft ²
Surface Horizontal Width:		577.078 ft
Surface Average Height:		126.27 ft

Method: spencer

	FS	1.940670
Center:		1357.951, 1053.414
Radius:		407.705
Left Slip Surface Endpoint:		983.533, 892.081
Right Slip Surface Endpoint:		1577.156, 709.652
Resisting Moment:		9.52647e+08 lb-ft
Driving Moment:		4.90886e+08 lb-ft
Resisting Horizontal Force:		2.04125e+06 lb
Driving Horizontal Force:		1.05183e+06 lb
Total Slice Area:		66038.9 ft ²
Surface Horizontal Width:		593.624 ft
Surface Average Height:		111.247 ft

Method: gle/morgenstern-price

FS	1.933010
Center:	1357.951, 1074.730
Radius:	420.574
Left Slip Surface Endpoint:	979.311, 891.659
Right Slip Surface Endpoint:	1570.553, 711.849
Resisting Moment:	9.44916e+08 lb-ft
Driving Moment:	4.88831e+08 lb-ft
Resisting Horizontal Force:	1.97787e+06 lb
Driving Horizontal Force:	1.02321e+06 lb
Total Slice Area:	62422 ft ²
Surface Horizontal Width:	591.241 ft
Surface Average Height:	105.578 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	3990
Number of Invalid Surfaces:	817

Error Codes

Error Code -103 reported for 11 surfaces
 Error Code -110 reported for 368 surfaces
 Error Code -112 reported for 172 surfaces
 Error Code -114 reported for 266 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4005
Number of Invalid Surfaces:	802

Error Codes

Error Code -103 reported for 11 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 368 surfaces
 Error Code -112 reported for 155 surfaces
 Error Code -114 reported for 266 surfaces

Method: spencer

Number of Valid Surfaces:	3977
Number of Invalid Surfaces:	830

Error Codes

Error Code -103 reported for 11 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 368 surfaces
 Error Code -111 reported for 8 surfaces
 Error Code -112 reported for 175 surfaces
 Error Code -114 reported for 266 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 3982

Number of Invalid Surfaces: 825

Error Codes

Error Code -103 reported for 11 surfaces

Error Code -108 reported for 2 surfaces

Error Code -110 reported for 368 surfaces

Error Code -111 reported for 2 surfaces

Error Code -112 reported for 176 surfaces

Error Code -114 reported for 266 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-114 = Surface with Reverse Curvature.

Entity Information

◆ Circ - thin layers

Shared Entities

Type	Coordinates (x,y)
	1688.75, 745.08
	1685.18, 744.943
	1685.18, 744.943
	1685.16, 744.936
	1685.16, 744.935
	1685.15, 744.933
	1685.14, 744.929
	1685.08, 744.911
	1664.79, 738.147
	1578.23, 709.294
	1032, 891
	1026.16, 893
	1025.72, 893.15
	1018.58, 895.585
	1003.75, 894.103
	1002.72, 894
	1002.59, 893.986
	983.533, 892.081
	982.719, 892

963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229

270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014

External Boundary

2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335

1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
1960.65, 739.995
1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
1940.44, 740
1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001
1923.26, 740
1923.21, 740
1920.59, 740.087
1917.66, 740.181
1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351

1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539
1705.77, 744.739
1703.67, 744.781
1695.56, 744.943

Material Boundary

371.139, 747.994
371.563, 747.853
384.979, 743.38
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684
698.374, 684.674
703.436, 684.721
840.146, 686
1002.59, 687.5
1031.85, 687.778
1056.47, 688
1070.79, 688.133
1076.9, 688.19
1113.49, 688.53
1198.73, 689.322
1487.03, 692
1546.99, 692.557
1550.33, 693.668
1688.75, 739.809
1696.74, 742.47
1703.25, 744.639
1703.64, 744.772
1703.65, 744.774
1703.66, 744.776
1703.67, 744.781

Material Boundary

371.563, 747.853
386.035, 748.141
390.055, 746.801
398.858, 743.867
399.741, 743.572
406.921, 741.178
409.057, 740.466
417.336, 737.706
429.767, 733.562
437.517, 730.978
447.048, 727.801
513.014, 705.81
558.684, 690.525
562.742, 689.207
564.035, 689.194
567.392, 689.194
591.838, 689.21
626.364, 689.21
695.699, 689.663
697.255, 689.673
736.378, 689.929
748.11, 690.039
840.147, 690.9
877.73, 691.247
1002.59, 692.4
1030.76, 692.667
1031.85, 692.678
1033.44, 692.692
1033.47, 692.692
1056.47, 692.9
1104.66, 693.348
1147.07, 693.742
1212.44, 694.349
1487.04, 696.9
1532.11, 697.319
1546.04, 697.45
1546.34, 697.453
1550.38, 698.797
1653.06, 733.024
1688.78, 744.929
1695.55, 744.793
1703.25, 744.639

	369.041, 747.954
	369.564, 747.963
	371.196, 747.996
	378.927, 748.15
	379.447, 748.16
	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625
	420.368, 737.91
	432.54, 733.852
	440.132, 731.322
	447.096, 729
	503.929, 710.053
	539.503, 697.951
	558.611, 691.762
	558.865, 691.679
	562.929, 690.36
	564.036, 690.348
	567.84, 690.349
	591.835, 690.364
Material Boundary	623.213, 690.364
	626.363, 690.364
	694.207, 690.807
	696.794, 690.824
	748.23, 691.161
	758.969, 691.241
	840.146, 692
	840.75, 692.006
	878.605, 692.355
	1002.59, 693.5
	1031.66, 693.776
	1031.69, 693.776
	1031.85, 693.778
	1056.47, 694
	1104.1, 694.443
	1146.52, 694.837
	1211.99, 695.445
	1271.75, 696
	1271.77, 696
	1289.59, 696.166
	1484.09, 698.02
	1487.03, 698.047
	1488.81, 698.064
	1546.16, 698.605
	1550.33, 699.993
	1555.18, 701.611
	1578.23, 709.294

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08


	378.927, 748.15
	386.841, 750.704
	390.794, 751.693
	391.025, 751.751
	391.998, 751.994
	392.991, 752.242
	394.005, 752.495
	395.042, 752.754
	396.101, 753.019
	397.183, 753.289
	398.289, 753.566
	399.521, 753.875
	412.966, 757.235
	447.787, 765.936
	564.727, 795.159
	627.055, 810.743
	685.799, 825.43
	713.413, 832.333
	737.4, 838.33
	760.264, 844.046
	782.078, 849.5
Material Boundary	802.913, 854.709
	822.833, 859.689
	840.838, 864.191
	841.905, 864.458
	860.28, 869.051
	877.824, 873.437
	912.579, 882.124
	912.983, 882.164
	913.393, 882.205
	922.986, 883.164
	923.806, 883.246
	942.991, 885.164
	943.81, 885.246
	962.997, 887.164
	963.813, 887.246
	983.002, 889.164
	983.816, 889.246
	1002.87, 891.15
	1003.01, 891.164
	1004.04, 891.267
	1022.87, 893.15
	1024.07, 893.15
	1025.72, 893.15

Material Boundary

379.447, 748.16
386.882, 750.56
390.83, 751.547
391.061, 751.605
392.034, 751.848
393.027, 752.096
394.042, 752.35
395.078, 752.609
396.137, 752.873
397.22, 753.144
398.326, 753.42
399.558, 753.729
413.002, 757.089
447.823, 765.791
564.764, 795.013
627.091, 810.598
685.835, 825.284
713.449, 832.187
737.436, 838.184
760.301, 843.901
782.115, 849.354
802.949, 854.563
822.869, 859.544
840.874, 864.045
841.942, 864.312
860.317, 868.905
877.86, 873.291
912.605, 881.976
912.998, 882.015
913.408, 882.056
923, 883.015
923.821, 883.097
943.006, 885.015
943.825, 885.097
963.012, 887.015
963.828, 887.096
983.017, 889.015
983.831, 889.096
1002.88, 891.001
1003.02, 891.015
1004.05, 891.118
1022.87, 893
1024.07, 893
1026.16, 893

	386.057, 748.292
	387.433, 748.636
	391.315, 749.607
	391.546, 749.665
	392.519, 749.908
	393.512, 750.156
	394.527, 750.409
	395.563, 750.668
	396.622, 750.933
	397.705, 751.204
	398.812, 751.48
	400.043, 751.789
	413.487, 755.149
	448.308, 763.851
	565.249, 793.073
	627.576, 808.658
	686.32, 823.344
	713.934, 830.247
	737.922, 836.244
	760.786, 841.96
	782.6, 847.414
Material Boundary	803.434, 852.623
	823.354, 857.603
	841.359, 862.105
	842.427, 862.372
	860.802, 866.965
	878.345, 871.351
	912.948, 880
	913.197, 880.025
	913.606, 880.066
	923.199, 881.025
	924.02, 881.107
	943.205, 883.025
	944.023, 883.107
	963.211, 885.025
	964.027, 885.106
	983.216, 887.025
	984.03, 887.106
	1003.08, 889.011
	1003.22, 889.025
	1004.25, 889.128
	1022.97, 891
	1024.07, 891
	1032, 891
Material Boundary	371.139, 747.994
	371.196, 747.996

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:  Wadsworth Till
	178.514, 731.719	
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	

198.554, 738.399
198.861, 738.501
199.339, 738.661
200.776, 739.14
200.997, 739.213
201.651, 739.431
201.708, 739.45
202.375, 739.673
202.96, 739.868
203.066, 739.903
203.951, 740.198
204.163, 740.269
205.695, 740.779
206.113, 740.918
206.403, 741.015
206.704, 741.116
206.992, 741.211
207.162, 741.268
207.356, 741.333
213.598, 743.414
219.127, 745.257
223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203

	385.911, 743.07	
	390.102, 741.672	
	392.178, 740.98	
	393.625, 740.498	
	395.889, 739.743	
	404.687, 736.81	
	418.13, 732.329	
	426.507, 729.536	
	447.096, 722.672	
	507.505, 702.534	
	563.515, 683.862	
	563.605, 683.831	
	563.976, 683.705	
	564.036, 683.685	
	564.179, 683.637	
	564.311, 683.592	
	564.361, 683.575	
	564.491, 683.575	
	564.857, 683.576	
	586.189, 683.61	
	626.363, 684	
	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.33, 740.212	
	1764.53, 739.813	
	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
Water Table	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	

1773.36, 737.944
1779.28, 737.93
1787.81, 737.786
1792.3, 737.722
1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383

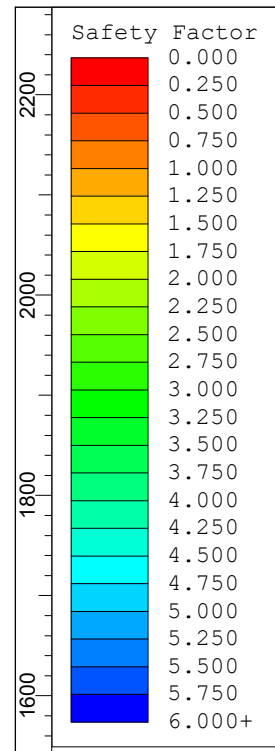
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2048.08, 751.704
2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
2054.59, 753.317
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2070.07, 757.147
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2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493

	2243.14, 734.24 2246.4, 734.069 2300, 733.229	
Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1555.88, 701.845 1556.2, 700.896 1569.82, 705.436 1578.49, 708.327 1578.54, 708.185	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

SLOPE STABILITY
SECTION A-A' - HORIZONTAL EXPANSION **(NORTH SLOPE)**

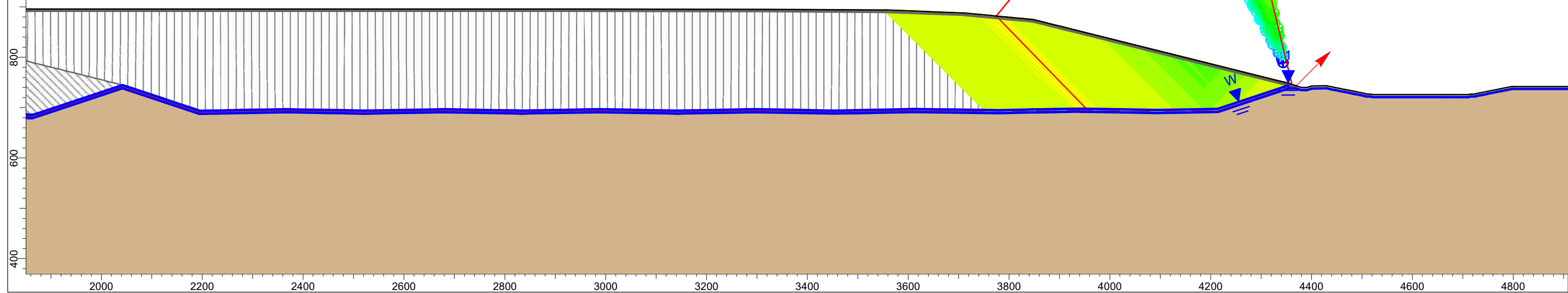
GLOBAL LINER STABILITY ANALYSIS

**COMPLETE BUILD-OUT / FINAL LANDFORM
BLOCK ANALYSIS OF LINER SYSTEM
(TRANSLATIONAL SLOPE FAILURE)**



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Short Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Bishop simplified
 Factor of Safety: 1.738
 Axis Location: 4203.418, 1400.588
 Left Slip Surface Endpoint: 3774.393, 881.957
 Right Slip Surface Endpoint: 4360.907, 746.190

1.738

Slide Analysis Information

A-A_final_north_ST

Project Summary

File Name:	A-A_final_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:12m:03.634s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1

Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1

Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.738030
Axis Location:	4203.418, 1400.588
Left Slip Surface Endpoint:	3774.393, 881.957
Right Slip Surface Endpoint:	4360.907, 746.190
Resisting Moment:	1.08098e+09 lb-ft
Driving Moment:	6.2196e+08 lb-ft
Total Slice Area:	52042.4 ft2
Surface Horizontal Width:	586.514 ft
Surface Average Height:	88.7316 ft

Method: janbu corrected

FS	1.760990
Axis Location:	4203.418, 1400.588
Left Slip Surface Endpoint:	3774.393, 881.957
Right Slip Surface Endpoint:	4360.907, 746.190
Resisting Horizontal Force:	1.43568e+06 lb
Driving Horizontal Force:	815273 lb
Total Slice Area:	52042.4 ft2
Surface Horizontal Width:	586.514 ft
Surface Average Height:	88.7316 ft

Method: spencer

FS	1.821320
Axis Location:	4186.238, 1446.790
Left Slip Surface Endpoint:	3730.756, 886.112
Right Slip Surface Endpoint:	4361.491, 745.997
Resisting Moment:	1.24661e+09 lb-ft
Driving Moment:	6.84454e+08 lb-ft
Resisting Horizontal Force:	1.48704e+06 lb
Driving Horizontal Force:	816461 lb
Total Slice Area:	56177.1 ft ²
Surface Horizontal Width:	630.735 ft
Surface Average Height:	89.0661 ft

Method: gle/morgenstern-price

FS	1.819090
Axis Location:	4190.916, 1436.686
Left Slip Surface Endpoint:	3740.937, 885.141
Right Slip Surface Endpoint:	4362.164, 745.775
Resisting Moment:	1.24034e+09 lb-ft
Driving Moment:	6.81848e+08 lb-ft
Resisting Horizontal Force:	1.48512e+06 lb
Driving Horizontal Force:	816411 lb
Total Slice Area:	57221 ft ²
Surface Horizontal Width:	621.227 ft
Surface Average Height:	92.1097 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
3774.39	881.957
3954.26	696.749
3963.45	696.44
4024.47	695.397
4071.54	694.593
4094.56	694.261
4151.61	695.262
4172.87	695.634
4213.72	696.502
4287.96	721.074
4346.98	740.737
4357.25	744.154
4360.91	746.19

Method: janbu corrected

X	Y
3774.39	881.957
3954.26	696.749
3963.45	696.44
4024.47	695.397
4071.54	694.593
4094.56	694.261
4151.61	695.262
4172.87	695.634
4213.72	696.502
4287.96	721.074
4346.98	740.737
4357.25	744.154
4360.91	746.19

Method: spencer

X	Y
3730.76	886.112
3949.28	696.833
3963.05	696.446
4023.52	695.413
4071.81	694.588
4093.12	694.236
4151.17	695.254
4172.66	695.631
4213.72	696.502
4287.94	721.069
4346.98	740.734
4357.26	744.158
4361.49	745.997

Method: gle/morgenstern-price

X	Y
3740.94	885.141
3856.43	765.938
3938.02	697.024
3961.75	696.469
3964.7	696.418
4024.69	695.394
4068.42	694.646
4092.93	694.233
4151.64	695.262
4172.75	695.633
4213.72	696.502
4286.68	720.648
4346.58	740.611
4347.5	740.924
4362.16	745.775

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4753
 Number of Invalid Surfaces: 251

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 6 surfaces
 Error Code -110 reported for 242 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4755
 Number of Invalid Surfaces: 249

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 4 surfaces
 Error Code -110 reported for 242 surfaces

Method: spencer

Number of Valid Surfaces: 4411
 Number of Invalid Surfaces: 593

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 44 surfaces
 Error Code -110 reported for 242 surfaces
 Error Code -111 reported for 304 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4118
 Number of Invalid Surfaces: 886

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 54 surfaces
 Error Code -110 reported for 242 surfaces
 Error Code -111 reported for 587 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	4801.25, 742.054
	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4486.94, 732
	4481.94, 733
	4481.87, 733
	4479.43, 733.489
	4476.94, 734
	4476.94, 734
	4473.66, 734.657
	4466.94, 736
	4466.94, 736
	4466.94, 736
	4464.31, 736.526
	4461.97, 736.994
	4456.92, 738
	4447.57, 739.873
	4442.27, 740.935
	4431.95, 743
	4429.16, 743.558
	4410.47, 743.103
	4399.15, 742.903
	4395.49, 741.691
	4390.15, 739.897
	4386.69, 739.881
	4380.14, 739.897
	4377.03, 740.918
	4371.14, 742.896
	4365, 744.839
	4354.12, 748.427
	4347.22, 750.154
	4333.51, 753.581
	4307.9, 760
	4147.28, 800.154
	4034.9, 828.248
	3943.28, 851.154
	3927.9, 855
	3907.85, 860.012

3887.89, 865
3862.48, 871.353
3847.89, 875
3838.52, 875.876
3826.49, 877
3817.3, 877.874
3805.47, 879
3795.83, 879.918
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3774.39, 881.957
3763.43, 883
3753.28, 883.966
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3742.72, 884.971
3742.42, 885
3736.08, 885.603
3731.91, 886
3712.33, 887.896
3711.25, 888
3660.7, 890
3608.3, 892
3555.9, 894
3501.92, 894.208
3324.04, 895.343
3093.34, 895.755
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2486.56, 896
2475.13, 896
2234.57, 896
2195.59, 896
2181.65, 896
2141.81, 896
2127.47, 896
1858.87, 896
1616.33, 896
1495.01, 896
1458.28, 896
1456.86, 896.353
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1454.84, 896.853
1447.37, 898.703
1446.76, 898.853
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1438.69, 900.853
1431.08, 902.74
1430.62, 902.853
1429.15, 903.217
1429.15, 903.218
1422.55, 904.853
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1414.48, 906.853
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1390.3, 912.844
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1386.77, 913.717
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1340.81, 917.854
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1219.79, 925
1211.96, 924.561
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1194.12, 922.975
1192.25, 922.788
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External Boundary

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998.336, 886.082
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Material Boundary

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	425.234, 740.088 425.659, 739.946 439.628, 735.289 445.572, 733.308 448.135, 732.454 451.572, 731.308 452.741, 730.918 455.017, 730.16 457.579, 729.307 460.127, 728.458 464.508, 727 464.51, 727 468.673, 725.614 475.6, 723.307 478.136, 722.463 481.611, 721.307 484.14, 720.465 487.618, 719.307 491.807, 717.914 493.629, 717.307 497.265, 716.097 505.64, 713.307 507.449, 712.705 511.648, 711.307 513.452, 710.707 517.656, 709.307 519.455, 708.708 523.669, 707.306 527.287, 706.108 535.86, 703.305 537.89, 702.622 541.663, 701.318 543.799, 700.607 547.704, 699.307 549.811, 698.606 553.711, 697.307	

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976.071, 689.127
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674.961, 802.298
682.111, 804.086
685.598, 804.957
690.111, 806.086
693.564, 806.949

698.111,	808.086
702.685,	809.229
706.111,	810.086
713.227,	811.865
713.234,	811.867
714.111,	812.086
714.999,	812.308
715.007,	812.31
722.112,	814.086
726.775,	815.254
730.101,	816.086
734.809,	817.263
738.101,	818.086
742.842,	819.271
746.101,	820.086
753.167,	821.852
753.175,	821.854
754.101,	822.086
755.035,	822.319
755.043,	822.321
762.101,	824.086
766.886,	825.282
770.101,	826.086
777.704,	827.987
777.707,	827.987
778.101,	828.086
778.761,	828.251
786.101,	830.086
793.117,	831.84
793.125,	831.842
794.101,	832.086
795.089,	832.333
802.101,	834.086
808.438,	835.67
810.101,	836.086
811.81,	836.513
818.101,	838.086
819.843,	838.521
826.101,	840.086
833.041,	841.821
834.101,	842.086
835.202,	842.361
842.101,	844.086
849.167,	845.852
850.101,	846.086
851.073,	846.329
858.101,	848.086
859.116,	848.34
866.101,	850.086
867.161,	850.351
874.101,	852.086
880.995,	853.809
882.101,	854.086
883.256,	854.375
884.146,	854.597
890.101,	856.086
895.862,	857.526
898.101,	858.086
904.848,	859.773


Material Boundary

906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
964.913, 874.789
970.101, 876.086
973.942, 877.046
978.101, 878.086
982.774, 879.254
986.101, 880.086
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989.058, 880.825
989.174, 880.854
990.419, 881.165
994.101, 882.086
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999.144, 883.347
1001, 883.811
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1002.54, 884.195
1002.57, 884.202
1002.93, 884.294
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1036.44, 892.671
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1043.67, 894.478
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1058.1, 898.086
1064.74, 899.745
1066.1, 900.086
1071.43, 901.419
1071.45, 901.423

1074.1, 902.086
1076.99, 902.808
1077.01, 902.812
1082.1, 904.086
1088.96, 905.8
1090.1, 906.086
1097.03, 907.819
1098.1, 908.086
1105.11, 909.839
1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
1114.25, 912.124
1131.8, 913.879
1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
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1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718
1219.87, 922.15
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1247.67, 922.15
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1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
1340.6, 915.011
1349.31, 914.473
1353.82, 914.144
1372.95, 912.621
1380.54, 912.136
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1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
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1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
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1414.79, 903.84
1421.86, 902.087
1428.46, 900.451
1428.47, 900.45
1429.93, 900.087
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	1438.54, 897.955
	1446.08, 896.087
	1446.69, 895.936
	1454.15, 894.087
	1456.17, 893.588
	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
	1616.33, 893.15
	1858.87, 893.15
	2127.47, 893.15
	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
	2234.57, 893.15
	2475.13, 893.15
	2486.56, 893.15
	2956.54, 893.15
	3093.33, 892.905
	3324.03, 892.493
	3501.9, 891.358
	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4481.94, 733
	4486.94, 732

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	
	689.108, 680.721	
	700.659, 681.002	
	711.292, 681.261	
	721.531, 681.51	
	724.458, 681.581	

731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
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992.201, 692.586
995.001, 693.111
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1053.89, 701.352

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	1069.64, 701.862	
	1072.74, 701.68	
	1077.4, 701.408	
	1085.39, 700.951	
	1093.76, 700.535	
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	1105.21, 699.039	
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	1108.62, 698.694	
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	1274.47, 687.001	
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	1380.15, 687.893	
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	1495.01, 687	
	1500.34, 687	
Water Table	1516.14, 686.278	
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	1692.76, 685	
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	1781.09, 683.397	
	1807.08, 683	
	1816.05, 683	
	1826.62, 683	
	1845.4, 682.209	
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	1862.97, 679	
	1893.13, 688.871	
	1989.22, 720.9	
	2041.9, 738.472	
	2194.18, 687.752	
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	2366.91, 690.776	
	2388.53, 690.405	
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	2475.13, 688.921	
	2503.25, 688.435	
	2521.93, 688.112	

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2620.01, 689.813
2676.93, 690.8
2698.09, 690.434
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2824.98, 688.239
2831.93, 688.119
2850.97, 688.449
2908.97, 689.455
2965.99, 690.443
2986.93, 690.807
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3064.92, 689.458
3122.98, 688.453
3141.93, 688.125
3160.82, 688.453
3218.92, 689.461
3276.19, 690.454
3296.93, 690.814
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3470.67, 688.457
3501.92, 688.999
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3660.7, 690.618
3690.85, 690.24
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3731.91, 689.696
3737.4, 689.624
3756.29, 689.378
3778.84, 689.083
3829.56, 690.08
3847.89, 690.44
3856.6, 690.594
3876.52, 690.946
3891.68, 691.214
3937.75, 692.028
3943.28, 691.934
3967.79, 691.516
4028.7, 690.475
4075.88, 689.669
4092.79, 689.38
4147.28, 690.336
4166.21, 690.668
4214.57, 691.517
4293.87, 717.935
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4375.46, 736.173
4379.34, 734.899

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4410.58, 738.104
4428.73, 738.546
4430.97, 738.097
4441.29, 736.032
4446.59, 734.97
4455.94, 733.097
4460.99, 732.09
4463.46, 731.597
4465.14, 731.261
4465.96, 731.097
4472.68, 729.754
4475.95, 729.1
4478.43, 728.588
4481.38, 728
4481.45, 728
4483.36, 727.618
4485.96, 727.097
4490.7, 726.149
4495.96, 725.097
4497.12, 724.865
4498.46, 724.597
4499.81, 724.327
4500.96, 724.097
4503.69, 723.551
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4506.46, 722.809
4508.48, 722
4513.9, 722
4516.95, 722
4519.28, 721.534
4521.95, 721
4656.84, 721
4685.73, 721
4711.04, 721
4713.3, 721.452
4716.04, 722
4718.38, 722
4724.51, 722
4726.29, 722.711
4727.48, 723.187
4728.79, 723.45
4732.03, 724.097
4732.91, 724.274
4734.53, 724.597
4735.41, 724.773
4737.03, 725.097
4740.55, 725.801
4747.03, 727.097
4748.79, 727.448
4752.03, 728.097
4753.78, 728.448
4757.03, 729.097
4760.53, 729.797
4763.5, 730.39
4767.03, 731.097

	<p>4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703 560.534, 701.359 567.615, 699 570.071, 698.182 573.623, 697 577.692, 695.645 579.631, 695 583.706, 693.643 583.706, 693.643 585.639, 693</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

594.551, 690.033
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599.571, 688.362
603.662, 687
608.129, 685.513
609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
700.514, 687
768.454, 688.654
835.534, 687
874.622, 687
885.02, 687
885.837, 687
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914.512, 685.056
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918.713, 684.952
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933.943, 685.923
944.266, 687
947.065, 687.849
951.886, 689
965.137, 692.608
966.644, 693
967.06, 693.116
971.617, 694.286
974.456, 694.923
974.842, 695
982.543, 696.691
984.141, 697
990.827, 698.433
993.853, 699
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1014.48, 703.184
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1054.51, 707.425
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1067.29, 707.858
1069.81, 707.862
1084.55, 707
1094.3, 706.515
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1109.31, 704.655
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1116.8, 703.68
1119.86, 703.126
1120.45, 703
1128.59, 701.46
1130.69, 701
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1140.35, 699
1147.15, 697.628

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	1152.34, 696.587	
	1158.74, 695	
	1164.46, 693.516	
	1180.87, 693.158	
	1199.19, 693	
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	1692.86, 691	
	1719.06, 690.137	
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	1816.05, 689	
	1826.74, 689	
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	1846.65, 688.162	
	1850.41, 686.948	
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	1856.55, 685	
	1858.87, 685	
	1861.85, 685	
	1868.81, 687.228	
	1874.47, 689	
	1874.47, 689	
	1875.79, 689.44	
	1880.49, 691	
	1880.54, 691	
	1881.86, 691.439	
	1886.54, 693	
	1891.23, 694.563	
	1892.54, 695	
	1893.8, 695.436	
	1893.84, 695.436	
	1898.54, 697	
Piezoline	1899.79, 697.434	
	1899.84, 697.434	
	1904.54, 699	
	1905.79, 699.433	
	1905.84, 699.433	
	1910.54, 701	
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	1911.83, 701.431	
	1916.54, 703	
	1917.78, 703.43	
	1917.83, 703.429	
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1947.76, 713.422
1947.81, 713.422
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1953.76, 715.42
1953.8, 715.42
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1959.76, 717.419
1959.8, 717.419
1964.54, 719
1965.76, 719.417
1965.79, 719.417
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1971.79, 721.416
1976.54, 723
1977.76, 723.414
1977.79, 723.414
1982.54, 725
1987.31, 726.589
1988.52, 727
1988.54, 727
1993.32, 728.59
1994.52, 729
1994.54, 729
1995.75, 729.41
1995.77, 729.41
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2001.74, 731.408
2001.77, 731.408
2006.55, 733
2007.74, 733.406
2007.77, 733.406
2012.55, 735
2013.73, 735.405
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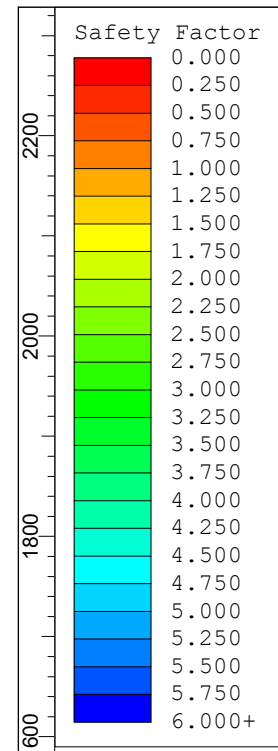
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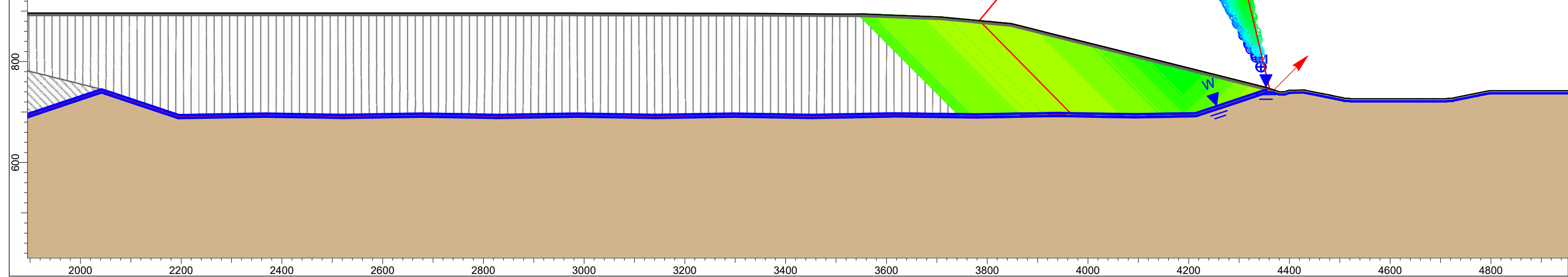
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	4368.83, 742.903	

Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Short Term / Static Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8

Method: Bishop simplified
Factor of Safety: 2.127
Axis Location: 4207.149, 1389.709
Left Slip Surface Endpoint: 3784.451, 881.000
Right Slip Surface Endpoint: 4360.497, 746.325



Slide Analysis Information

A-A_final_north_ST

Project Summary

File Name:	A-A_final_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:09m:07.240s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading







Advanced seismic analysis:	No
Staged pseudostatic analysis:	No


Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0
Existing Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465

Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.127390
Axis Location:	4207.149, 1389.709
Left Slip Surface Endpoint:	3784.451, 881.000
Right Slip Surface Endpoint:	4360.497, 746.325
Resisting Moment:	1.05585e+09 lb-ft
Driving Moment:	4.96314e+08 lb-ft
Total Slice Area:	50052.7 ft2
Surface Horizontal Width:	576.046 ft
Surface Average Height:	86.8901 ft

Method: janbu corrected

FS	2.161960
Axis Location:	4207.149, 1389.709
Left Slip Surface Endpoint:	3784.451, 881.000
Right Slip Surface Endpoint:	4360.497, 746.325
Resisting Horizontal Force:	1.42178e+06 lb
Driving Horizontal Force:	657634 lb
Total Slice Area:	50052.7 ft2
Surface Horizontal Width:	576.046 ft
Surface Average Height:	86.8901 ft

Method: spencer

FS	2.241000
Axis Location:	4188.360, 1437.635
Left Slip Surface Endpoint:	3738.516, 885.371
Right Slip Surface Endpoint:	4360.265, 746.402
Resisting Moment:	1.24003e+09 lb-ft
Driving Moment:	5.53339e+08 lb-ft
Resisting Horizontal Force:	1.48699e+06 lb
Driving Horizontal Force:	663542 lb
Total Slice Area:	55419.1 ft2
Surface Horizontal Width:	621.749 ft
Surface Average Height:	89.1343 ft

Method: gle/morgenstern-price

FS	2.244370
Axis Location:	4187.938, 1445.456
Left Slip Surface Endpoint:	3732.849, 885.910
Right Slip Surface Endpoint:	4362.521, 745.658
Resisting Moment:	1.29292e+09 lb-ft
Driving Moment:	5.76074e+08 lb-ft
Resisting Horizontal Force:	1.53181e+06 lb
Driving Horizontal Force:	682511 lb
Total Slice Area:	57027.2 ft ²
Surface Horizontal Width:	629.672 ft
Surface Average Height:	90.5665 ft

Global Minimum Coordinates**Method: bishop simplified**

X	Y
3784.45	881
3966.58	696.536
4024.94	695.39
4071.47	694.594
4092.81	694.23
4150.86	695.249
4178.73	695.738
4213.73	696.503
4288.71	721.484
4344.39	739.878
4355.64	743.619
4360.5	746.325

Method: janbu corrected

X	Y
3784.45	881
3966.58	696.536
4024.94	695.39
4071.47	694.594
4092.81	694.23
4150.86	695.249
4178.73	695.738
4213.73	696.503
4288.71	721.484
4344.39	739.878
4355.64	743.619
4360.5	746.325

Method: spencer

X	Y
3738.52	885.371
3950.25	696.817
3961.54	696.472
4024.6	695.395
4071.5	694.594
4092.8	694.23
4151.6	695.262
4172.27	695.625
4213.72	696.502
4288.26	721.174
4346.38	740.692
4354.72	743.301
4360.26	746.402

Method: gle/morgenstern-price

X	Y
3732.85	885.91
3876.39	752.578
3943.07	696.976
3943.32	696.895
3963.47	696.439
4024.75	695.393
4074.07	694.55
4095.38	694.275
4151.55	695.261
4172.71	695.632
4213.72	696.502
4288.21	721.16
4348.82	741.412
4351.35	742.275
4362.52	745.658

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4758
 Number of Invalid Surfaces: 246

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 5 surfaces
 Error Code -110 reported for 238 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4759

Number of Invalid Surfaces: 245

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 4 surfaces

Error Code -110 reported for 238 surfaces

Method: spencer

Number of Valid Surfaces: 4289

Number of Invalid Surfaces: 715

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 66 surfaces

Error Code -110 reported for 238 surfaces

Error Code -111 reported for 408 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4131

Number of Invalid Surfaces: 873

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 64 surfaces

Error Code -110 reported for 238 surfaces

Error Code -111 reported for 568 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
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	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355

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4685.73, 726
4656.84, 726
4522.44, 726
4520.26, 726.437
4517.44, 727
4513.9, 727
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978.101, 878.086
982.774, 879.254
986.101, 880.086
987.201, 880.361
989.058, 880.825
989.174, 880.854
990.419, 881.165
994.101, 882.086
999.027, 883.317
999.144, 883.347
1001, 883.811
1002.1, 884.086
1002.22, 884.117
1002.54, 884.195
1002.57, 884.202
1002.93, 884.294
1010.1, 886.086
1016.33, 887.642
1018.1, 888.086
1024.39, 889.659
1026.1, 890.086
1031.92, 891.541
1031.94, 891.544
1034.1, 892.086
1036.43, 892.667
1036.44, 892.671
1042.1, 894.086
1043.67, 894.478
1050.1, 896.086
1051.6, 896.461

1058.1, 898.086
1064.74, 899.745
1066.1, 900.086
1071.43, 901.419
1071.45, 901.423
1074.1, 902.086
1076.99, 902.808
1077.01, 902.812
1082.1, 904.086
1088.96, 905.8
1090.1, 906.086
1097.03, 907.819
1098.1, 908.086
1105.11, 909.839
1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
1114.25, 912.124
1131.8, 913.879
1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718
1219.87, 922.15
1223.74, 922.15
1239.2, 922.15
1244.03, 922.15
1247.67, 922.15
1263.87, 920.811
1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
1340.6, 915.011
1349.31, 914.473
1353.82, 914.144
1372.95, 912.621
1380.54, 912.136
1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
1397.65, 908.087
1397.77, 908.056
1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
1414.78, 903.841
1414.79, 903.84

1421.86,	902.087
1428.46,	900.451
1428.47,	900.45
1429.93,	900.087
1430.39,	899.974
1438.01,	898.087
1438.54,	897.955
1446.08,	896.087
1446.69,	895.936
1454.15,	894.087
1456.17,	893.588
1456.17,	893.587
1457.93,	893.15
1495.01,	893.15
1616.33,	893.15
1858.87,	893.15
2127.47,	893.15
2141.81,	893.15
2181.65,	893.15
2195.59,	893.15
2234.57,	893.15
2475.13,	893.15
2486.56,	893.15
2956.54,	893.15
3093.33,	892.905
3324.03,	892.493
3501.9,	891.358
3555.84,	891.15
3608.19,	889.152
3660.59,	887.152
3711.06,	885.155
3712.05,	885.059
3731.64,	883.163
3735.81,	882.765
3742.15,	882.163
3742.45,	882.134
3742.79,	882.102
3753.01,	881.129
3763.16,	880.163
3774.12,	879.12
3784.18,	878.163
3795.56,	877.08
3805.2,	876.163
3817.03,	875.037
3826.22,	874.163
3838.25,	873.038
3847.41,	872.183
3861.79,	868.588
3887.2,	862.235
3907.15,	857.247
3927.2,	852.235
3942.59,	848.389
4034.21,	825.483
4146.59,	797.389
4307.21,	757.235
4332.82,	750.816
4346.52,	747.389
4353.25,	745.706
4353.34,	745.686

	4361.26, 743.112
Material Boundary	4481.94, 733 4486.94, 732

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	

675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316

	1015.48, 697.262	
	1026.16, 698.595	
	1029.95, 699.071	
	1037.13, 700.174	
	1049.53, 700.993	
	1053.89, 701.352	
	1060.67, 701.77	
	1068.84, 701.86	
	1069.64, 701.862	
	1072.74, 701.68	
	1077.4, 701.408	
	1085.39, 700.951	
	1093.76, 700.535	
	1103.93, 699.206	
	1105.21, 699.039	
	1108.51, 698.705	
	1108.62, 698.694	
	1115.88, 697.749	
	1119.27, 697.117	
	1126.35, 695.778	
	1134.85, 694.035	
	1143.12, 692.32	
	1144.75, 691.993	
	1145.42, 691.857	
	1146.43, 691.649	
	1147.98, 691.316	
	1148.3, 691.248	
	1149.18, 691.068	
	1151.08, 690.719	
	1156.36, 689.409	
	1157.27, 689.184	
	1163.63, 687.532	
	1180.77, 687.159	
	1199.14, 687	
	1219.87, 686.821	
	1247.6, 686.53	
	1274.47, 687.001	
	1354.82, 688.409	
	1380.15, 687.893	
	1449.6, 687	
	1461.71, 687	
	1495.01, 687	
	1500.34, 687	
Water Table	1516.14, 686.278	
	1637.24, 685	
	1692.76, 685	
	1718.93, 684.138	
	1781.09, 683.397	
	1807.08, 683	
	1816.05, 683	
	1826.62, 683	
	1845.4, 682.209	
	1854.88, 679	
	1862.97, 679	
	1893.13, 688.871	
	1989.22, 720.9	
	2041.9, 738.472	
	2194.18, 687.752	
	2229.99, 688.38	

2366.91, 690.776
2388.53, 690.405
2447.68, 689.392
2475.13, 688.921
2503.25, 688.435
2521.93, 688.112
2580.04, 689.12
2600.94, 689.482
2620.01, 689.813
2676.93, 690.8
2698.09, 690.434
2754.87, 689.452
2812.87, 688.448
2824.98, 688.239
2831.93, 688.119
2850.97, 688.449
2908.97, 689.455
2965.99, 690.443
2986.93, 690.807
3007.8, 690.446
3064.92, 689.458
3122.98, 688.453
3141.93, 688.125
3160.82, 688.453
3218.92, 689.461
3276.19, 690.454
3296.93, 690.814
3317.6, 690.456
3374.97, 689.464
3433.13, 688.457
3451.93, 688.132
3470.67, 688.457
3501.92, 688.999
3529.71, 689.545
3555.9, 690.059
3593.11, 690.795
3608.3, 691.095
3613.84, 691.204
3660.7, 690.618
3690.85, 690.24
3711.25, 689.984
3731.91, 689.696
3737.4, 689.624
3756.29, 689.378
3778.84, 689.083
3829.56, 690.08
3847.89, 690.44
3856.6, 690.594
3876.52, 690.946
3891.68, 691.214
3937.75, 692.028
3943.28, 691.934
3967.79, 691.516
4028.7, 690.475
4075.88, 689.669
4092.79, 689.38
4147.28, 690.336
4166.21, 690.668
4214.57, 691.517

4293.87, 717.935
4347.22, 735.704
4355, 738.297
4369.59, 738.142
4375.46, 736.173
4379.34, 734.899
4386.69, 734.881
4390.98, 734.901
4397.07, 736.947
4400, 737.917
4410.58, 738.104
4428.73, 738.546
4430.97, 738.097
4441.29, 736.032
4446.59, 734.97
4455.94, 733.097
4460.99, 732.09
4463.46, 731.597
4465.14, 731.261
4465.96, 731.097
4472.68, 729.754
4475.95, 729.1
4478.43, 728.588
4481.38, 728
4481.45, 728
4483.36, 727.618
4485.96, 727.097
4490.7, 726.149
4495.96, 725.097
4497.12, 724.865
4498.46, 724.597
4499.81, 724.327
4500.96, 724.097
4503.69, 723.551
4505.51, 723.187
4506.46, 722.809
4508.48, 722
4513.9, 722
4516.95, 722
4519.28, 721.534
4521.95, 721
4656.84, 721
4685.73, 721
4711.04, 721
4713.3, 721.452
4716.04, 722
4718.38, 722
4724.51, 722
4726.29, 722.711
4727.48, 723.187
4728.79, 723.45
4732.03, 724.097
4732.91, 724.274
4734.53, 724.597
4735.41, 724.773
4737.03, 725.097
4740.55, 725.801
4747.03, 727.097
4748.79, 727.448

	<p>4752.03, 728.097 4753.78, 728.448 4757.03, 729.097 4760.53, 729.797 4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703 560.534, 701.359 567.615, 699 570.071, 698.182</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

573.623, 697
577.692, 695.645
579.631, 695
583.706, 693.643
583.706, 693.643
585.639, 693
594.551, 690.033
597.655, 689
599.571, 688.362
603.662, 687
608.129, 685.513
609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
700.514, 687
768.454, 688.654
835.534, 687
874.622, 687
885.02, 687
885.837, 687
914.501, 685.056
914.512, 685.056
916.783, 685
918.713, 684.952
920.67, 685
922.861, 685.058
931.35, 685.433
933.943, 685.923
944.266, 687
947.065, 687.849
951.886, 689
965.137, 692.608
966.644, 693
967.06, 693.116
971.617, 694.286
974.456, 694.923
974.842, 695
982.543, 696.691
984.141, 697
990.827, 698.433
993.853, 699
1011.96, 702.672
1013.62, 703
1014.48, 703.184
1029.03, 705
1036.48, 706.144
1049.43, 707
1054.51, 707.425
1060.93, 707.795
1067.29, 707.858
1069.81, 707.862
1084.55, 707
1094.3, 706.515
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1114.62, 703.964
1116.8, 703.68
1119.86, 703.126

	1120.45, 703	
	1128.59, 701.46	
	1130.69, 701	
	1136.6, 699.804	
	1140.35, 699	
	1147.15, 697.628	
	1150.1, 697	
	1152.34, 696.587	
	1158.74, 695	
	1164.46, 693.516	
	1180.87, 693.158	
	1199.19, 693	
	1219.93, 692.821	
	1247.58, 692.53	
	1274.37, 693	
	1354.83, 694.411	
	1380.25, 693.892	
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	1461.71, 693	
	1495.01, 693	
	1500.48, 693	
	1516.31, 692.277	
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	1692.86, 691	
	1719.06, 690.137	
	1781.17, 689.396	
	1807.13, 689	
	1816.05, 689	
	1826.74, 689	
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	1846.65, 688.162	
	1850.41, 686.948	
	1855.35, 685.225	
	1856.02, 685	
	1856.55, 685	
	1858.87, 685	
	1861.85, 685	
	1868.81, 687.228	
	1874.47, 689	
	1874.47, 689	
	1875.79, 689.44	
	1880.49, 691	
	1880.54, 691	
	1881.86, 691.439	
	1886.54, 693	
	1891.23, 694.563	
	1892.54, 695	
	1893.8, 695.436	
	1893.84, 695.436	
	1898.54, 697	
	1899.79, 697.434	
	1899.84, 697.434	
	1904.54, 699	
	1905.79, 699.433	
	1905.84, 699.433	
	1910.54, 701	
	1911.78, 701.431	
	1911.83, 701.431	
	1916.54, 703	
Piezoline		

1917.78, 703.43
1917.83, 703.429
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1923.82, 705.428
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1935.82, 709.425
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1941.76, 711.423
1941.81, 711.423
1946.54, 713
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1947.81, 713.422
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1953.8, 715.42
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1965.76, 719.417
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1970.54, 721
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1971.79, 721.416
1976.54, 723
1977.76, 723.414
1977.79, 723.414
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1988.52, 727
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1994.54, 729
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1995.77, 729.41
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2006.55, 733
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2013.76, 735.405
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2019.73, 737.403
2019.76, 737.403
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2025.75, 739.402
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2031.73, 741.4
2036.53, 743

2041.91, 744.8
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2075.51, 733.686
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2176.55, 700
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2195.27, 693.772
2195.49, 693.776
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2208.3, 694
2235.83, 694.482
2276.51, 695.194
2295.76, 695.531
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2366.91, 696.776
2387.58, 696.422
2475.13, 694.921
2521.93, 694.112
2580.01, 695.119
2599.02, 695.449
2655.66, 696.431
2676.93, 696.8
2698.12, 696.433
2756.78, 695.419
2775.37, 695.097
2824.98, 694.239
2831.93, 694.119
2870.81, 694.793
2896.34, 695.236
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2965.96, 696.443
2986.93, 696.807
3005.86, 696.479
3026.7, 696.119
3084.07, 695.126
3141.93, 694.125
3160.86, 694.454
3218.92, 695.461
3276.16, 696.453
3296.93, 696.814
3315.7, 696.489
3373.9, 695.482
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3451.93, 694.132
3472.59, 694.49
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3555.9, 696.059
3592.57, 696.784
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3613.84, 697.204
3660.7, 696.618
3690.91, 696.239
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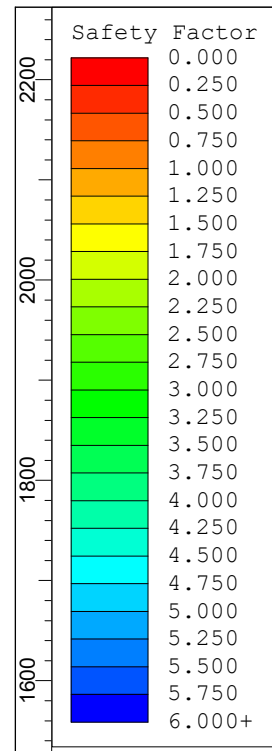
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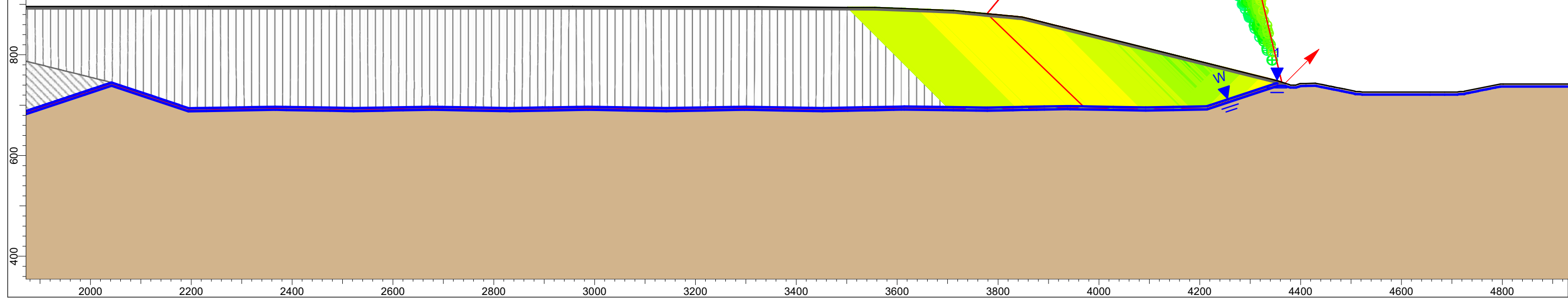
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4354.13, 743.197
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4368.83, 742.903



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Final Buildout / Global Liner Stability
NonCircular - Liner Block Search
Long Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
Existing Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3



Method: Bishop simplified
 Factor of Safety: 1.628
 Axis Location: 4206.235, 1398.148
 Left Slip Surface Endpoint: 3778.018, 881.612
 Right Slip Surface Endpoint: 4362.534, 745.654

Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:08m:56.753s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1

Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1

Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.627820
Axis Location:	4206.235, 1398.148	
Left Slip Surface Endpoint:	3778.018, 881.612	
Right Slip Surface Endpoint:	4362.534, 745.654	
Resisting Moment:	9.96668e+08 lb-ft	
Driving Moment:	6.12271e+08 lb-ft	
Total Slice Area:	50249.9 ft ²	
Surface Horizontal Width:	584.515 ft	
Surface Average Height:	85.9685 ft	

Method: janbu corrected

	FS	1.643350
Axis Location:	4209.187, 1390.534	
Left Slip Surface Endpoint:	3785.293, 880.920	
Right Slip Surface Endpoint:	4362.542, 745.651	
Resisting Horizontal Force:	1.31816e+06 lb	
Driving Horizontal Force:	802120 lb	
Total Slice Area:	49439.4 ft ²	
Surface Horizontal Width:	577.249 ft	
Surface Average Height:	85.6466 ft	

Method: spencer

FS	1.710850
Axis Location:	4188.962, 1441.780
Left Slip Surface Endpoint:	3736.084, 885.603
Right Slip Surface Endpoint:	4362.177, 745.771
Resisting Moment:	1.1447e+09 lb-ft
Driving Moment:	6.69084e+08 lb-ft
Resisting Horizontal Force:	1.39117e+06 lb
Driving Horizontal Force:	813141 lb
Total Slice Area:	53911.6 ft ²
Surface Horizontal Width:	626.093 ft
Surface Average Height:	86.1079 ft

Method: gle/morgenstern-price

FS	1.713160
Axis Location:	4191.987, 1434.678
Left Slip Surface Endpoint:	3743.057, 884.939
Right Slip Surface Endpoint:	4362.421, 745.691
Resisting Moment:	1.15033e+09 lb-ft
Driving Moment:	6.71465e+08 lb-ft
Resisting Horizontal Force:	1.40056e+06 lb
Driving Horizontal Force:	817531 lb
Total Slice Area:	55227.6 ft ²
Surface Horizontal Width:	619.363 ft
Surface Average Height:	89.1683 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
3778.02	881.612
3970.55	696.469
4023.62	695.412
4072.53	694.576
4096.96	694.304
4151.36	695.257
4172.8	695.634
4213.72	696.502
4289.23	721.591
4345.65	740.293
4355.55	743.59
4362.53	745.654

Method: janbu corrected

X	Y
3785.29	880.92
3972.85	696.43
4023.62	695.412
4072.53	694.576
4094.16	694.255
4151.36	695.257
4172.85	695.635
4213.72	696.502
4286.81	720.692
4345.64	740.29
4355.55	743.589
4362.54	745.651

Method: spencer

X	Y
3736.08	885.603
3935.76	723.062
3971.74	696.449
4014.42	695.569
4077.97	694.483
4095.67	694.281
4151.49	695.26
4174.31	695.66
4213.75	696.503
4287.99	721.086
4347.72	740.984
4352.24	742.492
4362.18	745.771

Method: gle/morgenstern-price

X	Y
3743.06	884.939
3867.53	766.943
3959.55	696.657
4024.4	695.399
4071.5	694.594
4094	694.252
4145.62	695.157
4172.74	695.633
4213.72	696.502
4288.12	721.127
4348.33	741.194
4353.45	742.905
4362.42	745.691

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4769

Number of Invalid Surfaces: 235

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 3 surfaces

Error Code -110 reported for 229 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4766

Number of Invalid Surfaces: 238

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 6 surfaces

Error Code -110 reported for 229 surfaces

Method: spencer

Number of Valid Surfaces: 4381

Number of Invalid Surfaces: 623

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 49 surfaces

Error Code -110 reported for 229 surfaces

Error Code -111 reported for 342 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4131

Number of Invalid Surfaces: 873

Error Codes

Error Code -106 reported for 3 surfaces

Error Code -108 reported for 44 surfaces

Error Code -110 reported for 229 surfaces

Error Code -111 reported for 597 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	4801.13, 742.054
	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4486.94, 732
	4481.94, 733
	4481.87, 733
	4476.94, 734
	4476.94, 734
	4471.48, 735.093
	4466.94, 736
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	4466.94, 736
	4464.31, 736.526
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	4456.94, 738
	4451.39, 739.111
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	4442.27, 740.935
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	4429.16, 743.558
	4410.47, 743.103
	4399.15, 742.903
	4395.49, 741.691
	4390.15, 739.897
	4386.69, 739.881
	4380.14, 739.897
	4377.03, 740.918
	4371.14, 742.896
	4365, 744.839
	4354.12, 748.427
	4347.22, 750.154
	4333.51, 753.581
	4307.9, 760
	4147.28, 800.154
	4034.9, 828.248
	3943.28, 851.154
	3927.9, 855
	3907.85, 860.012
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	3862.48, 871.353
	3847.89, 875

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1313.97, 917.047
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1340.6, 915.011
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1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
1389.57, 910.087
1389.61, 910.077
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1397.77, 908.056
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1414.79, 903.84
1421.86, 902.087
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1429.93, 900.087
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	1438.01, 898.087
	1438.54, 897.955
	1446.08, 896.087
	1446.69, 895.936
	1454.15, 894.087
	1456.17, 893.588
	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
	1616.33, 893.15
	1858.87, 893.15
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	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
	2234.57, 893.15
	2475.13, 893.15
	2486.56, 893.15
	2956.54, 893.15
	3093.33, 892.905
	3324.03, 892.493
	3501.9, 891.358
	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4442.27, 740.935
	4456.92, 738
	4464.31, 736.526
	4466.94, 736

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	
	689.108, 680.721	
	700.659, 681.002	
	711.292, 681.261	




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724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
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839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
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953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
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Water Table

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3218.92, 689.461
3276.19, 690.454
3296.93, 690.814
3317.6, 690.456
3374.97, 689.464
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3451.93, 688.132
3470.67, 688.457
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3593.11, 690.795
3608.3, 691.095
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3660.7, 690.618
3690.85, 690.24
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4369.59, 738.142

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4428.73, 738.546
4430.97, 738.097
4441.29, 736.032
4446.59, 734.97
4455.94, 733.097
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4465.14, 731.261
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4481.45, 728
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4485.96, 727.097
4490.7, 726.149
4495.96, 725.097
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4498.46, 724.597
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4500.96, 724.097
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4505.51, 723.187
4506.46, 722.809
4508.48, 722
4513.9, 722
4516.95, 722
4519.28, 721.534
4521.95, 721
4656.84, 721
4685.73, 721
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4713.3, 721.452
4716.04, 722
4718.38, 722
4724.51, 722
4726.29, 722.711
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4734.53, 724.597
4735.41, 724.773
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4740.55, 725.801
4747.03, 727.097
4748.79, 727.448
4752.03, 728.097
4753.78, 728.448
4757.03, 729.097
4760.53, 729.797

	<p>4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703 560.534, 701.359 567.615, 699 570.071, 698.182 573.623, 697 577.692, 695.645 579.631, 695 583.706, 693.643</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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585.639, 693
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599.571, 688.362
603.662, 687
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609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
700.514, 687
768.454, 688.654
835.534, 687
874.622, 687
885.02, 687
885.837, 687
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914.512, 685.056
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933.943, 685.923
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947.065, 687.849
951.886, 689
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984.141, 697
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993.853, 699
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1119.86, 703.126
1120.45, 703
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	1899.79, 697.434	
Piezoline	1899.84, 697.434	
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	425.446, 740.017 435.797, 740.214 440.59, 740.214 447.446, 737.929 450.009, 737.075 453.446, 735.929 454.615, 735.539 456.889, 734.782 459.451, 733.929 462, 733.08 465.468, 731.925 465.47, 731.925 470.544, 730.236 477.472, 727.929 480.007, 727.085 483.482, 725.929 486.011, 725.087 489.489, 723.929 493.677, 722.536 495.5, 721.929 499.136, 720.718 507.512, 717.929 509.321, 717.327 513.52, 715.929 515.323, 715.328 519.528, 713.929 521.326, 713.33 525.536, 711.929 529.138, 710.736 537.726, 707.929 539.803, 707.23 543.567, 705.929 545.671, 705.229 549.575, 703.929 551.683, 703.227 555.583, 701.929 560.51, 700.288 567.591, 697.929 570.047, 697.111 573.599, 695.929 577.668, 694.574	

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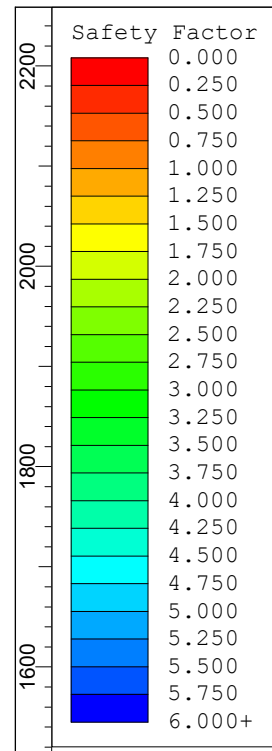
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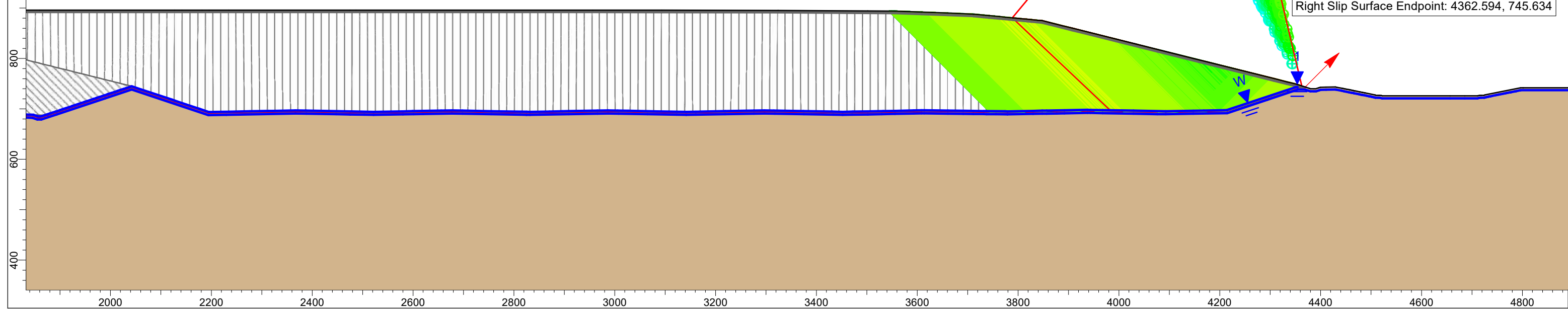
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Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Long Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
Existing Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3



Method: Bishop simplified
 Factor of Safety: 1.984
 Axis Location: 4210.531, 1387.212
 Left Slip Surface Endpoint: 3788.506, 880.614
 Right Slip Surface Endpoint: 4362.594, 745.634

Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:08m:23.24s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading







Advanced seismic analysis:	No
Staged pseudostatic analysis:	No


Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
Existing Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0

Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.984410
Axis Location:	4210.531, 1387.212	
Left Slip Surface Endpoint:	3788.506, 880.614	
Right Slip Surface Endpoint:	4362.594, 745.634	
Resisting Moment:	9.67497e+08 lb-ft	
Driving Moment:	4.8755e+08 lb-ft	
Total Slice Area:	48190.2 ft2	
Surface Horizontal Width:	574.088 ft	
Surface Average Height:	83.9422 ft	

Method: janbu corrected

	FS	2.008640
Axis Location:	4212.653, 1381.722	
Left Slip Surface Endpoint:	3793.747, 880.115	
Right Slip Surface Endpoint:	4362.595, 745.633	
Resisting Horizontal Force:	1.30624e+06 lb	
Driving Horizontal Force:	650310 lb	
Total Slice Area:	47753 ft2	
Surface Horizontal Width:	568.848 ft	
Surface Average Height:	83.9468 ft	

Method: spencer

	FS	2.092910
Axis Location:	4191.785, 1434.475	
Left Slip Surface Endpoint:	3743.057, 884.939	
Right Slip Surface Endpoint:	4362.177, 745.771	
Resisting Moment:	1.14844e+09 lb-ft	
Driving Moment:	5.48727e+08 lb-ft	
Resisting Horizontal Force:	1.40338e+06 lb	
Driving Horizontal Force:	670537 lb	
Total Slice Area:	53099.8 ft2	
Surface Horizontal Width:	619.12 ft	
Surface Average Height:	85.7665 ft	

Method: gle/morgenstern-price

FS	2.100150
Axis Location:	4197.715, 1420.379
Left Slip Surface Endpoint:	3756.846, 883.627
Right Slip Surface Endpoint:	4362.595, 745.633
Resisting Moment:	1.11824e+09 lb-ft
Driving Moment:	5.32459e+08 lb-ft
Resisting Horizontal Force:	1.38197e+06 lb
Driving Horizontal Force:	658035 lb
Total Slice Area:	53078.3 ft ²
Surface Horizontal Width:	605.749 ft
Surface Average Height:	87.6242 ft

Global Minimum Coordinates**Method: bishop simplified**

X	Y
3788.51	880.614
3983.54	696.247
4026.94	695.355
4071.54	694.593
4094.45	694.259
4151.29	695.257
4171.12	695.604
4213.68	696.49
4288.76	721.501
4346.44	740.712
4355.33	743.493
4362.59	745.634

Method: janbu corrected

X	Y
3793.75	880.115
3983.54	696.247
4023.63	695.412
4072.53	694.576
4094.09	694.264
4151.36	695.257
4172.85	695.635
4213.68	696.49
4288.76	721.501
4346.5	740.733
4355.32	743.489
4362.59	745.633

Method: spencer

X	Y
3743.06	884.939
3898.31	755.21
3977.46	696.351
4014.16	695.573
4077.92	694.484
4095.67	694.281
4151.49	695.26
4173.51	695.646
4213.72	696.502
4288	721.088
4347.71	740.979
4352.24	742.492
4362.18	745.771

Method: gle/morgenstern-price

X	Y
3756.85	883.627
3872.04	772.143
3972.44	696.437
4024.56	695.396
4074.25	694.547
4094.24	694.256
4149.75	695.229
4172.75	695.633
4213.72	696.502
4288.12	721.127
4347.48	741.05
4353.61	742.964
4362.6	745.633

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4761
 Number of Invalid Surfaces: 243

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 238 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4761
 Number of Invalid Surfaces: 243

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 238 surfaces

Method: spencer

Number of Valid Surfaces: 4271
 Number of Invalid Surfaces: 733

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 48 surfaces
 Error Code -110 reported for 238 surfaces
 Error Code -111 reported for 444 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4127
 Number of Invalid Surfaces: 877

Error Codes

Error Code -106 reported for 3 surfaces
 Error Code -108 reported for 55 surfaces
 Error Code -110 reported for 238 surfaces
 Error Code -111 reported for 581 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**Block Search - thin layers****Shared Entities**

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	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727

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1094.3, 706.515

Material Boundary

1105.9, 705
1109.31, 704.655
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1119.86, 703.126
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1128.59, 701.46
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1147.15, 697.628
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1152.34, 696.587
1158.74, 695
1164.46, 693.516
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1354.83, 694.411
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1461.71, 693
1495.01, 693
1500.48, 693
1516.31, 692.277
1637.28, 691
1692.86, 691
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1816.05, 689
1826.74, 689
1846.47, 688.169
1868.81, 687.228
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1875.79, 689.44
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1891.23, 694.563
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1893.8, 695.436
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1899.79, 697.434
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1905.79, 699.433
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1911.78, 701.431
1911.83, 701.431
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1923.77, 705.428
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1929.77, 707.426
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1935.76, 709.425
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1941.76, 711.423
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1947.76, 713.422
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1987.31, 726.589
1988.52, 727
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2018.55, 737
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Material Boundary	1457.67, 891 1495.01, 891 1616.33, 891 1858.87, 891 2127.47, 891 2141.81, 891 2181.65, 891 2195.59, 891 2234.57, 891 2475.13, 891 2486.56, 891 2956.54, 891 3093.33, 890.755 3324.02, 890.343 3501.89, 889.208 3555.79, 889 3608.11, 887.004 3660.51, 885.004 3710.91, 883.01 3711.85, 882.919 3731.43, 881.023 3735.61, 880.625 3741.94, 880.022 3742.25, 879.993 3742.58, 879.961 3752.81, 878.988 3762.96, 878.022 3773.92, 876.98 3783.98, 876.022 3795.35, 874.94 3804.99, 874.022 3816.83, 872.896 3826.02, 872.022 3838.05, 870.897 3847.04, 870.057 3861.26, 866.502 3886.68, 860.149 3906.63, 855.161 3926.68, 850.149 3942.07, 846.303 4033.69, 823.397 4146.07, 795.303 4306.68, 755.149 4332.29, 748.731 4346, 745.303 4352.73, 743.621 4354.12, 743.273
	426.87, 740.119 442.157, 743.94 449.793, 745.849 450.157, 745.94 450.483, 746.022 454.153, 746.94 458.147, 747.94 461.473, 748.773 466.139, 749.94 473.455, 751.767 474.149, 751.941 474.848, 752.114

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538.115, 767.936
538.372, 768
538.376, 768
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553.397, 771.755
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593.377, 781.753
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674.997, 802.153
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722.148, 813.94
726.812, 815.109
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753.211, 821.709
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755.079, 822.176
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777.725, 827.837
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793.153, 831.694
793.161, 831.696
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802.137, 833.94
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811.847, 836.368
818.137, 837.94
819.879, 838.376
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833.077, 841.675
834.137, 841.94
835.239, 842.216
842.137, 843.94
849.204, 845.707
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851.109, 846.183
858.137, 847.94
859.153, 848.194
866.137, 849.94
867.198, 850.205
874.137, 851.94
881.031, 853.664
882.137, 853.94
883.293, 854.229
884.182, 854.452
890.137, 855.94
895.898, 857.381
898.137, 857.94
904.884, 859.627
906.137, 859.94
912.782, 861.602
914.137, 861.94
915.557, 862.295

Material Boundary

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923.565, 864.297
930.137, 865.94
936.197, 867.455
938.137, 867.94
944.598, 869.556
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951.605, 871.307
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958.68, 873.076
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964.95, 874.643
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973.979, 876.901
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986.137, 879.94
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989.211, 880.709
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1002.97, 884.148
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	3742.77, 881.952
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	4307.17, 757.09
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	4353.3, 745.541
	4360.73, 743.124
	426.201, 740.106
	442.121, 744.086
	449.756, 745.995
	450.121, 746.086
	450.446, 746.167
	454.117, 747.086
	458.111, 748.086
	461.436, 748.919

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525.707, 764.991
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538.357, 768.15
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554.833, 772.27
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594.104, 782.086
594.853, 782.275
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618.11, 788.086
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626.101, 790.086
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634.101, 792.086
634.901, 792.286
642.101, 794.086
645.858, 795.025
650.112, 796.086
654.377, 797.152
658.111, 798.086
662.404, 799.162
666.101, 800.086
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674.112, 802.086
674.953, 802.296
674.961, 802.298
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685.598, 804.957
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693.564, 806.949

698.111, 808.086
702.685, 809.229
706.111, 810.086
713.227, 811.865
713.234, 811.867
714.111, 812.086
714.999, 812.308
715.007, 812.31
722.112, 814.086
726.775, 815.254
730.101, 816.086
734.809, 817.263
738.101, 818.086
742.842, 819.271
746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
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778.761, 828.251
786.101, 830.086
793.117, 831.84
793.125, 831.842
794.101, 832.086
795.089, 832.333
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818.101, 838.086
819.843, 838.521
826.101, 840.086
833.041, 841.821
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835.202, 842.361
842.101, 844.086
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851.073, 846.329
858.101, 848.086
859.116, 848.34
866.101, 850.086
867.161, 850.351
874.101, 852.086
880.995, 853.809
882.101, 854.086
883.256, 854.375
884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773


Material Boundary

906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
964.913, 874.789
970.101, 876.086
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987.201, 880.361
989.058, 880.825
989.174, 880.854
990.419, 881.165
994.101, 882.086
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999.144, 883.347
1001, 883.811
1002.1, 884.086
1002.22, 884.117
1002.54, 884.195
1002.57, 884.202
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1031.94, 891.544
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1036.44, 892.671
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1043.67, 894.478
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1064.74, 899.745
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1071.45, 901.423

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1076.99, 902.808
1077.01, 902.812
1082.1, 904.086
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1110.72, 911.241
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1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
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1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
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1380.54, 912.136
1386.13, 910.94
1389.43, 910.122
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1397.65, 908.087
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1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
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1429.93, 900.087
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	1446.69, 895.936
	1454.15, 894.087
	1456.17, 893.588
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	1457.93, 893.15
	1495.01, 893.15
	1616.33, 893.15
	1858.87, 893.15
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	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
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	2486.56, 893.15
	2956.54, 893.15
	3093.33, 892.905
	3324.03, 892.493
	3501.9, 891.358
	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
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	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4442.27, 740.935
	4456.92, 738
	4464.31, 736.526
	4466.94, 736

Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
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	<p>4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703 560.534, 701.359 567.615, 699 570.071, 698.182 573.623, 697 577.692, 695.645 579.631, 695 583.706, 693.643</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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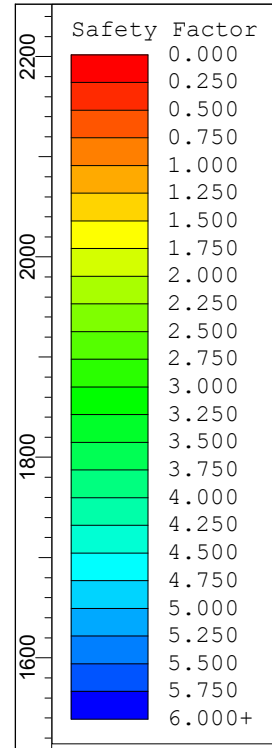
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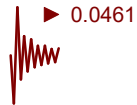
SLOPE STABILITY
SECTION A-A' - HORIZONTAL EXPANSION (**NORTH SLOPE**)

GLOBAL LINER STABILITY ANALYSIS

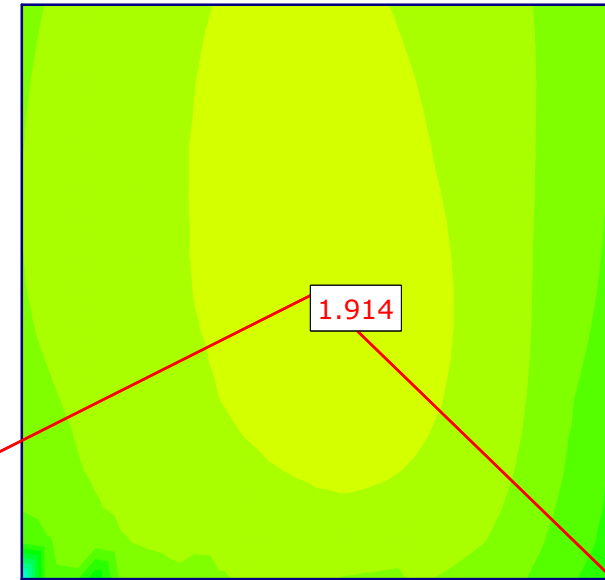
**COMPLETE BUILD-OUT / FINAL LANDFORM
CIRCULAR ANALYSIS OF WASTE AND FOUNDATION
(ROTATIONAL SLOPE FAILURE)**



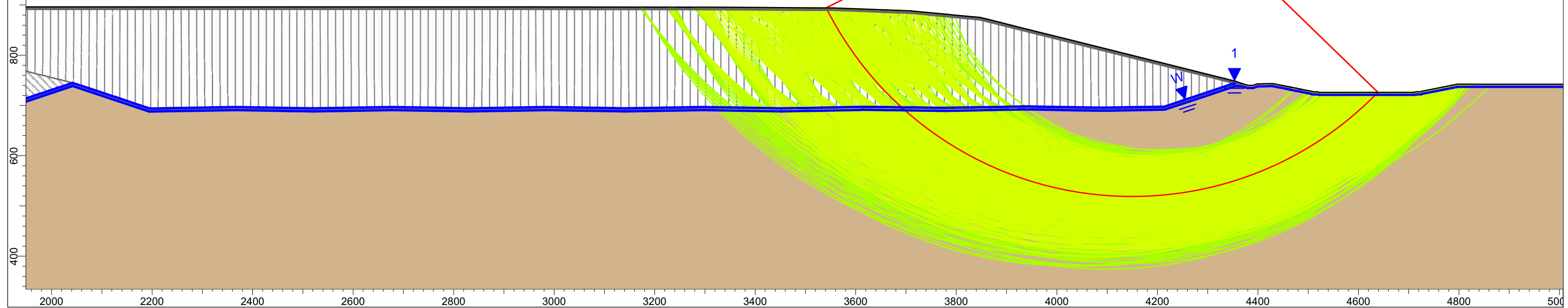
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Stability
Circular - Grid Search
Short Term / Seismic Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.914
 Center: 4149.758, 1200.262
 Radius: 681.294
 Left Slip Surface Endpoint: 3541.152, 894.057
 Right Slip Surface Endpoint: 4638.877, 726.000



Slide Analysis Information

A-A_final_north_ST

Project Summary

File Name:	A-A_final_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:28.779s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.995640
Center:	4174.347, 1415.305	
Radius:	880.076	
Left Slip Surface Endpoint:	3464.954, 894.444	
Right Slip Surface Endpoint:	4722.773, 727.000	
Resisting Moment:	5.68851e+09 lb-ft	
Driving Moment:	2.85047e+09 lb-ft	
Total Slice Area:	237548 ft ²	
Surface Horizontal Width:	1257.82 ft	
Surface Average Height:	188.857 ft	

Method: janbu corrected

	FS	1.914250
Center:	4149.758, 1200.262	
Radius:	681.294	
Left Slip Surface Endpoint:	3541.152, 894.057	
Right Slip Surface Endpoint:	4638.877, 726.000	
Resisting Horizontal Force:	5.76186e+06 lb	
Driving Horizontal Force:	3.00999e+06 lb	
Total Slice Area:	225430 ft ²	
Surface Horizontal Width:	1097.72 ft	
Surface Average Height:	205.361 ft	

Method: spencer

	FS	1.983240
Center:	4174.347, 1415.305	
Radius:	880.076	
Left Slip Surface Endpoint:	3464.954, 894.444	
Right Slip Surface Endpoint:	4722.773, 727.000	
Resisting Moment:	5.65318e+09 lb-ft	
Driving Moment:	2.85047e+09 lb-ft	
Resisting Horizontal Force:	5.85127e+06 lb	
Driving Horizontal Force:	2.95036e+06 lb	
Total Slice Area:	237548 ft ²	
Surface Horizontal Width:	1257.82 ft	
Surface Average Height:	188.857 ft	

Method: gle/morgenstern-price

FS	1.988770
Center:	4174.347, 1415.305
Radius:	880.076
Left Slip Surface Endpoint:	3464.954, 894.444
Right Slip Surface Endpoint:	4722.773, 727.000
Resisting Moment:	5.66893e+09 lb-ft
Driving Moment:	2.85047e+09 lb-ft
Resisting Horizontal Force:	5.86176e+06 lb
Driving Horizontal Force:	2.94744e+06 lb
Total Slice Area:	237548 ft ²
Surface Horizontal Width:	1257.82 ft
Surface Average Height:	188.857 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	3981
Number of Invalid Surfaces:	694

Error Codes

Error Code -103 reported for 497 surfaces
 Error Code -110 reported for 56 surfaces
 Error Code -112 reported for 141 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4020
Number of Invalid Surfaces:	655

Error Codes

Error Code -103 reported for 497 surfaces
 Error Code -110 reported for 56 surfaces
 Error Code -112 reported for 102 surfaces

Method: spencer

Number of Valid Surfaces:	3977
Number of Invalid Surfaces:	698

Error Codes

Error Code -103 reported for 497 surfaces
 Error Code -110 reported for 56 surfaces
 Error Code -111 reported for 1 surface
 Error Code -112 reported for 144 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	3979
Number of Invalid Surfaces:	696

Error Codes

Error Code -103 reported for 497 surfaces
 Error Code -110 reported for 56 surfaces
 Error Code -112 reported for 143 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

Entity Information

Circ - Thin Layers

Shared Entities

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	4801.2, 742.054
	4796.47, 742.054
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	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4486.94, 732
	4484.34, 732.521
	4481.94, 733
	4481.87, 733
	4479.43, 733.489
	4476.94, 734
	4476.94, 734
	4473.66, 734.657
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	4464.44, 736.5
	4464.31, 736.526

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Material Boundary	1457.67, 891 1495.01, 891 1616.33, 891 1858.87, 891 2127.47, 891 2141.81, 891 2181.65, 891 2195.59, 891 2234.57, 891 2475.13, 891 2486.56, 891 2956.54, 891 3093.33, 890.755 3324.02, 890.343 3501.89, 889.208 3555.79, 889 3608.11, 887.004 3660.51, 885.004 3710.91, 883.01 3711.85, 882.919 3731.43, 881.023 3735.61, 880.625 3741.94, 880.022 3742.25, 879.993 3742.58, 879.961 3752.81, 878.988 3762.96, 878.022 3773.92, 876.98 3783.98, 876.022 3795.35, 874.94 3804.99, 874.022 3816.83, 872.896 3826.02, 872.022 3838.05, 870.897 3847.04, 870.057 3861.26, 866.502 3886.68, 860.149 3906.63, 855.161 3926.68, 850.149 3942.07, 846.303 4033.69, 823.397 4146.07, 795.303 4306.68, 755.149 4332.29, 748.731 4346, 745.303 4352.73, 743.621 4354.12, 743.273
	426.87, 740.119 442.157, 743.94 449.793, 745.849 450.157, 745.94 450.483, 746.022 454.153, 746.94 458.147, 747.94 461.473, 748.773 466.139, 749.94 473.455, 751.767 474.149, 751.941 474.848, 752.114

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525.743, 764.846
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538.115, 767.936
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538.376, 768
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883.293, 854.229
884.182, 854.452
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Material Boundary

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	450.121, 746.086
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693.564, 806.949

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713.234, 811.867
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722.112, 814.086
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730.101, 816.086
734.809, 817.263
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746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
766.886, 825.282
770.101, 826.086
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777.707, 827.987
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778.761, 828.251
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842.101, 844.086
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851.073, 846.329
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866.101, 850.086
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874.101, 852.086
880.995, 853.809
882.101, 854.086
883.256, 854.375
884.146, 854.597
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898.101, 858.086
904.848, 859.773

Material Boundary


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912.746, 861.747
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915.521, 862.441
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923.528, 864.443
930.101, 866.086
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938.101, 868.086
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951.569, 871.453
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1162.12, 916.911
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	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
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	2956.54, 893.15
	3093.33, 892.905
	3324.03, 892.493
	3501.9, 891.358
	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
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	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4484.34, 732.521 4486.94, 732
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Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	

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731.21, 681.745
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768.453, 682.652
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876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
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916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
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948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262

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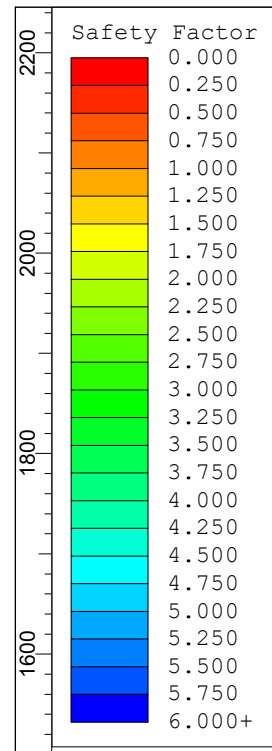
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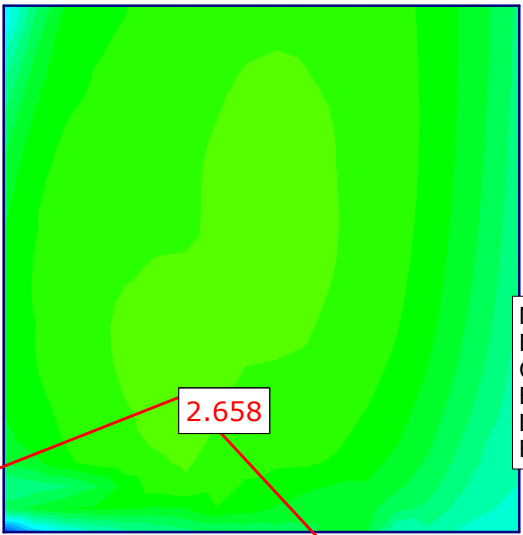
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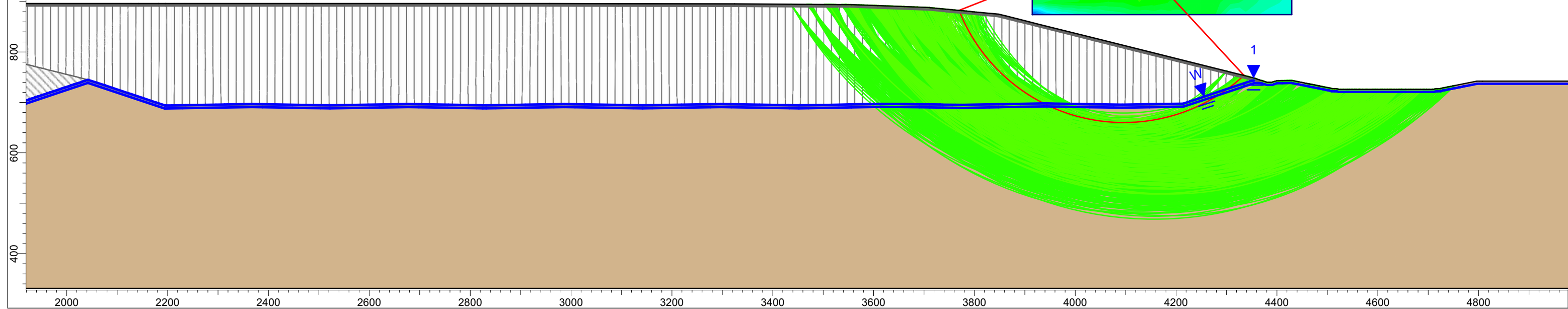


Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Stability
Circular - Grid Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
Existing Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 2.658
 Center: 4096.195, 1011.247
 Radius: 351.281
 Left Slip Surface Endpoint: 3769.383, 882.434
 Right Slip Surface Endpoint: 4334.649, 753.296



Slide Analysis Information

A-A_final_north_ST

Project Summary

File Name:	A-A_final_north_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:06.868s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	2.817890
Center:	4187.018, 1331.145	
Radius:	737.247	
Left Slip Surface Endpoint:	3594.440, 892.529	
Right Slip Surface Endpoint:	4608.126, 726.000	
Resisting Moment:	3.41617e+09 lb-ft	
Driving Moment:	1.21231e+09 lb-ft	
Total Slice Area:	148678 ft ²	
Surface Horizontal Width:	1013.69 ft	
Surface Average Height:	146.67 ft	

Method: janbu corrected

	FS	2.657970
Center:	4096.195, 1011.247	
Radius:	351.281	
Left Slip Surface Endpoint:	3769.383, 882.434	
Right Slip Surface Endpoint:	4334.649, 753.296	
Resisting Horizontal Force:	2.25304e+06 lb	
Driving Horizontal Force:	847656 lb	
Total Slice Area:	65199.7 ft ²	
Surface Horizontal Width:	565.265 ft	
Surface Average Height:	115.344 ft	

Method: spencer

	FS	2.790070
Center:	4187.018, 1262.596	
Radius:	655.725	
Left Slip Surface Endpoint:	3647.078, 890.520	
Right Slip Surface Endpoint:	4563.900, 726.000	
Resisting Moment:	2.64477e+09 lb-ft	
Driving Moment:	9.47923e+08 lb-ft	
Resisting Horizontal Force:	3.6442e+06 lb	
Driving Horizontal Force:	1.30613e+06 lb	
Total Slice Area:	126309 ft ²	
Surface Horizontal Width:	916.821 ft	
Surface Average Height:	137.769 ft	

Method: gle/morgenstern-price

	FS	2.783710
Center:		4065.921, 1079.796
Radius:		425.001
Left Slip Surface Endpoint:		3686.158, 888.993
Right Slip Surface Endpoint:		4337.219, 752.653
Resisting Moment:		1.20644e+09 lb-ft
Driving Moment:		4.33393e+08 lb-ft
Resisting Horizontal Force:		2.50833e+06 lb
Driving Horizontal Force:		901074 lb
Total Slice Area:		80851.1 ft ²
Surface Horizontal Width:		651.061 ft
Surface Average Height:		124.184 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	4115
Number of Invalid Surfaces:	637

Error Codes

Error Code -103 reported for 209 surfaces
 Error Code -110 reported for 69 surfaces
 Error Code -112 reported for 282 surfaces
 Error Code -114 reported for 77 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4142
Number of Invalid Surfaces:	610

Error Codes

Error Code -103 reported for 209 surfaces
 Error Code -108 reported for 3 surfaces
 Error Code -110 reported for 69 surfaces
 Error Code -112 reported for 252 surfaces
 Error Code -114 reported for 77 surfaces

Method: spencer

Number of Valid Surfaces:	4100
Number of Invalid Surfaces:	652

Error Codes

Error Code -103 reported for 209 surfaces
 Error Code -108 reported for 3 surfaces
 Error Code -110 reported for 69 surfaces
 Error Code -112 reported for 294 surfaces
 Error Code -114 reported for 77 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4101

Number of Invalid Surfaces: 651

Error Codes

Error Code -103 reported for 209 surfaces

Error Code -108 reported for 3 surfaces

Error Code -110 reported for 69 surfaces

Error Code -112 reported for 293 surfaces

Error Code -114 reported for 77 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-112 = The coefficient $M\text{-}\alpha = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-114 = Surface with Reverse Curvature.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
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	4710.54, 726
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	4506.94, 728
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594.104, 782.086
594.853, 782.275
602.111, 784.086
606.157, 785.097
610.11, 786.086
614.194, 787.107
618.11, 788.086
622.223, 789.116
626.101, 790.086
633.31, 791.888
634.101, 792.086
634.901, 792.286
642.101, 794.086
645.858, 795.025
650.112, 796.086
654.377, 797.152
658.111, 798.086
662.404, 799.162
666.101, 800.086
673.276, 801.878
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674.953, 802.296
674.961, 802.298
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690.111, 806.086
693.564, 806.949

698.111,	808.086
702.685,	809.229
706.111,	810.086
713.227,	811.865
713.234,	811.867
714.111,	812.086
714.999,	812.308
715.007,	812.31
722.112,	814.086
726.775,	815.254
730.101,	816.086
734.809,	817.263
738.101,	818.086
742.842,	819.271
746.101,	820.086
753.167,	821.852
753.175,	821.854
754.101,	822.086
755.035,	822.319
755.043,	822.321
762.101,	824.086
766.886,	825.282
770.101,	826.086
777.704,	827.987
777.707,	827.987
778.101,	828.086
778.761,	828.251
786.101,	830.086
793.117,	831.84
793.125,	831.842
794.101,	832.086
795.089,	832.333
802.101,	834.086
808.438,	835.67
810.101,	836.086
811.81,	836.513
818.101,	838.086
819.843,	838.521
826.101,	840.086
833.041,	841.821
834.101,	842.086
835.202,	842.361
842.101,	844.086
849.167,	845.852
850.101,	846.086
851.073,	846.329
858.101,	848.086
859.116,	848.34
866.101,	850.086
867.161,	850.351
874.101,	852.086
880.995,	853.809
882.101,	854.086
883.256,	854.375
884.146,	854.597
890.101,	856.086
895.862,	857.526
898.101,	858.086
904.848,	859.773

Material Boundary


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912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
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951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
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970.101, 876.086
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986.101, 880.086
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1162.12, 916.911
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1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
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1421.86, 902.087
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1428.47, 900.45
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	1446.69, 895.936
	1454.15, 894.087
	1456.17, 893.588
	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
	1616.33, 893.15
	1858.87, 893.15
	2127.47, 893.15
	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
	2234.57, 893.15
	2475.13, 893.15
	2486.56, 893.15
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	3093.33, 892.905
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	3555.84, 891.15
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	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
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	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
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	3763.16, 880.163
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	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
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	4034.21, 825.483
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	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4484.34, 732.521 4486.94, 732
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Scenario-based Entities




Type	Coordinates (x,y)	Static
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	0.809461, 723.908	 Wadsworth Till
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	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	

689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
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953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
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Water Table	1516.14, 686.278	
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	1692.76, 685	
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	1816.05, 683	
	1826.62, 683	
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	<p>4753.78, 728.448 4757.03, 729.097 4760.53, 729.797 4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703 560.534, 701.359 567.615, 699 570.071, 698.182 573.623, 697</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

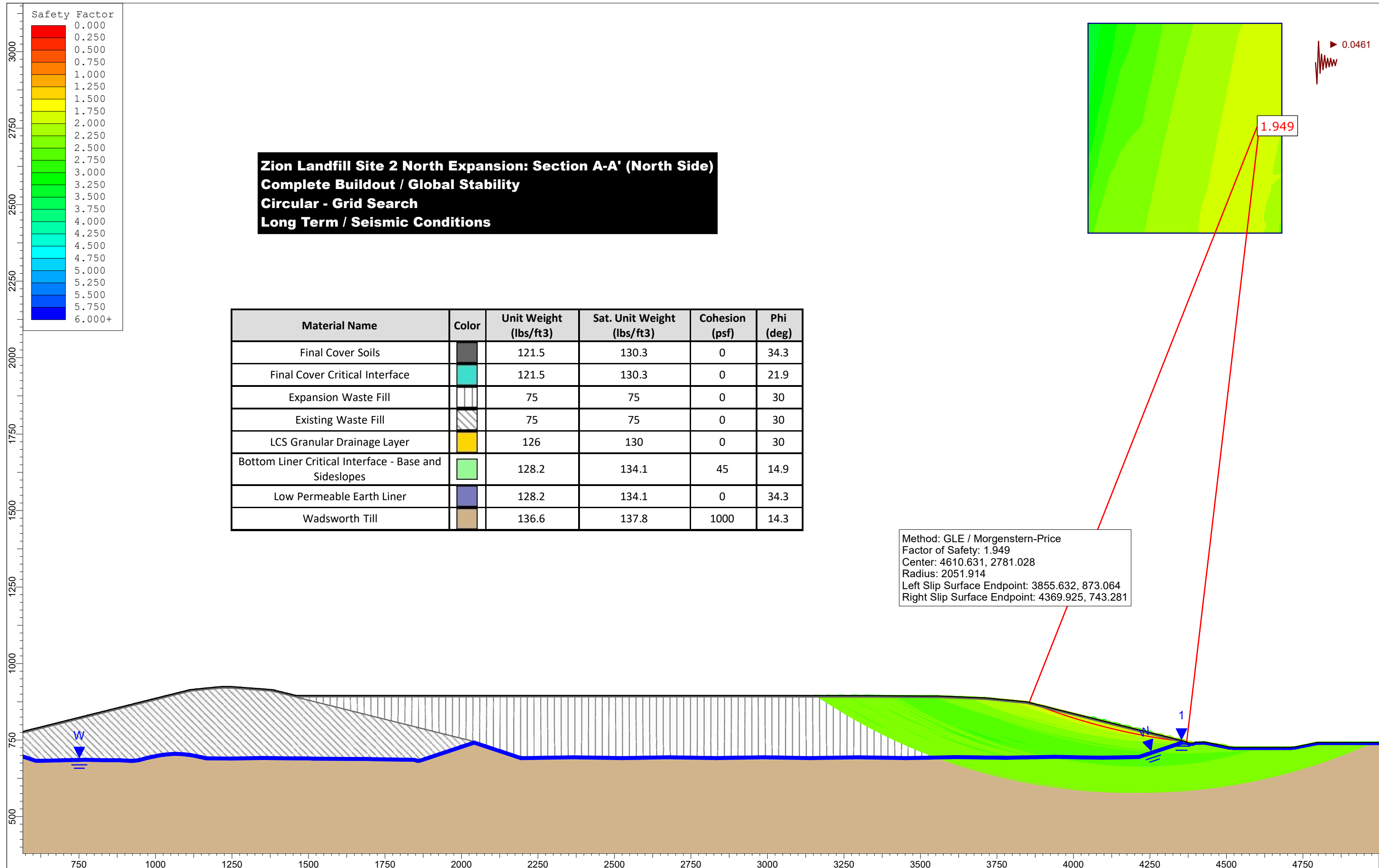
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Piezoline	1899.84, 697.434	
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3731.91, 695.696

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	3774.17, 695.144	
	3778.84, 695.083	
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	3847.89, 696.44	
	3856.62, 696.594	
	3891.54, 697.211	
	3937.75, 698.028	
	3943.28, 697.934	
	3967.87, 697.514	
	4028.8, 696.473	
	4092.79, 695.38	
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	4168.34, 696.706	
	4213.55, 697.499	
	4302.23, 727.041	
	4347.22, 741.988	
	4350.55, 743.096	
	4352.88, 743.177	
	4354.12, 743.273	



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Stability
Circular - Grid Search
Long Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
Existing Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3

Method: GLE / Morgenstern-Price
 Factor of Safety: 1.949
 Center: 4610.631, 2781.028
 Radius: 2051.914
 Left Slip Surface Endpoint: 3855.632, 873.064
 Right Slip Surface Endpoint: 4369.925, 743.281

Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:17.926s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	1.953020
Center:	4610.631, 2781.028	
Radius:	2051.914	
Left Slip Surface Endpoint:	3855.632, 873.064	
Right Slip Surface Endpoint:	4369.925, 743.281	
Resisting Moment:	6.8163e+08 lb-ft	
Driving Moment:	3.49014e+08 lb-ft	
Total Slice Area:	6389.84 ft2	
Surface Horizontal Width:	514.293 ft	
Surface Average Height:	12.4245 ft	

Method: janbu corrected

	FS	1.963790
Center:	4610.631, 2781.028	
Radius:	2051.914	
Left Slip Surface Endpoint:	3855.632, 873.064	
Right Slip Surface Endpoint:	4369.925, 743.281	
Resisting Horizontal Force:	324712 lb	
Driving Horizontal Force:	165350 lb	
Total Slice Area:	6389.84 ft2	
Surface Horizontal Width:	514.293 ft	
Surface Average Height:	12.4245 ft	

Method: spencer

	FS	1.950620
Center:	4610.631, 2781.028	
Radius:	2051.914	
Left Slip Surface Endpoint:	3855.632, 873.064	
Right Slip Surface Endpoint:	4369.925, 743.281	
Resisting Moment:	6.80791e+08 lb-ft	
Driving Moment:	3.49014e+08 lb-ft	
Resisting Horizontal Force:	321780 lb	
Driving Horizontal Force:	164963 lb	
Total Slice Area:	6389.84 ft2	
Surface Horizontal Width:	514.293 ft	
Surface Average Height:	12.4245 ft	

Method: gle/morgenstern-price

FS	1.949460
Center:	4610.631, 2781.028
Radius:	2051.914
Left Slip Surface Endpoint:	3855.632, 873.064
Right Slip Surface Endpoint:	4369.925, 743.281
Resisting Moment:	6.80387e+08 lb-ft
Driving Moment:	3.49014e+08 lb-ft
Resisting Horizontal Force:	321684 lb
Driving Horizontal Force:	165012 lb
Total Slice Area:	6389.84 ft ²
Surface Horizontal Width:	514.293 ft
Surface Average Height:	12.4245 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	4504
Number of Invalid Surfaces:	303

Error Codes

Error Code -106 reported for 10 surfaces
 Error Code -107 reported for 97 surfaces
 Error Code -110 reported for 196 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4504
Number of Invalid Surfaces:	303

Error Codes

Error Code -106 reported for 10 surfaces
 Error Code -107 reported for 97 surfaces
 Error Code -110 reported for 196 surfaces

Method: spencer

Number of Valid Surfaces:	4504
Number of Invalid Surfaces:	303

Error Codes

Error Code -106 reported for 10 surfaces
 Error Code -107 reported for 97 surfaces
 Error Code -110 reported for 196 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	4504
Number of Invalid Surfaces:	303

Error Codes

Error Code -106 reported for 10 surfaces
 Error Code -107 reported for 97 surfaces
 Error Code -110 reported for 196 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

Entity Information

Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	4805.86, 742.054
	4801.56, 742.054
	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4516.53, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4486.94, 732
	4481.94, 733
	4481.52, 733.084
	4476.94, 734
	4476.94, 734
	4471.48, 735.093
	4466.94, 736
	4466.94, 736
	4466.94, 736
	4464.31, 736.526
	4461.43, 737.102
	4456.94, 738
	4456.94, 738
	4451.39, 739.111
	4446.94, 740
	4442.27, 740.935
	4431.95, 743
	4429.16, 743.558

4410.47, 743.103
4399.15, 742.903
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4390.15, 739.897
4386.69, 739.881
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4377.03, 740.918
4371.14, 742.896
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4347.22, 750.154
4333.51, 753.581
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3943.28, 851.154
3927.9, 855
3907.85, 860.012
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3862.48, 871.353
3847.89, 875
3838.52, 875.876
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3753.28, 883.966
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3742.72, 884.971
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	442.157, 743.94
	449.793, 745.849
	450.157, 745.94
	450.483, 746.022
	454.153, 746.94
	458.147, 747.94
	461.473, 748.773
	466.139, 749.94
	473.455, 751.767
	474.149, 751.941
	474.848, 752.114

482.148, 753.94
486.653, 755.067
490.114, 755.94
493.637, 756.814
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501.644, 758.823
506.113, 759.94
513.419, 761.762
514.14, 761.94
514.848, 762.119
522.12, 763.94
525.743, 764.846
530.121, 765.94
538.115, 767.936
538.372, 768
538.376, 768
546.137, 769.94
553.397, 771.755
554.133, 771.94
554.87, 772.124
562.119, 773.94
566.015, 774.914
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574.042, 776.921
578.12, 777.94
582.069, 778.927
586.121, 779.94
593.377, 781.753
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594.889, 782.13
602.147, 783.941
606.193, 784.952
610.147, 785.94
614.23, 786.961
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626.137, 789.94
633.346, 791.743
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634.937, 792.14
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645.894, 794.88
650.148, 795.941
654.413, 797.007
658.147, 797.94
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666.137, 799.94
673.312, 801.732
674.148, 801.941
674.99, 802.151
674.997, 802.153
682.148, 803.94
685.634, 804.812
690.148, 805.94
693.601, 806.804
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715.036, 812.162
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722.148, 813.94
726.812, 815.109
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742.878, 819.125
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753.211, 821.709
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755.079, 822.176
762.137, 823.94
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770.137, 825.94
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793.153, 831.694
793.161, 831.696
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810.137, 835.94
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826.137, 839.94
833.077, 841.675
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835.239, 842.216
842.137, 843.94
849.204, 845.707
850.137, 845.94
851.109, 846.183
858.137, 847.94
859.153, 848.194
866.137, 849.94
867.198, 850.205
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881.031, 853.664
882.137, 853.94
883.293, 854.229
884.182, 854.452
890.137, 855.94
895.898, 857.381
898.137, 857.94
904.884, 859.627
906.137, 859.94
912.782, 861.602
914.137, 861.94
915.557, 862.295

Material Boundary

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923.565, 864.297
930.137, 865.94
936.197, 867.455
938.137, 867.94
944.598, 869.556
945.218, 869.71
945.978, 869.9
946.137, 869.94
948.322, 870.486
949.797, 870.855
950.928, 871.138
951.605, 871.307
954.137, 871.94
958.68, 873.076
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964.95, 874.643
970.137, 875.94
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982.81, 879.109
986.137, 879.94
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989.094, 880.68
989.211, 880.709
990.456, 881.02
994.137, 881.94
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999.18, 883.201
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1058.14, 897.94
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1071.49, 901.277
1074.14, 901.94
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1077.04, 902.666
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1098.14, 907.94
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1106.14, 909.94
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1162.14, 916.761
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1194.42, 919.99
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1223.74, 922
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	3742.77, 881.952
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	3763.15, 880.013
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	458.111, 748.086
	461.436, 748.919

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626.101, 790.086
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650.112, 796.086
654.377, 797.152
658.111, 798.086
662.404, 799.162
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693.564, 806.949

698.111,	808.086
702.685,	809.229
706.111,	810.086
713.227,	811.865
713.234,	811.867
714.111,	812.086
714.999,	812.308
715.007,	812.31
722.112,	814.086
726.775,	815.254
730.101,	816.086
734.809,	817.263
738.101,	818.086
742.842,	819.271
746.101,	820.086
753.167,	821.852
753.175,	821.854
754.101,	822.086
755.035,	822.319
755.043,	822.321
762.101,	824.086
766.886,	825.282
770.101,	826.086
777.704,	827.987
777.707,	827.987
778.101,	828.086
778.761,	828.251
786.101,	830.086
793.117,	831.84
793.125,	831.842
794.101,	832.086
795.089,	832.333
802.101,	834.086
808.438,	835.67
810.101,	836.086
811.81,	836.513
818.101,	838.086
819.843,	838.521
826.101,	840.086
833.041,	841.821
834.101,	842.086
835.202,	842.361
842.101,	844.086
849.167,	845.852
850.101,	846.086
851.073,	846.329
858.101,	848.086
859.116,	848.34
866.101,	850.086
867.161,	850.351
874.101,	852.086
880.995,	853.809
882.101,	854.086
883.256,	854.375
884.146,	854.597
890.101,	856.086
895.862,	857.526
898.101,	858.086
904.848,	859.773

Material Boundary


906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
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1001, 883.811
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1071.45, 901.423

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1077.01, 902.812
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1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
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1197.66, 920.465
1201.22, 920.776
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1298.92, 918.144
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1414.79, 903.84
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1428.47, 900.45
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1454.15, 894.087
1456.17, 893.588
1456.17, 893.587
1457.93, 893.15
1495.01, 893.15
1616.33, 893.15
1858.87, 893.15
2127.47, 893.15
2141.81, 893.15
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2234.57, 893.15
2475.13, 893.15
2486.56, 893.15
2956.54, 893.15
3093.33, 892.905
3324.03, 892.493
3501.9, 891.358
3555.84, 891.15
3608.19, 889.152
3660.59, 887.152
3711.06, 885.155
3712.05, 885.059
3731.64, 883.163
3735.81, 882.765
3742.15, 882.163
3742.45, 882.134
3742.79, 882.102
3753.01, 881.129
3763.16, 880.163
3774.12, 879.12
3784.18, 878.163
3795.56, 877.08
3805.2, 876.163
3817.03, 875.037
3826.22, 874.163
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3861.79, 868.588
3887.2, 862.235
3907.15, 857.247
3927.2, 852.235
3942.59, 848.389
4034.21, 825.483
4146.59, 797.389
4307.21, 757.235
4332.82, 750.816
4346.52, 747.389
4353.25, 745.706
4353.34, 745.686
4361.26, 743.112

Material Boundary	4442.27, 740.935 4456.92, 738 4464.31, 736.526 4464.44, 736.5 4466.94, 736
Material Boundary	4466.94, 736 4476.94, 734 4481.87, 733 4481.94, 733 4481.94, 733 4486.94, 732

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	




488.131, 719.138
522.779, 707.602
608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
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839.229, 681
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853.768, 681
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861.768, 681
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876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186

Water Table

964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262
1026.16, 698.595
1029.95, 699.071
1037.13, 700.174
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1060.67, 701.77
1068.84, 701.86
1069.64, 701.862
1072.74, 701.68
1077.4, 701.408
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1093.76, 700.535
1103.93, 699.206
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1108.51, 698.705
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1119.27, 697.117
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1147.98, 691.316
1148.3, 691.248
1149.18, 691.068
1151.08, 690.719
1156.36, 689.409
1157.27, 689.184
1163.63, 687.532
1180.77, 687.159
1199.14, 687
1219.87, 686.821
1247.6, 686.53
1274.47, 687.001
1354.82, 688.409
1380.15, 687.893
1449.6, 687
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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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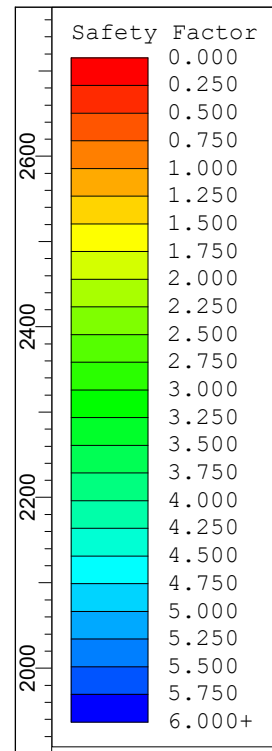
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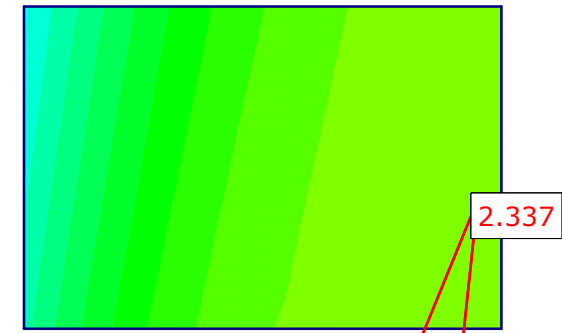
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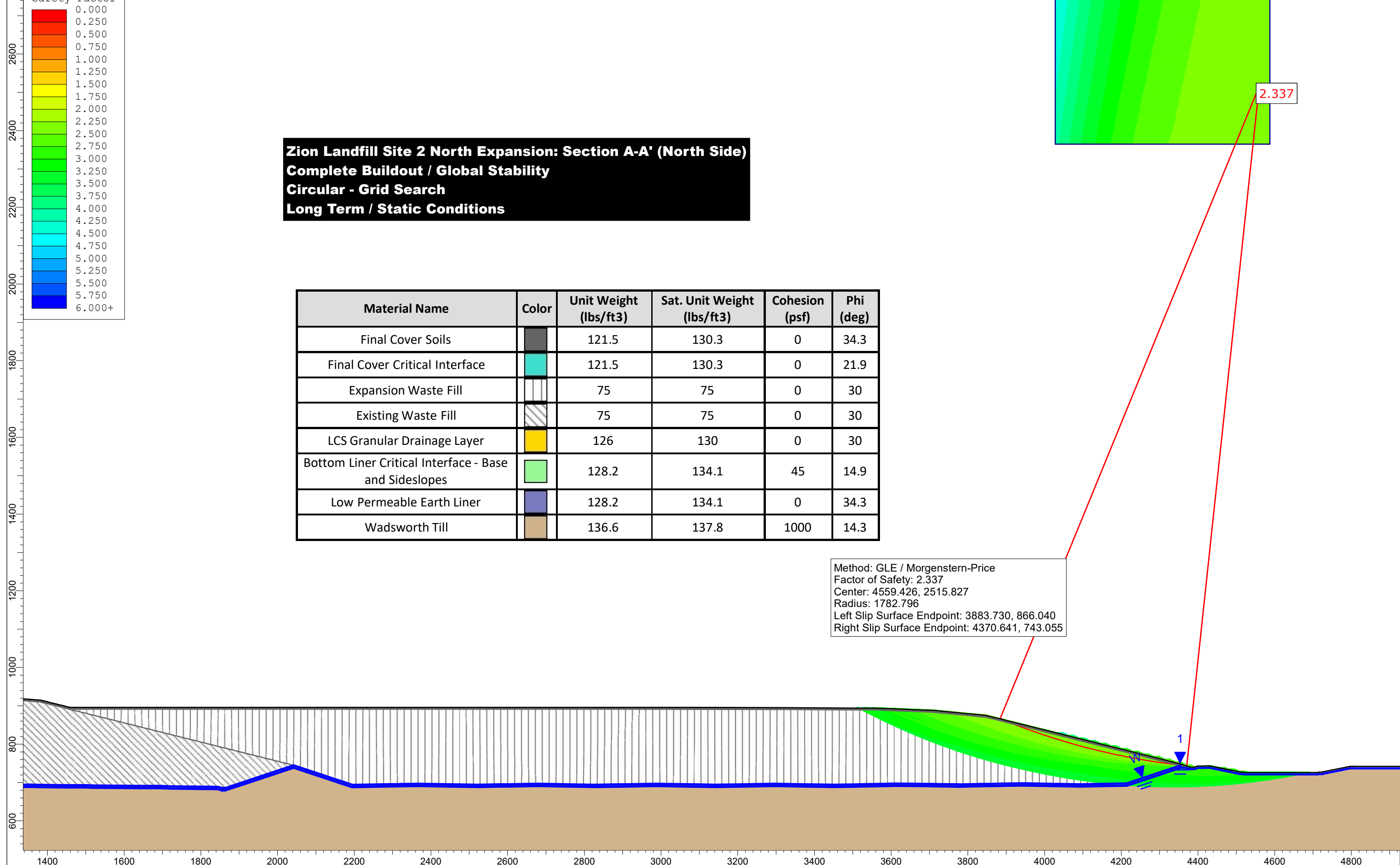


Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Complete Buildout / Global Stability
Circular - Grid Search
Long Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
Existing Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3



Method: GLE / Morgenstern-Price
 Factor of Safety: 2.337
 Center: 4559.426, 2515.827
 Radius: 1782.796
 Left Slip Surface Endpoint: 3883.730, 866.040
 Right Slip Surface Endpoint: 4370.641, 743.055



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:10.309s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No






Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
Existing Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	2.340870
Center:	4559.426, 2515.827	
Radius:	1782.796	
Left Slip Surface Endpoint:	3883.730, 866.040	
Right Slip Surface Endpoint:	4370.641, 743.055	
Resisting Moment:	5.82407e+08 lb-ft	
Driving Moment:	2.48799e+08 lb-ft	
Total Slice Area:	6248.77 ft2	
Surface Horizontal Width:	486.912 ft	
Surface Average Height:	12.8335 ft	

Method: janbu corrected

	FS	2.354170
Center:	4587.412, 2704.743	
Radius:	1972.918	
Left Slip Surface Endpoint:	3852.270, 873.905	
Right Slip Surface Endpoint:	4367.152, 744.159	
Resisting Horizontal Force:	337875 lb	
Driving Horizontal Force:	143522 lb	
Total Slice Area:	6612.43 ft2	
Surface Horizontal Width:	514.882 ft	
Surface Average Height:	12.8426 ft	

Method: spencer

	FS	2.338840
Center:	4559.426, 2515.827	
Radius:	1782.796	
Left Slip Surface Endpoint:	3883.730, 866.040	
Right Slip Surface Endpoint:	4370.641, 743.055	
Resisting Moment:	5.81903e+08 lb-ft	
Driving Moment:	2.48799e+08 lb-ft	
Resisting Horizontal Force:	316329 lb	
Driving Horizontal Force:	135250 lb	
Total Slice Area:	6248.77 ft2	
Surface Horizontal Width:	486.912 ft	
Surface Average Height:	12.8335 ft	

Method: gle/morgenstern-price

FS	2.337150
Center:	4559.426, 2515.827
Radius:	1782.796
Left Slip Surface Endpoint:	3883.730, 866.040
Right Slip Surface Endpoint:	4370.641, 743.055
Resisting Moment:	5.81481e+08 lb-ft
Driving Moment:	2.48799e+08 lb-ft
Resisting Horizontal Force:	316287 lb
Driving Horizontal Force:	135330 lb
Total Slice Area:	6248.77 ft ²
Surface Horizontal Width:	486.912 ft
Surface Average Height:	12.8335 ft

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces:	4165
Number of Invalid Surfaces:	686

Error Codes

Error Code -110 reported for 186 surfaces
 Error Code -115 reported for 500 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4165
Number of Invalid Surfaces:	686

Error Codes

Error Code -110 reported for 186 surfaces
 Error Code -115 reported for 500 surfaces

Method: spencer

Number of Valid Surfaces:	4165
Number of Invalid Surfaces:	686

Error Codes

Error Code -110 reported for 186 surfaces
 Error Code -115 reported for 500 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	4165
Number of Invalid Surfaces:	686

Error Codes

Error Code -110 reported for 186 surfaces
 Error Code -115 reported for 500 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

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	4712.32, 726.355
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	4656.84, 726
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	4516.53, 727
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Material Boundary	4466.94, 736 4476.94, 734 4481.87, 733 4481.94, 733 4481.94, 733 4486.94, 732

Scenario-based Entities

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	399.078, 735.005	
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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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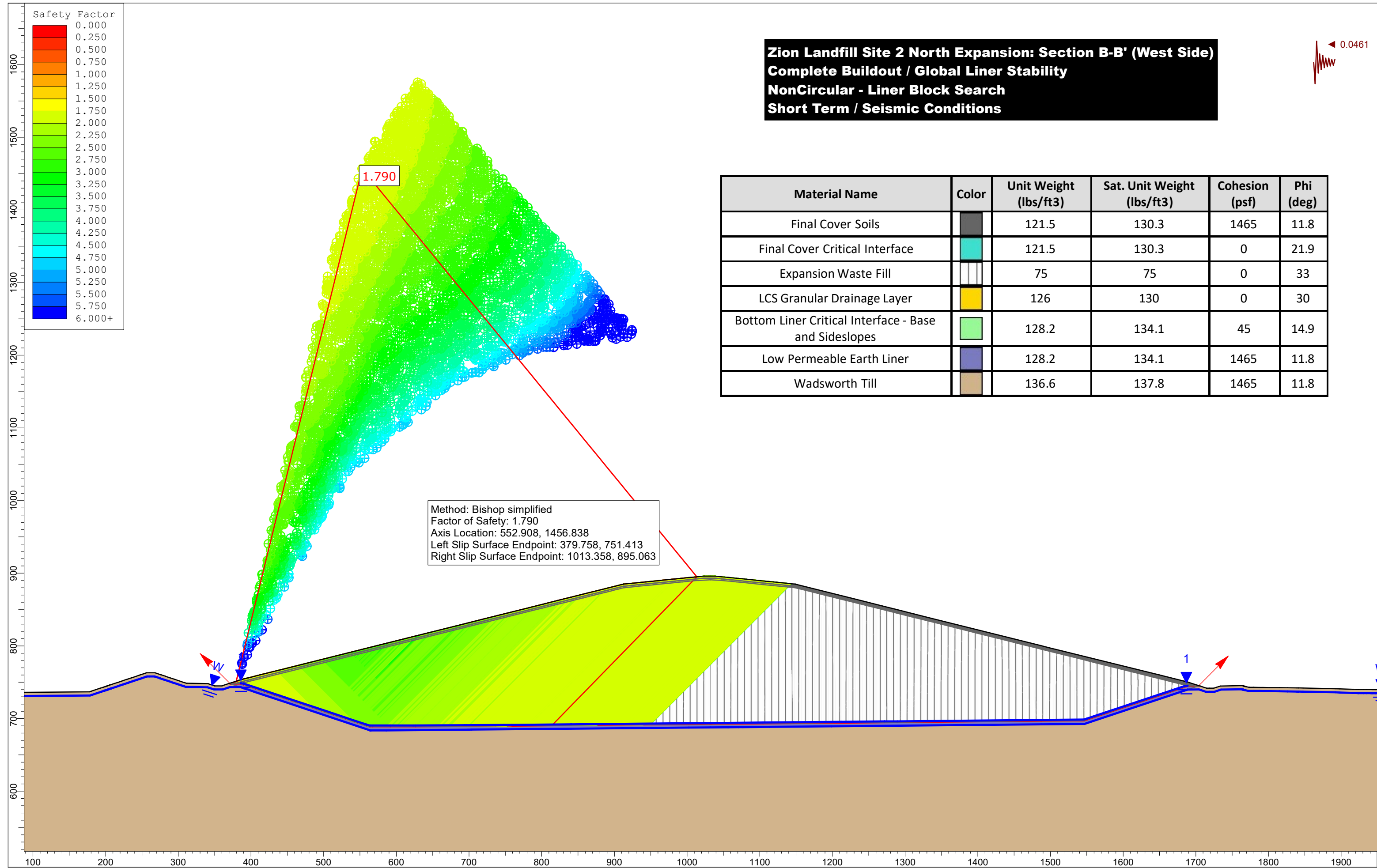
SLOPE STABILITY
SECTION **B-B'** - **HORIZONTAL EXPANSION (WEST SLOPE)**

GLOBAL LINER STABILITY ANALYSIS

**COMPLETE BUILD-OUT / FINAL LANDFORM
BLOCK ANALYSIS OF LINER SYSTEM
(TRANSLATIONAL SLOPE FAILURE)**

Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Short Term / Seismic Conditions

0.0461



Material Name	Color	Unit Weight (lbs/ft ³)	Sat. Unit Weight (lbs/ft ³)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	1465	11.8
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	33
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	1465	11.8
Wadsworth Till	Brown	136.6	137.8	1465	11.8

Method: Bishop simplified
 Factor of Safety: 1.790
 Axis Location: 552.908, 1456.838
 Left Slip Surface Endpoint: 379.758, 751.413
 Right Slip Surface Endpoint: 1013.358, 895.063

Slide Analysis Information

B-B_final_west_ST

Project Summary

File Name:	B-B_final_west_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:10m:02.182s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8

Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.789550
Axis Location:	552.908, 1456.838
Left Slip Surface Endpoint:	379.758, 751.413
Right Slip Surface Endpoint:	1013.358, 895.063
Resisting Moment:	1.41659e+09 lb-ft
Driving Moment:	7.91587e+08 lb-ft
Total Slice Area:	62740.3 ft ²
Surface Horizontal Width:	633.6 ft
Surface Average Height:	99.0219 ft

Method: janbu corrected

FS	1.810390
Axis Location:	547.693, 1443.011
Left Slip Surface Endpoint:	379.805, 751.428
Right Slip Surface Endpoint:	1000.226, 893.750
Resisting Horizontal Force:	1.67674e+06 lb
Driving Horizontal Force:	926173 lb
Total Slice Area:	60701.8 ft ²
Surface Horizontal Width:	620.422 ft
Surface Average Height:	97.8397 ft

Method: spencer

FS	1.891970
Axis Location:	570.371, 1494.094
Left Slip Surface Endpoint:	378.634, 751.050
Right Slip Surface Endpoint:	1049.764, 894.879
Resisting Moment:	1.58041e+09 lb-ft
Driving Moment:	8.35325e+08 lb-ft
Resisting Horizontal Force:	1.76466e+06 lb
Driving Horizontal Force:	932711 lb
Total Slice Area:	66629.2 ft ²
Surface Horizontal Width:	671.13 ft
Surface Average Height:	99.2792 ft

Method: gle/morgenstern-price

FS	1.898430
Axis Location:	571.731, 1493.884
Left Slip Surface Endpoint:	379.738, 751.407
Right Slip Surface Endpoint:	1050.517, 894.803
Resisting Moment:	1.5829e+09 lb-ft
Driving Moment:	8.33795e+08 lb-ft
Resisting Horizontal Force:	1.76511e+06 lb
Driving Horizontal Force:	929778 lb
Total Slice Area:	67403.1 ft ²
Surface Horizontal Width:	670.779 ft
Surface Average Height:	100.485 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
379.758	751.413
385.483	748.325
394.475	745.328
398.827	743.877
399.282	743.725
406.922	741.178
409.058	740.466
418.13	737.442
428.218	734.079
439.311	730.382
440.293	730.055
513.018	705.845
559.553	690.394
562.825	689.315
567.674	689.204
627.659	689.218
635.264	689.268
707.149	689.738
708.377	689.746
744.273	690.003
756.21	690.115
814.382	690.809
1013.36	895.063

Method: janbu corrected

X	Y
379.805	751.428
385.481	748.324
391.614	746.298
398.414	744.014
400.175	743.427
406.79	741.222
408.733	740.574
419.674	736.927
430.059	733.465
436.645	731.268
442.374	729.36
514.455	705.33
559.775	690.321
562.761	689.359
568.961	689.195
625.94	689.23
631.636	689.244
637.907	689.284
707.172	689.738
708.591	689.747
744.393	690.004
756.18	690.114
806.111	690.732
1000.23	893.75

Method: spencer

X	Y
378.634	751.05
382.392	749.357
392.038	746.14
398.57	743.961
400.026	743.477
406.875	741.193
409.772	740.228
418.094	737.453
427.913	734.18
437.762	730.897
444.566	728.629
510.879	706.524
559.204	690.357
562.666	689.314
569.429	689.197
629.562	689.23
637.748	689.283
706.564	689.735
708.708	689.748
744.264	690.002
755.89	690.111
815.519	690.82
1049.76	894.879

Method: gle/morgenstern-price

X	Y
379.738	751.407
384.907	748.508
387.099	747.795
399.009	743.816
399.922	743.512
408.661	740.597
408.95	740.502
410.569	739.961
430.935	733.173
438.139	730.771
445.935	728.172
513.932	705.504
556.902	691.121
562.774	689.358
570.87	689.207
612.528	689.212
626.416	689.209
637.823	689.284
704.484	689.72
710.202	689.758
742.454	689.986
756.485	690.117
822.402	690.884
1050.52	894.803

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4778
 Number of Invalid Surfaces: 227

Error Codes

Error Code -108 reported for 1 surface
 Error Code -110 reported for 226 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4778
 Number of Invalid Surfaces: 226

Error Codes

Error Code -110 reported for 226 surfaces

Method: spencer

Number of Valid Surfaces: 4442
 Number of Invalid Surfaces: 563

Error Codes

Error Code -108 reported for 50 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 287 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4092
 Number of Invalid Surfaces: 912

Error Codes

Error Code -108 reported for 50 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 636 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072

1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749

307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281

External Boundary

0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56

2020.89,	750.238
2018.62,	749.686
2006.76,	746.88
2004.23,	746.282
1997.83,	744.822
1995.37,	744.264
1991.58,	743.44
1989.28,	742.939
1986.9,	742.454
1984.74,	742.012
1983.22,	741.722
1982.37,	741.56
1981.18,	741.335
1980.18,	741.16
1978.26,	740.826
1976.28,	740.516
1975.78,	740.438
1973.88,	740.177
1973.61,	740.14
1971.91,	739.942
1971.78,	739.934
1971.67,	739.928
1970.03,	739.874
1969.97,	739.872
1966.4,	739.837
1962.15,	739.979
1962.12,	739.978
1960.65,	739.995
1960.65,	739.995
1960.64,	739.995
1958.84,	739.961
1958.8,	739.962
1955.97,	739.894
1955.8,	739.899
1952.02,	739.831
1947.2,	739.799
1942.5,	739.789
1940.44,	740
1939.92,	740.009
1935.35,	740.008
1934.96,	740.008
1930.81,	740.006
1930.4,	740.007
1927.26,	740.005
1923.77,	740.001
1923.7,	740.001
1923.26,	740
1923.21,	740
1920.59,	740.087
1917.66,	740.181
1916.72,	740.203
1915.68,	740.226
1898.08,	740.833
1892.28,	741.057
1847.51,	742
1841.58,	742.146
1841.29,	742.151
1840.84,	742.154
1840.05,	742.158

1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
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1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
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1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
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	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
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Material Boundary	1003.02, 891.015
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	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281 178.514, 731.719 180.265, 732.302 180.685, 732.443 181.358, 732.667 181.974, 732.872 194.152, 736.932 197.551, 738.064 198.448, 738.364 198.554, 738.399 198.861, 738.501 199.339, 738.661 200.776, 739.14 200.997, 739.213 201.651, 739.431 201.708, 739.45 202.375, 739.673 202.96, 739.868 203.066, 739.903 203.951, 740.198 204.163, 740.269 205.695, 740.779 206.113, 740.918 206.403, 741.015 206.704, 741.116 206.992, 741.211 207.162, 741.268 207.356, 741.333 213.598, 743.414 219.127, 745.257	Assigned to:  Wadsworth Till

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
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563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

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	703.436, 684.721	
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	1198.73, 689.322	
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	1546.99, 692.557	
	1550.33, 693.668	
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	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
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	1763.01, 740.319	
	1763.65, 740.104	
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	1763.78, 740.062	
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	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
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	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
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	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
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1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
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1917.52, 735.183
1920.42, 735.09
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1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
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1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
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1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
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1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

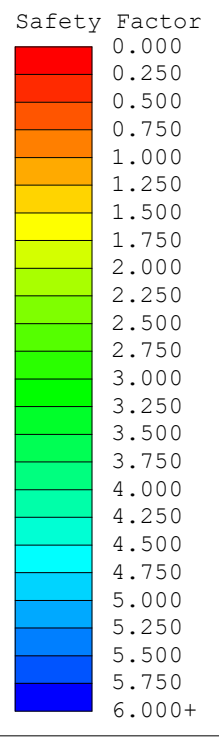
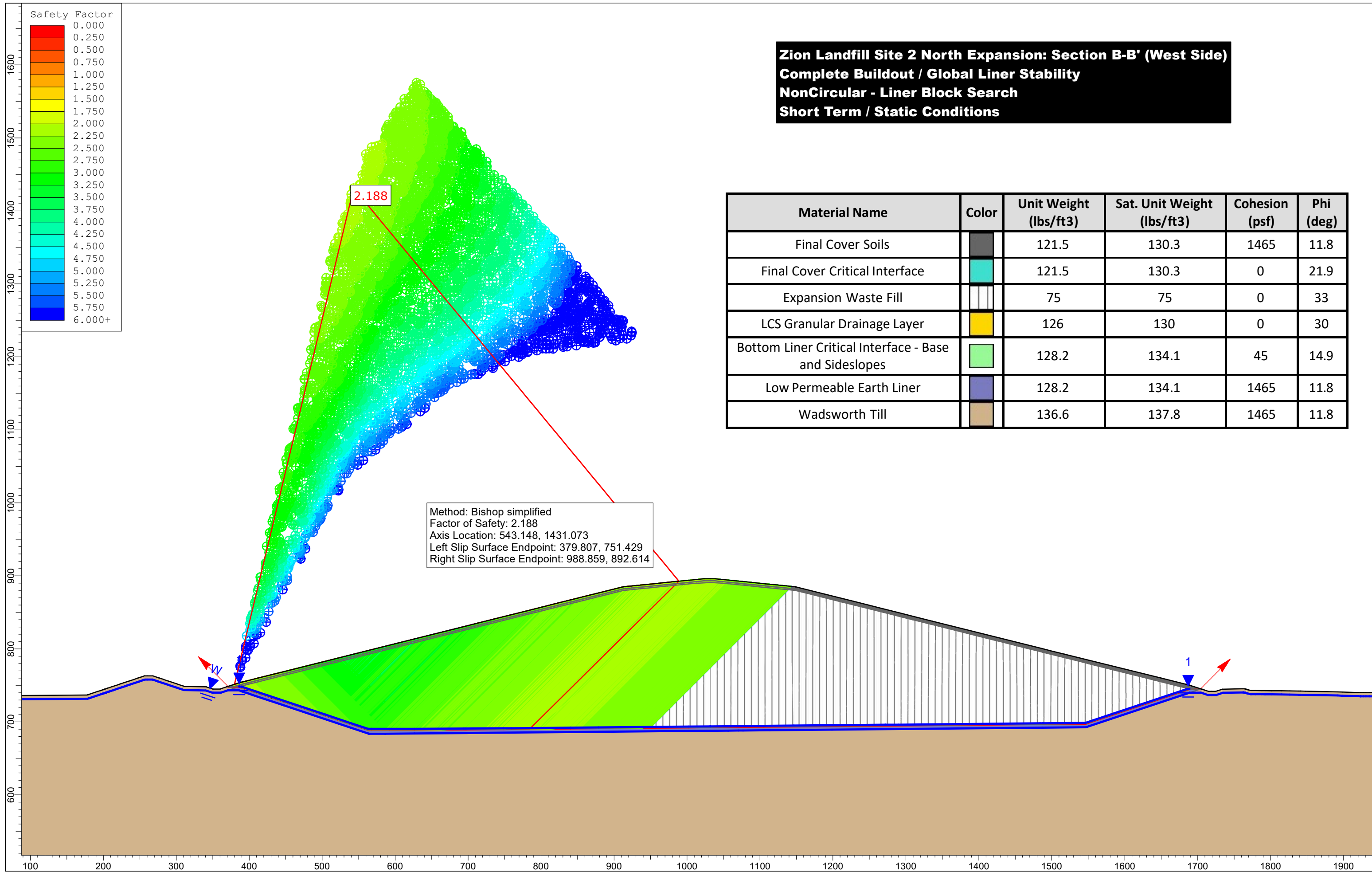
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2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
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	696.794, 690.824	
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	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
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1488.81, 698.064		
1546.16, 698.605		
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1550.34, 699.997		
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1685.08, 744.911		
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Block Search Polyline

371.35, 747.924
386.046, 748.217
390.079, 746.872
398.881, 743.938
399.765, 743.643
406.945, 741.249
409.081, 740.537
417.36, 737.777
429.79, 733.633
437.541, 731.05
447.072, 727.872
513.038, 705.881
558.707, 690.597
562.754, 689.282
564.036, 689.269
567.392, 689.269
591.838, 689.285
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695.699, 689.738
697.255, 689.748
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1030.76, 692.742
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1033.47, 692.767
1056.47, 692.975
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1212.44, 694.424
1487.03, 696.975
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1546.33, 697.528
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1653.04, 733.095
1688.76, 745.005
1695.56, 744.868
1703.46, 744.71

Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Short Term / Static Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	1465	11.8
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	33
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	1465	11.8
Wadsworth Till	Brown	136.6	137.8	1465	11.8

Method: Bishop simplified
 Factor of Safety: 2.188
 Axis Location: 543.148, 1431.073
 Left Slip Surface Endpoint: 379.807, 751.429
 Right Slip Surface Endpoint: 988.859, 892.614

Slide Analysis Information

B-B_final_west_ST

Project Summary

File Name:	B-B_final_west_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:16m:11.170s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465

Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.187580
Axis Location:	543.148, 1431.073
Left Slip Surface Endpoint:	379.807, 751.429
Right Slip Surface Endpoint:	988.859, 892.614
Resisting Moment:	1.30568e+09 lb-ft
Driving Moment:	5.96858e+08 lb-ft
Total Slice Area:	57455.3 ft ²
Surface Horizontal Width:	609.052 ft
Surface Average Height:	94.3357 ft

Method: janbu corrected

FS	2.220250
Axis Location:	543.148, 1431.073
Left Slip Surface Endpoint:	379.807, 751.429
Right Slip Surface Endpoint:	988.859, 892.614
Resisting Horizontal Force:	1.65578e+06 lb
Driving Horizontal Force:	745762 lb
Total Slice Area:	57455.3 ft ²
Surface Horizontal Width:	609.052 ft
Surface Average Height:	94.3357 ft

Method: spencer

FS	2.323780
Axis Location:	559.786, 1477.697
Left Slip Surface Endpoint:	377.841, 750.794
Right Slip Surface Endpoint:	1032.142, 896.000
Resisting Moment:	1.53492e+09 lb-ft
Driving Moment:	6.60529e+08 lb-ft
Resisting Horizontal Force:	1.74426e+06 lb
Driving Horizontal Force:	750614 lb
Total Slice Area:	62741.2 ft ²
Surface Horizontal Width:	654.3 ft
Surface Average Height:	95.8906 ft

Method: gle/morgenstern-price

FS	2.336710
Axis Location:	567.703, 1487.199
Left Slip Surface Endpoint:	379.882, 751.453
Right Slip Surface Endpoint:	1043.607, 895.494
Resisting Moment:	1.57445e+09 lb-ft
Driving Moment:	6.73789e+08 lb-ft
Resisting Horizontal Force:	1.76546e+06 lb
Driving Horizontal Force:	755533 lb
Total Slice Area:	65307.9 ft ²
Surface Horizontal Width:	663.725 ft
Surface Average Height:	98.396 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
379.807	751.429
385.472	748.322
387.637	747.627
399.031	743.967
399.528	743.801
407.968	740.973
408.584	740.754
416.588	738.114
431.915	733.004
436.847	731.36
451.844	726.36
513.894	705.674
558.556	690.725
562.759	689.359
568.914	689.195
585.003	689.21
626.3	689.209
629.988	689.233
707.208	689.738
708.693	689.748
744.952	690.009
756.167	690.114
784.667	690.532
988.859	892.614

Method: janbu corrected

X	Y
379.807	751.429
385.472	748.322
387.637	747.627
399.031	743.967
399.528	743.801
407.968	740.973
408.584	740.754
416.588	738.114
431.915	733.004
436.847	731.36
451.844	726.36
513.894	705.674
558.556	690.725
562.759	689.359
568.914	689.195
585.003	689.21
626.3	689.209
629.988	689.233
707.208	689.738
708.693	689.748
744.952	690.009
756.167	690.114
784.667	690.532
988.859	892.614

Method: spencer

X	Y
377.841	750.794
394.172	745.506
399.813	743.612
426.464	734.663
452.359	726.032
462.375	722.692
477.663	717.594
493.059	712.462
510.815	706.553
533.924	698.812
562.715	689.36
568.522	689.206
627.187	689.214
627.411	689.216
637.881	689.284
655.076	689.397
672.401	689.51
708.37	689.746
740.546	689.968
757.283	690.125
777.945	690.317
788.982	690.573
806.397	703.727
823.045	716.586
839.415	729.448
854.3	741.359
870.459	754.565
886.772	768.245
902.655	781.974
916.524	794.198
930.368	806.401
944.754	819.055
959.143	831.671
973.518	844.204
988.065	856.769
1006.96	872.813
1027.91	890.79
1032.14	896

Method: gle/morgenstern-price

X	Y
379.882	751.453
385.463	748.326
397.03	744.592
399.38	743.851
408.361	740.855
408.761	740.723
433.228	732.471
434.473	732.04
434.607	731.994
446.355	728.032
513.134	705.771
561.509	689.731
562.776	689.358
571.116	689.196
593.103	689.209
626.333	689.21
635.763	689.271
706.975	689.737
714.251	689.784
744.385	690.004
756.331	690.116
809.901	690.767
1043.61	895.494

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4772
 Number of Invalid Surfaces: 232

Error Codes

Error Code -108 reported for 6 surfaces
 Error Code -110 reported for 226 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4777
 Number of Invalid Surfaces: 227

Error Codes

Error Code -108 reported for 1 surface
 Error Code -110 reported for 226 surfaces

Method: spencer

Number of Valid Surfaces: 4338
 Number of Invalid Surfaces: 666

Error Codes

Error Code -108 reported for 61 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 379 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4149
 Number of Invalid Surfaces: 856

Error Codes

Error Code -108 reported for 74 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 556 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072

1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749

307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281

External Boundary

0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56

2020.89,	750.238
2018.62,	749.686
2006.76,	746.88
2004.23,	746.282
1997.83,	744.822
1995.37,	744.264
1991.58,	743.44
1989.28,	742.939
1986.9,	742.454
1984.74,	742.012
1983.22,	741.722
1982.37,	741.56
1981.18,	741.335
1980.18,	741.16
1978.26,	740.826
1976.28,	740.516
1975.78,	740.438
1973.88,	740.177
1973.61,	740.14
1971.91,	739.942
1971.78,	739.934
1971.67,	739.928
1970.03,	739.874
1969.97,	739.872
1966.4,	739.837
1962.15,	739.979
1962.12,	739.978
1960.65,	739.995
1960.65,	739.995
1960.64,	739.995
1958.84,	739.961
1958.8,	739.962
1955.97,	739.894
1955.8,	739.899
1952.02,	739.831
1947.2,	739.799
1942.5,	739.789
1940.44,	740
1939.92,	740.009
1935.35,	740.008
1934.96,	740.008
1930.81,	740.006
1930.4,	740.007
1927.26,	740.005
1923.77,	740.001
1923.7,	740.001
1923.26,	740
1923.21,	740
1920.59,	740.087
1917.66,	740.181
1916.72,	740.203
1915.68,	740.226
1898.08,	740.833
1892.28,	741.057
1847.51,	742
1841.58,	742.146
1841.29,	742.151
1840.84,	742.154
1840.05,	742.158

1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

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	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
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	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281 178.514, 731.719 180.265, 732.302 180.685, 732.443 181.358, 732.667 181.974, 732.872 194.152, 736.932 197.551, 738.064 198.448, 738.364 198.554, 738.399 198.861, 738.501 199.339, 738.661 200.776, 739.14 200.997, 739.213 201.651, 739.431 201.708, 739.45 202.375, 739.673 202.96, 739.868 203.066, 739.903 203.951, 740.198 204.163, 740.269 205.695, 740.779 206.113, 740.918 206.403, 741.015 206.704, 741.116 206.992, 741.211 207.162, 741.268 207.356, 741.333 213.598, 743.414 219.127, 745.257	Assigned to:  Wadsworth Till

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

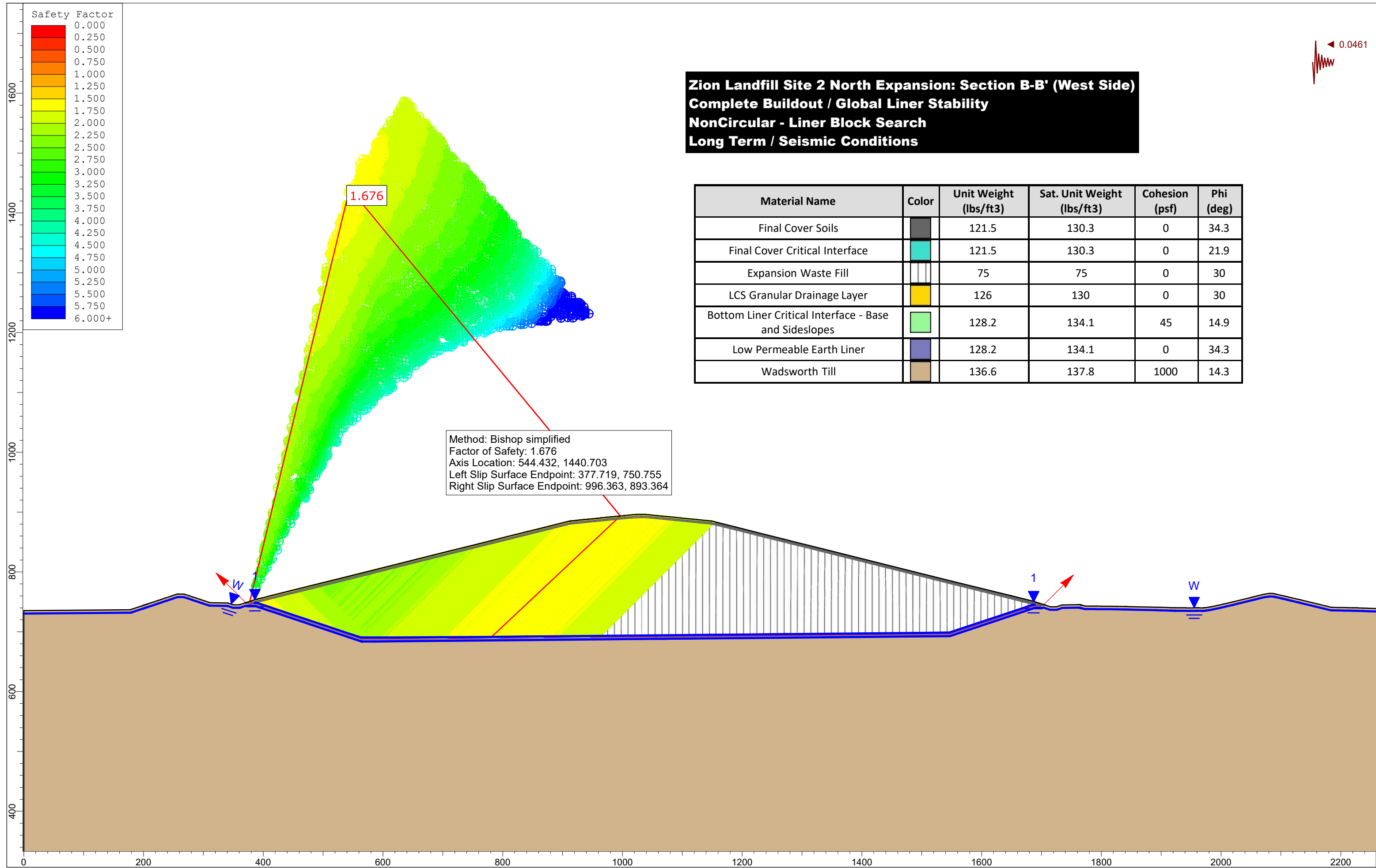
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1841.48, 737.147
1847.4, 737.001
1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
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1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
2022.07, 745.38
2048.08, 751.704
2048.4, 751.784
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2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
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2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
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2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
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2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
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2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
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2114.73, 752.349
2117.29, 751.74
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2131.93, 748.281
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2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
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	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
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1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
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1550.34, 699.997		
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1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

Block Search Polyline

371.35, 747.924
386.046, 748.217
390.079, 746.872
398.881, 743.938
399.765, 743.643
406.945, 741.249
409.081, 740.537
417.36, 737.777
429.79, 733.633
437.541, 731.05
447.072, 727.872
513.038, 705.881
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567.392, 689.269
591.838, 689.285
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697.255, 689.748
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1212.44, 694.424
1487.03, 696.975
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1546.33, 697.528
1550.35, 698.868
1653.04, 733.095
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1695.56, 744.868
1703.46, 744.71



Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Long Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	0	34.3
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	30
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	0	34.3
Wadsworth Till	Brown	136.6	137.8	1000	14.3

Method: Bishop simplified
 Factor of Safety: 1.676
 Axis Location: 544.432, 1440.703
 Left Slip Surface Endpoint: 377.719, 750.755
 Right Slip Surface Endpoint: 996.363, 893.364

Slide Analysis Information

B-B_final_west_LT

Project Summary

File Name:	B-B_final_west_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:13m:20.505s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8

Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.676000
Axis Location:	544.432, 1440.703
Left Slip Surface Endpoint:	377.719, 750.755
Right Slip Surface Endpoint:	996.363, 893.364
Resisting Moment:	1.23473e+09 lb-ft
Driving Moment:	7.36714e+08 lb-ft
Total Slice Area:	57758.4 ft ²
Surface Horizontal Width:	618.644 ft
Surface Average Height:	93.363 ft

Method: janbu corrected

FS	1.689770
Axis Location:	541.909, 1434.570
Left Slip Surface Endpoint:	377.556, 750.702
Right Slip Surface Endpoint:	990.392, 892.767
Resisting Horizontal Force:	1.54616e+06 lb
Driving Horizontal Force:	915015 lb
Total Slice Area:	57435.5 ft ²
Surface Horizontal Width:	612.836 ft
Surface Average Height:	93.7209 ft

Method: spencer

FS	1.778520
Axis Location:	566.584, 1490.203
Left Slip Surface Endpoint:	377.651, 750.733
Right Slip Surface Endpoint:	1044.801, 895.375
Resisting Moment:	1.44072e+09 lb-ft
Driving Moment:	8.10065e+08 lb-ft
Resisting Horizontal Force:	1.63953e+06 lb
Driving Horizontal Force:	921849 lb
Total Slice Area:	63222.6 ft ²
Surface Horizontal Width:	667.149 ft
Surface Average Height:	94.7653 ft

Method: gle/morgenstern-price

FS	1.783480
Axis Location:	561.862, 1481.782
Left Slip Surface Endpoint:	377.868, 750.803
Right Slip Surface Endpoint:	1036.249, 896.000
Resisting Moment:	1.50477e+09 lb-ft
Driving Moment:	8.43723e+08 lb-ft
Resisting Horizontal Force:	1.70362e+06 lb
Driving Horizontal Force:	955222 lb
Total Slice Area:	67428.4 ft ²
Surface Horizontal Width:	658.381 ft
Surface Average Height:	102.415 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
377.719	750.755
387.034	747.827
392.964	745.831
399.028	743.859
399.173	743.811
407.08	741.151
407.937	740.864
410.783	739.911
433.977	732.164
435.866	731.532
450.651	726.6
514.247	705.556
558.552	690.726
562.758	689.359
568.348	689.208
626.559	689.21
628.954	689.226
636.462	689.277
707.001	689.736
708.726	689.748
744.424	690.004
756.492	690.117
780.398	690.492
996.363	893.364

Method: janbu corrected

X	Y
377.556	750.702
384.955	748.488
392.635	746.022
398.789	743.951
399.65	743.661
406.896	741.22
408.906	740.542
416.046	738.136
431.718	732.912
435.144	731.77
453.43	725.674
512.305	706.046
556.258	691.404
562.76	689.359
568.855	689.194
590.401	689.209
627.036	689.214
638.238	689.287
707.235	689.738
709.126	689.751
744.429	690.004
776.778	690.308
782.484	690.514
990.392	892.767

Method: spencer

X	Y
377.651	750.733
382.874	749.183
392.316	746.07
398.459	744.012
400.057	743.477
406.822	741.212
409.781	740.225
418.456	737.333
429.148	733.768
438.569	730.628
444.489	728.655
511.852	706.197
559.288	690.409
562.772	689.358
570.363	689.203
626.43	689.22
630.095	689.234
637.994	689.285
704.92	689.723
710.565	689.76
743.365	689.995
754.337	690.097
787.469	690.557
1044.8	895.375

Method: gle/morgenstern-price

X	Y
377.868	750.803
392.384	746.072
414.47	738.661
435.998	731.485
455.499	724.983
475.009	718.48
494.511	711.978
514.007	705.479
536.228	698.041
560	690.19
562.774	689.358
571.439	689.197
586.673	689.207
629.292	689.228
647.122	689.345
664.567	689.459
681.792	689.573
699.623	689.688
724.003	689.849
744.39	690.004
767.006	690.215
788.43	690.417
804.723	690.568
815.087	690.817
832.346	703.221
848.814	715.428
866.471	728.89
881.483	740.738
895.714	752.386
910.826	765.288
927.655	780.216
945.683	796.683
963.563	813.564
981.004	830.722
997	847.254
1011.18	862.864
1022.99	877.239
1036.25	896

Valid and Invalid Surfaces**Method: bishop simplified**

Number of Valid Surfaces: 4774
Number of Invalid Surfaces: 230

Error Codes

Error Code -108 reported for 4 surfaces
Error Code -110 reported for 226 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4773
 Number of Invalid Surfaces: 231

Error Codes

Error Code -108 reported for 5 surfaces
 Error Code -110 reported for 226 surfaces

Method: spencer

Number of Valid Surfaces: 4391
 Number of Invalid Surfaces: 613

Error Codes

Error Code -108 reported for 65 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 322 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4076
 Number of Invalid Surfaces: 928

Error Codes

Error Code -108 reported for 36 surfaces
 Error Code -110 reported for 226 surfaces
 Error Code -111 reported for 666 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

- 108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).
- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649

1268.54, 855.013
1248.67, 860.028
1228.73, 865.012
1188.81, 874.99
1149.18, 884.939
1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054

395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646

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	200.794, 744.416
	200.126, 744.193
	200.07, 744.175
	199.416, 743.957
	199.195, 743.883
	197.758, 743.404
	197.28, 743.245
	196.972, 743.142
	196.867, 743.107
	195.97, 742.808
	192.571, 741.675
	180.393, 737.616
	179.777, 737.41
	179.104, 737.186
	178.684, 737.046
	177.683, 736.712
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	0, 330
	2300, 329.864
	2300, 738.23
	2246.57, 739.067
	2243.38, 739.234
	2237.66, 739.488
	2236.06, 739.552
	2228.45, 739.866
External Boundary	2227.6, 739.905
	2224.96, 740
	2223.17, 740.005
	2222.91, 740.007
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	2219.67, 740.014
	2212.87, 740.232
	2205.39, 740.356
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	2193.85, 740.574
	2186.16, 740.695
	2184.12, 740.751
	2173.86, 743.268
	2159.21, 746.831
	2152.58, 748.429
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	2133.09, 753.144
	2130.67, 753.722
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	2118.44, 756.605
	2115.88, 757.214
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	2110.94, 758.385
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	2106.6, 759.395
	2105.52, 759.642
	2103.96, 760
	2102.2, 760.402
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	2091.21, 762.906
	2090.82, 762.992

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1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278

	1733.77, 744.737 1728.36, 742.936 1724.77, 741.738 1718.76, 741.738 1714.77, 741.738 1709.37, 743.539
Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157

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	410.125, 741.325
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	440.132, 731.322
	447.096, 729
	503.929, 710.053
	539.503, 697.951
	558.611, 691.762
	558.865, 691.679
	562.929, 690.36
	564.036, 690.348
	567.84, 690.349
	591.835, 690.364
	623.213, 690.364
	626.363, 690.364
	694.207, 690.807
	696.794, 690.824
	748.23, 691.161
	758.969, 691.241
	840.146, 692
Material Boundary	840.75, 692.006
	878.605, 692.355
	1002.59, 693.5
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	1031.69, 693.776
	1031.85, 693.778
	1056.47, 694
	1104.1, 694.443
	1146.52, 694.837
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	1271.75, 696
	1271.77, 696
	1289.59, 696.166
	1484.09, 698.02
	1487.03, 698.047
	1488.81, 698.064
	1546.16, 698.605
	1550.33, 699.993
	1550.34, 699.997
	1664.79, 738.147
	1685.08, 744.911
	1685.14, 744.929
	1685.15, 744.933
	1685.16, 744.935
	1685.16, 744.936
	1685.18, 744.943
	1685.18, 744.943
	1688.75, 745.08
	1695.33, 744.948
	1695.56, 744.943
	1695.85, 744.937
	1703.67, 744.781
	1705.77, 744.739
	386.057, 748.292
	387.636, 748.688

389.31, 749.106
390.102, 749.304
391.02, 749.533
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394.551, 750.415
396.375, 750.871
398.239, 751.337
400.145, 751.813
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404.089, 752.798
406.128, 753.308
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626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890

Material Boundary

1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

<p>Material Boundary</p>	<p>371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639</p>
	<p>378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43</p>


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
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841.905, 864.458
860.28, 869.051
877.824, 873.437
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913.393, 882.205
922.986, 883.164
923.806, 883.246
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943.81, 885.246
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983.002, 889.164
983.816, 889.246
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1127.74, 884.219
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1148.69, 882.124
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	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
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	685.835, 825.284
	713.449, 832.187
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	760.301, 843.901
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	802.949, 854.563
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	912.998, 882.015
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	1031.85, 893
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	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
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	1122.95, 884.554

	1127.72, 884.07
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	1142.97, 882.553
	1147.91, 882.06
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	1148.66, 881.976
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	1247.94, 857.118
	1267.81, 852.104
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	1335.66, 834.996
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	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
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299.331, 747.189
305.137, 745.256
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308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
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350.337, 739.956
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370.6, 743.203
385.911, 743.07
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404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
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563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
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564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

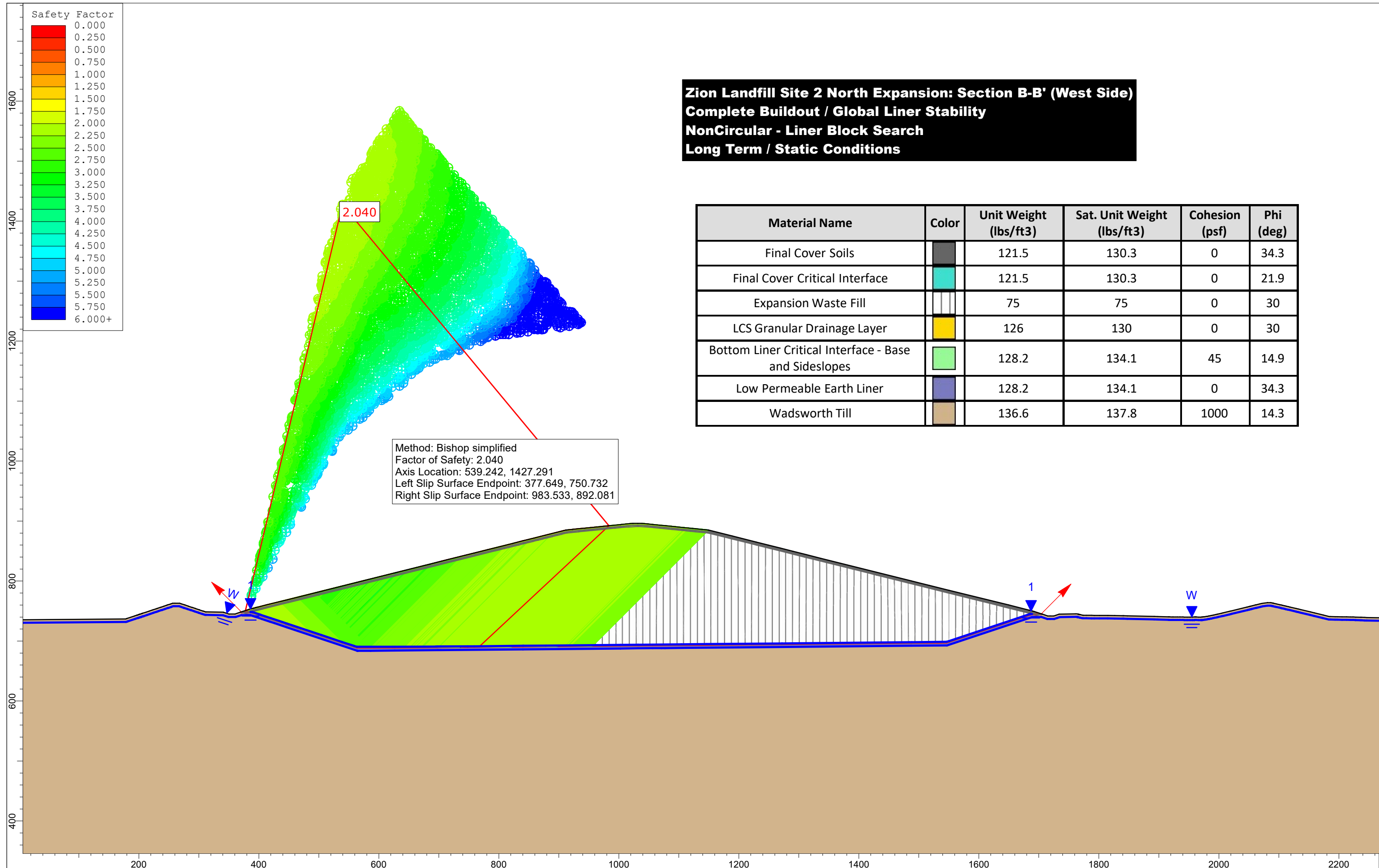
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	1546.99, 692.557	
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	1689.03, 739.9	
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	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
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	1763.72, 740.082	
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	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
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	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
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	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
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	1787.81, 737.786	
	1792.3, 737.722	

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1935.34, 735.008
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2127.76, 749.27
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Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
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Block Search Polyline	371.35, 747.924	
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	399.765, 743.643	
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	409.081, 740.537	
	417.36, 737.777	
	429.79, 733.633	
	437.541, 731.05	
	447.072, 727.872	
	513.038, 705.881	
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	567.392, 689.269	
	591.838, 689.285	
	626.364, 689.285	
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	697.255, 689.748	
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	748.109, 690.114	
	840.147, 690.975	
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1703.46, 744.71		



Slide Analysis Information

B-B_final_west_LT

Project Summary

File Name:	B-B_final_west_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:23m:05.172s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading







Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1000

Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.040230
Axis Location:	539.242, 1427.291
Left Slip Surface Endpoint:	377.649, 750.732
Right Slip Surface Endpoint:	983.533, 892.081
Resisting Moment:	1.1902e+09 lb-ft
Driving Moment:	5.83365e+08 lb-ft
Total Slice Area:	55221.5 ft ²
Surface Horizontal Width:	605.884 ft
Surface Average Height:	91.142 ft

Method: janbu corrected

FS	2.063370
Axis Location:	539.242, 1427.291
Left Slip Surface Endpoint:	377.649, 750.732
Right Slip Surface Endpoint:	983.533, 892.081
Resisting Horizontal Force:	1.53688e+06 lb
Driving Horizontal Force:	744843 lb
Total Slice Area:	55221.5 ft ²
Surface Horizontal Width:	605.884 ft
Surface Average Height:	91.142 ft

Method: spencer

FS	2.173900
Axis Location:	556.239, 1470.307
Left Slip Surface Endpoint:	377.960, 750.833
Right Slip Surface Endpoint:	1024.851, 896.000
Resisting Moment:	1.42446e+09 lb-ft
Driving Moment:	6.55253e+08 lb-ft
Resisting Horizontal Force:	1.64633e+06 lb
Driving Horizontal Force:	757317 lb
Total Slice Area:	61532.6 ft ²
Surface Horizontal Width:	646.891 ft
Surface Average Height:	95.1205 ft

Method: gle/morgenstern-price

FS	2.187820
Axis Location:	556.239, 1470.307
Left Slip Surface Endpoint:	377.960, 750.833
Right Slip Surface Endpoint:	1024.851, 896.000
Resisting Moment:	1.42049e+09 lb-ft
Driving Moment:	6.49271e+08 lb-ft
Resisting Horizontal Force:	1.64238e+06 lb
Driving Horizontal Force:	750694 lb
Total Slice Area:	61532.6 ft ²
Surface Horizontal Width:	646.891 ft
Surface Average Height:	95.1205 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
377.649	750.732
387.166	747.792
392.166	746.198
399.275	743.847
399.735	743.695
406.872	741.328
408.75	740.687
419.95	736.993
430.333	733.531
437.208	731.24
455.697	725.074
514.718	705.398
557.701	691.01
562.666	689.36
569.194	689.195
599.794	689.214
626.425	689.209
637.112	689.279
707.096	689.738
708.563	689.747
744.406	690.004
755.216	690.115
767.076	690.368
983.533	892.081

Method: janbu corrected

X	Y
377.649	750.732
387.166	747.792
392.166	746.198
399.275	743.847
399.735	743.695
406.872	741.328
408.75	740.687
419.95	736.993
430.333	733.531
437.208	731.24
455.697	725.074
514.718	705.398
557.701	691.01
562.666	689.36
569.194	689.195
599.794	689.214
626.425	689.209
637.112	689.279
707.096	689.738
708.563	689.747
744.406	690.004
755.216	690.115
767.076	690.368
983.533	892.081

Method: spencer

X	Y
377.96	750.833
403.577	742.294
430.101	733.451
458.359	724.03
484.022	715.506
513.512	705.643
536.231	698.039
561.452	689.769
561.582	689.728
562.772	689.357
577.318	689.2
586.77	689.206
643.984	689.324
663.841	689.454
693.281	689.647
706.613	689.735
726.241	689.862
744.296	690.003
761.404	690.196
774.647	690.438
804.335	710.414
816.04	718.474
831.237	729.134
846.948	740.435
865.001	753.857
882.102	767.11
897.653	779.746
912.434	792.404
927.666	805.931
942.767	819.572
957.263	832.909
971.03	845.776
983.927	858.011
995.9	869.458
1007.11	880.094
1024.85	896

Method: gle/morgenstern-price

X	Y
377.96	750.833
403.577	742.294
430.101	733.451
458.359	724.03
484.022	715.506
513.512	705.643
536.231	698.039
561.452	689.769
561.582	689.728
562.772	689.357
577.318	689.2
586.77	689.206
643.984	689.324
663.841	689.454
693.281	689.647
706.613	689.735
726.241	689.862
744.296	690.003
761.404	690.196
774.647	690.438
804.335	710.414
816.04	718.474
831.237	729.134
846.948	740.435
865.001	753.857
882.102	767.11
897.653	779.746
912.434	792.404
927.666	805.931
942.767	819.572
957.263	832.909
971.03	845.776
983.927	858.011
995.9	869.458
1007.11	880.094
1024.85	896

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4775
 Number of Invalid Surfaces: 229

Error Codes

Error Code -108 reported for 1 surface
 Error Code -110 reported for 228 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4773

Number of Invalid Surfaces: 231

Error Codes

Error Code -108 reported for 3 surfaces

Error Code -110 reported for 228 surfaces

Method: spencer

Number of Valid Surfaces: 4313

Number of Invalid Surfaces: 691

Error Codes

Error Code -108 reported for 48 surfaces

Error Code -110 reported for 228 surfaces

Error Code -111 reported for 415 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4134

Number of Invalid Surfaces: 870

Error Codes

Error Code -108 reported for 67 surfaces

Error Code -110 reported for 228 surfaces

Error Code -111 reported for 575 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028

1228.73, 865.012
1188.81, 874.99
1149.18, 884.939
1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519

393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416

External Boundary

200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
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2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
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2115.88, 757.214
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2110.94, 758.385
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2106.6, 759.395
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2102.2, 760.402
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2089.21, 763.348

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2086.22, 764
2081.11, 764
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2020.89, 750.238
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1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
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1975.78, 740.438
1973.88, 740.177
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1970.03, 739.874
1969.97, 739.872
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1958.8, 739.962
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1952.02, 739.831
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1934.96, 740.008
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1840.84, 742.154
1840.05, 742.158
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1800.98, 742.614
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1765.27, 744.837
1765.25, 744.842
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1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936

	1724.77, 741.738 1718.76, 741.738 1714.77, 741.738 1709.37, 743.539
Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962

	403.086, 743.672
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	412.226, 740.625
	420.368, 737.91
	432.54, 733.852
	440.132, 731.322
	447.096, 729
	503.929, 710.053
	539.503, 697.951
	558.611, 691.762
	558.865, 691.679
	562.929, 690.36
	564.036, 690.348
	567.84, 690.349
	591.835, 690.364
	623.213, 690.364
	626.363, 690.364
	694.207, 690.807
	696.794, 690.824
	748.23, 691.161
	758.969, 691.241
	840.146, 692
Material Boundary	840.75, 692.006
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	1002.59, 693.5
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	1031.69, 693.776
	1031.85, 693.778
	1056.47, 694
	1104.1, 694.443
	1146.52, 694.837
	1211.99, 695.445
	1271.75, 696
	1271.77, 696
	1289.59, 696.166
	1484.09, 698.02
	1487.03, 698.047
	1488.81, 698.064
	1546.16, 698.605
	1550.33, 699.993
	1550.34, 699.997
	1664.79, 738.147
	1685.08, 744.911
	1685.14, 744.929
	1685.15, 744.933
	1685.16, 744.935
	1685.16, 744.936
	1685.18, 744.943
	1685.18, 744.943
	1688.75, 745.08
	1695.33, 744.948
	1695.56, 744.943
	1695.85, 744.937
	1703.67, 744.781
	1705.77, 744.739
	386.057, 748.292
	387.636, 748.688
	389.31, 749.106
	390.102, 749.304

	391.02, 749.533
	392.766, 749.97
	394.551, 750.415
	396.375, 750.871
	398.239, 751.337
	400.145, 751.813
	402.095, 752.3
	404.089, 752.798
	406.128, 753.308
	411.367, 754.619
	447.096, 763.546
	449.685, 764.193
	564.036, 792.77
	626.363, 808.356
	792.922, 850
	793.256, 850.084
	800.923, 852
	801.257, 852.084
	808.924, 854
	809.258, 854.084
	816.925, 856
	817.259, 856.084
	824.926, 858
	825.261, 858.084
	832.927, 860
	833.262, 860.084
	840.146, 861.804
	840.929, 862
	841.263, 862.084
	848.931, 864
	849.265, 864.083
	856.933, 866
	857.267, 866.083
	864.935, 868
	865.269, 868.083
	872.938, 870
	873.27, 870.083
	880.94, 872
	881.272, 872.083
	888.942, 874
	889.273, 874.083
	896.944, 876
	897.274, 876.082
	904.947, 878
	905.275, 878.082
	912.949, 880.002
	913.77, 880.084
	932.954, 882
	933.767, 882.081
	952.96, 884
	953.77, 884.081
	972.965, 886
	973.774, 886.081
	992.971, 888
	993.788, 888.082
	1002.59, 888.961
	1012.97, 890
	1015.9, 890
	1031.85, 890.91
Material Boundary	

1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

<p>Material Boundary</p>	<p>371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639</p>
	<p>378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43</p>


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281 178.514, 731.719 180.265, 732.302 180.685, 732.443 181.358, 732.667 181.974, 732.872 194.152, 736.932 197.551, 738.064 198.448, 738.364 198.554, 738.399 198.861, 738.501 199.339, 738.661 200.776, 739.14 200.997, 739.213 201.651, 739.431 201.708, 739.45 202.375, 739.673 202.96, 739.868 203.066, 739.903 203.951, 740.198 204.163, 740.269 205.695, 740.779 206.113, 740.918 206.403, 741.015 206.704, 741.116 206.992, 741.211 207.162, 741.268 207.356, 741.333 213.598, 743.414 219.127, 745.257	Assigned to:  Wadsworth Till

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
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	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
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1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
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1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
2022.07, 745.38
2048.08, 751.704
2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
2054.59, 753.317
2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

Block Search Polyline	371.35, 747.924 386.046, 748.217 390.079, 746.872 398.881, 743.938 399.765, 743.643 406.945, 741.249 409.081, 740.537 417.36, 737.777 429.79, 733.633 437.541, 731.05 447.072, 727.872 513.038, 705.881 558.707, 690.597 562.754, 689.282 564.036, 689.269 567.392, 689.269 591.838, 689.285 626.364, 689.285 695.699, 689.738 697.255, 689.748 736.377, 690.004 748.109, 690.114 840.147, 690.975 877.73, 691.322 1002.59, 692.475 1030.76, 692.742 1031.85, 692.753 1033.44, 692.767 1033.47, 692.767 1056.47, 692.975 1104.66, 693.423 1147.07, 693.817 1212.44, 694.424 1487.03, 696.975 1532.11, 697.394 1546.04, 697.525 1546.33, 697.528 1550.35, 698.868 1653.04, 733.095 1688.76, 745.005 1695.56, 744.868 1703.46, 744.71	
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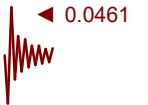
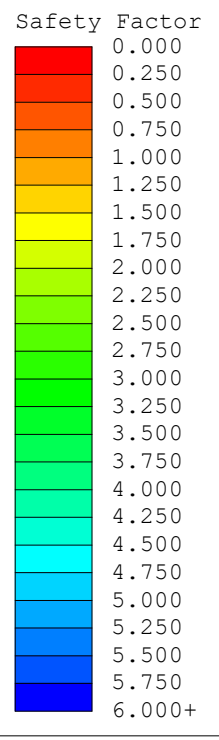
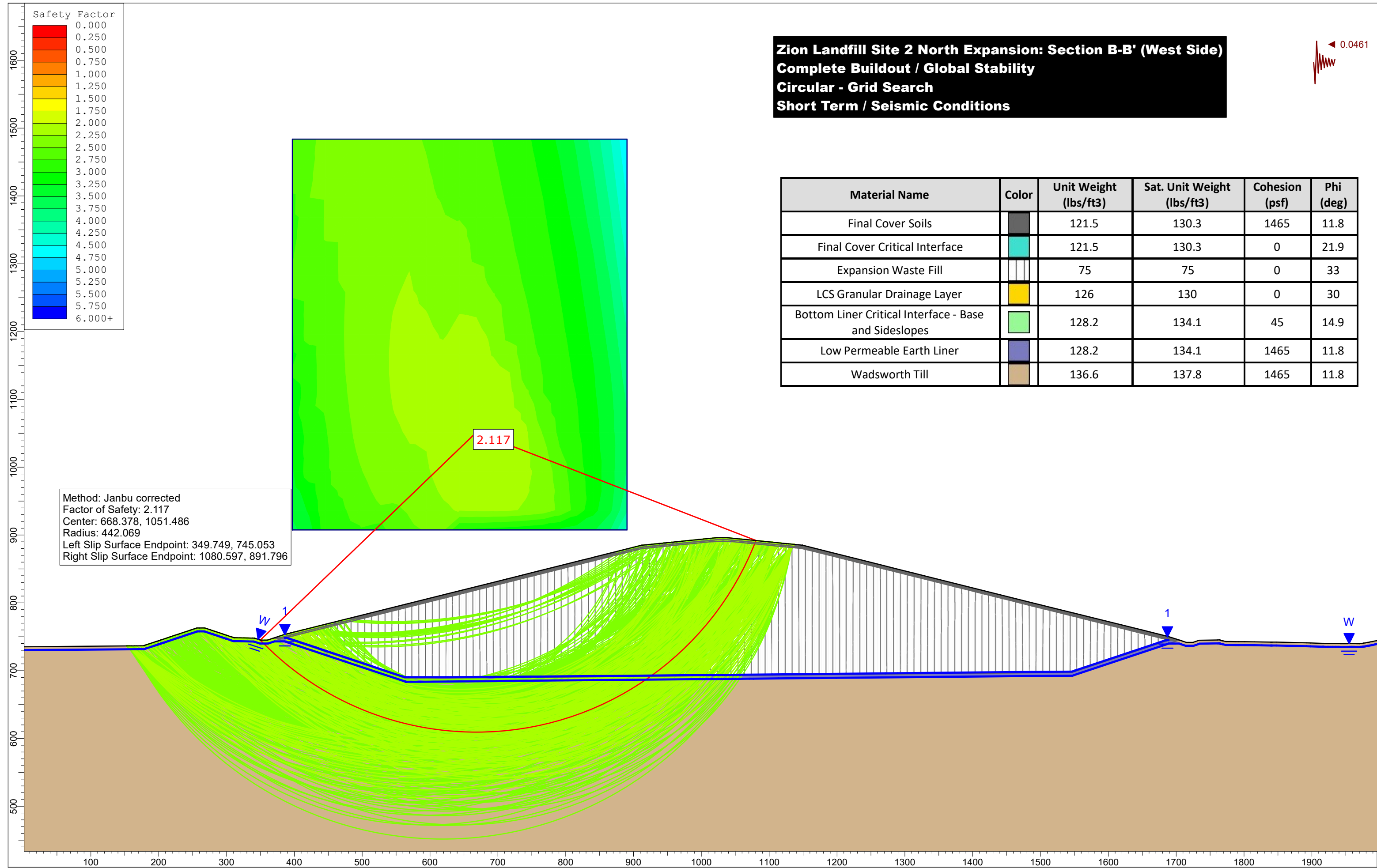
SLOPE STABILITY
SECTION **B-B'** - **HORIZONTAL** EXPANSION (**WEST SLOPE**)

GLOBAL LINER STABILITY ANALYSIS

**COMPLETE BUILD-OUT / FINAL LANDFORM
CIRCULAR ANALYSIS OF WASTE AND FOUNDATION
(ROTATIONAL SLOPE FAILURE)**

Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Stability
Circular - Grid Search
Short Term / Seismic Conditions

0.0461

Method: Janbu corrected
 Factor of Safety: 2.117
 Center: 668.378, 1051.486
 Radius: 442.069
 Left Slip Surface Endpoint: 349.749, 745.053
 Right Slip Surface Endpoint: 1080.597, 891.796

Material Name	Color	Unit Weight (lbs/ft ³)	Sat. Unit Weight (lbs/ft ³)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	1465	11.8
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	33
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	1465	11.8
Wadsworth Till	Brown	136.6	137.8	1465	11.8

2.117

Slide Analysis Information

B-B_final_west_ST

Project Summary

File Name:	B-B_final_west_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:09.797s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
	Analysis Methods Used
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m_{\alpha} < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.293920
Center:	668.378, 1166.761
Radius:	545.507
Left Slip Surface Endpoint:	318.286, 748.414
Right Slip Surface Endpoint:	1136.227, 886.234
Resisting Moment:	2.14635e+09 lb-ft
Driving Moment:	9.35667e+08 lb-ft
Total Slice Area:	122816 ft ²
Surface Horizontal Width:	817.94 ft
Surface Average Height:	150.153 ft

Method: janbu corrected

FS	2.116930
Center:	668.378, 1051.486
Radius:	442.069
Left Slip Surface Endpoint:	349.749, 745.053
Right Slip Surface Endpoint:	1080.597, 891.796
Resisting Horizontal Force:	3.42353e+06 lb
Driving Horizontal Force:	1.61722e+06 lb
Total Slice Area:	117562 ft ²
Surface Horizontal Width:	730.848 ft
Surface Average Height:	160.857 ft

Method: spencer

FS	2.255600
Center:	668.378, 1137.942
Radius:	509.361
Left Slip Surface Endpoint:	340.710, 747.964
Right Slip Surface Endpoint:	1112.539, 888.603
Resisting Moment:	1.84966e+09 lb-ft
Driving Moment:	8.20034e+08 lb-ft
Resisting Horizontal Force:	3.22567e+06 lb
Driving Horizontal Force:	1.43007e+06 lb
Total Slice Area:	112513 ft ²
Surface Horizontal Width:	771.829 ft
Surface Average Height:	145.774 ft

Method: gle/morgenstern-price

FS	2.258740
Center:	668.378, 1137.942
Radius:	509.361
Left Slip Surface Endpoint:	340.710, 747.964
Right Slip Surface Endpoint:	1112.539, 888.603
Resisting Moment:	1.85224e+09 lb-ft
Driving Moment:	8.20034e+08 lb-ft
Resisting Horizontal Force:	3.22712e+06 lb
Driving Horizontal Force:	1.42873e+06 lb
Total Slice Area:	112513 ft ²
Surface Horizontal Width:	771.829 ft
Surface Average Height:	145.774 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4527
Number of Invalid Surfaces:	324

Error Codes

Error Code -110 reported for 92 surfaces
 Error Code -112 reported for 142 surfaces
 Error Code -115 reported for 90 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4554
Number of Invalid Surfaces:	297

Error Codes

Error Code -108 reported for 1 surface
 Error Code -110 reported for 92 surfaces
 Error Code -112 reported for 114 surfaces
 Error Code -115 reported for 90 surfaces

Method: spencer

Number of Valid Surfaces:	4509
Number of Invalid Surfaces:	342

Error Codes

Error Code -108 reported for 1 surface
 Error Code -110 reported for 92 surfaces
 Error Code -111 reported for 16 surfaces
 Error Code -112 reported for 143 surfaces
 Error Code -115 reported for 90 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4524

Number of Invalid Surfaces: 327

Error Codes

Error Code -108 reported for 1 surface

Error Code -110 reported for 92 surfaces

Error Code -112 reported for 144 surfaces

Error Code -115 reported for 90 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-}\alpha = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888

1107.98,	889.059
1098.56,	890
1087.91,	891.065
1078.56,	892
1067.84,	893.072
1058.55,	894
1056.47,	894.208
1047.76,	895.079
1038.54,	896
1031.85,	896
1031.28,	896
1024.07,	896
1022.73,	896
1003.75,	894.103
1002.72,	894
1002.59,	893.986
983.533,	892.081
982.719,	892
963.529,	890.082
962.713,	890
943.526,	888.082
942.708,	888
923.523,	886.082
922.702,	886
913.109,	885.041
912.699,	885
912.114,	884.941
912.089,	884.939
877.133,	876.202
859.589,	871.816
841.214,	867.222
840.146,	866.956
822.141,	862.454
802.222,	857.474
781.387,	852.265
759.573,	846.811
736.709,	841.095
712.721,	835.098
685.108,	828.195
626.363,	813.508
564.036,	797.924
447.096,	768.701
412.275,	760
398.83,	756.64
397.597,	756.33
396.492,	756.054
395.41,	755.784
394.351,	755.519
393.315,	755.26
392.3,	755.007
391.307,	754.759
390.334,	754.516
390.102,	754.458
386.057,	753.446
369.041,	747.954
360.315,	745.046
360.041,	744.955
350.343,	744.956
350.039,	744.956

341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616

External Boundary

179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758

2051.46,	757.69
2050.07,	757.348
2049.83,	757.288
2047.2,	756.637
2046.89,	756.56
2020.89,	750.238
2018.62,	749.686
2006.76,	746.88
2004.23,	746.282
1997.83,	744.822
1995.37,	744.264
1991.58,	743.44
1989.28,	742.939
1986.9,	742.454
1984.74,	742.012
1983.22,	741.722
1982.37,	741.56
1981.18,	741.335
1980.18,	741.16
1978.26,	740.826
1976.28,	740.516
1975.78,	740.438
1973.88,	740.177
1973.61,	740.14
1971.91,	739.942
1971.78,	739.934
1971.67,	739.928
1970.03,	739.874
1969.97,	739.872
1966.4,	739.837
1962.15,	739.979
1962.12,	739.978
1960.65,	739.995
1960.65,	739.995
1960.64,	739.995
1958.84,	739.961
1958.8,	739.962
1955.97,	739.894
1955.8,	739.899
1952.02,	739.831
1947.2,	739.799
1942.5,	739.789
1940.44,	740
1939.92,	740.009
1935.35,	740.008
1934.96,	740.008
1930.81,	740.006
1930.4,	740.007
1927.26,	740.005
1923.77,	740.001
1923.7,	740.001
1923.26,	740
1923.21,	740
1920.59,	740.087
1917.66,	740.181
1916.72,	740.203
1915.68,	740.226
1898.08,	740.833
1892.28,	741.057

1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

<p>Material Boundary</p>	<p>371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639</p>
	<p>378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43</p>


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
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	1147.91, 882.06
	1147.94, 882.048
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	1148.66, 881.976
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	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
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564.361, 683.575
564.491, 683.575
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586.189, 683.61
626.363, 684

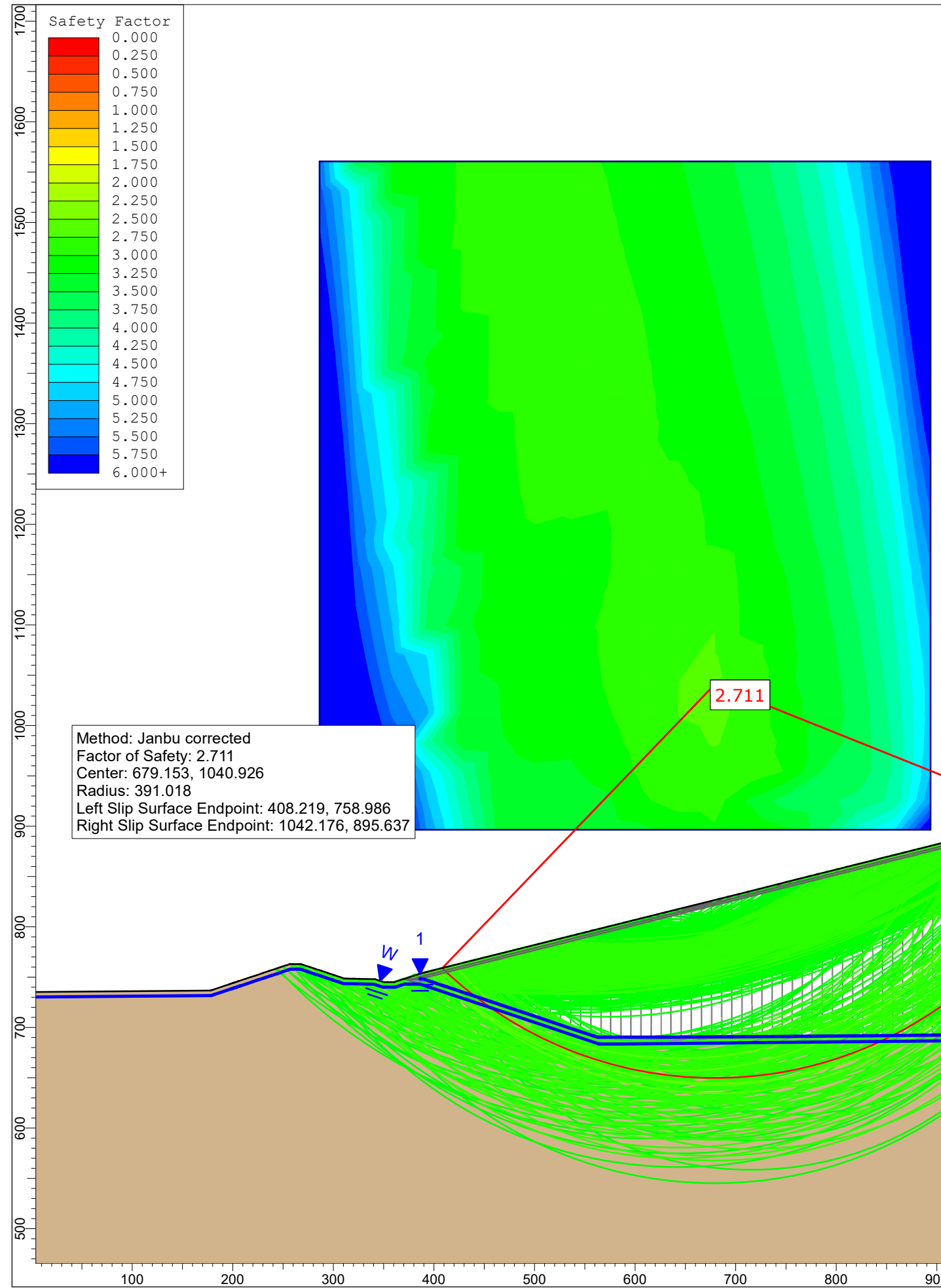
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	703.436, 684.721	
	840.146, 686	
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	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
2022.07, 745.38
2048.08, 751.704
2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
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2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
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2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Liner Stability
Circular - Grid Search
Short Term / Static Conditions



Method: Janbu corrected
 Factor of Safety: 2.711
 Center: 679.153, 1040.926
 Radius: 391.018
 Left Slip Surface Endpoint: 408.219, 758.986
 Right Slip Surface Endpoint: 1042.176, 895.637

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8

Slide Analysis Information

B-B_final_west_ST

Project Summary

File Name:	B-B_final_west_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:08.829s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No






Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	2.834820
Center:	464.751, 1560.631	
Radius:	815.796	
Left Slip Surface Endpoint:	374.792, 749.810	
Right Slip Surface Endpoint:	923.568, 886.087	
Resisting Moment:	8.70461e+08 lb-ft	
Driving Moment:	3.0706e+08 lb-ft	
Total Slice Area:	19850.7 ft ²	
Surface Horizontal Width:	548.776 ft	
Surface Average Height:	36.1727 ft	

Method: janbu corrected

	FS	2.710950
Center:	679.153, 1040.926	
Radius:	391.018	
Left Slip Surface Endpoint:	408.219, 758.986	
Right Slip Surface Endpoint:	1042.176, 895.637	
Resisting Horizontal Force:	2.79708e+06 lb	
Driving Horizontal Force:	1.03177e+06 lb	
Total Slice Area:	84204.3 ft ²	
Surface Horizontal Width:	633.957 ft	
Surface Average Height:	132.823 ft	

Method: spencer

	FS	2.831730
Center:	464.751, 1560.631	
Radius:	815.796	
Left Slip Surface Endpoint:	374.792, 749.810	
Right Slip Surface Endpoint:	923.568, 886.087	
Resisting Moment:	8.69513e+08 lb-ft	
Driving Moment:	3.0706e+08 lb-ft	
Resisting Horizontal Force:	1.02219e+06 lb	
Driving Horizontal Force:	360978 lb	
Total Slice Area:	19850.7 ft ²	
Surface Horizontal Width:	548.776 ft	
Surface Average Height:	36.1727 ft	

Method: gle/morgenstern-price

FS	2.834960
Center:	464.751, 1560.631
Radius:	815.796
Left Slip Surface Endpoint:	374.792, 749.810
Right Slip Surface Endpoint:	923.568, 886.087
Resisting Moment:	8.70503e+08 lb-ft
Driving Moment:	3.0706e+08 lb-ft
Resisting Horizontal Force:	1.02329e+06 lb
Driving Horizontal Force:	360955 lb
Total Slice Area:	19850.7 ft ²
Surface Horizontal Width:	548.776 ft
Surface Average Height:	36.1727 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4166
Number of Invalid Surfaces:	586

Error Codes

Error Code -107 reported for 76 surfaces
 Error Code -110 reported for 125 surfaces
 Error Code -112 reported for 126 surfaces
 Error Code -115 reported for 259 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4181
Number of Invalid Surfaces:	571

Error Codes

Error Code -107 reported for 76 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 125 surfaces
 Error Code -112 reported for 109 surfaces
 Error Code -115 reported for 259 surfaces

Method: spencer

Number of Valid Surfaces:	4158
Number of Invalid Surfaces:	594

Error Codes

Error Code -107 reported for 76 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 125 surfaces
 Error Code -111 reported for 4 surfaces
 Error Code -112 reported for 128 surfaces
 Error Code -115 reported for 259 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4162

Number of Invalid Surfaces: 590

Error Codes

Error Code -107 reported for 76 surfaces

Error Code -108 reported for 2 surfaces

Error Code -110 reported for 125 surfaces

Error Code -111 reported for 2 surfaces

Error Code -112 reported for 126 surfaces

Error Code -115 reported for 259 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-107 = Total driving moment or total driving force is negative. This will occur if the wrong failure direction is specified, or if high external or anchor loads are applied against the failure direction.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi)/F) < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539

1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954

360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142

External Boundary

196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334

2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
1960.65, 739.995
1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
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1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001
1923.26, 740
1923.21, 740
1920.59, 740.087
1917.66, 740.181

1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary	386.057, 748.292
	390.102, 746.943
	398.905, 744.009
	399.789, 743.714
	406.969, 741.321
	409.105, 740.609
	417.384, 737.849
	429.814, 733.705
	437.565, 731.121
	447.096, 727.943
	513.062, 705.952
	558.73, 690.668
	562.767, 689.357
	564.036, 689.344
	567.392, 689.344
	591.838, 689.36
	626.363, 689.36
	695.698, 689.813
	697.254, 689.823
	736.377, 690.079
	748.109, 690.189
	840.146, 691.05
	877.729, 691.397
	1002.59, 692.55
	1030.76, 692.817
	1031.85, 692.828
	1033.44, 692.842
	1033.47, 692.842
	1056.47, 693.05
	1104.66, 693.498
1147.07, 693.892	
1212.44, 694.499	
1487.03, 697.05	
1532.11, 697.469	
1546.04, 697.6	
1546.32, 697.603	
1550.33, 698.939	
1653.01, 733.166	
1688.75, 745.08	

<p>Material Boundary</p>	<p>371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639</p>
	<p>378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43</p>


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

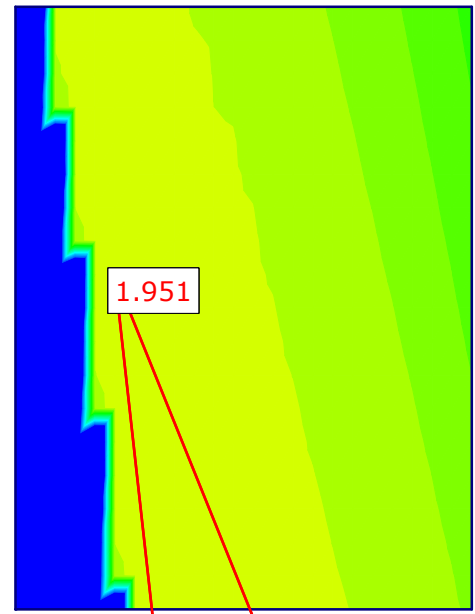
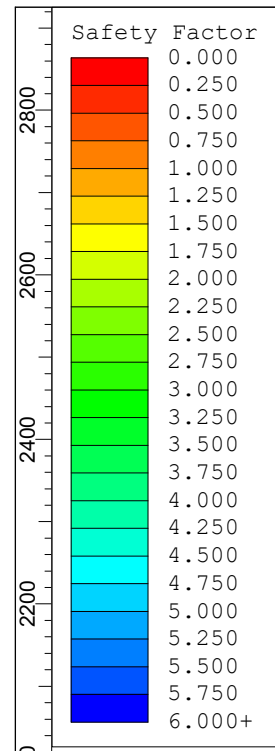
223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
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	1763.89, 740.024	
	1764, 739.989	
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	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

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1838.69, 737.163
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1840.81, 737.155
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2086.73, 758.77
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2088.96, 758.282
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2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

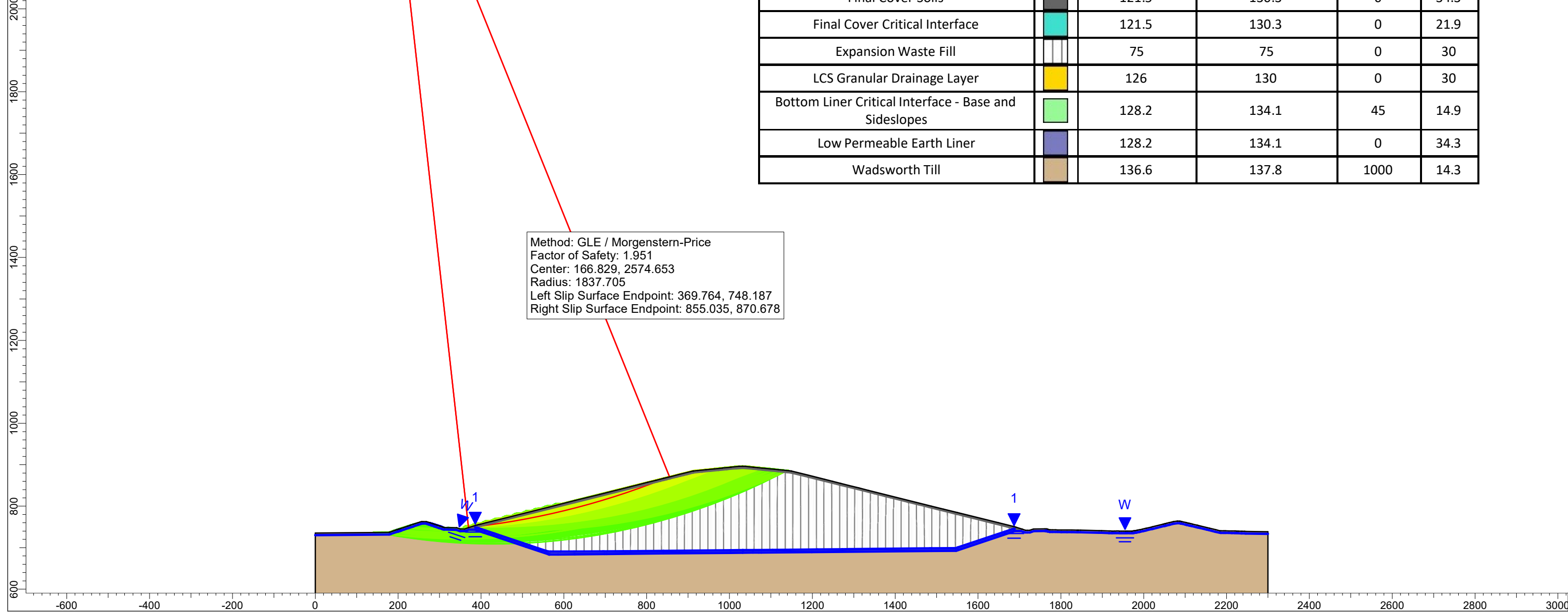


Zion Landfill Site 2 North Expansion: Section B-B' (West Side)
Complete Buildout / Global Stability
Circular - Grid Search
Long Term / Seismic Conditions

0.0461

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3

Method: GLE / Morgenstern-Price
 Factor of Safety: 1.951
 Center: 166.829, 2574.653
 Radius: 1837.705
 Left Slip Surface Endpoint: 369.764, 748.187
 Right Slip Surface Endpoint: 855.035, 870.678



Slide Analysis Information

B-B_final_west_LT

Project Summary

File Name:	B-B_final_west_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:09.381s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.954990
Center:	166.829, 2574.653
Radius:	1837.705
Left Slip Surface Endpoint:	369.764, 748.187
Right Slip Surface Endpoint:	855.035, 870.678
Resisting Moment:	5.73103e+08 lb-ft
Driving Moment:	2.93149e+08 lb-ft
Total Slice Area:	5990.16 ft ²
Surface Horizontal Width:	485.271 ft
Surface Average Height:	12.344 ft

Method: janbu corrected

FS	1.965850
Center:	166.829, 2574.653
Radius:	1837.705
Left Slip Surface Endpoint:	369.764, 748.187
Right Slip Surface Endpoint:	855.035, 870.678
Resisting Horizontal Force:	304922 lb
Driving Horizontal Force:	155109 lb
Total Slice Area:	5990.16 ft ²
Surface Horizontal Width:	485.271 ft
Surface Average Height:	12.344 ft

Method: spencer

FS	1.952320
Center:	166.829, 2574.653
Radius:	1837.705
Left Slip Surface Endpoint:	369.764, 748.187
Right Slip Surface Endpoint:	855.035, 870.678
Resisting Moment:	5.72319e+08 lb-ft
Driving Moment:	2.93149e+08 lb-ft
Resisting Horizontal Force:	302035 lb
Driving Horizontal Force:	154706 lb
Total Slice Area:	5990.16 ft ²
Surface Horizontal Width:	485.271 ft
Surface Average Height:	12.344 ft

Method: gle/morgenstern-price

FS	1.951040
Center:	166.829, 2574.653
Radius:	1837.705
Left Slip Surface Endpoint:	369.764, 748.187
Right Slip Surface Endpoint:	855.035, 870.678
Resisting Moment:	5.71944e+08 lb-ft
Driving Moment:	2.93149e+08 lb-ft
Resisting Horizontal Force:	301937 lb
Driving Horizontal Force:	154757 lb
Total Slice Area:	5990.16 ft ²
Surface Horizontal Width:	485.271 ft
Surface Average Height:	12.344 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	3894
Number of Invalid Surfaces:	1122

Error Codes

Error Code -106 reported for 22 surfaces
 Error Code -110 reported for 167 surfaces
 Error Code -115 reported for 933 surfaces

Method: janbu corrected

Number of Valid Surfaces:	3894
Number of Invalid Surfaces:	1122

Error Codes

Error Code -106 reported for 22 surfaces
 Error Code -110 reported for 167 surfaces
 Error Code -115 reported for 933 surfaces

Method: spencer

Number of Valid Surfaces:	3894
Number of Invalid Surfaces:	1122

Error Codes

Error Code -106 reported for 22 surfaces
 Error Code -110 reported for 167 surfaces
 Error Code -115 reported for 933 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	3894
Number of Invalid Surfaces:	1122

Error Codes

Error Code -106 reported for 22 surfaces
 Error Code -110 reported for 167 surfaces
 Error Code -115 reported for 933 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072
	1058.55, 894
	1056.47, 894.208
	1047.76, 895.079
	1038.54, 896
	1031.85, 896
	1031.28, 896
	1024.07, 896
	1022.73, 896
	1003.75, 894.103
	1002.72, 894

1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
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712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
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397.597, 756.33
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393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
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360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
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311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
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300.708, 752
300.446, 752.087
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294.195, 754.168
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288.155, 756.178
282.681, 758

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275.984, 760.229
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267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
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246.36, 759.605
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236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
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External Boundary

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1960.65, 739.995
1960.64, 739.995
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1958.8, 739.962
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1955.8, 739.899
1952.02, 739.831
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1942.5, 739.789
1940.44, 740
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1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001
1923.26, 740
1923.21, 740
1920.59, 740.087
1917.66, 740.181
1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777

1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
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1252.48, 854.135
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1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

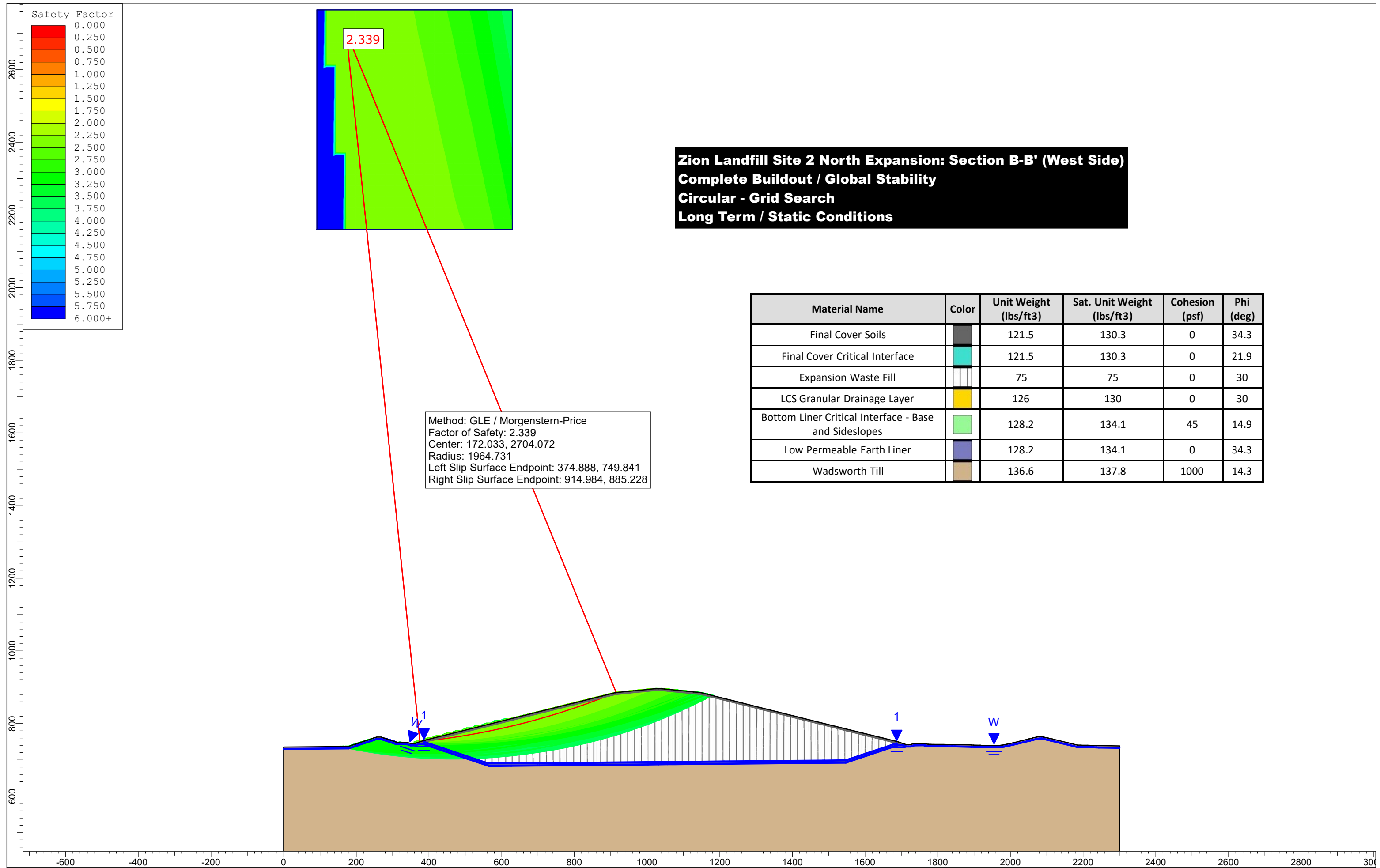
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225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
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	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
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1841.22, 737.151
1841.48, 737.147
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1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
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2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
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2072.75, 757.81
2081.18, 759
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2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
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2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		



Slide Analysis Information

B-B_final_west_LT

Project Summary

File Name:	B-B_final_west_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:07.738s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Right to Left

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.342460
Center:	172.033, 2704.072
Radius:	1964.731
Left Slip Surface Endpoint:	374.888, 749.841
Right Slip Surface Endpoint:	914.984, 885.228
Resisting Moment:	7.7592e+08 lb-ft
Driving Moment:	3.31241e+08 lb-ft
Total Slice Area:	7692.55 ft ²
Surface Horizontal Width:	540.095 ft
Surface Average Height:	14.243 ft

Method: janbu corrected

FS	2.355780
Center:	172.033, 2704.072
Radius:	1964.731
Left Slip Surface Endpoint:	374.888, 749.841
Right Slip Surface Endpoint:	914.984, 885.228
Resisting Horizontal Force:	386256 lb
Driving Horizontal Force:	163961 lb
Total Slice Area:	7692.55 ft ²
Surface Horizontal Width:	540.095 ft
Surface Average Height:	14.243 ft

Method: spencer

FS	2.340410
Center:	172.033, 2704.072
Radius:	1964.731
Left Slip Surface Endpoint:	374.888, 749.841
Right Slip Surface Endpoint:	914.984, 885.228
Resisting Moment:	7.75241e+08 lb-ft
Driving Moment:	3.31241e+08 lb-ft
Resisting Horizontal Force:	382440 lb
Driving Horizontal Force:	163407 lb
Total Slice Area:	7692.55 ft ²
Surface Horizontal Width:	540.095 ft
Surface Average Height:	14.243 ft

Method: gle/morgenstern-price

FS	2.338690
Center:	172.033, 2704.072
Radius:	1964.731
Left Slip Surface Endpoint:	374.888, 749.841
Right Slip Surface Endpoint:	914.984, 885.228
Resisting Moment:	7.74671e+08 lb-ft
Driving Moment:	3.31241e+08 lb-ft
Resisting Horizontal Force:	382390 lb
Driving Horizontal Force:	163506 lb
Total Slice Area:	7692.55 ft ²
Surface Horizontal Width:	540.095 ft
Surface Average Height:	14.243 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	3842
Number of Invalid Surfaces:	1009

Error Codes

Error Code -106 reported for 1 surface
 Error Code -110 reported for 175 surfaces
 Error Code -115 reported for 833 surfaces

Method: janbu corrected

Number of Valid Surfaces:	3842
Number of Invalid Surfaces:	1009

Error Codes

Error Code -106 reported for 1 surface
 Error Code -110 reported for 175 surfaces
 Error Code -115 reported for 833 surfaces

Method: spencer

Number of Valid Surfaces:	3842
Number of Invalid Surfaces:	1009

Error Codes

Error Code -106 reported for 1 surface
 Error Code -110 reported for 175 surfaces
 Error Code -115 reported for 833 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	3842
Number of Invalid Surfaces:	1009

Error Codes

Error Code -106 reported for 1 surface
 Error Code -110 reported for 175 surfaces
 Error Code -115 reported for 833 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072
	1058.55, 894
	1056.47, 894.208
	1047.76, 895.079
	1038.54, 896
	1031.85, 896
	1031.28, 896
	1024.07, 896
	1022.73, 896
	1003.75, 894.103
	1002.72, 894

1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758

282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
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2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740

External Boundary

2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
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2159.21, 746.831
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2128.9, 754.137
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2110.94, 758.385
2109.47, 758.729
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2103.96, 760
2102.2, 760.402
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2091.21, 762.906
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2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
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2049.83, 757.288
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2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
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2004.23, 746.282
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1995.37, 744.264
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1984.74, 742.012

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1975.78, 740.438
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1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
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1958.8, 739.962
1955.97, 739.894
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1934.96, 740.008
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1800.98, 742.614
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1773.36, 742.944
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1766.59, 744.396
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1766.11, 744.557
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1765.58, 744.732
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1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
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1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
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1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary

432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
539.503, 697.951
558.611, 691.762
558.865, 691.679
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567.84, 690.349
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696.794, 690.824
748.23, 691.161
758.969, 691.241
840.146, 692
840.75, 692.006
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1002.59, 693.5
1031.66, 693.776
1031.69, 693.776
1031.85, 693.778
1056.47, 694
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1146.52, 694.837
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1271.77, 696
1289.59, 696.166
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1487.03, 698.047
1488.81, 698.064
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1550.34, 699.997
1664.79, 738.147
1685.08, 744.911
1685.14, 744.929
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1685.16, 744.935
1685.16, 744.936
1685.18, 744.943
1685.18, 744.943
1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415
396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
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953.77, 884.081
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973.774, 886.081
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1015.9, 890
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1138.86, 880.946
1138.94, 880.946
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1170.76, 874.561
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1189.01, 870
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1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
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1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary	386.057, 748.292
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	398.905, 744.009
	399.789, 743.714
	406.969, 741.321
	409.105, 740.609
	417.384, 737.849
	429.814, 733.705
	437.565, 731.121
	447.096, 727.943
	513.062, 705.952
	558.73, 690.668
	562.767, 689.357
	564.036, 689.344
	567.392, 689.344
	591.838, 689.36
	626.363, 689.36
	695.698, 689.813
	697.254, 689.823
	736.377, 690.079
	748.109, 690.189
	840.146, 691.05
	877.729, 691.397
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	1033.47, 692.842
	1056.47, 693.05
	1104.66, 693.498
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1212.44, 694.499	
1487.03, 697.05	
1532.11, 697.469	
1546.04, 697.6	
1546.32, 697.603	
1550.33, 698.939	
1653.01, 733.166	
1688.75, 745.08	

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
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912.983, 882.164
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923.806, 883.246
942.991, 885.164
943.81, 885.246
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963.813, 887.246
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983.816, 889.246
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1078.27, 889.164
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1122.97, 884.703
1127.74, 884.219
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1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
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1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

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	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
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	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
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	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
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	912.998, 882.015
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	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
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	963.828, 887.096
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	983.831, 889.096
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Material Boundary	1003.02, 891.015
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	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
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	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
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	1127.72, 884.07
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	1147.91, 882.06
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	1148.66, 881.976
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	1247.94, 857.118
	1267.81, 852.104
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	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

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	703.436, 684.721	
	840.146, 686	
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	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
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	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
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	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
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	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
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1840.81, 737.155
1841.22, 737.151
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1897.9, 735.837
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1955.95, 734.892
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1958.82, 734.96
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1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
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1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

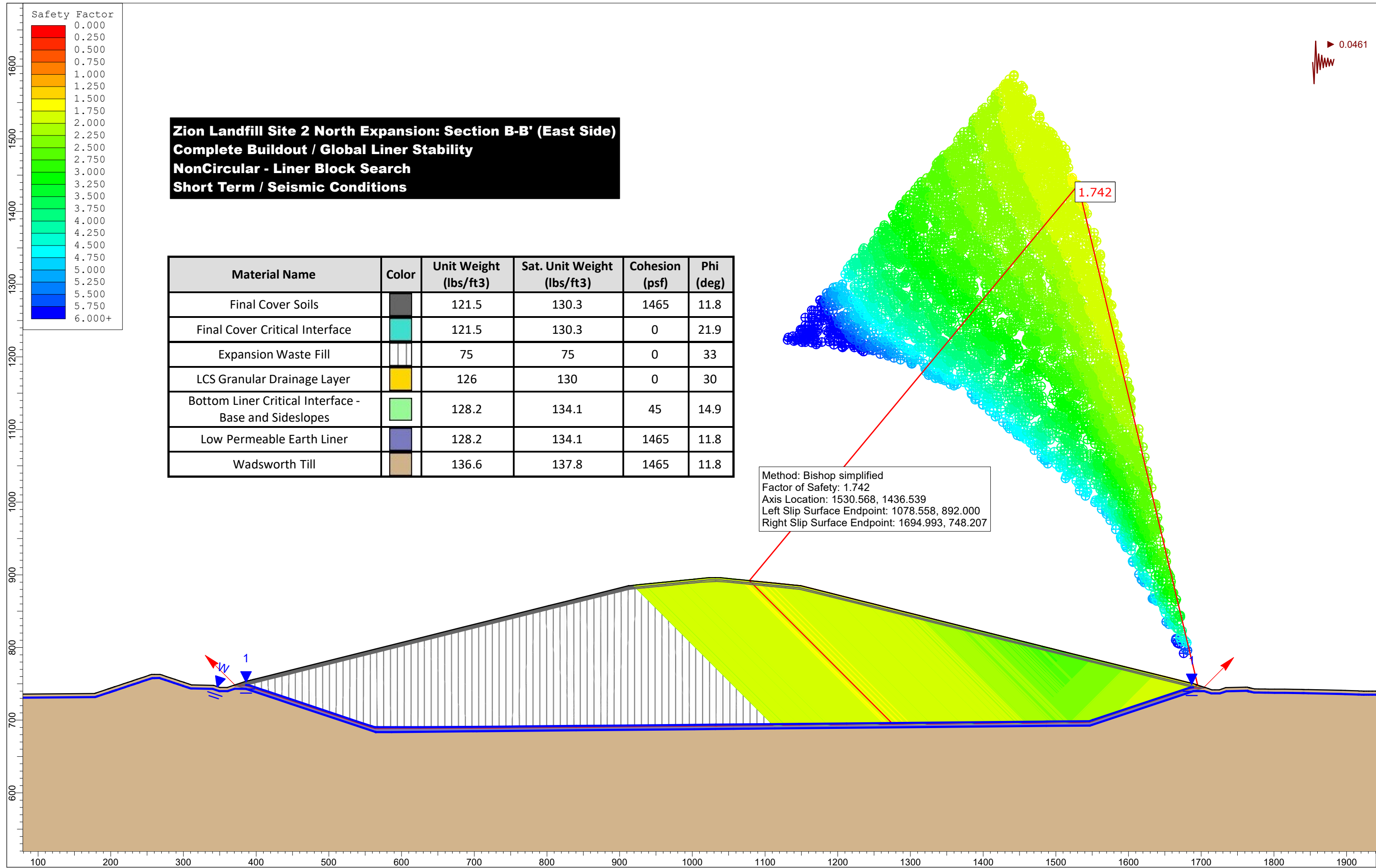
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2135.49, 747.427
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2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
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2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
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	840.75, 692.006	
	878.605, 692.355	
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	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
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1487.03, 698.047		
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1685.16, 744.935		
1685.16, 744.936		
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1688.75, 745.08		

SLOPE STABILITY
SECTION **B-B'** – **HORIZONTAL EXPANSION (EAST SLOPE)**

GLOBAL LINER STABILITY ANALYSIS

**COMPLETE BUILD-OUT / FINAL LANDFORM
BLOCK ANALYSIS OF LINER SYSTEM
(TRANSLATIONAL SLOPE FAILURE)**



Slide Analysis Information

B-B_final_east_ST

Project Summary

File Name:	B-B_final_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:01m:56.878s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8

Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.741940
Axis Location:	1530.568, 1436.539
Left Slip Surface Endpoint:	1078.558, 892.000
Right Slip Surface Endpoint:	1694.993, 748.207
Resisting Moment:	1.23694e+09 lb-ft
Driving Moment:	7.10097e+08 lb-ft
Total Slice Area:	56242.6 ft ²
Surface Horizontal Width:	616.435 ft
Surface Average Height:	91.2384 ft

Method: janbu corrected

FS	1.765060
Axis Location:	1533.217, 1429.509
Left Slip Surface Endpoint:	1085.233, 891.333
Right Slip Surface Endpoint:	1694.967, 748.216
Resisting Horizontal Force:	1.53701e+06 lb
Driving Horizontal Force:	870798 lb
Total Slice Area:	55440 ft ²
Surface Horizontal Width:	609.734 ft
Surface Average Height:	90.9249 ft

Method: spencer

FS	1.828350
Axis Location:	1513.436, 1485.868
Left Slip Surface Endpoint:	1032.940, 896.000
Right Slip Surface Endpoint:	1697.033, 747.551
Resisting Moment:	1.42797e+09 lb-ft
Driving Moment:	7.81018e+08 lb-ft
Resisting Horizontal Force:	1.62618e+06 lb
Driving Horizontal Force:	889426 lb
Total Slice Area:	60172.7 ft ²
Surface Horizontal Width:	664.092 ft
Surface Average Height:	90.609 ft

Method: gle/morgenstern-price

FS	1.832750
Axis Location:	1508.684, 1490.866
Left Slip Surface Endpoint:	1026.420, 896.000
Right Slip Surface Endpoint:	1695.218, 748.135
Resisting Moment:	1.46717e+09 lb-ft
Driving Moment:	8.00529e+08 lb-ft
Resisting Horizontal Force:	1.65424e+06 lb
Driving Horizontal Force:	902602 lb
Total Slice Area:	62648.5 ft ²
Surface Horizontal Width:	668.798 ft
Surface Average Height:	93.6733 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1078.56	892
1275.4	695.084
1495.13	696.975
1540.18	697.394
1543.26	697.474
1546.33	697.607
1549.28	698.588
1652.16	732.726
1689.32	745.112
1694.99	748.207

Method: janbu corrected

X	Y
1085.23	891.333
1277.55	695.104
1492.5	696.95
1540.07	697.394
1543.2	697.473
1546.33	697.607
1550.43	698.945
1652.8	732.936
1689.31	745.112
1694.97	748.216

Method: spencer

X	Y
1032.94	896
1277.28	695.101
1495.1	696.974
1537.63	697.371
1546.27	697.595
1549.63	698.549
1652.41	732.807
1684.99	743.729
1697.03	747.551

Method: gle/morgenstern-price

X	Y
1026.42	896
1259.32	694.935
1495.15	696.975
1539.98	697.391
1543.15	697.484
1546.32	697.607
1552.16	699.39
1654.35	733.495
1690.71	745.573
1695.22	748.135

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4744
 Number of Invalid Surfaces: 260

Error Codes

Error Code -106 reported for 2 surfaces
 Error Code -108 reported for 1 surface
 Error Code -110 reported for 257 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4742
 Number of Invalid Surfaces: 262

Error Codes

Error Code -106 reported for 2 surfaces
 Error Code -108 reported for 3 surfaces
 Error Code -110 reported for 257 surfaces

Method: spencer

Number of Valid Surfaces: 4370

Number of Invalid Surfaces: 635

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 63 surfaces

Error Code -110 reported for 257 surfaces

Error Code -111 reported for 313 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4085

Number of Invalid Surfaces: 919

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 46 surfaces

Error Code -110 reported for 257 surfaces

Error Code -111 reported for 614 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939

1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759

390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
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300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957

External Boundary

199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
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2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764

2080.83, 764
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2062.14, 760.334
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2053.39, 758.173
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2020.89, 750.238
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1995.37, 744.264
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1983.22, 741.722
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1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
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1955.97, 739.894
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1952.02, 739.831
1947.2, 739.799
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1934.96, 740.008
1930.81, 740.006
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1920.59, 740.087
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1841.58, 742.146
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1840.05, 742.158
1838.74, 742.163
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1800.98, 742.614
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1773.36, 742.944
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1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
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1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
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1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738

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Material Boundary	371.139, 747.994
	371.35, 747.924
	371.563, 747.853
	384.979, 743.38
	385.911, 743.07
	390.102, 741.672
	392.178, 740.98
	393.625, 740.498
	395.889, 739.743
	404.687, 736.81
	418.13, 732.329
	426.507, 729.536
	447.096, 722.672
	507.505, 702.534
	563.515, 683.862
	563.605, 683.831
	563.976, 683.705
	564.036, 683.685
	564.179, 683.637
	564.311, 683.592
	564.361, 683.575
	564.491, 683.575
	564.857, 683.576
	586.189, 683.61
	626.363, 684
	698.374, 684.674
	703.436, 684.721
	840.146, 686
	1002.59, 687.5
	1031.85, 687.778
	1056.47, 688
	1070.79, 688.133
	1076.9, 688.19
	1113.49, 688.53
	1198.73, 689.322
	1487.03, 692
	1546.99, 692.557
	1550.33, 693.668
	1688.75, 739.809
	1696.74, 742.47
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1703.64, 744.772	
1703.65, 744.774	
1703.66, 744.776	
1703.67, 744.781	
	369.041, 747.954
	369.564, 747.963
	371.299, 747.998
	378.927, 748.15
	379.447, 748.16
	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625

Material Boundary	420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415

	396.375, 750.871
	398.239, 751.337
	400.145, 751.813
	402.095, 752.3
	404.089, 752.798
	406.128, 753.308
	411.367, 754.619
	447.096, 763.546
	449.685, 764.193
	564.036, 792.77
	626.363, 808.356
	792.922, 850
	793.256, 850.084
	800.923, 852
	801.257, 852.084
	808.924, 854
	809.258, 854.084
	816.925, 856
	817.259, 856.084
	824.926, 858
	825.261, 858.084
	832.927, 860
	833.262, 860.084
	840.146, 861.804
	840.929, 862
	841.263, 862.084
	848.931, 864
	849.265, 864.083
	856.933, 866
	857.267, 866.083
	864.935, 868
	865.269, 868.083
	872.938, 870
	873.27, 870.083
	880.94, 872
	881.272, 872.083
	888.942, 874
	889.273, 874.083
	896.944, 876
	897.274, 876.082
	904.947, 878
	905.275, 878.082
	912.949, 880.002
	913.77, 880.084
	932.954, 882
	933.767, 882.081
	952.96, 884
	953.77, 884.081
	972.965, 886
	973.774, 886.081
	992.971, 888
	993.788, 888.082
	1002.59, 888.961
	1012.97, 890
	1015.9, 890
	1031.85, 890.91
	1032.05, 890.921
	1048.3, 890
	1056.47, 889.183
Material Boundary	

1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
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943.81, 885.246
962.997, 887.164
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983.816, 889.246
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1067.55, 890.236
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1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
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	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
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238.4, 751.681
243.127, 753.257
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249.127, 755.257
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255.127, 757.257
257.07, 757.904
259.897, 757.906
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269.085, 757.256
274.405, 755.485
275.094, 755.256
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281.102, 753.256
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287.111, 751.256
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	1550.33, 693.668	
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	1763.81, 740.051	
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	1763.89, 740.024	
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	1764.14, 739.942	
	1764.53, 739.813	
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	1764.59, 739.79	
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	1765.03, 739.645	
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	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
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	1787.81, 737.786	
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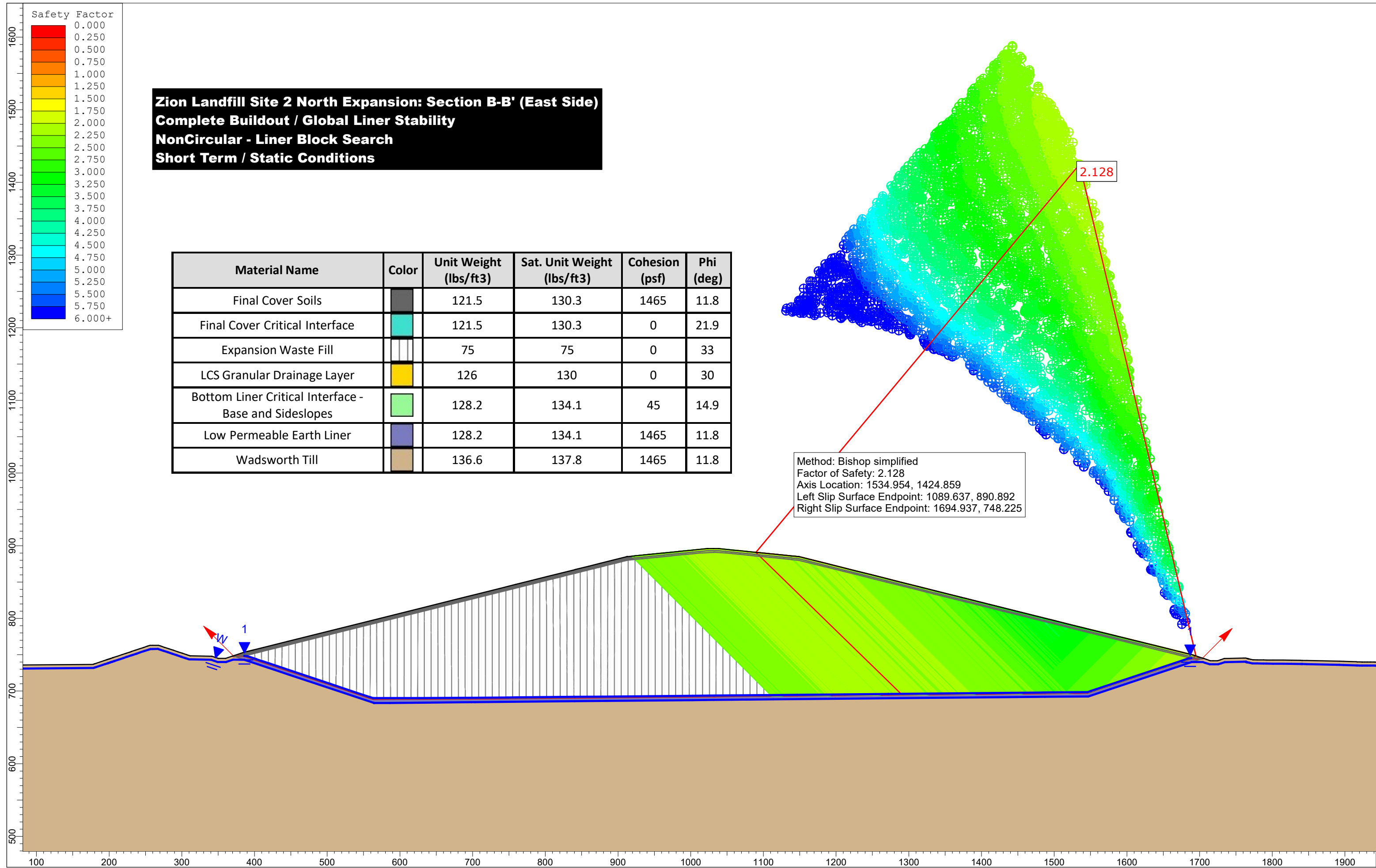
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Block Search Polyline

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Slide Analysis Information

B-B_final_east_ST

Project Summary

File Name:	B-B_final_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:15m:18.148s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465

Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.127690
Axis Location:	1534.954, 1424.859
Left Slip Surface Endpoint:	1089.637, 890.892
Right Slip Surface Endpoint:	1694.937, 748.225
Resisting Moment:	1.20242e+09 lb-ft
Driving Moment:	5.65132e+08 lb-ft
Total Slice Area:	53763.6 ft ²
Surface Horizontal Width:	605.3 ft
Surface Average Height:	88.8214 ft

Method: janbu corrected

FS	2.162230
Axis Location:	1536.880, 1419.764
Left Slip Surface Endpoint:	1094.478, 890.408
Right Slip Surface Endpoint:	1694.924, 748.230
Resisting Horizontal Force:	1.52477e+06 lb
Driving Horizontal Force:	705182 lb
Total Slice Area:	53190.9 ft ²
Surface Horizontal Width:	600.445 ft
Surface Average Height:	88.5858 ft

Method: spencer

FS	2.236230
Axis Location:	1525.697, 1454.849
Left Slip Surface Endpoint:	1062.592, 893.596
Right Slip Surface Endpoint:	1696.836, 747.614
Resisting Moment:	1.33114e+09 lb-ft
Driving Moment:	5.95262e+08 lb-ft
Resisting Horizontal Force:	1.56726e+06 lb
Driving Horizontal Force:	700852 lb
Total Slice Area:	55970.6 ft ²
Surface Horizontal Width:	634.244 ft
Surface Average Height:	88.2477 ft

Method: gle/morgenstern-price

FS	2.246610
Axis Location:	1525.697, 1454.849
Left Slip Surface Endpoint:	1062.592, 893.596
Right Slip Surface Endpoint:	1696.836, 747.614
Resisting Moment:	1.32761e+09 lb-ft
Driving Moment:	5.90939e+08 lb-ft
Resisting Horizontal Force:	1.56405e+06 lb
Driving Horizontal Force:	696184 lb
Total Slice Area:	55970.6 ft ²
Surface Horizontal Width:	634.244 ft
Surface Average Height:	88.2477 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1089.64	890.892
1290.39	695.223
1495.05	696.974
1540.09	697.393
1543.21	697.476
1546.33	697.607
1550.79	699.092
1653.91	733.465
1689.33	745.111
1694.94	748.225

Method: janbu corrected

X	Y
1094.48	890.408
1291.88	695.237
1495.18	696.975
1540.14	697.394
1543.24	697.477
1546.33	697.607
1550.79	699.092
1653.91	733.465
1689.33	745.11
1694.92	748.23

Method: spencer

X	Y
1062.59	893.596
1066.46	888.277
1086.11	872.038
1102.12	858.441
1117.51	845.008
1132.59	831.668
1146.03	819.762
1160.52	806.928
1174.71	794.599
1189.62	782.048
1204.93	769.498
1218.85	758.339
1234	746.407
1248.59	735.091
1265.41	722.243
1292.63	701.811
1301.51	695.327
1331.36	695.454
1359.65	695.717
1382.52	695.928
1404.98	696.137
1432.24	696.39
1450.12	696.557
1472.41	696.764
1495.08	696.975
1517.93	697.186
1546.12	697.542
1549.3	698.56
1575.81	707.275
1592.69	712.901
1612.3	719.437
1627.79	724.602
1640.51	728.841
1653.92	733.31
1668.88	738.297
1680.6	742.205
1696.84	747.614

Method: gle/morgenstern-price

X	Y
1062.59	893.596
1066.46	888.277
1086.11	872.038
1102.12	858.441
1117.51	845.008
1132.59	831.668
1146.03	819.762
1160.52	806.928
1174.71	794.599
1189.62	782.048
1204.93	769.498
1218.85	758.339
1234	746.407
1248.59	735.091
1265.41	722.243
1292.63	701.811
1301.51	695.327
1331.36	695.454
1359.65	695.717
1382.52	695.928
1404.98	696.137
1432.24	696.39
1450.12	696.557
1472.41	696.764
1495.08	696.975
1517.93	697.186
1546.12	697.542
1549.3	698.56
1575.81	707.275
1592.69	712.901
1612.3	719.437
1627.79	724.602
1640.51	728.841
1653.92	733.31
1668.88	738.297
1680.6	742.205
1696.84	747.614

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4737
 Number of Invalid Surfaces: 267

Error Codes

Error Code -106 reported for 2 surfaces
 Error Code -108 reported for 8 surfaces
 Error Code -110 reported for 257 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4745

Number of Invalid Surfaces: 259

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -110 reported for 257 surfaces

Method: spencer

Number of Valid Surfaces: 4301

Number of Invalid Surfaces: 703

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 57 surfaces

Error Code -110 reported for 257 surfaces

Error Code -111 reported for 387 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4146

Number of Invalid Surfaces: 858

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 58 surfaces

Error Code -110 reported for 257 surfaces

Error Code -111 reported for 541 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244

1336.39, 837.906
1312.53, 843.912
1289.79, 849.649
1268.54, 855.013
1248.67, 860.028
1228.73, 865.012
1188.81, 874.99
1149.18, 884.939
1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760

398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523

	202.582, 745.012
	202.37, 744.941
	201.485, 744.646
	201.379, 744.611
	200.794, 744.416
	200.126, 744.193
	200.07, 744.175
	199.416, 743.957
	199.195, 743.883
	197.758, 743.404
	197.28, 743.245
	196.972, 743.142
	196.867, 743.107
	195.97, 742.808
	192.571, 741.675
	180.393, 737.616
	179.777, 737.41
	179.104, 737.186
	178.684, 737.046
	177.683, 736.712
	0, 735.281
	0, 330
	2300, 329.864
	2300, 738.23
	2246.57, 739.067
	2243.38, 739.234
	2237.66, 739.488
	2236.06, 739.552
	2228.45, 739.866
External Boundary	2227.6, 739.905
	2224.96, 740
	2223.17, 740.005
	2222.91, 740.007
	2220.2, 740.014
	2219.67, 740.014
	2212.87, 740.232
	2205.39, 740.356
	2197.23, 740.483
	2193.85, 740.574
	2186.16, 740.695
	2184.12, 740.751
	2173.86, 743.268
	2159.21, 746.831
	2152.58, 748.429
	2136.66, 752.288
	2133.09, 753.144
	2130.67, 753.722
	2128.9, 754.137
	2118.44, 756.605
	2115.88, 757.214
	2112.57, 758
	2110.94, 758.385
	2109.47, 758.729
	2106.6, 759.395
	2105.52, 759.642
	2103.96, 760
	2102.2, 760.402
	2101.25, 760.619
	2095.2, 762

2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
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2046.89, 756.56
2020.89, 750.238
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2006.76, 746.88
2004.23, 746.282
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1995.37, 744.264
1991.58, 743.44
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1986.9, 742.454
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1983.22, 741.722
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1976.28, 740.516
1975.78, 740.438
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1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
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1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
1940.44, 740

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1916.72, 740.203
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1840.84, 742.154
1840.05, 742.158
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1814.92, 742.579
1800.98, 742.614
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1766.72, 744.351
1766.68, 744.366
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1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
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1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845

	1765.23, 744.848 1763.77, 745.335 1760.89, 745.278 1733.77, 744.737 1728.36, 742.936 1724.77, 741.738 1718.76, 741.738 1714.77, 741.738 1709.37, 743.539
Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16

	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625
	420.368, 737.91
	432.54, 733.852
	440.132, 731.322
	447.096, 729
	503.929, 710.053
	539.503, 697.951
	558.611, 691.762
	558.865, 691.679
	562.929, 690.36
	564.036, 690.348
	567.84, 690.349
	591.835, 690.364
	623.213, 690.364
	626.363, 690.364
	694.207, 690.807
	696.794, 690.824
	748.23, 691.161
	758.969, 691.241
	840.146, 692
Material Boundary	840.75, 692.006
	878.605, 692.355
	1002.59, 693.5
	1031.66, 693.776
	1031.69, 693.776
	1031.85, 693.778
	1056.47, 694
	1104.1, 694.443
	1146.52, 694.837
	1211.99, 695.445
	1271.75, 696
	1271.77, 696
	1289.59, 696.166
	1484.09, 698.02
	1487.03, 698.047
	1488.81, 698.064
	1546.16, 698.605
	1550.33, 699.993
	1550.34, 699.997
	1664.79, 738.147
	1685.08, 744.911
	1685.14, 744.929
	1685.15, 744.933
	1685.16, 744.935
	1685.16, 744.936
	1685.18, 744.943
	1685.18, 744.943
	1688.75, 745.08
	1695.33, 744.948
	1695.56, 744.943
	1695.85, 744.937
	1703.67, 744.781

	1705.77, 744.739
	386.057, 748.292
	387.636, 748.688
	389.31, 749.106
	390.102, 749.304
	391.02, 749.533
	392.766, 749.97
	394.551, 750.415
	396.375, 750.871
	398.239, 751.337
	400.145, 751.813
	402.095, 752.3
	404.089, 752.798
	406.128, 753.308
	411.367, 754.619
	447.096, 763.546
	449.685, 764.193
	564.036, 792.77
	626.363, 808.356
	792.922, 850
	793.256, 850.084
	800.923, 852
	801.257, 852.084
	808.924, 854
	809.258, 854.084
	816.925, 856
	817.259, 856.084
	824.926, 858
	825.261, 858.084
	832.927, 860
	833.262, 860.084
	840.146, 861.804
	840.929, 862
	841.263, 862.084
	848.931, 864
	849.265, 864.083
	856.933, 866
	857.267, 866.083
	864.935, 868
	865.269, 868.083
	872.938, 870
	873.27, 870.083
	880.94, 872
	881.272, 872.083
	888.942, 874
	889.273, 874.083
	896.944, 876
	897.274, 876.082
	904.947, 878
	905.275, 878.082
	912.949, 880.002
	913.77, 880.084
	932.954, 882
	933.767, 882.081
	952.96, 884
	953.77, 884.081
	972.965, 886
	973.774, 886.081
	992.971, 888
Material Boundary	

993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
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943.81, 885.246
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1067.55, 890.236
1078.27, 889.164
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1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
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1148.69, 882.124
1188.12, 872.225
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1267.84, 852.25
1289.09, 846.886
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	1695.85, 744.937
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	386.882, 750.56
	390.83, 751.547
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	392.034, 751.848
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	397.22, 753.144
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	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
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	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
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Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281 178.514, 731.719 180.265, 732.302 180.685, 732.443 181.358, 732.667 181.974, 732.872 194.152, 736.932 197.551, 738.064 198.448, 738.364 198.554, 738.399 198.861, 738.501 199.339, 738.661 200.776, 739.14 200.997, 739.213 201.651, 739.431 201.708, 739.45 202.375, 739.673 202.96, 739.868 203.066, 739.903 203.951, 740.198 204.163, 740.269 205.695, 740.779 206.113, 740.918 206.403, 741.015 206.704, 741.116 206.992, 741.211 207.162, 741.268 207.356, 741.333 213.598, 743.414 219.127, 745.257	Assigned to:  Wadsworth Till

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
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237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
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269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
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293.12, 749.256
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299.331, 747.189
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306.093, 744.938
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311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
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	1550.33, 693.668	
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	1689.03, 739.9	
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	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
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	1763.77, 740.065	
	1763.78, 740.062	
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	1763.81, 740.051	
	1763.83, 740.046	
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	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
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	1764.59, 739.79	
	1764.67, 739.764	
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	1765.03, 739.645	
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	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
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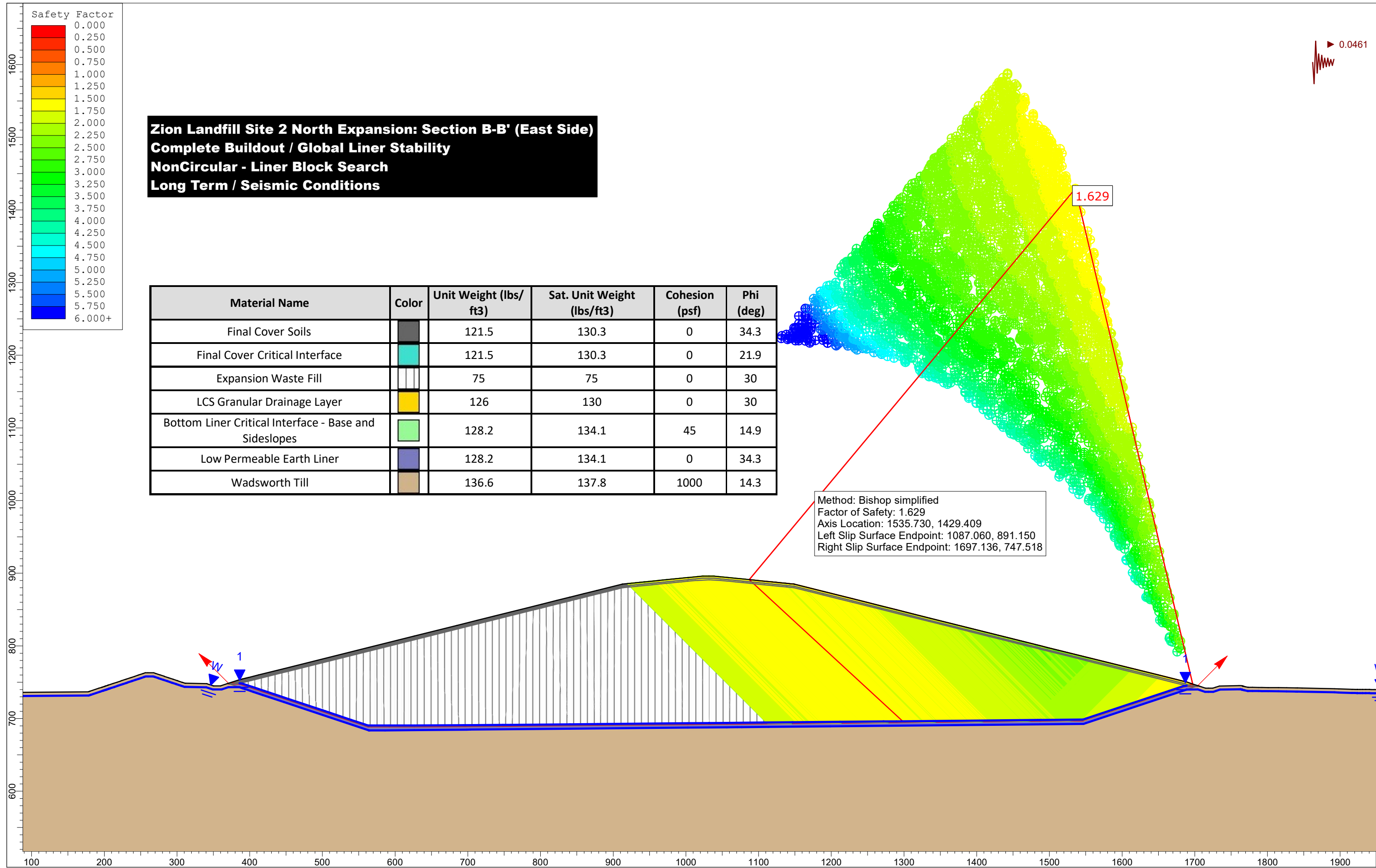
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1923.71, 735.001
1923.77, 735.001
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1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
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1977.05, 735.576
1979.08, 735.893
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1983.3, 736.648
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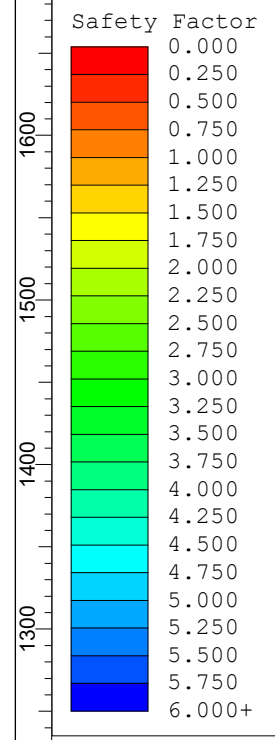
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	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
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	591.835, 690.364	
	623.213, 690.364	
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	696.794, 690.824	
	748.23, 691.161	
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Block Search Polyline

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1212.44, 694.424
1487.03, 696.975
1532.11, 697.394
1546.04, 697.525
1546.33, 697.528
1550.35, 698.868
1653.04, 733.095
1688.76, 745.005
1695.56, 744.868
1703.46, 744.71



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Complete Buildout / Global Liner Stability
NonCircular - Liner Block Search
Long Term / Seismic Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3

Method: Bishop simplified
 Factor of Safety: 1.629
 Axis Location: 1535.730, 1429.409
 Left Slip Surface Endpoint: 1087.060, 891.150
 Right Slip Surface Endpoint: 1697.136, 747.518

Slide Analysis Information

B-B_final_east_LT

Project Summary

File Name:	B-B_final_east_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:11m:48.151s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials


Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0


Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8

Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.628560
Axis Location:	1535.730, 1429.409
Left Slip Surface Endpoint:	1087.060, 891.150
Right Slip Surface Endpoint:	1697.136, 747.518
Resisting Moment:	1.1107e+09 lb-ft
Driving Moment:	6.82015e+08 lb-ft
Total Slice Area:	53122.5 ft ²
Surface Horizontal Width:	610.075 ft
Surface Average Height:	87.0753 ft

Method: janbu corrected

FS	1.645100
Axis Location:	1536.926, 1426.298
Left Slip Surface Endpoint:	1090.031, 890.853
Right Slip Surface Endpoint:	1697.145, 747.515
Resisting Horizontal Force:	1.43216e+06 lb
Driving Horizontal Force:	870562 lb
Total Slice Area:	53274.4 ft ²
Surface Horizontal Width:	607.115 ft
Surface Average Height:	87.7502 ft

Method: spencer

FS	1.712550
Axis Location:	1517.338, 1477.691
Left Slip Surface Endpoint:	1041.079, 895.747
Right Slip Surface Endpoint:	1697.138, 747.517
Resisting Moment:	1.27503e+09 lb-ft
Driving Moment:	7.44517e+08 lb-ft
Resisting Horizontal Force:	1.48644e+06 lb
Driving Horizontal Force:	867966 lb
Total Slice Area:	56811.7 ft ²
Surface Horizontal Width:	656.06 ft
Surface Average Height:	86.5953 ft

Method: gle/morgenstern-price

FS	1.721050
Axis Location:	1514.667, 1483.983
Left Slip Surface Endpoint:	1035.021, 896.000
Right Slip Surface Endpoint:	1697.265, 747.476
Resisting Moment:	1.30793e+09 lb-ft
Driving Moment:	7.59964e+08 lb-ft
Resisting Horizontal Force:	1.51209e+06 lb
Driving Horizontal Force:	878587 lb
Total Slice Area:	58275.2 ft ²
Surface Horizontal Width:	662.245 ft
Surface Average Height:	87.9964 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1087.06	891.15
1299.36	695.307
1495.08	696.975
1540.18	697.393
1543.23	697.467
1546.27	697.588
1549.61	698.699
1653.11	733.041
1691.8	745.938
1697.14	747.518

Method: janbu corrected

X	Y
1090.03	890.853
1295.2	695.268
1495.18	696.975
1540.1	697.394
1543.22	697.476
1546.33	697.607
1549.22	698.568
1653.61	733.205
1691.79	745.935
1697.15	747.515

Method: spencer

X	Y
1041.08	895.747
1302.65	695.337
1495.15	696.974
1540.04	697.394
1543.18	697.488
1546.32	697.607
1551.41	699.142
1653.56	733.19
1691.91	745.975
1697.14	747.517

Method: gle/morgenstern-price

X	Y
1035.02	896
1293.91	695.256
1494.97	696.974
1540.18	697.394
1543.25	697.487
1546.31	697.607
1549.46	698.492
1652.28	732.765
1689.88	745.297
1697.27	747.476

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4742
Number of Invalid Surfaces: 262

Error Codes

Error Code -106 reported for 2 surfaces
Error Code -108 reported for 4 surfaces
Error Code -110 reported for 256 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4739
Number of Invalid Surfaces: 265

Error Codes

Error Code -106 reported for 2 surfaces
Error Code -108 reported for 7 surfaces
Error Code -110 reported for 256 surfaces

Method: spencer

Number of Valid Surfaces: 4366

Number of Invalid Surfaces: 638

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 55 surfaces

Error Code -110 reported for 256 surfaces

Error Code -111 reported for 325 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4061

Number of Invalid Surfaces: 943

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 53 surfaces

Error Code -110 reported for 256 surfaces

Error Code -111 reported for 632 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information**◆ Block Search - thin layers****Shared Entities**

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939

1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
1031.28, 896
1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759

390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957

External Boundary

199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764

2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
1960.65, 739.995
1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
1940.44, 740
1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001

1923.26, 740
1923.21, 740
1920.59, 740.087
1917.66, 740.181
1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944
1769.62, 743.777
1768.75, 744
1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738

	1709.37, 743.539
Material Boundary	371.139, 747.994
	371.35, 747.924
	371.563, 747.853
	384.979, 743.38
	385.911, 743.07
	390.102, 741.672
	392.178, 740.98
	393.625, 740.498
	395.889, 739.743
	404.687, 736.81
	418.13, 732.329
	426.507, 729.536
	447.096, 722.672
	507.505, 702.534
	563.515, 683.862
	563.605, 683.831
	563.976, 683.705
	564.036, 683.685
	564.179, 683.637
	564.311, 683.592
	564.361, 683.575
	564.491, 683.575
	564.857, 683.576
	586.189, 683.61
	626.363, 684
	698.374, 684.674
	703.436, 684.721
	840.146, 686
	1002.59, 687.5
	1031.85, 687.778
	1056.47, 688
	1070.79, 688.133
	1076.9, 688.19
	1113.49, 688.53
	1198.73, 689.322
	1487.03, 692
	1546.99, 692.557
	1550.33, 693.668
	1688.75, 739.809
	1696.74, 742.47
	1703.25, 744.639
1703.64, 744.772	
1703.65, 744.774	
1703.66, 744.776	
1703.67, 744.781	
	369.041, 747.954
	369.564, 747.963
	371.299, 747.998
	378.927, 748.15
	379.447, 748.16
	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625

Material Boundary

420.368, 737.91
432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
539.503, 697.951
558.611, 691.762
558.865, 691.679
562.929, 690.36
564.036, 690.348
567.84, 690.349
591.835, 690.364
623.213, 690.364
626.363, 690.364
694.207, 690.807
696.794, 690.824
748.23, 691.161
758.969, 691.241
840.146, 692
840.75, 692.006
878.605, 692.355
1002.59, 693.5
1031.66, 693.776
1031.69, 693.776
1031.85, 693.778
1056.47, 694
1104.1, 694.443
1146.52, 694.837
1211.99, 695.445
1271.75, 696
1271.77, 696
1289.59, 696.166
1484.09, 698.02
1487.03, 698.047
1488.81, 698.064
1546.16, 698.605
1550.33, 699.993
1550.34, 699.997
1664.79, 738.147
1685.08, 744.911
1685.14, 744.929
1685.15, 744.933
1685.16, 744.935
1685.16, 744.936
1685.18, 744.943
1685.18, 744.943
1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415

	396.375, 750.871
	398.239, 751.337
	400.145, 751.813
	402.095, 752.3
	404.089, 752.798
	406.128, 753.308
	411.367, 754.619
	447.096, 763.546
	449.685, 764.193
	564.036, 792.77
	626.363, 808.356
	792.922, 850
	793.256, 850.084
	800.923, 852
	801.257, 852.084
	808.924, 854
	809.258, 854.084
	816.925, 856
	817.259, 856.084
	824.926, 858
	825.261, 858.084
	832.927, 860
	833.262, 860.084
	840.146, 861.804
	840.929, 862
	841.263, 862.084
	848.931, 864
	849.265, 864.083
	856.933, 866
	857.267, 866.083
	864.935, 868
	865.269, 868.083
	872.938, 870
	873.27, 870.083
	880.94, 872
	881.272, 872.083
	888.942, 874
	889.273, 874.083
	896.944, 876
	897.274, 876.082
	904.947, 878
	905.275, 878.082
	912.949, 880.002
	913.77, 880.084
	932.954, 882
	933.767, 882.081
	952.96, 884
	953.77, 884.081
	972.965, 886
	973.774, 886.081
	992.971, 888
	993.788, 888.082
	1002.59, 888.961
	1012.97, 890
	1015.9, 890
	1031.85, 890.91
	1032.05, 890.921
	1048.3, 890
	1056.47, 889.183
Material Boundary	

1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
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	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
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	1267.81, 852.104
	1289.06, 846.74
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	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

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225.127, 747.257
229.854, 748.832
231.127, 749.257
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237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
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275.094, 755.256
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299.331, 747.189
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626.363, 684

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	1070.79, 688.133	
	1076.9, 688.19	
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	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
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	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
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	1763.68, 740.094	
	1763.69, 740.092	
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	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
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	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
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	1764.14, 739.942	
	1764.53, 739.813	
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	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
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	1787.81, 737.786	
	1792.3, 737.722	

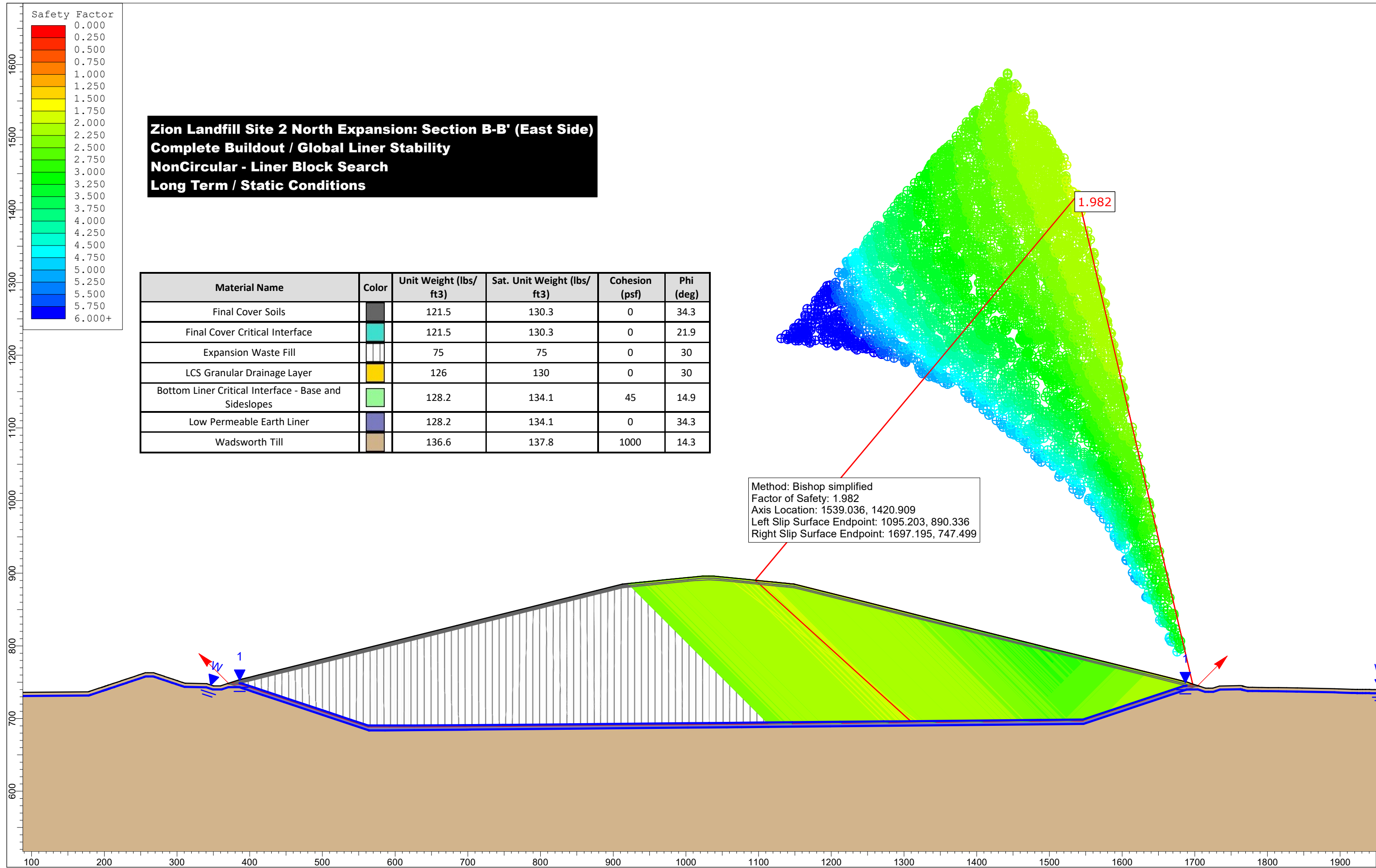
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1923.71, 735.001
1923.77, 735.001
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1935.34, 735.008
1939.87, 735.009
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1971.89, 734.933
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1972.35, 734.958
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1976.51, 735.491
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1983.3, 736.648
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2017.72, 744.335

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2081.18, 759
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2086.73, 758.77
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2089.74, 758.111
2090.12, 758.026
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2186.05, 735.696
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2220.19, 735.014
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2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91 432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
-----------	---	---

Block Search Polyline

371.35, 747.924
386.046, 748.217
390.079, 746.872
398.881, 743.938
399.765, 743.643
406.945, 741.249
409.081, 740.537
417.36, 737.777
429.79, 733.633
437.541, 731.05
447.072, 727.872
513.038, 705.881
558.707, 690.597
562.754, 689.282
564.036, 689.269
567.392, 689.269
591.838, 689.285
626.364, 689.285
695.699, 689.738
697.255, 689.748
736.377, 690.004
748.109, 690.114
840.147, 690.975
877.73, 691.322
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1033.44, 692.767
1033.47, 692.767
1056.47, 692.975
1104.66, 693.423
1147.07, 693.817
1212.44, 694.424
1487.03, 696.975
1532.11, 697.394
1546.04, 697.525
1546.33, 697.528
1550.35, 698.868
1653.04, 733.095
1688.76, 745.005
1695.56, 744.868
1703.46, 744.71



Slide Analysis Information

B-B_final_east_LT

Project Summary

File Name:	B-B_final_east_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:05m:52.122s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Non-Circular Block Search
Number of Surfaces:	5000
Multiple Groups:	Disabled
Pseudo-Random Surfaces:	Enabled
Convex Surfaces Only:	Disabled
Left Projection Angle (Start Angle) [deg]:	135
Left Projection Angle (End Angle) [deg]:	135
Right Projection Angle (Start Angle) [deg]:	45
Right Projection Angle (End Angle) [deg]:	45
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading







Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1000

Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.982430
Axis Location:	1539.036, 1420.909
Left Slip Surface Endpoint:	1095.203, 890.336
Right Slip Surface Endpoint:	1697.195, 747.499
Resisting Moment:	1.08928e+09 lb-ft
Driving Moment:	5.49466e+08 lb-ft
Total Slice Area:	51335.7 ft ²
Surface Horizontal Width:	601.992 ft
Surface Average Height:	85.2765 ft

Method: janbu corrected

FS	2.007580
Axis Location:	1541.877, 1413.509
Left Slip Surface Endpoint:	1102.265, 889.630
Right Slip Surface Endpoint:	1697.213, 747.493
Resisting Horizontal Force:	1.39965e+06 lb
Driving Horizontal Force:	697182 lb
Total Slice Area:	50438.9 ft ²
Surface Horizontal Width:	594.948 ft
Surface Average Height:	84.7787 ft

Method: spencer

FS	2.090860
Axis Location:	1523.579, 1461.617
Left Slip Surface Endpoint:	1056.470, 894.208
Right Slip Surface Endpoint:	1697.240, 747.484
Resisting Moment:	1.21774e+09 lb-ft
Driving Moment:	5.82411e+08 lb-ft
Resisting Horizontal Force:	1.44794e+06 lb
Driving Horizontal Force:	692509 lb
Total Slice Area:	53754.4 ft ²
Surface Horizontal Width:	640.771 ft
Surface Average Height:	83.8903 ft

Method: gle/morgenstern-price

FS	2.105400
Axis Location:	1519.968, 1471.271
Left Slip Surface Endpoint:	1047.321, 895.123
Right Slip Surface Endpoint:	1697.299, 747.465
Resisting Moment:	1.26161e+09 lb-ft
Driving Moment:	5.99226e+08 lb-ft
Resisting Horizontal Force:	1.48271e+06 lb
Driving Horizontal Force:	704239 lb
Total Slice Area:	55243.5 ft ²
Surface Horizontal Width:	649.978 ft
Surface Average Height:	84.993 ft

Global Minimum Coordinates

Method: bishop simplified

X	Y
1095.2	890.336
1310.16	695.407
1495.16	696.975
1540.23	697.394
1546.09	697.527
1550.92	699.137
1658.72	735.067
1687.14	744.429
1697.19	747.499

Method: janbu corrected

X	Y
1102.27	889.63
1313.29	695.325
1495.18	696.975
1539.64	697.389
1546.09	697.526
1551.55	699.346
1657.41	734.632
1690.06	745.35
1697.21	747.493

Method: spencer

X	Y
1056.47	894.208
1319.69	695.496
1500.72	697.026
1539.38	697.387
1542.87	697.483
1546.32	697.606
1552.13	699.383
1652.56	732.86
1690.28	745.432
1697.24	747.484

Method: gle/morgenstern-price

X	Y
1047.32	895.123
1312.78	695.431
1495.04	696.973
1538.45	697.379
1546.09	697.526
1549.59	698.532
1652.65	732.89
1689.33	745.114
1697.3	747.465

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4745
 Number of Invalid Surfaces: 260

Error Codes

Error Code -106 reported for 2 surfaces
 Error Code -108 reported for 4 surfaces
 Error Code -110 reported for 254 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4746
 Number of Invalid Surfaces: 259

Error Codes

Error Code -106 reported for 2 surfaces
 Error Code -108 reported for 3 surfaces
 Error Code -110 reported for 254 surfaces

Method: spencer

Number of Valid Surfaces: 4279

Number of Invalid Surfaces: 725

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 54 surfaces

Error Code -110 reported for 254 surfaces

Error Code -111 reported for 415 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4136

Number of Invalid Surfaces: 869

Error Codes

Error Code -106 reported for 2 surfaces

Error Code -108 reported for 56 surfaces

Error Code -110 reported for 254 surfaces

Error Code -111 reported for 557 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-106 = Average slice width is less than $0.0001 * (\text{maximum horizontal extent of soil region})$. This limitation is imposed to avoid numerical errors which may result from too many slices, or too small a slip region.

-108 = Total driving moment or total driving force < 0.1 . This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-111 = safety factor equation did not converge

Entity Information

◆ Block Search - thin layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939

1148.86, 884.971
1148.57, 885
1148.54, 885.012
1143.27, 885.539
1138.57, 886
1128.02, 887.055
1123.25, 887.539
1118.57, 888
1107.98, 889.059
1098.56, 890
1087.91, 891.065
1078.56, 892
1067.84, 893.072
1058.55, 894
1056.47, 894.208
1047.76, 895.079
1038.54, 896
1031.85, 896
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1024.07, 896
1022.73, 896
1003.75, 894.103
1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759

390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957

External Boundary

199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
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2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764

2080.83, 764
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2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
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1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
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1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
1927.26, 740.005
1923.77, 740.001
1923.7, 740.001

1923.26, 740
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1920.59, 740.087
1917.66, 740.181
1916.72, 740.203
1915.68, 740.226
1898.08, 740.833
1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
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1773.36, 742.944
1769.62, 743.777
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1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
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1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738

	1709.37, 743.539
Material Boundary	371.139, 747.994
	371.35, 747.924
	371.563, 747.853
	384.979, 743.38
	385.911, 743.07
	390.102, 741.672
	392.178, 740.98
	393.625, 740.498
	395.889, 739.743
	404.687, 736.81
	418.13, 732.329
	426.507, 729.536
	447.096, 722.672
	507.505, 702.534
	563.515, 683.862
	563.605, 683.831
	563.976, 683.705
	564.036, 683.685
	564.179, 683.637
	564.311, 683.592
	564.361, 683.575
	564.491, 683.575
	564.857, 683.576
	586.189, 683.61
	626.363, 684
	698.374, 684.674
	703.436, 684.721
	840.146, 686
	1002.59, 687.5
	1031.85, 687.778
	1056.47, 688
	1070.79, 688.133
	1076.9, 688.19
	1113.49, 688.53
	1198.73, 689.322
	1487.03, 692
	1546.99, 692.557
	1550.33, 693.668
	1688.75, 739.809
	1696.74, 742.47
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1703.64, 744.772	
1703.65, 744.774	
1703.66, 744.776	
1703.67, 744.781	
	369.041, 747.954
	369.564, 747.963
	371.299, 747.998
	378.927, 748.15
	379.447, 748.16
	386.057, 748.292
	386.203, 748.286
	389.631, 748.157
	390.102, 748
	402.215, 743.962
	403.086, 743.672
	410.125, 741.325
	412.226, 740.625

Material Boundary

420.368, 737.91
432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
539.503, 697.951
558.611, 691.762
558.865, 691.679
562.929, 690.36
564.036, 690.348
567.84, 690.349
591.835, 690.364
623.213, 690.364
626.363, 690.364
694.207, 690.807
696.794, 690.824
748.23, 691.161
758.969, 691.241
840.146, 692
840.75, 692.006
878.605, 692.355
1002.59, 693.5
1031.66, 693.776
1031.69, 693.776
1031.85, 693.778
1056.47, 694
1104.1, 694.443
1146.52, 694.837
1211.99, 695.445
1271.75, 696
1271.77, 696
1289.59, 696.166
1484.09, 698.02
1487.03, 698.047
1488.81, 698.064
1546.16, 698.605
1550.33, 699.993
1550.34, 699.997
1664.79, 738.147
1685.08, 744.911
1685.14, 744.929
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1685.16, 744.935
1685.16, 744.936
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1685.18, 744.943
1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415

	396.375, 750.871
	398.239, 751.337
	400.145, 751.813
	402.095, 752.3
	404.089, 752.798
	406.128, 753.308
	411.367, 754.619
	447.096, 763.546
	449.685, 764.193
	564.036, 792.77
	626.363, 808.356
	792.922, 850
	793.256, 850.084
	800.923, 852
	801.257, 852.084
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	809.258, 854.084
	816.925, 856
	817.259, 856.084
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	825.261, 858.084
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	833.262, 860.084
	840.146, 861.804
	840.929, 862
	841.263, 862.084
	848.931, 864
	849.265, 864.083
	856.933, 866
	857.267, 866.083
	864.935, 868
	865.269, 868.083
	872.938, 870
	873.27, 870.083
	880.94, 872
	881.272, 872.083
	888.942, 874
	889.273, 874.083
	896.944, 876
	897.274, 876.082
	904.947, 878
	905.275, 878.082
	912.949, 880.002
	913.77, 880.084
	932.954, 882
	933.767, 882.081
	952.96, 884
	953.77, 884.081
	972.965, 886
	973.774, 886.081
	992.971, 888
	993.788, 888.082
	1002.59, 888.961
	1012.97, 890
	1015.9, 890
	1031.85, 890.91
	1032.05, 890.921
	1048.3, 890
	1056.47, 889.183
Material Boundary	

1057.55, 889.075
1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
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1189.01, 870
1211.61, 864.353
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1334.07, 833.743
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1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
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1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
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877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
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1148.69, 882.124
1188.12, 872.225
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1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07 1138.27, 883.015 1142.97, 882.553 1147.91, 882.06 1147.94, 882.048 1148.56, 881.986 1148.66, 881.976 1188.08, 872.08 1228, 862.102 1247.94, 857.118 1267.81, 852.104 1289.06, 846.74 1311.8, 841.003 1335.66, 834.996 1486.31, 797.334 1497.43, 794.552 1516.51, 789.734 1549.61, 781.577 1649, 757.085 1683.06, 748.565 1687.92, 747.349 1691.01, 746.338 1695.33, 744.948
Material Boundary	371.139, 747.994 371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281 178.514, 731.719 180.265, 732.302 180.685, 732.443 181.358, 732.667 181.974, 732.872 194.152, 736.932 197.551, 738.064 198.448, 738.364 198.554, 738.399 198.861, 738.501 199.339, 738.661 200.776, 739.14 200.997, 739.213 201.651, 739.431 201.708, 739.45 202.375, 739.673 202.96, 739.868 203.066, 739.903 203.951, 740.198 204.163, 740.269 205.695, 740.779 206.113, 740.918 206.403, 741.015 206.704, 741.116 206.992, 741.211 207.162, 741.268 207.356, 741.333 213.598, 743.414 219.127, 745.257	Assigned to:  Wadsworth Till

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
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	1689.03, 739.9	
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	1725.58, 736.737	
	1729.95, 738.193	
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	1763.69, 740.092	
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	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
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Water Table	1764.53, 739.811	
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	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
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	1767.72, 739.101	
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	1787.81, 737.786	
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2220.19, 735.014
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2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
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Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
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	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
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	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
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	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
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	840.75, 692.006	
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	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
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1488.81, 698.064		
1546.16, 698.605		
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1550.34, 699.997		
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1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

Block Search Polyline

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567.392, 689.269
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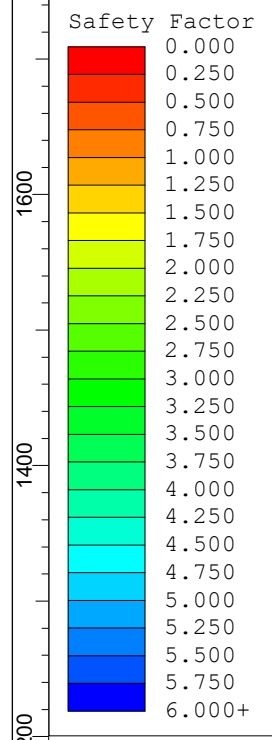
SLOPE STABILITY
SECTION **B-B'** - **HORIZONTAL EXPANSION (EAST SLOPE)**

GLOBAL LINER STABILITY ANALYSIS

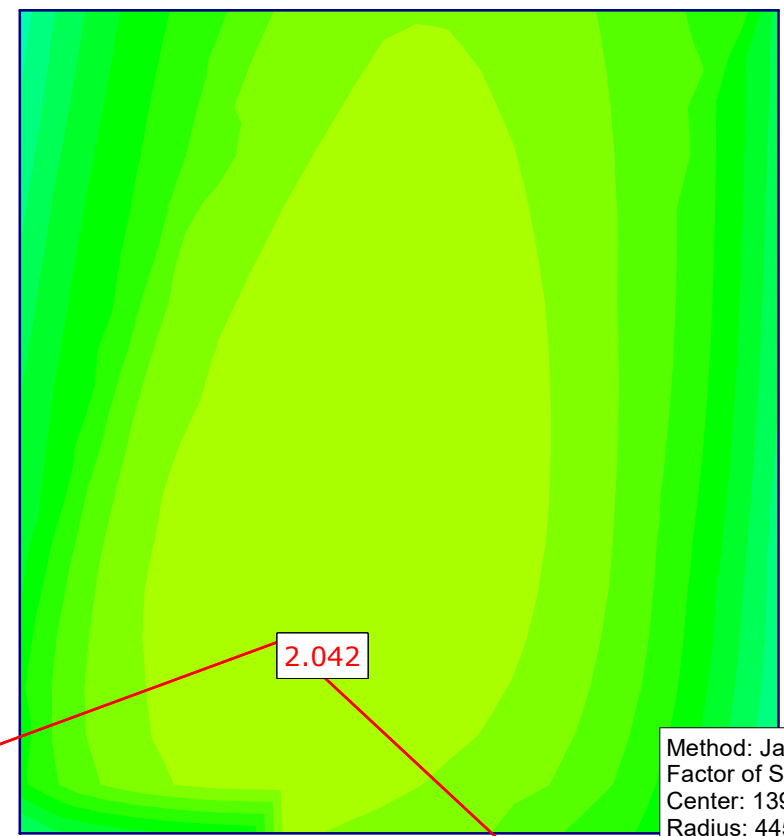
**COMPLETE BUILD-OUT / FINAL LANDFORM
CIRCULAR ANALYSIS OF WASTE AND FOUNDATION
(ROTATIONAL SLOPE FAILURE)**

Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Complete Buildout / Global Stability
Circular - Grid Search
Short Term / Seismic Conditions

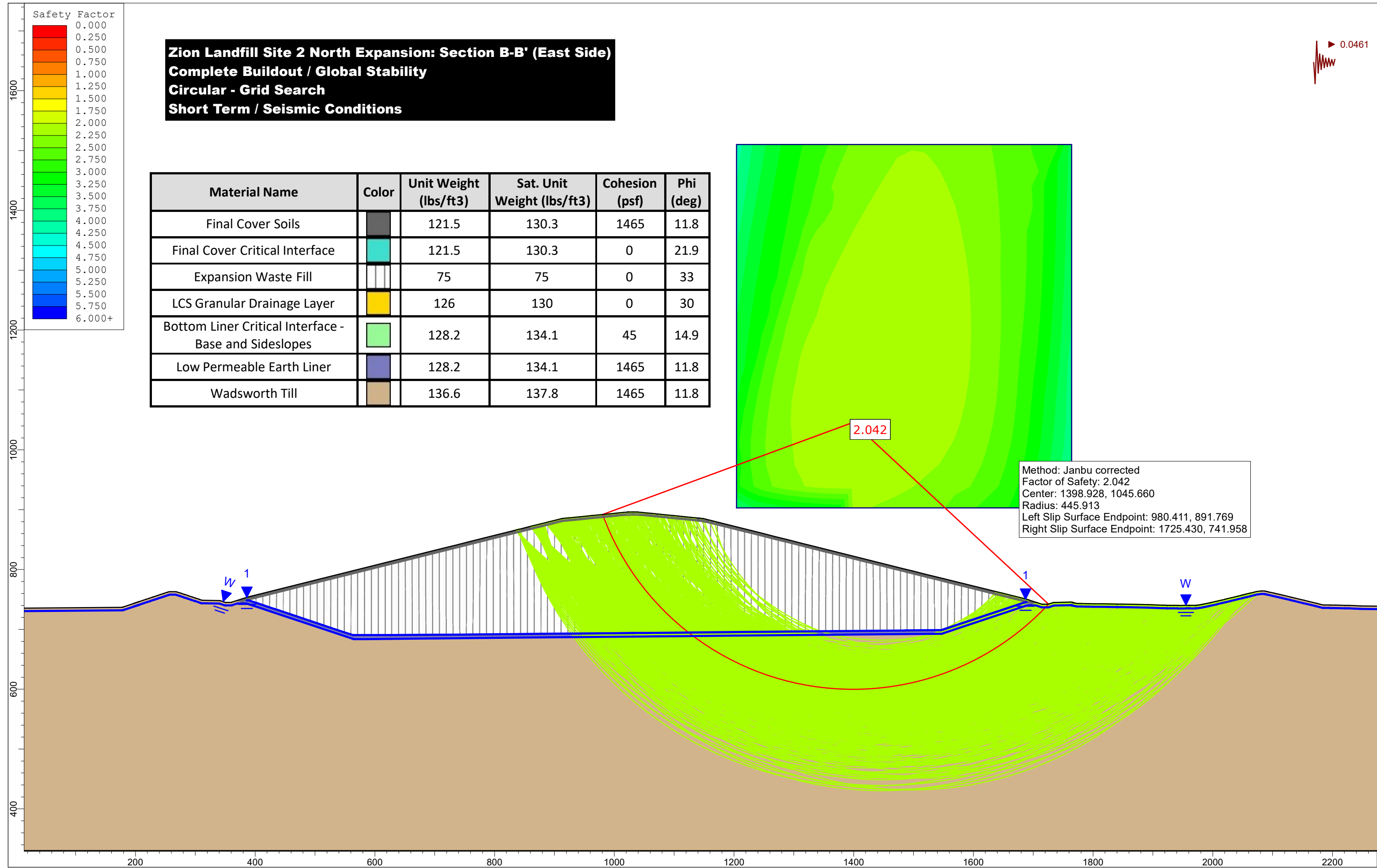
0.0461



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	1465	11.8
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	33
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	1465	11.8
Wadsworth Till		136.6	137.8	1465	11.8



Method: Janbu corrected
Factor of Safety: 2.042
Center: 1398.928, 1045.660
Radius: 445.913
Left Slip Surface Endpoint: 980.411, 891.769
Right Slip Surface Endpoint: 1725.430, 741.958



Slide Analysis Information

B-B_final_east_ST

Project Summary

File Name:	B-B_final_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:09.252s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.184350
Center:	1471.936, 1224.256
Radius:	636.197
Left Slip Surface Endpoint:	932.500, 886.980
Right Slip Surface Endpoint:	1885.935, 741.191
Resisting Moment:	2.93398e+09 lb-ft
Driving Moment:	1.34318e+09 lb-ft
Total Slice Area:	152552 ft ²
Surface Horizontal Width:	953.435 ft
Surface Average Height:	160.002 ft

Method: janbu corrected

FS	2.041600
Center:	1398.928, 1045.660
Radius:	445.913
Left Slip Surface Endpoint:	980.411, 891.769
Right Slip Surface Endpoint:	1725.430, 741.958
Resisting Horizontal Force:	3.52703e+06 lb
Driving Horizontal Force:	1.72759e+06 lb
Total Slice Area:	124354 ft ²
Surface Horizontal Width:	745.019 ft
Surface Average Height:	166.913 ft

Method: spencer

FS	2.158890
Center:	1447.600, 1188.537
Radius:	566.686
Left Slip Surface Endpoint:	965.740, 890.303
Right Slip Surface Endpoint:	1797.352, 742.659
Resisting Moment:	2.13041e+09 lb-ft
Driving Moment:	9.86809e+08 lb-ft
Resisting Horizontal Force:	3.36456e+06 lb
Driving Horizontal Force:	1.55847e+06 lb
Total Slice Area:	116844 ft ²
Surface Horizontal Width:	831.613 ft
Surface Average Height:	140.503 ft

Method: gle/morgenstern-price

FS	2.161150
Center:	1447.600, 1188.537
Radius:	566.686
Left Slip Surface Endpoint:	965.740, 890.303
Right Slip Surface Endpoint:	1797.352, 742.659
Resisting Moment:	2.13265e+09 lb-ft
Driving Moment:	9.86809e+08 lb-ft
Resisting Horizontal Force:	3.36756e+06 lb
Driving Horizontal Force:	1.55822e+06 lb
Total Slice Area:	116844 ft ²
Surface Horizontal Width:	831.613 ft
Surface Average Height:	140.503 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4466
Number of Invalid Surfaces:	286

Error Codes

Error Code -103 reported for 16 surfaces
 Error Code -110 reported for 101 surfaces
 Error Code -112 reported for 164 surfaces
 Error Code -115 reported for 5 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4490
Number of Invalid Surfaces:	262

Error Codes

Error Code -103 reported for 16 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 101 surfaces
 Error Code -112 reported for 138 surfaces
 Error Code -115 reported for 5 surfaces

Method: spencer

Number of Valid Surfaces:	4453
Number of Invalid Surfaces:	299

Error Codes

Error Code -103 reported for 16 surfaces
 Error Code -108 reported for 2 surfaces
 Error Code -110 reported for 101 surfaces
 Error Code -112 reported for 175 surfaces
 Error Code -115 reported for 5 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4456

Number of Invalid Surfaces: 296

Error Codes

Error Code -103 reported for 16 surfaces

Error Code -108 reported for 2 surfaces

Error Code -110 reported for 101 surfaces

Error Code -112 reported for 172 surfaces

Error Code -115 reported for 5 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539

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1128.02, 887.055
1123.25, 887.539
1118.57, 888
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1087.91, 891.065
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1058.55, 894
1056.47, 894.208
1047.76, 895.079
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1031.28, 896
1024.07, 896
1022.73, 896
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982.719, 892
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962.713, 890
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942.708, 888
923.523, 886.082
922.702, 886
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912.089, 884.939
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841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
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759.573, 846.811
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223.546, 752
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200.126, 744.193
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197.758, 743.404
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External Boundary

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2227.6, 739.905
2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
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2102.2, 760.402
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2092.07, 762.714
2091.21, 762.906
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2086.22, 764
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2080.83, 764
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2020.89, 750.238
2018.62, 749.686
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1997.83, 744.822
1995.37, 744.264
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1989.28, 742.939
1986.9, 742.454
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1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
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1923.77, 740.001
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1916.72, 740.203
1915.68, 740.226
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1892.28, 741.057
1847.51, 742
1841.58, 742.146
1841.29, 742.151
1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
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1779.33, 742.929
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1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
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1765.27, 744.837
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1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary	432.54, 733.852 440.132, 731.322 447.096, 729 503.929, 710.053 539.503, 697.951 558.611, 691.762 558.865, 691.679 562.929, 690.36 564.036, 690.348 567.84, 690.349 591.835, 690.364 623.213, 690.364 626.363, 690.364 694.207, 690.807 696.794, 690.824 748.23, 691.161 758.969, 691.241 840.146, 692 840.75, 692.006 878.605, 692.355 1002.59, 693.5 1031.66, 693.776 1031.69, 693.776 1031.85, 693.778 1056.47, 694 1104.1, 694.443 1146.52, 694.837 1211.99, 695.445 1271.75, 696 1271.77, 696 1289.59, 696.166 1484.09, 698.02 1487.03, 698.047 1488.81, 698.064 1546.16, 698.605 1550.33, 699.993 1550.34, 699.997 1664.79, 738.147 1685.08, 744.911 1685.14, 744.929 1685.15, 744.933 1685.16, 744.935 1685.16, 744.936 1685.18, 744.943 1685.18, 744.943 1688.75, 745.08 1695.33, 744.948 1695.56, 744.943 1695.85, 744.937 1703.67, 744.781 1705.77, 744.739
	386.057, 748.292 387.636, 748.688 389.31, 749.106 390.102, 749.304 391.02, 749.533 392.766, 749.97 394.551, 750.415 396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
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1293.27, 843.94
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1309.04, 840
1334.07, 833.743
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1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
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1267.84, 852.25
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1311.83, 841.149
1335.69, 835.141
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1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

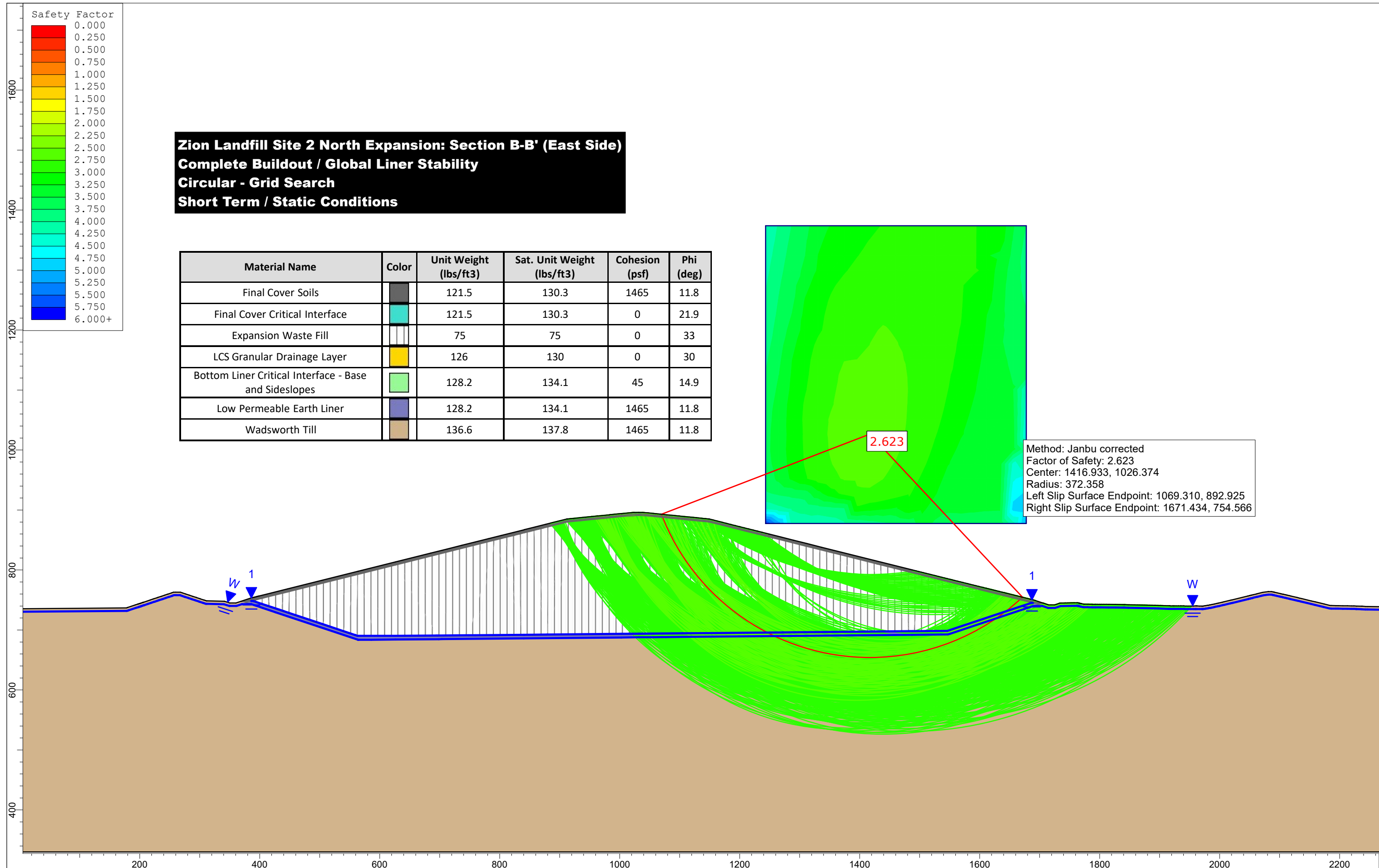
223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
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	1734.63, 739.753	
	1760.99, 740.279	
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	1763.65, 740.104	
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	1763.68, 740.094	
	1763.69, 740.092	
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	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
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	1763.88, 740.028	
	1763.89, 740.024	
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	1764.53, 739.813	
Water Table	1764.53, 739.811	
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	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
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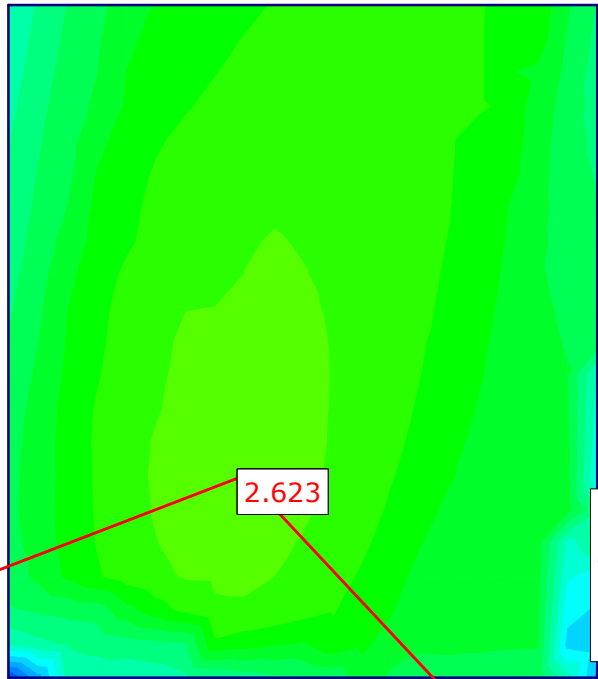
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Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
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	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
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	420.368, 737.91	
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	696.794, 690.824	
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	1056.47, 694	
	1104.1, 694.443	
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	1211.99, 695.445	
	1271.75, 696	
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	1289.59, 696.166	
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1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
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1550.34, 699.997		
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1685.16, 744.935		
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1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Complete Buildout / Global Liner Stability
Circular - Grid Search
Short Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	1465	11.8
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	33
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	1465	11.8
Wadsworth Till	Brown	136.6	137.8	1465	11.8



Method: Janbu corrected
 Factor of Safety: 2.623
 Center: 1416.933, 1026.374
 Radius: 372.358
 Left Slip Surface Endpoint: 1069.310, 892.925
 Right Slip Surface Endpoint: 1671.434, 754.566

Slide Analysis Information

B-B_final_east_ST

Project Summary

File Name:	B-B_final_east_ST.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:09.411s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No






Materials

Final Cover Soils

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	None
Ru Value	0

Final Cover Critical Interface

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0
Expansion Waste Fill	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	33
Water Surface	None
Ru Value	0
LCS Granular Drainage Layer	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1
Bottom Liner Critical Interface - Base and Sideslopes	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1
Low Permeable Earth Liner	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Piezometric Line 1
Hu Value	1
Wadsworth Till	
Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1465
Friction Angle [deg]	11.8
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

	FS	2.806140
Center:	1395.203, 1100.816	
Radius:	444.985	
Left Slip Surface Endpoint:	1001.275, 893.855	
Right Slip Surface Endpoint:	1673.931, 753.941	
Resisting Moment:	1.36541e+09 lb-ft	
Driving Moment:	4.8658e+08 lb-ft	
Total Slice Area:	85002.4 ft ²	
Surface Horizontal Width:	672.657 ft	
Surface Average Height:	126.368 ft	

Method: janbu corrected

	FS	2.622640
Center:	1416.933, 1026.374	
Radius:	372.358	
Left Slip Surface Endpoint:	1069.310, 892.925	
Right Slip Surface Endpoint:	1671.434, 754.566	
Resisting Horizontal Force:	2.50845e+06 lb	
Driving Horizontal Force:	956461 lb	
Total Slice Area:	74383 ft ²	
Surface Horizontal Width:	602.124 ft	
Surface Average Height:	123.534 ft	

Method: spencer

	FS	2.756500
Center:	1395.203, 1100.816	
Radius:	444.985	
Left Slip Surface Endpoint:	1001.275, 893.855	
Right Slip Surface Endpoint:	1673.931, 753.941	
Resisting Moment:	1.34126e+09 lb-ft	
Driving Moment:	4.8658e+08 lb-ft	
Resisting Horizontal Force:	2.66748e+06 lb	
Driving Horizontal Force:	967704 lb	
Total Slice Area:	85002.4 ft ²	
Surface Horizontal Width:	672.657 ft	
Surface Average Height:	126.368 ft	

Method: gle/morgenstern-price

FS	2.747190
Center:	1395.203, 1100.816
Radius:	444.985
Left Slip Surface Endpoint:	1001.275, 893.855
Right Slip Surface Endpoint:	1673.931, 753.941
Resisting Moment:	1.33673e+09 lb-ft
Driving Moment:	4.8658e+08 lb-ft
Resisting Horizontal Force:	2.65782e+06 lb
Driving Horizontal Force:	967467 lb
Total Slice Area:	85002.4 ft ²
Surface Horizontal Width:	672.657 ft
Surface Average Height:	126.368 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4216
Number of Invalid Surfaces:	635

Error Codes

Error Code -103 reported for 112 surfaces
 Error Code -110 reported for 81 surfaces
 Error Code -112 reported for 339 surfaces
 Error Code -114 reported for 103 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4250
Number of Invalid Surfaces:	601

Error Codes

Error Code -103 reported for 112 surfaces
 Error Code -108 reported for 4 surfaces
 Error Code -110 reported for 81 surfaces
 Error Code -112 reported for 301 surfaces
 Error Code -114 reported for 103 surfaces

Method: spencer

Number of Valid Surfaces:	4204
Number of Invalid Surfaces:	647

Error Codes

Error Code -103 reported for 112 surfaces
 Error Code -108 reported for 4 surfaces
 Error Code -110 reported for 81 surfaces
 Error Code -112 reported for 347 surfaces
 Error Code -114 reported for 103 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4205

Number of Invalid Surfaces: 646

Error Codes

Error Code -103 reported for 112 surfaces

Error Code -108 reported for 4 surfaces

Error Code -110 reported for 81 surfaces

Error Code -112 reported for 346 surfaces

Error Code -114 reported for 103 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-112 = The coefficient $M\text{-Alpha} = \cos(\alpha)(1 + \tan(\alpha)\tan(\phi))/F < 0.2$ for the final iteration of the safety factor calculation. This screens out some slip surfaces which may not be valid in the context of the analysis, in particular, deep seated slip surfaces with many high negative base angle slices in the passive zone.

-114 = Surface with Reverse Curvature.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539

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962.713, 890
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840.146, 866.956
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1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary

432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
539.503, 697.951
558.611, 691.762
558.865, 691.679
562.929, 690.36
564.036, 690.348
567.84, 690.349
591.835, 690.364
623.213, 690.364
626.363, 690.364
694.207, 690.807
696.794, 690.824
748.23, 691.161
758.969, 691.241
840.146, 692
840.75, 692.006
878.605, 692.355
1002.59, 693.5
1031.66, 693.776
1031.69, 693.776
1031.85, 693.778
1056.47, 694
1104.1, 694.443
1146.52, 694.837
1211.99, 695.445
1271.75, 696
1271.77, 696
1289.59, 696.166
1484.09, 698.02
1487.03, 698.047
1488.81, 698.064
1546.16, 698.605
1550.33, 699.993
1550.34, 699.997
1664.79, 738.147
1685.08, 744.911
1685.14, 744.929
1685.15, 744.933
1685.16, 744.935
1685.16, 744.936
1685.18, 744.943
1685.18, 744.943
1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415
396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

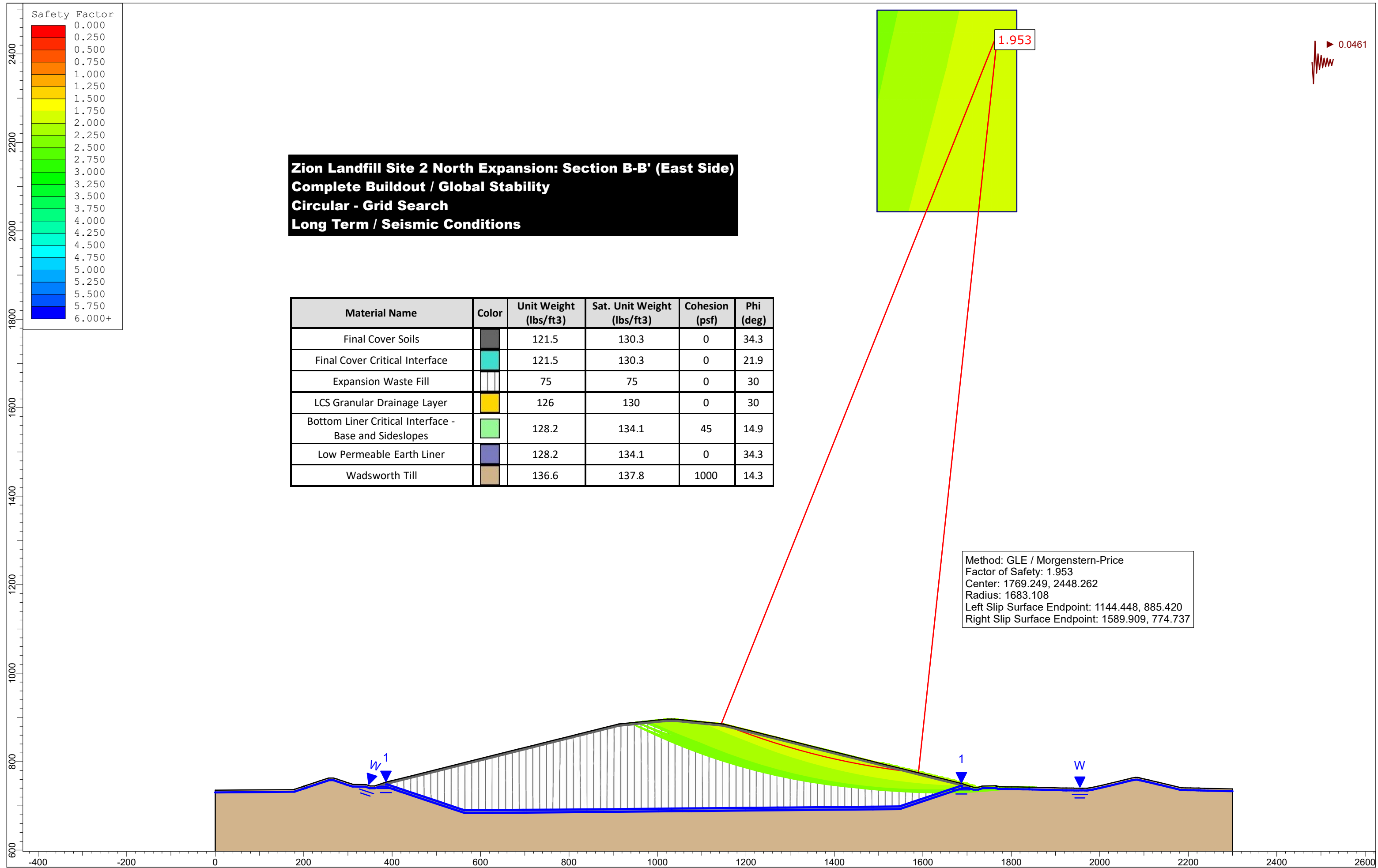
223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
2022.07, 745.38
2048.08, 751.704
2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
2054.59, 753.317
2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Complete Buildout / Global Stability
Circular - Grid Search
Long Term / Seismic Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils	Grey	121.5	130.3	0	34.3
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9
Expansion Waste Fill	White with vertical lines	75	75	0	30
LCS Granular Drainage Layer	Yellow	126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9
Low Permeable Earth Liner	Dark Blue	128.2	134.1	0	34.3
Wadsworth Till	Brown	136.6	137.8	1000	14.3

Method: GLE / Morgenstern-Price
 Factor of Safety: 1.953
 Center: 1769.249, 2448.262
 Radius: 1683.108
 Left Slip Surface Endpoint: 1144.448, 885.420
 Right Slip Surface Endpoint: 1589.909, 774.737

Slide Analysis Information

B-B_final_east_LT

Project Summary

File Name:	B-B_final_east_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:08.413s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	136.6
Saturated Unit Weight [lbs/ft ³]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	1.956520
Center:	1769.249, 2448.262
Radius:	1683.108
Left Slip Surface Endpoint:	1144.448, 885.420
Right Slip Surface Endpoint:	1589.909, 774.737
Resisting Moment:	4.33332e+08 lb-ft
Driving Moment:	2.2148e+08 lb-ft
Total Slice Area:	4903.29 ft ²
Surface Horizontal Width:	445.461 ft
Surface Average Height:	11.0072 ft

Method: janbu corrected

FS	1.967460
Center:	1769.249, 2448.262
Radius:	1683.108
Left Slip Surface Endpoint:	1144.448, 885.420
Right Slip Surface Endpoint:	1589.909, 774.737
Resisting Horizontal Force:	251735 lb
Driving Horizontal Force:	127950 lb
Total Slice Area:	4903.29 ft ²
Surface Horizontal Width:	445.461 ft
Surface Average Height:	11.0072 ft

Method: spencer

FS	1.953960
Center:	1769.249, 2448.262
Radius:	1683.108
Left Slip Surface Endpoint:	1144.448, 885.420
Right Slip Surface Endpoint:	1589.909, 774.737
Resisting Moment:	4.32763e+08 lb-ft
Driving Moment:	2.2148e+08 lb-ft
Resisting Horizontal Force:	249357 lb
Driving Horizontal Force:	127616 lb
Total Slice Area:	4903.29 ft ²
Surface Horizontal Width:	445.461 ft
Surface Average Height:	11.0072 ft

Method: gle/morgenstern-price

FS	1.952660
Center:	1769.249, 2448.262
Radius:	1683.108
Left Slip Surface Endpoint:	1144.448, 885.420
Right Slip Surface Endpoint:	1589.909, 774.737
Resisting Moment:	4.32475e+08 lb-ft
Driving Moment:	2.2148e+08 lb-ft
Resisting Horizontal Force:	249272 lb
Driving Horizontal Force:	127658 lb
Total Slice Area:	4903.29 ft ²
Surface Horizontal Width:	445.461 ft
Surface Average Height:	11.0072 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4137
Number of Invalid Surfaces:	670

Error Codes

Error Code -110 reported for 183 surfaces
 Error Code -115 reported for 487 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4137
Number of Invalid Surfaces:	670

Error Codes

Error Code -110 reported for 183 surfaces
 Error Code -115 reported for 487 surfaces

Method: spencer

Number of Valid Surfaces:	4137
Number of Invalid Surfaces:	670

Error Codes

Error Code -110 reported for 183 surfaces
 Error Code -115 reported for 487 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	4137
Number of Invalid Surfaces:	670

Error Codes

Error Code -110 reported for 183 surfaces
 Error Code -115 reported for 487 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072
	1058.55, 894
	1056.47, 894.208
	1047.76, 895.079
	1038.54, 896
	1031.85, 896
	1031.28, 896
	1024.07, 896
	1022.73, 896
	1003.75, 894.103
	1002.72, 894
	1002.59, 893.986
	983.533, 892.081
	982.719, 892
	963.529, 890.082
	962.713, 890
	943.526, 888.082
	942.708, 888
	923.523, 886.082
	922.702, 886

913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178
282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641

External Boundary

247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
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2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
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2236.06, 739.552
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2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
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2193.85, 740.574
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2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
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2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
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2103.96, 760
2102.2, 760.402
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2092.07, 762.714
2091.21, 762.906
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2090.04, 763.165
2089.21, 763.348
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2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
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2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
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2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454
1984.74, 742.012
1983.22, 741.722
1982.37, 741.56
1981.18, 741.335
1980.18, 741.16
1978.26, 740.826
1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14

1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
1962.15, 739.979
1962.12, 739.978
1960.65, 739.995
1960.65, 739.995
1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
1952.02, 739.831
1947.2, 739.799
1942.5, 739.789
1940.44, 740
1939.92, 740.009
1935.35, 740.008
1934.96, 740.008
1930.81, 740.006
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1923.77, 740.001
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1841.58, 742.146
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1838.74, 742.163
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1814.92, 742.579
1800.98, 742.614
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1787.88, 742.786
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1768.54, 744.035
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1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary

432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
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696.794, 690.824
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758.969, 691.241
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1146.52, 694.837
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1271.75, 696
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1289.59, 696.166
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1487.03, 698.047
1488.81, 698.064
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1685.08, 744.911
1685.14, 744.929
1685.15, 744.933
1685.16, 744.935
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1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415
396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
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1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
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1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
1229.02, 860
1229.02, 860
1252.48, 854.135
1269.03, 850
1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Seismic
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

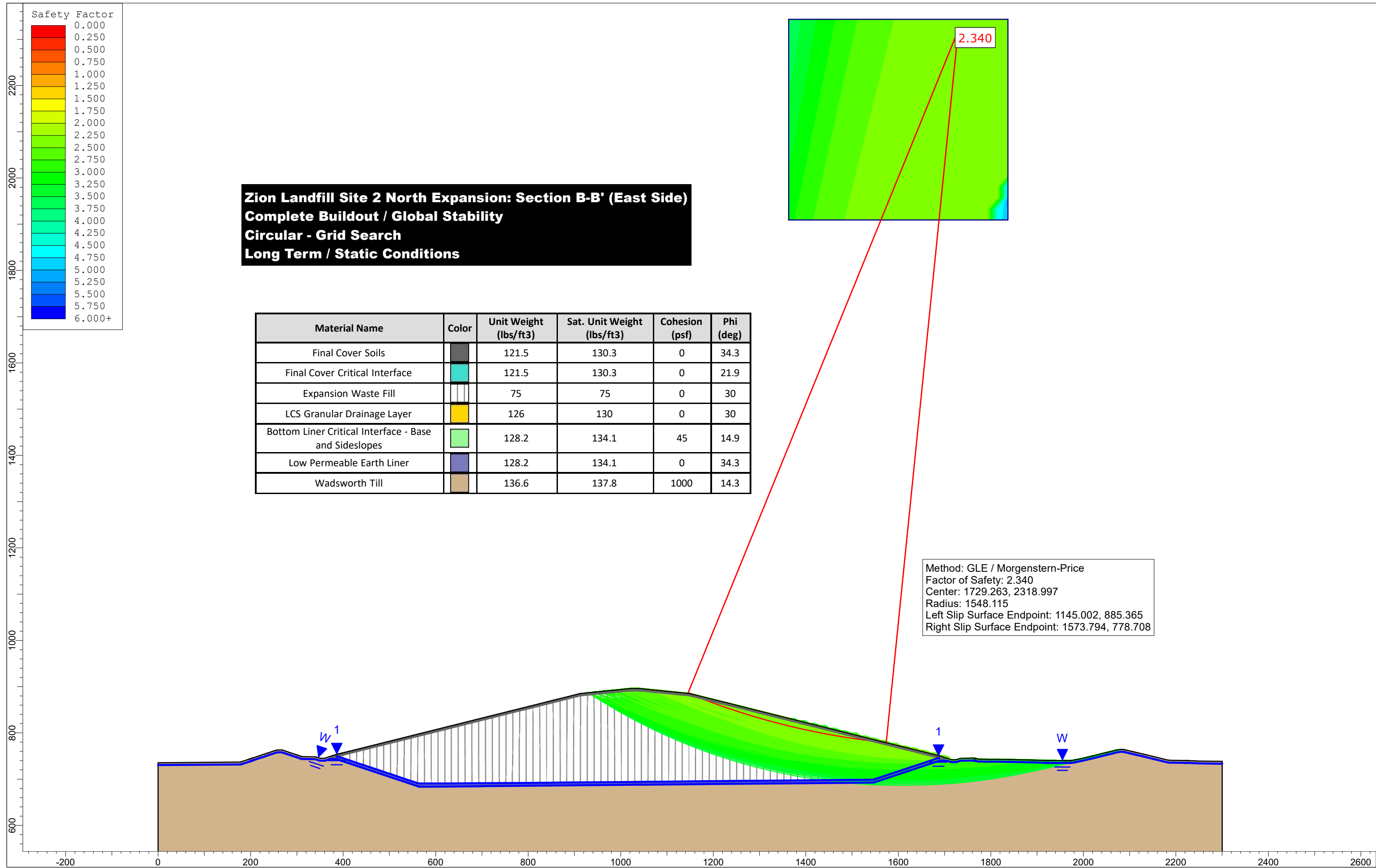
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229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
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1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
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1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
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2063.34, 755.481
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2081.18, 759
2085.68, 759
2086.73, 758.77
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2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
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2102.84, 755.126
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2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
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2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
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1550.34, 699.997		
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1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		



Zion Landfill Site 2 North Expansion: Section B-B' (East Side)
Complete Buildout / Global Stability
Circular - Grid Search
Long Term / Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)
Final Cover Soils		121.5	130.3	0	34.3
Final Cover Critical Interface		121.5	130.3	0	21.9
Expansion Waste Fill		75	75	0	30
LCS Granular Drainage Layer		126	130	0	30
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9
Low Permeable Earth Liner		128.2	134.1	0	34.3
Wadsworth Till		136.6	137.8	1000	14.3

Method: GLE / Morgenstern-Price
 Factor of Safety: 2.340
 Center: 1729.263, 2318.997
 Radius: 1548.115
 Left Slip Surface Endpoint: 1145.002, 885.365
 Right Slip Surface Endpoint: 1573.794, 778.708

Slide Analysis Information

B-B_final_east_LT

Project Summary

File Name:	B-B_final_east_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:08.725s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	None

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3

Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1

Global Minimums

Method: bishop simplified

FS	2.343420
Center:	1729.263, 2318.997
Radius:	1548.115
Left Slip Surface Endpoint:	1145.002, 885.365
Right Slip Surface Endpoint:	1573.794, 778.708
Resisting Moment:	3.9008e+08 lb-ft
Driving Moment:	1.66457e+08 lb-ft
Total Slice Area:	4750.35 ft ²
Surface Horizontal Width:	428.792 ft
Surface Average Height:	11.0784 ft

Method: janbu corrected

FS	2.356840
Center:	1729.263, 2318.997
Radius:	1548.115
Left Slip Surface Endpoint:	1145.002, 885.365
Right Slip Surface Endpoint:	1573.794, 778.708
Resisting Horizontal Force:	246421 lb
Driving Horizontal Force:	104556 lb
Total Slice Area:	4750.35 ft ²
Surface Horizontal Width:	428.792 ft
Surface Average Height:	11.0784 ft

Method: spencer

FS	2.341450
Center:	1729.263, 2318.997
Radius:	1548.115
Left Slip Surface Endpoint:	1145.002, 885.365
Right Slip Surface Endpoint:	1573.794, 778.708
Resisting Moment:	3.89752e+08 lb-ft
Driving Moment:	1.66457e+08 lb-ft
Resisting Horizontal Force:	243978 lb
Driving Horizontal Force:	104199 lb
Total Slice Area:	4750.35 ft ²
Surface Horizontal Width:	428.792 ft
Surface Average Height:	11.0784 ft

Method: gle/morgenstern-price

FS	2.339700
Center:	1729.263, 2318.997
Radius:	1548.115
Left Slip Surface Endpoint:	1145.002, 885.365
Right Slip Surface Endpoint:	1573.794, 778.708
Resisting Moment:	3.8946e+08 lb-ft
Driving Moment:	1.66457e+08 lb-ft
Resisting Horizontal Force:	243942 lb
Driving Horizontal Force:	104262 lb
Total Slice Area:	4750.35 ft ²
Surface Horizontal Width:	428.792 ft
Surface Average Height:	11.0784 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4411
Number of Invalid Surfaces:	396

Error Codes

Error Code -109 reported for 2 surfaces
 Error Code -110 reported for 77 surfaces
 Error Code -115 reported for 317 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4411
Number of Invalid Surfaces:	396

Error Codes

Error Code -109 reported for 2 surfaces
 Error Code -110 reported for 77 surfaces
 Error Code -115 reported for 317 surfaces

Method: spencer

Number of Valid Surfaces:	4411
Number of Invalid Surfaces:	396

Error Codes

Error Code -109 reported for 2 surfaces
 Error Code -110 reported for 77 surfaces
 Error Code -115 reported for 317 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces:	4411
Number of Invalid Surfaces:	396

Error Codes

Error Code -109 reported for 2 surfaces
 Error Code -110 reported for 77 surfaces
 Error Code -115 reported for 317 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-109 = Soiltype for slice base not located. This error should occur very rarely, if at all. It may occur if a very low number of slices is combined with certain soil geometries, such that the midpoint of a slice base is actually outside the soil region, even though the slip surface is wholly within the soil region.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-115 = Surface too shallow, below the minimum depth.

Entity Information

◆ Circ - Thin Layers

Shared Entities

Type	Coordinates (x,y)
	1705.77, 744.739
	1691.94, 749.191
	1688.75, 750.234
	1683.79, 751.475
	1649.72, 759.997
	1550.33, 784.49
	1517.24, 792.645
	1498.17, 797.461
	1487.03, 800.244
	1336.39, 837.906
	1312.53, 843.912
	1289.79, 849.649
	1268.54, 855.013
	1248.67, 860.028
	1228.73, 865.012
	1188.81, 874.99
	1149.18, 884.939
	1148.86, 884.971
	1148.57, 885
	1148.54, 885.012
	1143.27, 885.539
	1138.57, 886
	1128.02, 887.055
	1123.25, 887.539
	1118.57, 888
	1107.98, 889.059
	1098.56, 890
	1087.91, 891.065
	1078.56, 892
	1067.84, 893.072
	1058.55, 894
	1056.47, 894.208
	1047.76, 895.079
	1038.54, 896
	1031.85, 896
	1031.28, 896
	1024.07, 896
	1022.73, 896
	1003.75, 894.103

1002.72, 894
1002.59, 893.986
983.533, 892.081
982.719, 892
963.529, 890.082
962.713, 890
943.526, 888.082
942.708, 888
923.523, 886.082
922.702, 886
913.109, 885.041
912.699, 885
912.114, 884.941
912.089, 884.939
877.133, 876.202
859.589, 871.816
841.214, 867.222
840.146, 866.956
822.141, 862.454
802.222, 857.474
781.387, 852.265
759.573, 846.811
736.709, 841.095
712.721, 835.098
685.108, 828.195
626.363, 813.508
564.036, 797.924
447.096, 768.701
412.275, 760
398.83, 756.64
397.597, 756.33
396.492, 756.054
395.41, 755.784
394.351, 755.519
393.315, 755.26
392.3, 755.007
391.307, 754.759
390.334, 754.516
390.102, 754.458
386.057, 753.446
369.041, 747.954
360.315, 745.046
360.041, 744.955
350.343, 744.956
350.039, 744.956
341.31, 747.867
341.038, 747.958
312.062, 748.539
311.035, 748.562
309.721, 749
307.672, 749.682
306.716, 750
300.91, 751.933
300.708, 752
300.446, 752.087
294.699, 754
294.195, 754.168
288.69, 756
288.155, 756.178

282.681, 758
282.067, 758.204
276.673, 760
275.984, 760.229
270.664, 762
267.902, 762.919
259.888, 762.906
256.257, 762.904
253.546, 762
252.468, 761.641
247.546, 760
246.36, 759.605
241.546, 758
236.819, 756.424
235.546, 756
230.819, 754.424
229.546, 754
228.273, 753.576
223.546, 752
222.273, 751.576
217.546, 750
212.017, 748.157
205.774, 746.076
205.581, 746.012
205.411, 745.955
205.123, 745.859
204.822, 745.759
204.532, 745.662
204.114, 745.523
202.582, 745.012
202.37, 744.941
201.485, 744.646
201.379, 744.611
200.794, 744.416
200.126, 744.193
200.07, 744.175
199.416, 743.957
199.195, 743.883
197.758, 743.404
197.28, 743.245
196.972, 743.142
196.867, 743.107
195.97, 742.808
192.571, 741.675
180.393, 737.616
179.777, 737.41
179.104, 737.186
178.684, 737.046
177.683, 736.712
0, 735.281
0, 330
2300, 329.864
2300, 738.23
2246.57, 739.067
2243.38, 739.234
2237.66, 739.488
2236.06, 739.552
2228.45, 739.866
2227.6, 739.905

External Boundary

2224.96, 740
2223.17, 740.005
2222.91, 740.007
2220.2, 740.014
2219.67, 740.014
2212.87, 740.232
2205.39, 740.356
2197.23, 740.483
2193.85, 740.574
2186.16, 740.695
2184.12, 740.751
2173.86, 743.268
2159.21, 746.831
2152.58, 748.429
2136.66, 752.288
2133.09, 753.144
2130.67, 753.722
2128.9, 754.137
2118.44, 756.605
2115.88, 757.214
2112.57, 758
2110.94, 758.385
2109.47, 758.729
2106.6, 759.395
2105.52, 759.642
2103.96, 760
2102.2, 760.402
2101.25, 760.619
2095.2, 762
2092.07, 762.714
2091.21, 762.906
2090.82, 762.992
2090.04, 763.165
2089.21, 763.348
2087.8, 763.655
2086.22, 764
2081.11, 764
2080.83, 764
2071.8, 762.725
2068.87, 762
2062.14, 760.334
2060.79, 760
2055.71, 758.743
2053.39, 758.173
2052.69, 758
2051.46, 757.69
2050.07, 757.348
2049.83, 757.288
2047.2, 756.637
2046.89, 756.56
2020.89, 750.238
2018.62, 749.686
2006.76, 746.88
2004.23, 746.282
1997.83, 744.822
1995.37, 744.264
1991.58, 743.44
1989.28, 742.939
1986.9, 742.454

1984.74, 742.012
1983.22, 741.722
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1981.18, 741.335
1980.18, 741.16
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1976.28, 740.516
1975.78, 740.438
1973.88, 740.177
1973.61, 740.14
1971.91, 739.942
1971.78, 739.934
1971.67, 739.928
1970.03, 739.874
1969.97, 739.872
1966.4, 739.837
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1960.64, 739.995
1958.84, 739.961
1958.8, 739.962
1955.97, 739.894
1955.8, 739.899
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1947.2, 739.799
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1934.96, 740.008
1930.81, 740.006
1930.4, 740.007
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1923.77, 740.001
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1923.21, 740
1920.59, 740.087
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1841.58, 742.146
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1840.84, 742.154
1840.05, 742.158
1838.74, 742.163
1826.34, 742.37
1814.92, 742.579
1800.98, 742.614
1792.37, 742.721
1787.88, 742.786
1779.33, 742.929
1773.39, 742.943
1773.36, 742.944

1769.62, 743.777
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1768.54, 744.035
1766.72, 744.351
1766.68, 744.366
1766.61, 744.388
1766.59, 744.396
1766.25, 744.508
1766.18, 744.534
1766.15, 744.543
1766.11, 744.554
1766.11, 744.557
1765.72, 744.685
1765.58, 744.732
1765.48, 744.767
1765.46, 744.772
1765.43, 744.781
1765.41, 744.79
1765.39, 744.795
1765.38, 744.799
1765.36, 744.806
1765.35, 744.809
1765.3, 744.824
1765.3, 744.826
1765.29, 744.828
1765.27, 744.835
1765.27, 744.837
1765.25, 744.842
1765.24, 744.845
1765.23, 744.848
1763.77, 745.335
1760.89, 745.278
1733.77, 744.737
1728.36, 742.936
1724.77, 741.738
1718.76, 741.738
1714.77, 741.738
1709.37, 743.539

Material Boundary	371.139, 747.994 371.35, 747.924 371.563, 747.853 384.979, 743.38 385.911, 743.07 390.102, 741.672 392.178, 740.98 393.625, 740.498 395.889, 739.743 404.687, 736.81 418.13, 732.329 426.507, 729.536 447.096, 722.672 507.505, 702.534 563.515, 683.862 563.605, 683.831 563.976, 683.705 564.036, 683.685 564.179, 683.637 564.311, 683.592 564.361, 683.575 564.491, 683.575 564.857, 683.576 586.189, 683.61 626.363, 684 698.374, 684.674 703.436, 684.721 840.146, 686 1002.59, 687.5 1031.85, 687.778 1056.47, 688 1070.79, 688.133 1076.9, 688.19 1113.49, 688.53 1198.73, 689.322 1487.03, 692 1546.99, 692.557 1550.33, 693.668 1688.75, 739.809 1696.74, 742.47 1703.25, 744.639 1703.64, 744.772 1703.65, 744.774 1703.66, 744.776 1703.67, 744.781
	369.041, 747.954 369.564, 747.963 371.299, 747.998 378.927, 748.15 379.447, 748.16 386.057, 748.292 386.203, 748.286 389.631, 748.157 390.102, 748 402.215, 743.962 403.086, 743.672 410.125, 741.325 412.226, 740.625 420.368, 737.91

Material Boundary

432.54, 733.852
440.132, 731.322
447.096, 729
503.929, 710.053
539.503, 697.951
558.611, 691.762
558.865, 691.679
562.929, 690.36
564.036, 690.348
567.84, 690.349
591.835, 690.364
623.213, 690.364
626.363, 690.364
694.207, 690.807
696.794, 690.824
748.23, 691.161
758.969, 691.241
840.146, 692
840.75, 692.006
878.605, 692.355
1002.59, 693.5
1031.66, 693.776
1031.69, 693.776
1031.85, 693.778
1056.47, 694
1104.1, 694.443
1146.52, 694.837
1211.99, 695.445
1271.75, 696
1271.77, 696
1289.59, 696.166
1484.09, 698.02
1487.03, 698.047
1488.81, 698.064
1546.16, 698.605
1550.33, 699.993
1550.34, 699.997
1664.79, 738.147
1685.08, 744.911
1685.14, 744.929
1685.15, 744.933
1685.16, 744.935
1685.16, 744.936
1685.18, 744.943
1685.18, 744.943
1688.75, 745.08
1695.33, 744.948
1695.56, 744.943
1695.85, 744.937
1703.67, 744.781
1705.77, 744.739
386.057, 748.292
387.636, 748.688
389.31, 749.106
390.102, 749.304
391.02, 749.533
392.766, 749.97
394.551, 750.415
396.375, 750.871

Material Boundary

398.239, 751.337
400.145, 751.813
402.095, 752.3
404.089, 752.798
406.128, 753.308
411.367, 754.619
447.096, 763.546
449.685, 764.193
564.036, 792.77
626.363, 808.356
792.922, 850
793.256, 850.084
800.923, 852
801.257, 852.084
808.924, 854
809.258, 854.084
816.925, 856
817.259, 856.084
824.926, 858
825.261, 858.084
832.927, 860
833.262, 860.084
840.146, 861.804
840.929, 862
841.263, 862.084
848.931, 864
849.265, 864.083
856.933, 866
857.267, 866.083
864.935, 868
865.269, 868.083
872.938, 870
873.27, 870.083
880.94, 872
881.272, 872.083
888.942, 874
889.273, 874.083
896.944, 876
897.274, 876.082
904.947, 878
905.275, 878.082
912.949, 880.002
913.77, 880.084
932.954, 882
933.767, 882.081
952.96, 884
953.77, 884.081
972.965, 886
973.774, 886.081
992.971, 888
993.788, 888.082
1002.59, 888.961
1012.97, 890
1015.9, 890
1031.85, 890.91
1032.05, 890.921
1048.3, 890
1056.47, 889.183
1057.55, 889.075

1068.31, 888
1079.05, 886.926
1088.31, 886
1098.98, 884.933
1108.31, 884
1118.92, 882.939
1128.32, 882
1138.86, 880.946
1138.94, 880.946
1148.32, 880
1170.76, 874.561
1189.01, 870
1189.01, 870
1211.61, 864.353
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1229.02, 860
1252.48, 854.135
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1269.03, 850
1293.27, 843.94
1309.04, 840
1309.04, 840
1334.07, 833.743
1349.05, 830
1349.05, 830
1374.81, 823.56
1389.05, 820
1389.05, 820
1415.4, 813.414
1429.06, 810
1429.06, 810
1455.94, 803.282
1469.07, 800
1482.16, 796.729
1487.03, 795.51
1509.08, 790
1521.9, 786.796
1549.09, 780
1550.33, 779.69
1561.74, 776.837
1589.1, 770
1601.64, 766.866
1629.1, 760
1641.53, 756.892
1669.11, 749.997
1687.47, 745.402
1688.75, 745.08

Material Boundary

386.057, 748.292
390.102, 746.943
398.905, 744.009
399.789, 743.714
406.969, 741.321
409.105, 740.609
417.384, 737.849
429.814, 733.705
437.565, 731.121
447.096, 727.943
513.062, 705.952
558.73, 690.668
562.767, 689.357
564.036, 689.344
567.392, 689.344
591.838, 689.36
626.363, 689.36
695.698, 689.813
697.254, 689.823
736.377, 690.079
748.109, 690.189
840.146, 691.05
877.729, 691.397
1002.59, 692.55
1030.76, 692.817
1031.85, 692.828
1033.44, 692.842
1033.47, 692.842
1056.47, 693.05
1104.66, 693.498
1147.07, 693.892
1212.44, 694.499
1487.03, 697.05
1532.11, 697.469
1546.04, 697.6
1546.32, 697.603
1550.33, 698.939
1653.01, 733.166
1688.75, 745.08

Material Boundary	371.563, 747.853 386.035, 748.141 390.055, 746.801 398.858, 743.867 399.741, 743.572 406.921, 741.178 409.057, 740.466 417.336, 737.706 429.767, 733.562 437.517, 730.978 447.048, 727.801 513.014, 705.81 558.684, 690.525 562.742, 689.207 564.035, 689.194 567.392, 689.194 591.838, 689.21 626.364, 689.21 695.699, 689.663 697.255, 689.673 736.378, 689.929 748.11, 690.039 840.147, 690.9 877.73, 691.247 1002.59, 692.4 1030.76, 692.667 1031.85, 692.678 1033.44, 692.692 1033.47, 692.692 1056.47, 692.9 1104.66, 693.348 1147.07, 693.742 1212.44, 694.349 1487.04, 696.9 1532.11, 697.319 1546.04, 697.45 1546.34, 697.453 1550.38, 698.797 1653.06, 733.024 1688.78, 744.929 1695.55, 744.793 1703.25, 744.639
	378.927, 748.15 386.841, 750.704 390.794, 751.693 391.025, 751.751 391.998, 751.994 392.991, 752.242 394.005, 752.495 395.042, 752.754 396.101, 753.019 397.183, 753.289 398.289, 753.566 399.521, 753.875 412.966, 757.235 447.787, 765.936 564.727, 795.159 627.055, 810.743 685.799, 825.43


Material Boundary

713.413, 832.333
737.4, 838.33
760.264, 844.046
782.078, 849.5
802.913, 854.709
822.833, 859.689
840.838, 864.191
841.905, 864.458
860.28, 869.051
877.824, 873.437
912.579, 882.124
912.983, 882.164
913.393, 882.205
922.986, 883.164
923.806, 883.246
942.991, 885.164
943.81, 885.246
962.997, 887.164
963.813, 887.246
983.002, 889.164
983.816, 889.246
1002.87, 891.15
1003.01, 891.164
1004.04, 891.267
1022.87, 893.15
1024.07, 893.15
1031.28, 893.15
1031.85, 893.15
1038.4, 893.15
1047.48, 892.243
1056.19, 891.373
1058.27, 891.164
1067.55, 890.236
1078.27, 889.164
1087.62, 888.229
1098.28, 887.164
1107.69, 886.223
1118.28, 885.164
1122.97, 884.703
1127.74, 884.219
1138.29, 883.164
1142.99, 882.702
1147.94, 882.207
1147.97, 882.196
1148.58, 882.135
1148.69, 882.124
1188.12, 872.225
1228.04, 862.248
1247.98, 857.264
1267.84, 852.25
1289.09, 846.886
1311.83, 841.149
1335.69, 835.141
1486.34, 797.479
1497.47, 794.697
1516.55, 789.88
1549.65, 781.723
1649.04, 757.231
1683.09, 748.711

	1687.96, 747.493
	1691.06, 746.481
	1695.85, 744.937
	379.447, 748.16
	386.882, 750.56
	390.83, 751.547
	391.061, 751.605
	392.034, 751.848
	393.027, 752.096
	394.042, 752.35
	395.078, 752.609
	396.137, 752.873
	397.22, 753.144
	398.326, 753.42
	399.558, 753.729
	413.002, 757.089
	447.823, 765.791
	564.764, 795.013
	627.091, 810.598
	685.835, 825.284
	713.449, 832.187
	737.436, 838.184
	760.301, 843.901
	782.115, 849.354
	802.949, 854.563
	822.869, 859.544
	840.874, 864.045
	841.942, 864.312
	860.317, 868.905
	877.86, 873.291
	912.605, 881.976
	912.998, 882.015
	913.408, 882.056
	923, 883.015
	923.821, 883.097
	943.006, 885.015
	943.825, 885.097
	963.012, 887.015
	963.828, 887.096
	983.017, 889.015
	983.831, 889.096
	1002.88, 891.001
Material Boundary	1003.02, 891.015
	1004.05, 891.118
	1022.87, 893
	1024.07, 893
	1031.28, 893
	1031.85, 893
	1038.39, 893
	1047.46, 892.093
	1056.17, 891.223
	1058.26, 891.015
	1067.54, 890.087
	1078.26, 889.015
	1087.61, 888.08
	1098.26, 887.015
	1107.68, 886.073
	1118.27, 885.015
	1122.95, 884.554

	1127.72, 884.07
	1138.27, 883.015
	1142.97, 882.553
	1147.91, 882.06
	1147.94, 882.048
	1148.56, 881.986
	1148.66, 881.976
	1188.08, 872.08
	1228, 862.102
	1247.94, 857.118
	1267.81, 852.104
	1289.06, 846.74
	1311.8, 841.003
	1335.66, 834.996
	1486.31, 797.334
	1497.43, 794.552
	1516.51, 789.734
	1549.61, 781.577
	1649, 757.085
	1683.06, 748.565
	1687.92, 747.349
	1691.01, 746.338
	1695.33, 744.948
Material Boundary	371.139, 747.994
	371.299, 747.998

Scenario-based Entities




Type	Coordinates (x,y)	Static
	-2.66e-14, 730.281	Assigned to:
	178.514, 731.719	 Wadsworth Till
	180.265, 732.302	
	180.685, 732.443	
	181.358, 732.667	
	181.974, 732.872	
	194.152, 736.932	
	197.551, 738.064	
	198.448, 738.364	
	198.554, 738.399	
	198.861, 738.501	
	199.339, 738.661	
	200.776, 739.14	
	200.997, 739.213	
	201.651, 739.431	
	201.708, 739.45	
	202.375, 739.673	
	202.96, 739.868	
	203.066, 739.903	
	203.951, 740.198	
	204.163, 740.269	
	205.695, 740.779	
	206.113, 740.918	
	206.403, 741.015	
	206.704, 741.116	
	206.992, 741.211	
	207.162, 741.268	
	207.356, 741.333	
	213.598, 743.414	
	219.127, 745.257	

223.854, 746.832
225.127, 747.257
229.854, 748.832
231.127, 749.257
232.4, 749.681
237.127, 751.257
238.4, 751.681
243.127, 753.257
247.941, 754.861
249.127, 755.257
254.049, 756.897
255.127, 757.257
257.07, 757.904
259.897, 757.906
267.095, 757.918
269.085, 757.256
274.405, 755.485
275.094, 755.256
280.488, 753.46
281.102, 753.256
286.576, 751.434
287.111, 751.256
292.616, 749.424
293.12, 749.256
298.867, 747.343
299.129, 747.256
299.331, 747.189
305.137, 745.256
306.093, 744.938
308.139, 744.257
310.17, 743.58
311.956, 743.54
340.176, 742.974
349.223, 739.958
350.337, 739.956
360.847, 739.955
361.891, 740.301
370.6, 743.203
385.911, 743.07
390.102, 741.672
392.178, 740.98
393.625, 740.498
395.889, 739.743
404.687, 736.81
418.13, 732.329
426.507, 729.536
447.096, 722.672
507.505, 702.534
563.515, 683.862
563.605, 683.831
563.976, 683.705
564.036, 683.685
564.179, 683.637
564.311, 683.592
564.361, 683.575
564.491, 683.575
564.857, 683.576
586.189, 683.61
626.363, 684

	698.374, 684.674	
	703.436, 684.721	
	840.146, 686	
	1002.59, 687.5	
	1031.85, 687.778	
	1056.47, 688	
	1070.79, 688.133	
	1076.9, 688.19	
	1113.49, 688.53	
	1198.73, 689.322	
	1487.03, 692	
	1546.99, 692.557	
	1550.33, 693.668	
	1688.75, 739.809	
	1689.03, 739.9	
	1704.21, 739.987	
	1707.78, 738.796	
	1713.96, 736.738	
	1718.76, 736.738	
	1725.58, 736.737	
	1729.95, 738.193	
	1734.63, 739.753	
	1760.99, 740.279	
	1763.01, 740.319	
	1763.65, 740.104	
	1763.66, 740.102	
	1763.67, 740.098	
	1763.68, 740.094	
	1763.69, 740.092	
	1763.71, 740.084	
	1763.72, 740.082	
	1763.72, 740.081	
	1763.77, 740.065	
	1763.78, 740.062	
	1763.8, 740.056	
	1763.81, 740.051	
	1763.83, 740.046	
	1763.85, 740.038	
	1763.88, 740.028	
	1763.89, 740.024	
	1764, 739.989	
	1764.14, 739.942	
	1764.53, 739.813	
Water Table	1764.53, 739.811	
	1764.57, 739.8	
	1764.59, 739.79	
	1764.67, 739.764	
	1765.01, 739.653	
	1765.03, 739.645	
	1765.1, 739.622	
	1765.5, 739.489	
	1767.7, 739.105	
	1767.72, 739.101	
	1768.46, 738.913	
	1772.79, 737.949	
	1773.36, 737.944	
	1779.28, 737.93	
	1787.81, 737.786	
	1792.3, 737.722	

1800.94, 737.614
1814.87, 737.579
1826.26, 737.371
1838.69, 737.163
1840.03, 737.158
1840.81, 737.155
1841.22, 737.151
1841.48, 737.147
1847.4, 737.001
1892.14, 736.059
1897.9, 735.837
1915.54, 735.228
1916.61, 735.205
1917.52, 735.183
1920.42, 735.09
1923.11, 735.001
1923.25, 735
1923.71, 735.001
1923.77, 735.001
1927.27, 735.005
1930.39, 735.007
1930.8, 735.006
1934.96, 735.008
1935.34, 735.008
1939.87, 735.009
1940.15, 735.004
1942.25, 734.789
1947.23, 734.799
1952.08, 734.831
1955.77, 734.898
1955.95, 734.892
1958.79, 734.961
1958.82, 734.96
1960.66, 734.994
1962.1, 734.978
1966.34, 734.837
1970.07, 734.872
1970.19, 734.876
1971.89, 734.933
1972.06, 734.942
1972.35, 734.958
1974.24, 735.179
1974.56, 735.224
1976.51, 735.491
1977.05, 735.576
1979.08, 735.893
1981.03, 736.234
1982.08, 736.415
1983.3, 736.648
1984.15, 736.81
1985.71, 737.106
1987.91, 737.555
1990.31, 738.046
1992.64, 738.554
1996.46, 739.383
1998.94, 739.947
2005.36, 741.412
2007.91, 742.015
2017.72, 744.335

2019.79, 744.824
2022.07, 745.38
2048.08, 751.704
2048.4, 751.784
2051.03, 752.434
2051.27, 752.494
2052.67, 752.839
2053.9, 753.148
2054.59, 753.317
2056.91, 753.888
2061.99, 755.146
2063.34, 755.481
2070.07, 757.147
2072.75, 757.81
2081.18, 759
2085.68, 759
2086.73, 758.77
2088.14, 758.464
2088.96, 758.282
2089.74, 758.111
2090.12, 758.026
2090.97, 757.836
2094.09, 757.125
2100.14, 755.745
2101.08, 755.528
2102.84, 755.126
2104.41, 754.768
2105.48, 754.523
2108.33, 753.859
2109.8, 753.517
2111.42, 753.135
2114.73, 752.349
2117.29, 751.74
2127.76, 749.27
2129.51, 748.856
2131.93, 748.281
2135.49, 747.427
2151.4, 743.569
2158.03, 741.971
2172.67, 738.411
2183.45, 735.767
2186.05, 735.696
2193.75, 735.575
2197.12, 735.484
2205.31, 735.357
2212.75, 735.233
2219.59, 735.014
2220.19, 735.014
2222.89, 735.007
2223.15, 735.005
2224.86, 735
2227.4, 734.909
2228.23, 734.871
2235.86, 734.556
2237.44, 734.493
2243.14, 734.24
2246.4, 734.069
2300, 733.229

Piezoline	386.057, 748.292	Assigned to:  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner
	386.203, 748.286	
	389.631, 748.157	
	390.102, 748	
	402.215, 743.962	
	403.086, 743.672	
	410.125, 741.325	
	412.226, 740.625	
	420.368, 737.91	
	432.54, 733.852	
	440.132, 731.322	
	447.096, 729	
	503.929, 710.053	
	539.503, 697.951	
	558.611, 691.762	
	558.865, 691.679	
	562.929, 690.36	
	564.036, 690.348	
	567.84, 690.349	
	591.835, 690.364	
	623.213, 690.364	
	626.363, 690.364	
	694.207, 690.807	
	696.794, 690.824	
	748.23, 691.161	
	758.969, 691.241	
	840.146, 692	
	840.75, 692.006	
	878.605, 692.355	
	1002.59, 693.5	
	1031.66, 693.776	
	1031.69, 693.776	
	1031.85, 693.778	
	1056.47, 694	
	1104.1, 694.443	
	1146.52, 694.837	
	1211.99, 695.445	
	1271.75, 696	
	1271.77, 696	
	1289.59, 696.166	
1484.09, 698.02		
1487.03, 698.047		
1488.81, 698.064		
1546.16, 698.605		
1550.33, 699.993		
1550.34, 699.997		
1664.79, 738.147		
1685.08, 744.911		
1685.14, 744.929		
1685.15, 744.933		
1685.16, 744.935		
1685.16, 744.936		
1685.18, 744.943		
1685.18, 744.943		
1688.75, 745.08		

J.2-D Rapid Drawdown of Detention Basin (Global Stability Analysis)



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: SHEAR STRENGTH EVALUATION OF WADSWORTH TILL DURING RAPID DRAWDOWN OF DETENTION BASIN (GLOBAL STABILITY ANALYSIS)

Problem Statement

Determine the factor of safety of the horizontal expansion during rapid drawdown conditions of the detention basin, with the landfill in final buildout (after final cover placement).

Background

The rapid drawdown condition arises when submerged slopes experience a rapid reduction in water level. The reduction in water level removes the stabilizing force from the weight of the water and the pore pressure of the basin foundation material (Wadsworth Till) will be slow to dissipate. These scenarios will reduce the slope stability of the basin.

This calculation is developed to identify the lowest factor of safety assuming that rapid drawdown of the detention basin occurs with the force of the fully constructed landfill behind it (worst case scenario).

Given

- Hydrogeologic and Design Drawings contained in this Application.
- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this Application.
- Peak horizontal ground acceleration (PGHA) determined from USGS National Seismic Hazard Mapping Project website (value of 0.0461g for horizontal acceleration — please refer to **Appendix J.1**).
- Appendix J.2-C** contained in this Application.
- Computer model SLIDE - An Interactive Slope Stability Program, version 9.006, developed by Rocscience, Inc. was used for the stability analyses.

Assumptions

Selected Critical Cross Section

Critical Cross Section A-A' - Cross Section A-A' as shown on **Figure Nos. 1 and 2** in **Appendix J.2-C** is a critical cross section oriented overall from south to north, starting in the existing landfill and moving north through the vertical expansion and then horizontal expansion, and then through the detention basin. A critical cross section is used to be conservative, as it captures the steepest possible slopes and peak elevations. The cross section is characterized by the following features:

- Peak final landform elevation of 896 feet MSL for the horizontal and vertical expansions;
- Maximum waste column thickness of 198 feet located at peak final landform elevation of approximately 896 feet MSL in the horizontal expansion area (Cell 11);
- Maximum waste column thickness of 206 feet located at final landform elevation of approximately 896 feet MSL in the vertical expansion area (Cell 7);



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- Final cover side slopes of 4H:1V with a slope of 10H:1V across the plateau;
- Cell excavation side slopes of 3H:1V in the horizontal and vertical expansion areas; and
- Detention basin slopes and depth.

Landfill Stages Analyzed and Modes of Failure

Stability of the landfill was analyzed during final buildout (following final cover placement) conditions and during rapid drawdown conditions of the detention basin. There are three methods of rapid drawdown analyses in SLIDE with two of the methods having different interpolation methods:

- Duncan and Wright (3-stage) (1990)
 - o VandenBerge, Wright (2016)
 - o Duncan, Wright and Wong (1990)
- Lowe and Karafiath (1960)
 - o VandenBerge, Wright (2016)
 - o Duncan, Wright and Wong (1990)
- Army Corps (2-Stage) (1970)

These methods relate the undrained strength of the soil (after drawdown) to the pre-drawdown strength.

The stability of the waste mass and foundation after rapid drawdown was evaluated within the SLIDE model using the rotational (circular) failure. This uses a grid search to find the most critical circular failure surfaces within the waste mass and foundation. The grid search was performed in an iterative manner by the SLIDE model user. Each time the user adjusted / fine-tuned the grid to the point where the model generated the absolute lowest factor of safety.

Limit Equilibrium Analysis Methods

The limit equilibrium analysis methods used in the SLIDE model analyses included the Bishop Simplified Method, the Janbu Corrected Method, the Spencer Method, and the GLE (Generalized Limit Equilibrium) / Morgenstern-Price Method. The lowest factor of safety from the four methods used is reported on the SLIDE plot for each modeled scenario (attached pages) and on the summary table on the following page. All of the modeled scenarios are graphically presented on the SLIDE plots provided in the attached pages.



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Failure Conditions

In a rapid drawdown model it is typical to model initial soil loading as effective stress conditions (drained, consolidated). As rapid drawdown occurs, the pore-water pressures present in the soil do not have time to dissipate. Therefore, post-drawdown conditions of the soil become undrained and therefore less stable, and the total stress conditions are assumed (undrained, consolidated).

The model assumes long-term material properties for the pre-drawdown conditions. The long-term shear strength of the Wadsworth Till is obtained from the total stress consolidated undrained (CU) triaxial shear strength test. Therefore, the initial Wadsworth Till conditions are consolidated and undrained. This accurately reflects the groundwater conditions in final buildout, as the groundwater table is consistently high. Short-term shear strength values were used for the Wadsworth Till to mimic the drop in shear strength that occurs after rapid drawdown. The short-term shear strength value in the model are obtained from the unconsolidated undrained (UU) triaxial shear strength test. This is a conservative assumption, as short-term conditions conservatively assume that the waste is placed before the foundation soils are allowed to consolidate and dissipate excess pore-water pressures.

The stability analyses were performed under static and seismic conditions with the assumption that rapid drawdown of the detention basin during final buildout conditions. All material properties are from **Appendices J.1, J.2-A and J.2-C**.

Global Stability Analysis of Rapid Drawdown of the Detention Basin

Global stability of the landfill and detention basin was analyzed to determine if an adequate factor of safety is achieved if rapid drawdown were to occur.

The stability of the landform was evaluated using the unit weights and shear strengths as described in **Appendix J.1** and **Appendix J.2-C**. All materials were modeled under long-term shear strength parameters. After rapid drawdown occurs, the Wadsworth Till was modeled under short-term shear strength parameters. This switch in shear strength of the Wadsworth Till was to model the loss of strength of the Till after rapid drawdown has occurred. Both shear strength conditions are conservative and derived from site specific testing, as described in **Appendix J.1**.

The initial water table (pre-drawdown) was assumed to be located in contact with the bottom the low permeable earth liner and 5-feet below the ground surface (beyond the limits of the landfill). The initial water level in the detention basin was modeled as being 1-foot below the crest of the detention basin. The post-drawdown water level in the detention basin was modeled as five feet below the base of the detention basin. The groundwater was modeled as tying into the water elevation within the detention basin in the initial and post-drawdown water elevations. This is conservative and represents the worst-case groundwater scenario across the site. A liquid level representing a 1-foot leachate level was modeled as a piezometric surface, at the top of the LCS drainage layer.



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TITLE: SHEAR STRENGTH EVALUATION OF WADSWORTH TILL DURING RAPID DRAWDOWN OF DETENTION BASIN (GLOBAL STABILITY ANALYSIS)

Rapid Drawdown Results

The factors of safety for the landfill if rapid drawdown were to occur are shown in **Table J.2D-1**. The factors of safety meet the typical minimum requirements of 1.3 for seismic conditions and 1.5 for static conditions.

Table J.2D-1 Zion Landfill – Site 2 North Expansion Rapid Drawdown Conditions Stability Cross Section A-A' – Horizontal Expansion (northern slope)		
Analysis (Interpolation Method)	Factors of Safety	
	Short-Term Shear Strength	
	Seismic (>1.3)	Static (>1.5)
Duncan, Wright and Wong (VandenBerge, Wright)	1.951	2.657
Duncan, Wright and Wong (Duncan, Wright and Wong)	1.950	2.655
Lowe and Karafiath (VandenBerge, Wright)	2.076	2.771
Lowe and Karafiath (Duncan, Wright and Wong)	2.084	2.728
Army Corp of Engineers (NA)	1.862	2.584

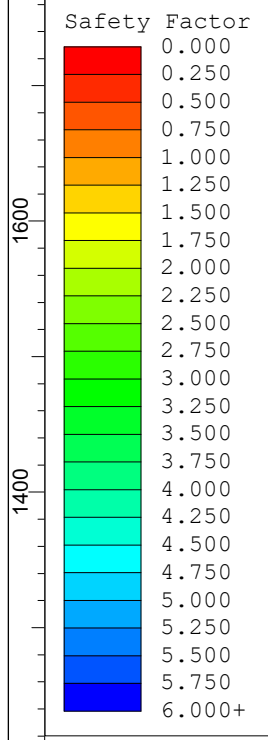
SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION

**RAPID DRAWDOWN GLOBAL STABILITY
DUNCAN, WRIGHT AND WONG ANALYSIS
VANDENBERGE, WRIGHT INTERPOLATION**

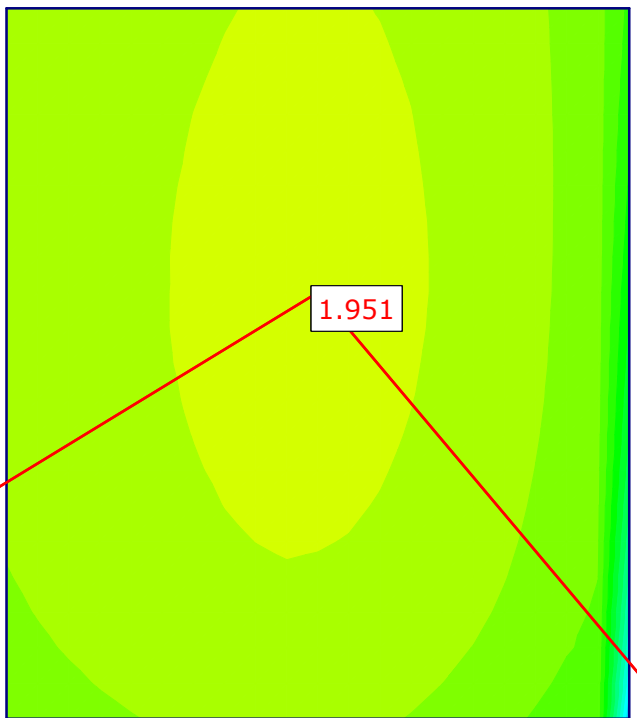
**COMPLETE BUILDOUT LANDFORM
CIRCULAR ANALYSIS
(ROTATIONAL SLOPE FAILURE)**

Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Duncan, Wright and Wong Analysis - VandenBerge, Wright Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Seismic Conditions

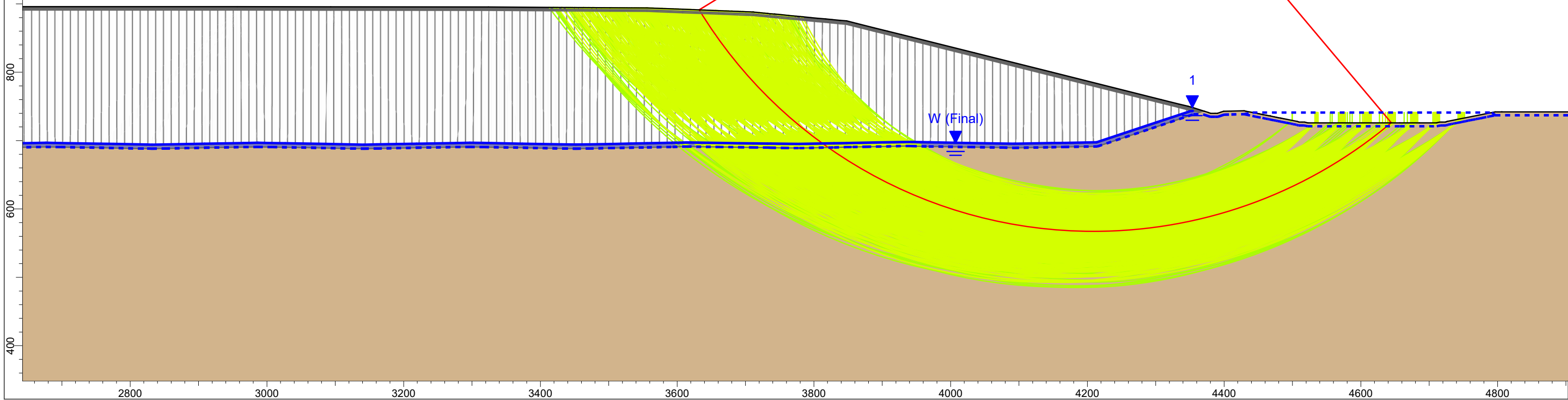
0.0461



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils	[Dark Grey]	121.5	130.3	0	34.3	No		
Final Cover Critical Interface	[Cyan]	121.5	130.3	0	21.9	No		
Expansion Waste Fill	[Vertical Lines]	75	75	0	30	No		
Existing Waste Fill	[Diagonal Lines]	75	75	0	30	No		
LCS Granular Drainage Layer	[Yellow]	126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes	[Light Green]	128.2	134.1	45	14.9	No		
Low Permeable Earth Liner	[Blue]	128.2	134.1	0	34.3	No		
Wadsworth Till	[Brown]	136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.951
 Center: 4209.244, 1243.152
 Radius: 675.984
 Left Slip Surface Endpoint: 3632.178, 891.089
 Right Slip Surface Endpoint: 4644.570, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:53.705s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Duncan, Wright, Wong 3 Stage (1990)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb

Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	2.029960
Center:	4209.244, 1374.191	
Radius:	781.814	
Left Slip Surface Endpoint:	3593.392, 892.569	
Right Slip Surface Endpoint:	4646.374, 726.000	
Resisting Moment:	3.64157e+09 lb-ft	
Driving Moment:	1.79391e+09 lb-ft	
Total Slice Area:	154474 ft2	
Surface Horizontal Width:	1052.98 ft	
Surface Average Height:	146.702 ft	

Method: janbu corrected

	FS	1.950740
Center:	4209.244, 1243.152	
Radius:	675.984	
Left Slip Surface Endpoint:	3632.178, 891.089	
Right Slip Surface Endpoint:	4644.570, 726.000	
Resisting Horizontal Force:	4.69667e+06 lb	
Driving Horizontal Force:	2.40763e+06 lb	
Total Slice Area:	166182 ft2	
Surface Horizontal Width:	1012.39 ft	
Surface Average Height:	164.148 ft	

Method: spencer

FS	2.013800
Center:	4209.244, 1374.191
Radius:	781.814
Left Slip Surface Endpoint:	3593.392, 892.569
Right Slip Surface Endpoint:	4646.374, 726.000
Resisting Moment:	3.61257e+09 lb-ft
Driving Moment:	1.79391e+09 lb-ft
Resisting Horizontal Force:	4.25043e+06 lb
Driving Horizontal Force:	2.11065e+06 lb
Total Slice Area:	154474 ft ²
Surface Horizontal Width:	1052.98 ft
Surface Average Height:	146.702 ft

Method: gle/morgenstern-price

FS	2.015610
Center:	4209.244, 1374.191
Radius:	781.814
Left Slip Surface Endpoint:	3593.392, 892.569
Right Slip Surface Endpoint:	4646.374, 726.000
Resisting Moment:	3.61581e+09 lb-ft
Driving Moment:	1.79391e+09 lb-ft
Resisting Horizontal Force:	4.2543e+06 lb
Driving Horizontal Force:	2.11068e+06 lb
Total Slice Area:	154474 ft ²
Surface Horizontal Width:	1052.98 ft
Surface Average Height:	146.702 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4720
Number of Invalid Surfaces:	131

Error Codes

Error Code -103 reported for 87 surfaces
 Error Code -110 reported for 29 surfaces
 Error Code -118 reported for 15 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4720
Number of Invalid Surfaces:	131

Error Codes

Error Code -103 reported for 87 surfaces
 Error Code -110 reported for 29 surfaces
 Error Code -118 reported for 15 surfaces

Method: spencer

Number of Valid Surfaces: 4720

Number of Invalid Surfaces: 131

Error Codes

Error Code -103 reported for 87 surfaces

Error Code -110 reported for 29 surfaces

Error Code -118 reported for 15 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4720

Number of Invalid Surfaces: 131

Error Codes

Error Code -103 reported for 87 surfaces

Error Code -110 reported for 29 surfaces

Error Code -118 reported for 15 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-118 = Surface does not pass through the search focus

Entity Information**◆ DWW - Van****Shared Entities**

Type	Coordinates (x,y)
	4805.95, 742.054
	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4484.34, 732.521

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4481.87, 733
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4476.94, 734
4473.66, 734.657
4466.94, 736
4466.94, 736
4466.94, 736
4464.44, 736.5
4461.97, 736.994
4456.92, 738
4451.39, 739.111
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4442.27, 740.935
4431.95, 743
4429.16, 743.558
4410.47, 743.103
4399.15, 742.903
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4386.69, 739.881
4380.14, 739.897
4377.03, 740.918
4371.14, 742.896
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4333.51, 753.581
4307.9, 760
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3943.28, 851.154
3927.9, 855
3907.85, 860.012
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3862.48, 871.353
3847.89, 875
3838.52, 875.876
3826.49, 877
3817.3, 877.874
3805.47, 879
3795.83, 879.918
3784.45, 881
3774.39, 881.957
3763.43, 883
3753.28, 883.966
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3742.72, 884.971
3742.42, 885
3736.08, 885.603
3731.91, 886
3712.33, 887.896
3711.25, 888
3660.7, 890
3608.3, 892
3555.9, 894
3501.92, 894.208
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3093.34, 895.755

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2234.57, 896
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2141.81, 896
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External Boundary

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	426.87, 740.119

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465.482, 732
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551.706, 703.298
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609.67, 684
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628.175, 684.242
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768.454, 687.654
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874.622, 686
885.02, 686
885.837, 686

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967.06, 692.116
971.617, 693.286
974.456, 693.923
974.842, 694
982.543, 695.691
984.141, 696
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993.853, 698
1011.96, 701.672
1013.62, 702
1014.48, 702.184
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1036.48, 705.144
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1054.51, 706.425
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1119.86, 702.126
1120.45, 702
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1147.15, 696.628
1150.1, 696
1152.34, 695.587
1158.74, 694
1164.46, 692.516
1180.87, 692.158
1199.19, 692
1219.93, 691.821
1247.58, 691.53
1274.37, 692
1354.83, 693.411
1380.25, 692.892
1449.64, 692
1461.71, 692
1495.01, 692
1500.48, 692

Material Boundary

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1637.28, 690
1692.86, 690
1719.06, 689.137
1781.17, 688.396
1807.13, 688
1816.05, 688
1826.74, 688
1846.47, 687.169
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1855.02, 684.278
1855.86, 684
1856.55, 684
1858.87, 684
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1867.43, 685.773
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1868.4, 686.102
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1875.79, 688.44
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1881.86, 690.439
1886.54, 692
1891.23, 693.563
1892.54, 694
1893.8, 694.436
1893.84, 694.436
1898.54, 696
1899.79, 696.434
1899.84, 696.434
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1905.79, 698.433
1905.84, 698.433
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1911.78, 700.431
1911.83, 700.431
1916.54, 702
1917.78, 702.43
1917.83, 702.429
1922.54, 704
1923.77, 704.428
1923.82, 704.428
1928.54, 706
1929.77, 706.426
1929.82, 706.426
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1935.76, 708.425
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1965.76, 718.417
1965.79, 718.417
1970.54, 720
1971.76, 720.416
1971.79, 720.416
1976.54, 722
1977.76, 722.414
1977.79, 722.414
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1987.31, 725.589
1988.52, 726
1988.54, 726
1993.32, 727.59
1994.52, 728
1994.54, 728
1995.75, 728.41
1995.77, 728.41
2000.55, 730
2001.74, 730.408
2001.77, 730.408
2006.55, 732
2007.74, 732.406
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2018.55, 736
2019.73, 736.403
2019.76, 736.403
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2031.73, 740.4
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2521.93, 693.112
2580.01, 694.119
2600.94, 694.482
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	3555.83, 891
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713.234, 811.867
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714.999, 812.308
715.007, 812.31
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726.775, 815.254
730.101, 816.086
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746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
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811.81, 836.513
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826.101, 840.086
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842.101, 844.086
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850.101, 846.086
851.073, 846.329
858.101, 848.086
859.116, 848.34
866.101, 850.086
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874.101, 852.086
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882.101, 854.086
883.256, 854.375
884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773


Material Boundary

906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
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946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
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962.101, 874.086
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1036.44, 892.671
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1066.1, 900.086
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1071.45, 901.423

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1106.1, 910.086
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1278.63, 919.622
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	1446.69, 895.936
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	3712.05, 885.059
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	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
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	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
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	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	
	689.108, 680.721	
	700.659, 681.002	
	711.292, 681.261	
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	724.458, 681.581	

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760.228, 682.452
768.453, 682.652
775.361, 682.481
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792.858, 682.05
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853.768, 681
858.565, 681
861.768, 681
864.838, 681
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876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
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914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
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923.072, 679.061
924.878, 679.141
925.393, 679.164
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953.372, 683.186
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983.137, 690.678
992.201, 692.586
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Water Table

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	-9e-16, 723.896 0.809461, 723.908 1.15168, 723.913 1.45658, 723.917 1.93345, 723.924 4.30816, 723.96 12.6281, 724.084 24.2881, 724.257 113.381, 725.584 150.125, 726.131 251.754, 727.644 262.684, 727.807 264.857, 727.839 267.412, 727.877 267.74, 727.882 267.83, 727.883 268.093, 727.885 268.368, 727.882 269.061, 727.877 272.624, 727.847 281.529, 727.773 284.508, 727.748 284.959, 727.743 286.557, 727.708 289.734, 727.637 295.936, 727.498 301.316, 727.378 309.904, 727.186 313.768, 727.1 314.73, 727 317.794, 727 322.428, 727 327.892, 727 334.438, 727 352.838, 735.003 356.385, 735.013 366.425, 735.2 372.719, 735.318 379.181, 735.415 391.383, 735.573 399.078, 735.005 407.323, 734.789 412.029, 734.347 439.628, 735.289 488.131, 719.138 522.779, 707.602 608.698, 679 618.307, 679	

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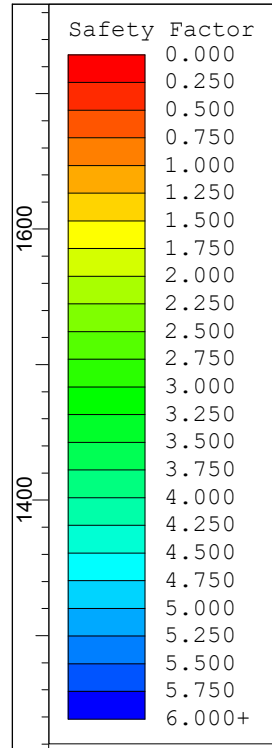
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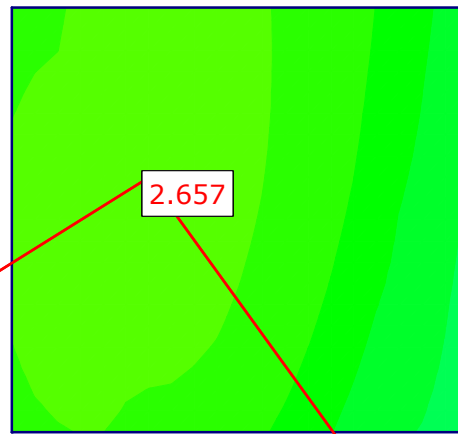
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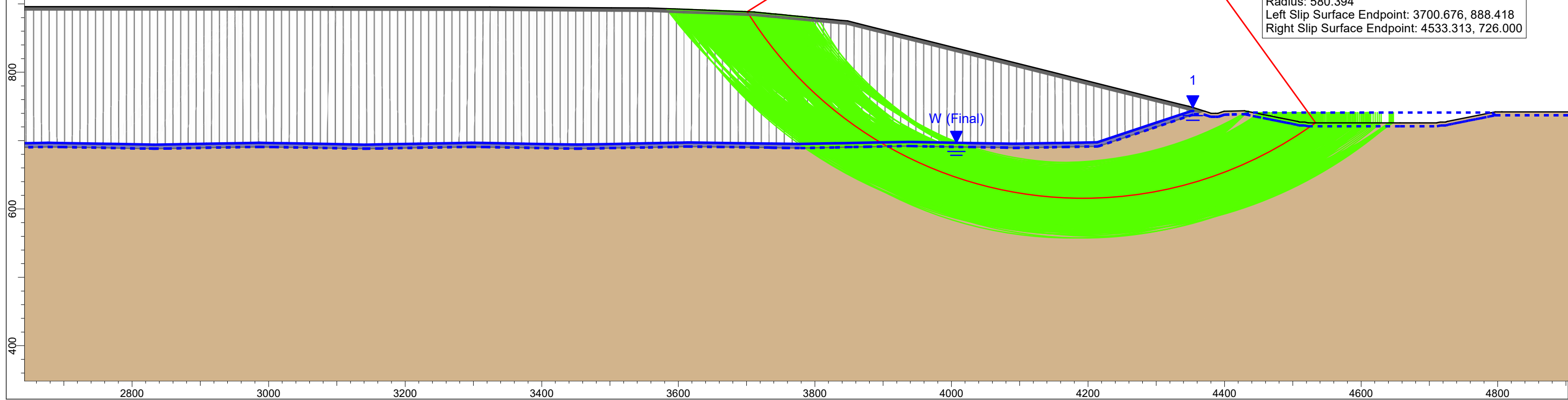


Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Duncan, Wright and Wong Analysis - VandenBerge, Wright Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Janbu corrected
 Factor of Safety: 2.657
 Center: 4192.842, 1196.039
 Radius: 580.394
 Left Slip Surface Endpoint: 3700.676, 888.418
 Right Slip Surface Endpoint: 4533.313, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:47.141s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Duncan, Wright, Wong 3 Stage (1990)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

FS	2.764060
Center:	4209.664, 1321.306
Radius:	696.993
Left Slip Surface Endpoint:	3662.194, 889.941
Right Slip Surface Endpoint:	4572.168, 726.000
Resisting Moment:	2.57999e+09 lb-ft
Driving Moment:	9.33404e+08 lb-ft
Total Slice Area:	112586 ft2
Surface Horizontal Width:	909.974 ft
Surface Average Height:	123.724 ft

Method: janbu corrected

FS	2.657280
Center:	4192.842, 1196.039
Radius:	580.394
Left Slip Surface Endpoint:	3700.676, 888.418
Right Slip Surface Endpoint:	4533.313, 726.000
Resisting Horizontal Force:	3.42358e+06 lb
Driving Horizontal Force:	1.28838e+06 lb
Total Slice Area:	109554 ft2
Surface Horizontal Width:	832.637 ft
Surface Average Height:	131.574 ft

Method: spencer

FS	2.735100
Center:	4226.486, 1321.306
Radius:	689.917
Left Slip Surface Endpoint:	3688.902, 888.884
Right Slip Surface Endpoint:	4575.192, 726.000
Resisting Moment:	2.35855e+09 lb-ft
Driving Moment:	8.62326e+08 lb-ft
Resisting Horizontal Force:	3.14768e+06 lb
Driving Horizontal Force:	1.15085e+06 lb
Total Slice Area:	102836 ft ²
Surface Horizontal Width:	886.29 ft
Surface Average Height:	116.029 ft

Method: gl/morgenstern-price

FS	2.731910
Center:	4226.486, 1321.306
Radius:	689.917
Left Slip Surface Endpoint:	3688.902, 888.884
Right Slip Surface Endpoint:	4575.192, 726.000
Resisting Moment:	2.3558e+09 lb-ft
Driving Moment:	8.62326e+08 lb-ft
Resisting Horizontal Force:	3.14725e+06 lb
Driving Horizontal Force:	1.15203e+06 lb
Total Slice Area:	102836 ft ²
Surface Horizontal Width:	886.29 ft
Surface Average Height:	116.029 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4830
Number of Invalid Surfaces:	21

Error Codes

Error Code -110 reported for 17 surfaces
 Error Code -118 reported for 4 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4829
Number of Invalid Surfaces:	22

Error Codes

Error Code -110 reported for 17 surfaces
 Error Code -118 reported for 4 surfaces
 Error Code -200 reported for 1 surface

Method: spencer

Number of Valid Surfaces: 4829

Number of Invalid Surfaces: 22

Error Codes

Error Code -110 reported for 17 surfaces

Error Code -118 reported for 4 surfaces

Error Code -200 reported for 1 surface

Method: gle/morgenstern-price

Number of Valid Surfaces: 4828

Number of Invalid Surfaces: 23

Error Codes

Error Code -108 reported for 1 surface

Error Code -110 reported for 17 surfaces

Error Code -118 reported for 4 surfaces

Error Code -200 reported for 1 surface

Error Code Descriptions

The following errors were encountered during the computation:

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-118 = Surface does not pass through the search focus

-200 = Factor of Safety <= min iteration value. Could mean 0 Normal/Shear resistance along part of the slip surface

Entity Information**DWW - Van****Shared Entities**

Type	Coordinates (x,y)
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755.043, 822.321
762.101, 824.086
766.886, 825.282
770.101, 826.086
777.704, 827.987
777.707, 827.987
778.101, 828.086
778.761, 828.251
786.101, 830.086
793.117, 831.84
793.125, 831.842
794.101, 832.086
795.089, 832.333
802.101, 834.086
808.438, 835.67
810.101, 836.086
811.81, 836.513
818.101, 838.086
819.843, 838.521
826.101, 840.086
833.041, 841.821
834.101, 842.086
835.202, 842.361
842.101, 844.086
849.167, 845.852
850.101, 846.086
851.073, 846.329
858.101, 848.086
859.116, 848.34
866.101, 850.086
867.161, 850.351
874.101, 852.086
880.995, 853.809
882.101, 854.086
883.256, 854.375
884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773


Material Boundary

906.101, 860.086
912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
949.761, 871.001
950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
964.913, 874.789
970.101, 876.086
973.942, 877.046
978.101, 878.086
982.774, 879.254
986.101, 880.086
987.201, 880.361
989.058, 880.825
989.174, 880.854
990.419, 881.165
994.101, 882.086
999.027, 883.317
999.144, 883.347
1001, 883.811
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1002.54, 884.195
1002.57, 884.202
1002.93, 884.294
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1016.33, 887.642
1018.1, 888.086
1024.39, 889.659
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1031.94, 891.544
1034.1, 892.086
1036.43, 892.667
1036.44, 892.671
1042.1, 894.086
1043.67, 894.478
1050.1, 896.086
1051.6, 896.461
1058.1, 898.086
1064.74, 899.745
1066.1, 900.086
1071.43, 901.419
1071.45, 901.423

1074.1, 902.086
1076.99, 902.808
1077.01, 902.812
1082.1, 904.086
1088.96, 905.8
1090.1, 906.086
1097.03, 907.819
1098.1, 908.086
1105.11, 909.839
1106.1, 910.086
1110.71, 911.237
1110.72, 911.241
1114.25, 912.124
1131.8, 913.879
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1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
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1219.87, 922.15
1223.74, 922.15
1239.2, 922.15
1244.03, 922.15
1247.67, 922.15
1263.87, 920.811
1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
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1340.6, 915.011
1349.31, 914.473
1353.82, 914.144
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1386.13, 910.94
1389.43, 910.122
1389.43, 910.122
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1389.61, 910.077
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1405.93, 906.034
1413.79, 904.087
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1414.79, 903.84
1421.86, 902.087
1428.46, 900.451
1428.47, 900.45
1429.93, 900.087
1430.39, 899.974

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	1438.54, 897.955
	1446.08, 896.087
	1446.69, 895.936
	1454.15, 894.087
	1456.17, 893.588
	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
	1616.33, 893.15
	1858.87, 893.15
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	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
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	2956.54, 893.15
	3093.33, 892.905
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	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738

Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	
	608.698, 679	
	618.307, 679	
	629.067, 679.262	
	642.777, 679.595	
	647.559, 679.711	
	660.405, 680.023	
	675.999, 680.402	
	689.108, 680.721	
	700.659, 681.002	
	711.292, 681.261	
	721.531, 681.51	
	724.458, 681.581	

731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
945.459, 681.092
948.634, 682.055
953.372, 683.186
964.682, 686.266
968.611, 687.32
970.51, 687.807
973.537, 688.568
983.137, 690.678
992.201, 692.586
995.001, 693.111
1005.88, 695.316
1015.48, 697.262
1026.16, 698.595
1029.95, 699.071
1037.13, 700.174
1049.53, 700.993
1053.89, 701.352

Water Table

1060.67, 701.77
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1072.74, 701.68
1077.4, 701.408
1085.39, 700.951
1093.76, 700.535
1103.93, 699.206
1105.21, 699.039
1108.51, 698.705
1108.62, 698.694
1115.88, 697.749
1119.27, 697.117
1126.35, 695.778
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1143.12, 692.32
1144.75, 691.993
1145.42, 691.857
1146.43, 691.649
1147.98, 691.316
1148.3, 691.248
1149.18, 691.068
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1156.36, 689.409
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1180.77, 687.159
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1274.47, 687.001
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1692.76, 685
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1816.05, 683
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3064.92, 689.458
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3218.92, 689.461
3276.19, 690.454
3296.93, 690.814
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4347.22, 735.704
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4375.46, 736.173
4379.34, 734.899




	4386.69, 734.881 4390.98, 734.901 4397.07, 736.947 4400, 737.917 4410.58, 738.104 4428.73, 738.546 4441.57, 741.074 4791.53, 741.074 4796.96, 737.054 5009.05, 737.054	
	-9e-16, 723.896 0.809461, 723.908 1.15168, 723.913 1.45658, 723.917 1.93345, 723.924 4.30816, 723.96 12.6281, 724.084 24.2881, 724.257 113.381, 725.584 150.125, 726.131 251.754, 727.644 262.684, 727.807 264.857, 727.839 267.412, 727.877 267.74, 727.882 267.83, 727.883 268.093, 727.885 268.368, 727.882 269.061, 727.877 272.624, 727.847 281.529, 727.773 284.508, 727.748 284.959, 727.743 286.557, 727.708 289.734, 727.637 295.936, 727.498 301.316, 727.378 309.904, 727.186 313.768, 727.1 314.73, 727 317.794, 727 322.428, 727 327.892, 727 334.438, 727 352.838, 735.003 356.385, 735.013 366.425, 735.2 372.719, 735.318 379.181, 735.415 391.383, 735.573 399.078, 735.005 407.323, 734.789 412.029, 734.347 439.628, 735.289 488.131, 719.138 522.779, 707.602 608.698, 679 618.307, 679	

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	<p>4737.03, 725.097 4740.55, 725.801 4747.03, 727.097 4748.79, 727.448 4752.03, 728.097 4753.78, 728.448 4757.03, 729.097 4760.53, 729.797 4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717 515.347, 716.4 519.551, 715 521.35, 714.401 525.559, 713 529.162, 711.807 537.749, 709 539.827, 708.301 543.591, 707 545.694, 706.3 549.599, 705 551.706, 704.298 555.606, 703</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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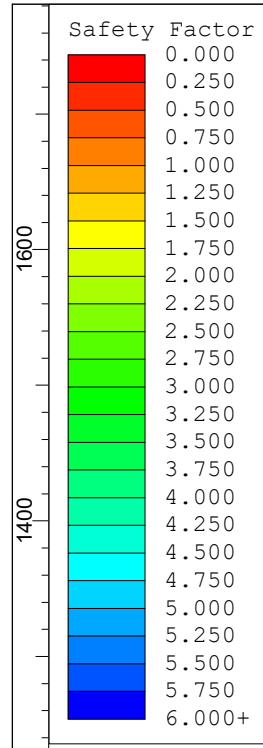
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SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION

**RAPID DRAWDOWN GLOBAL STABILITY
DUNCAN, WRIGHT AND WONG ANALYSIS
DUNCAN, WRIGHT AND WONG INTERPOLATION**

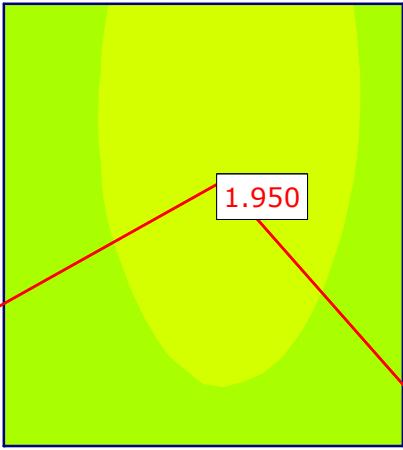
**COMPLETE BUILDOUT LANDFORM
CIRCULAR ANALYSIS
(ROTATIONAL SLOPE FAILURE)**



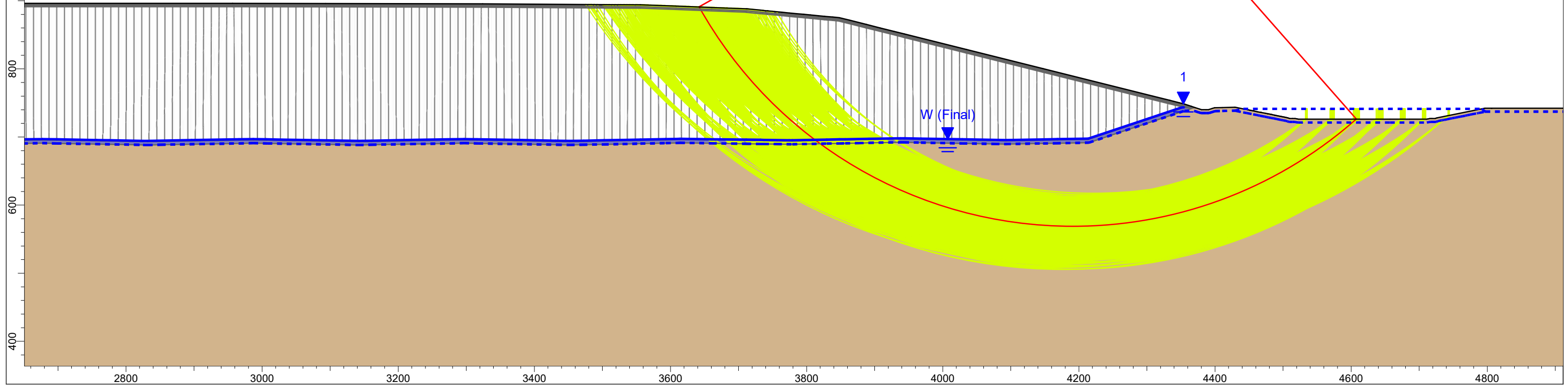
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Duncan, Wright and Wong Analysis and Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Seismic Conditions

0.0461

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils	[Grey]	121.5	130.3	0	34.3	No		
Final Cover Critical Interface	[Cyan]	121.5	130.3	0	21.9	No		
Expansion Waste Fill	[White with vertical lines]	75	75	0	30	No		
Existing Waste Fill	[White with diagonal lines]	75	75	0	30	No		
LCS Granular Drainage Layer	[Yellow]	126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes	[Light Green]	128.2	134.1	45	14.9	No		
Low Permeable Earth Liner	[Blue]	128.2	134.1	0	34.3	No		
Wadsworth Till	[Brown]	136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.950
 Center: 4191.167, 1198.526
 Radius: 629.838
 Left Slip Surface Endpoint: 3641.662, 890.727
 Right Slip Surface Endpoint: 4607.598, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:56.107s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Duncan, Wright, Wong 3 Stage (1990)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb

Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	2.031810
Center:		4220.618, 1329.029
Radius:		737.196
Left Slip Surface Endpoint:		3627.471, 891.268
Right Slip Surface Endpoint:		4644.663, 726.000
Resisting Moment:		3.29535e+09 lb-ft
Driving Moment:		1.62188e+09 lb-ft
Total Slice Area:		147681 ft2
Surface Horizontal Width:		1017.19 ft
Surface Average Height:		145.185 ft

Method: janbu corrected

	FS	1.950050
Center:		4191.167, 1198.526
Radius:		629.838
Left Slip Surface Endpoint:		3641.662, 890.727
Right Slip Surface Endpoint:		4607.598, 726.000
Resisting Horizontal Force:		4.52006e+06 lb
Driving Horizontal Force:		2.31791e+06 lb
Total Slice Area:		159979 ft2
Surface Horizontal Width:		965.935 ft
Surface Average Height:		165.621 ft

Method: spencer

FS	2.016870
Center:	4220.618, 1329.029
Radius:	737.196
Left Slip Surface Endpoint:	3627.471, 891.268
Right Slip Surface Endpoint:	4644.663, 726.000
Resisting Moment:	3.27112e+09 lb-ft
Driving Moment:	1.62188e+09 lb-ft
Resisting Horizontal Force:	4.0602e+06 lb
Driving Horizontal Force:	2.01312e+06 lb
Total Slice Area:	147681 ft ²
Surface Horizontal Width:	1017.19 ft
Surface Average Height:	145.185 ft

Method: gle/morgenstern-price

FS	2.017470
Center:	4205.893, 1329.029
Radius:	745.208
Left Slip Surface Endpoint:	3602.115, 892.236
Right Slip Surface Endpoint:	4643.719, 726.000
Resisting Moment:	3.51257e+09 lb-ft
Driving Moment:	1.74108e+09 lb-ft
Resisting Horizontal Force:	4.31346e+06 lb
Driving Horizontal Force:	2.13806e+06 lb
Total Slice Area:	159369 ft ²
Surface Horizontal Width:	1041.6 ft
Surface Average Height:	153.004 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4829
Number of Invalid Surfaces:	22

Error Codes

Error Code -103 reported for 22 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4829
Number of Invalid Surfaces:	22

Error Codes

Error Code -103 reported for 22 surfaces

Method: spencer

Number of Valid Surfaces:	4829
Number of Invalid Surfaces:	22

Error Codes

Error Code -103 reported for 22 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4829

Number of Invalid Surfaces: 22

Error Codes

Error Code -103 reported for 22 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

Entity Information

DWW - DWW**Shared Entities**

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	4710.54, 726
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	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
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	4484.34, 732.521
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Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
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	372.719, 735.318	
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	399.078, 735.005	
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DRAWDOWN LINE

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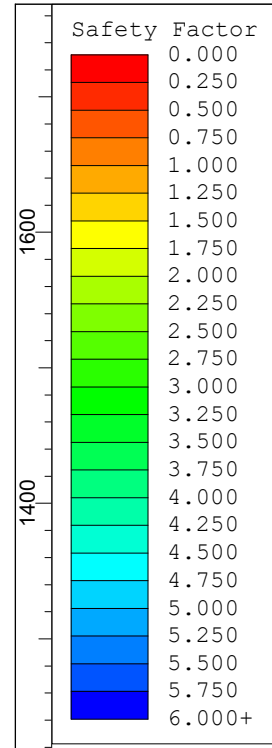
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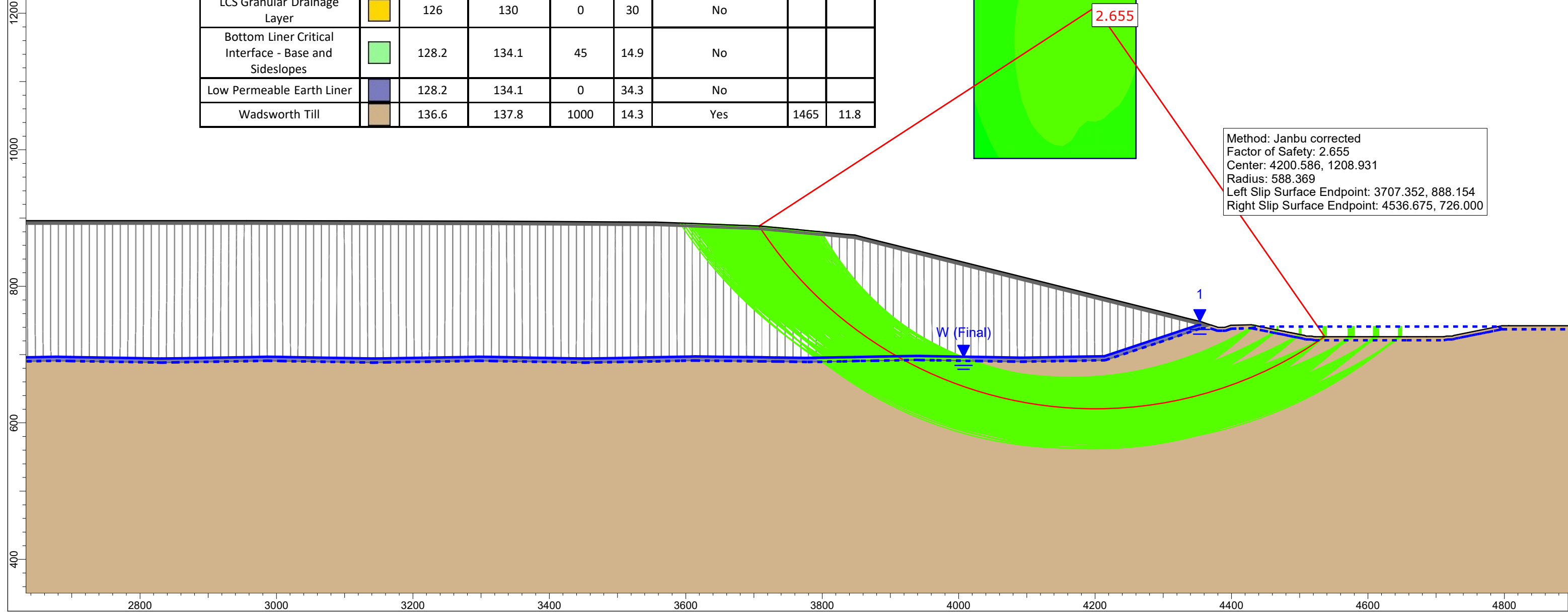
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3084.07, 695.126
3141.93, 694.125
3160.86, 694.454
3218.92, 695.461
3276.16, 696.453

	3296.93, 696.814 3315.7, 696.489 3373.9, 695.482 3431.21, 694.491 3451.93, 694.132 3472.59, 694.49 3501.92, 694.999 3532.41, 695.598 3555.9, 696.059 3592.57, 696.784 3608.3, 697.095 3613.84, 697.204 3660.7, 696.618 3690.91, 696.239 3711.25, 695.984 3731.91, 695.696 3736.97, 695.63 3774.17, 695.144 3778.84, 695.083 3829.52, 696.079 3847.89, 696.44 3856.62, 696.594 3891.54, 697.211 3937.75, 698.028 3943.28, 697.934 3967.87, 697.514 4028.8, 696.473 4092.79, 695.38 4147.28, 696.336 4168.34, 696.706 4213.55, 697.499 4302.23, 727.041 4347.22, 741.988 4350.55, 743.096 4352.88, 743.177 4354.12, 743.273	
Focus Search Window	4434.16, 723.545 4762.52, 723.545 4762.52, 752 4434.16, 742.558	

Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Duncan, Wright and Wong Analysis and Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Static Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:50.57s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Duncan, Wright, Wong 3 Stage (1990)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	2.769340
Center:		4200.586, 1282.848
Radius:		650.412
Left Slip Surface Endpoint:		3682.884, 889.122
Right Slip Surface Endpoint:		4536.676, 726.000
Resisting Moment:		2.22357e+09 lb-ft
Driving Moment:		8.02925e+08 lb-ft
Total Slice Area:		101740 ft2
Surface Horizontal Width:		853.792 ft
Surface Average Height:		119.162 ft

Method: janbu corrected

	FS	2.655280
Center:		4200.586, 1208.931
Radius:		588.369
Left Slip Surface Endpoint:		3707.352, 888.154
Right Slip Surface Endpoint:		4536.675, 726.000
Resisting Horizontal Force:		3.32984e+06 lb
Driving Horizontal Force:		1.25404e+06 lb
Total Slice Area:		105188 ft2
Surface Horizontal Width:		829.324 ft
Surface Average Height:		126.836 ft

Method: spencer

FS	2.738080
Center:	4200.586, 1282.848
Radius:	650.412
Left Slip Surface Endpoint:	3682.884, 889.122
Right Slip Surface Endpoint:	4536.676, 726.000
Resisting Moment:	2.19847e+09 lb-ft
Driving Moment:	8.02925e+08 lb-ft
Resisting Horizontal Force:	3.09967e+06 lb
Driving Horizontal Force:	1.13206e+06 lb
Total Slice Area:	101740 ft ²
Surface Horizontal Width:	853.792 ft
Surface Average Height:	119.162 ft

Method: gl/morgenstern-price

FS	2.737540
Center:	4212.455, 1282.848
Radius:	644.682
Left Slip Surface Endpoint:	3702.572, 888.343
Right Slip Surface Endpoint:	4537.316, 726.000
Resisting Moment:	2.05877e+09 lb-ft
Driving Moment:	7.52051e+08 lb-ft
Resisting Horizontal Force:	2.93527e+06 lb
Driving Horizontal Force:	1.07223e+06 lb
Total Slice Area:	94434.3 ft ²
Surface Horizontal Width:	834.744 ft
Surface Average Height:	113.13 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4782
Number of Invalid Surfaces:	69

Error Codes

Error Code -103 reported for 69 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4772
Number of Invalid Surfaces:	79

Error Codes

Error Code -103 reported for 69 surfaces

Error Code -108 reported for 7 surfaces

Error Code -200 reported for 3 surfaces

Method: spencer

Number of Valid Surfaces: 4770

Number of Invalid Surfaces: 81

Error Codes

Error Code -103 reported for 69 surfaces

Error Code -108 reported for 9 surfaces

Error Code -200 reported for 3 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4773

Number of Invalid Surfaces: 78

Error Codes

Error Code -103 reported for 69 surfaces

Error Code -108 reported for 6 surfaces

Error Code -200 reported for 3 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-200 = Factor of Safety <= min iteration value. Could mean 0 Normal/Shear resistance along part of the slip surface

Entity Information**DWW - DWW****Shared Entities**

Type	Coordinates (x,y)
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	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4484.34, 732.521

4481.94, 733
4481.87, 733
4479.43, 733.489
4476.94, 734
4476.94, 734
4473.66, 734.657
4466.94, 736
4466.94, 736
4466.94, 736
4464.44, 736.5
4461.97, 736.994
4456.94, 738
4456.94, 738
4451.39, 739.111
4447.57, 739.873
4442.27, 740.935
4431.95, 743
4429.16, 743.558
4410.47, 743.103
4399.15, 742.903
4395.49, 741.691
4390.15, 739.897
4386.69, 739.881
4380.14, 739.897
4377.03, 740.918
4371.14, 742.896
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4333.51, 753.581
4307.9, 760
4147.28, 800.154
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3943.28, 851.154
3927.9, 855
3907.85, 860.012
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3817.3, 877.874
3805.47, 879
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3774.39, 881.957
3763.43, 883
3753.28, 883.966
3743.06, 884.939
3742.72, 884.971
3742.42, 885
3736.08, 885.603
3731.91, 886
3712.33, 887.896
3711.25, 888
3660.7, 890
3608.3, 892
3555.9, 894
3501.92, 894.208

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2486.56, 896
2475.13, 896
2234.57, 896
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2181.65, 896
2141.81, 896
2127.47, 896
1858.87, 896
1616.33, 896
1495.01, 896
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1456.86, 896.353
1456.85, 896.354
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1431.08, 902.74
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1406.4, 908.853
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1390.12, 912.889
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1380.94, 914.966
1373.16, 915.464
1354.04, 916.986
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1247.79, 925
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1239.2, 925
1223.74, 925
1219.79, 925
1211.96, 924.561
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1197.39, 923.302

External Boundary

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1192.25, 922.788
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998.336, 886.082
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988.367, 883.59
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973.251, 879.811
969.409, 878.851
964.222, 877.554

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	449.065, 748.76
	441.43, 746.851
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	351.399, 739.824
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	331.196, 732
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577.692, 694.645
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618.233, 684
628.127, 684.241
628.175, 684.242
700.514, 686
768.454, 687.654
835.534, 686
874.622, 686

885.02, 686
885.837, 686
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914.512, 684.056
916.783, 684
918.713, 683.952
920.67, 684
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933.943, 684.923
944.266, 686
947.065, 686.849
951.886, 688
965.137, 691.608
966.644, 692
967.06, 692.116
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974.842, 694
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Material Boundary

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882.101, 854.086
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898.101, 858.086
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Material Boundary


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	3712.05, 885.059
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	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
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	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
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	3826.22, 874.163
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	3847.41, 872.183
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	3907.15, 857.247
	3927.2, 852.235
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	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	

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647.559, 679.711
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675.999, 680.402
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700.659, 681.002
711.292, 681.261
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724.458, 681.581
731.21, 681.745
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746.542, 682.118
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920.654, 678.998
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921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
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Water Table

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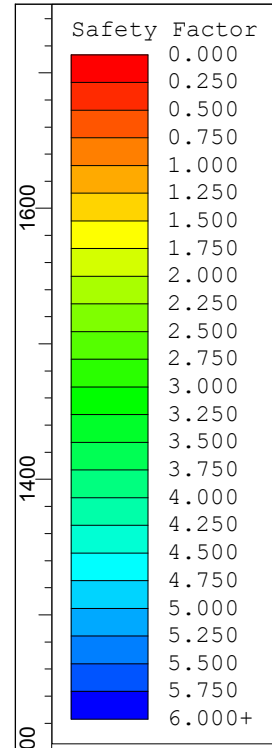
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SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION

**RAPID DRAWDOWN GLOBAL STABILITY
LOWE AND KARAFIATH ANALYSIS
VANDENBERGE, WRIGHT INTERPOLATION**

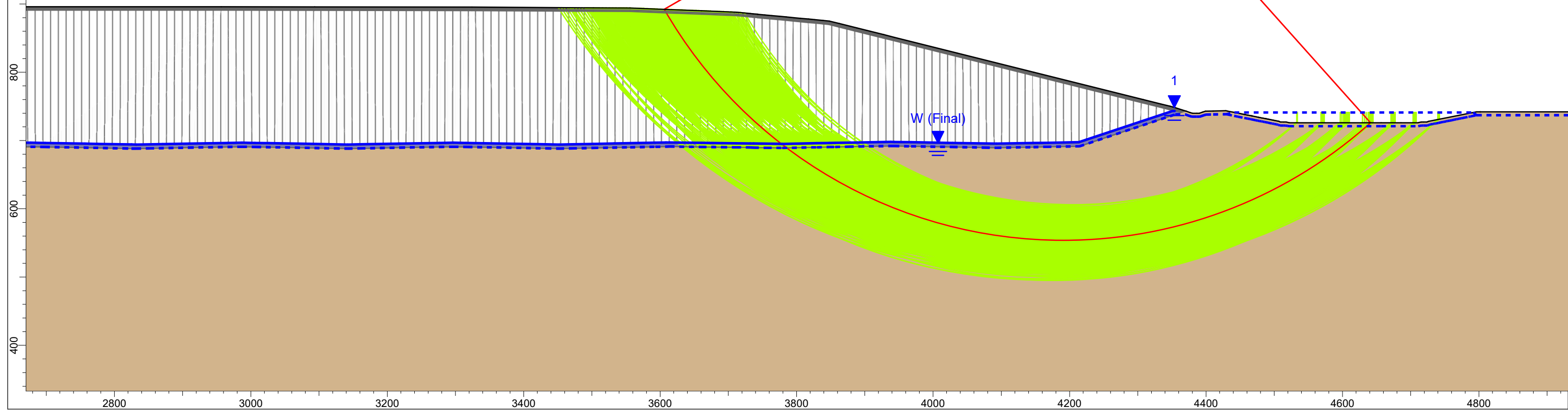
**COMPLETE BUILDOUT LANDFORM
CIRCULAR ANALYSIS
(ROTATIONAL SLOPE FAILURE)**



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Lowe and Karafiath Analysis - VandenBerge, Wright Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Seismic Conditions



Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils	Grey	121.5	130.3	0	34.3	No		
Final Cover Critical Interface	Cyan	121.5	130.3	0	21.9	No		
Expansion Waste Fill	White with vertical lines	75	75	0	30	No		
Existing Waste Fill	White with diagonal lines	75	75	0	30	No		
LCS Granular Drainage Layer	Yellow	126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes	Light Green	128.2	134.1	45	14.9	No		
Low Permeable Earth Liner	Dark Blue	128.2	134.1	0	34.3	No		
Wadsworth Till	Brown	136.6	137.8	1000	14.3	Yes	1465	11.8



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:56.819s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Lowe and Karafiath (1960)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb

Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

FS	2.172020
Center:	4205.188, 1368.202
Radius:	795.952
Left Slip Surface Endpoint:	3566.205, 893.607
Right Slip Surface Endpoint:	4675.419, 726.000
Resisting Moment:	4.44557e+09 lb-ft
Driving Moment:	2.04674e+09 lb-ft
Total Slice Area:	179145 ft2
Surface Horizontal Width:	1109.21 ft
Surface Average Height:	161.506 ft

Method: janbu corrected

FS	2.076110
Center:	4190.808, 1229.037
Radius:	675.133
Left Slip Surface Endpoint:	3605.764, 892.097
Right Slip Surface Endpoint:	4641.095, 726.000
Resisting Horizontal Force:	5.39345e+06 lb
Driving Horizontal Force:	2.59786e+06 lb
Total Slice Area:	182626 ft2
Surface Horizontal Width:	1035.33 ft
Surface Average Height:	176.394 ft

Method: spencer

FS	2.154950
Center:	4205.188, 1368.202
Radius:	777.420
Left Slip Surface Endpoint:	3590.149, 892.693
Right Slip Surface Endpoint:	4643.319, 726.000
Resisting Moment:	3.87576e+09 lb-ft
Driving Moment:	1.79854e+09 lb-ft
Resisting Horizontal Force:	4.58945e+06 lb
Driving Horizontal Force:	2.12973e+06 lb
Total Slice Area:	156252 ft ²
Surface Horizontal Width:	1053.17 ft
Surface Average Height:	148.363 ft

Method: gle/morgenstern-price

FS	2.156370
Center:	4205.188, 1368.202
Radius:	777.420
Left Slip Surface Endpoint:	3590.149, 892.693
Right Slip Surface Endpoint:	4643.319, 726.000
Resisting Moment:	3.87831e+09 lb-ft
Driving Moment:	1.79854e+09 lb-ft
Resisting Horizontal Force:	4.59425e+06 lb
Driving Horizontal Force:	2.13055e+06 lb
Total Slice Area:	156252 ft ²
Surface Horizontal Width:	1053.17 ft
Surface Average Height:	148.363 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4836
Number of Invalid Surfaces:	15

Error Codes

Error Code -103 reported for 15 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4836
Number of Invalid Surfaces:	15

Error Codes

Error Code -103 reported for 15 surfaces

Method: spencer

Number of Valid Surfaces:	4836
Number of Invalid Surfaces:	15

Error Codes

Error Code -103 reported for 15 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4836

Number of Invalid Surfaces: 15

Error Codes

Error Code -103 reported for 15 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

Entity Information

◆ L&K - Van**Shared Entities**

Type	Coordinates (x,y)
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	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4486.94, 732
	4484.34, 732.521
	4481.94, 733
	4481.87, 733
	4479.43, 733.489
	4476.94, 734
	4476.94, 734
	4473.66, 734.657
	4466.94, 736
	4466.94, 736
	4466.94, 736
	4464.44, 736.5
	4461.97, 736.994
	4461.43, 737.102
	4456.94, 738
	4456.94, 738
	4451.39, 739.111

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External Boundary

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Material Boundary

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	3470.7, 693.458
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	3660.7, 695.618
	3690.86, 695.24
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654.377, 797.152
658.111, 798.086
662.404, 799.162
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742.842, 819.271
746.101, 820.086
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753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
766.886, 825.282
770.101, 826.086
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777.707, 827.987
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826.101, 840.086
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849.167, 845.852
850.101, 846.086
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858.101, 848.086
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866.101, 850.086
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874.101, 852.086
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882.101, 854.086
883.256, 854.375
884.146, 854.597
890.101, 856.086
895.862, 857.526
898.101, 858.086
904.848, 859.773

Material Boundary


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915.521, 862.441
922.101, 864.086
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930.101, 866.086
936.16, 867.601
938.101, 868.086
944.562, 869.701
945.181, 869.856
945.941, 870.046
946.101, 870.086
948.285, 870.632
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950.891, 871.283
951.569, 871.453
954.101, 872.086
958.644, 873.222
960.237, 873.62
962.101, 874.086
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1002.57, 884.202
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1071.45, 901.423

1074.1, 902.086
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1106.1, 910.086
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1110.72, 911.241
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1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
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1212.16, 921.718
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1223.74, 922.15
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1380.54, 912.136
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1389.43, 910.122
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1414.79, 903.84
1421.86, 902.087
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1428.47, 900.45
1429.93, 900.087
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	1446.69, 895.936
	1454.15, 894.087
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	1456.17, 893.587
	1457.93, 893.15
	1495.01, 893.15
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	2141.81, 893.15
	2181.65, 893.15
	2195.59, 893.15
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	2475.13, 893.15
	2486.56, 893.15
	2956.54, 893.15
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	3555.84, 891.15
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	3660.59, 887.152
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	3712.05, 885.059
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	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	

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647.559, 679.711
660.405, 680.023
675.999, 680.402
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700.659, 681.002
711.292, 681.261
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731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
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853.768, 681
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879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
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914.27, 679.058
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920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
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968.611, 687.32

Water Table

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	-9e-16, 723.896 0.809461, 723.908 1.15168, 723.913 1.45658, 723.917 1.93345, 723.924 4.30816, 723.96 12.6281, 724.084 24.2881, 724.257 113.381, 725.584 150.125, 726.131 251.754, 727.644 262.684, 727.807 264.857, 727.839 267.412, 727.877 267.74, 727.882 267.83, 727.883 268.093, 727.885 268.368, 727.882 269.061, 727.877 272.624, 727.847 281.529, 727.773 284.508, 727.748 284.959, 727.743 286.557, 727.708 289.734, 727.637 295.936, 727.498 301.316, 727.378 309.904, 727.186 313.768, 727.1 314.73, 727 317.794, 727 322.428, 727 327.892, 727 334.438, 727 352.838, 735.003 356.385, 735.013	

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Drawdown Line

DRAWDOWN LINE

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2600.94, 689.482
2620.01, 689.813
2676.93, 690.8
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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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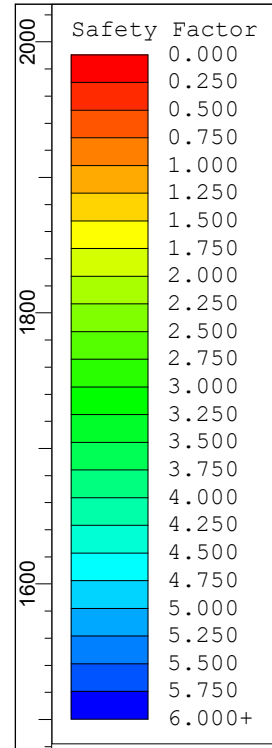
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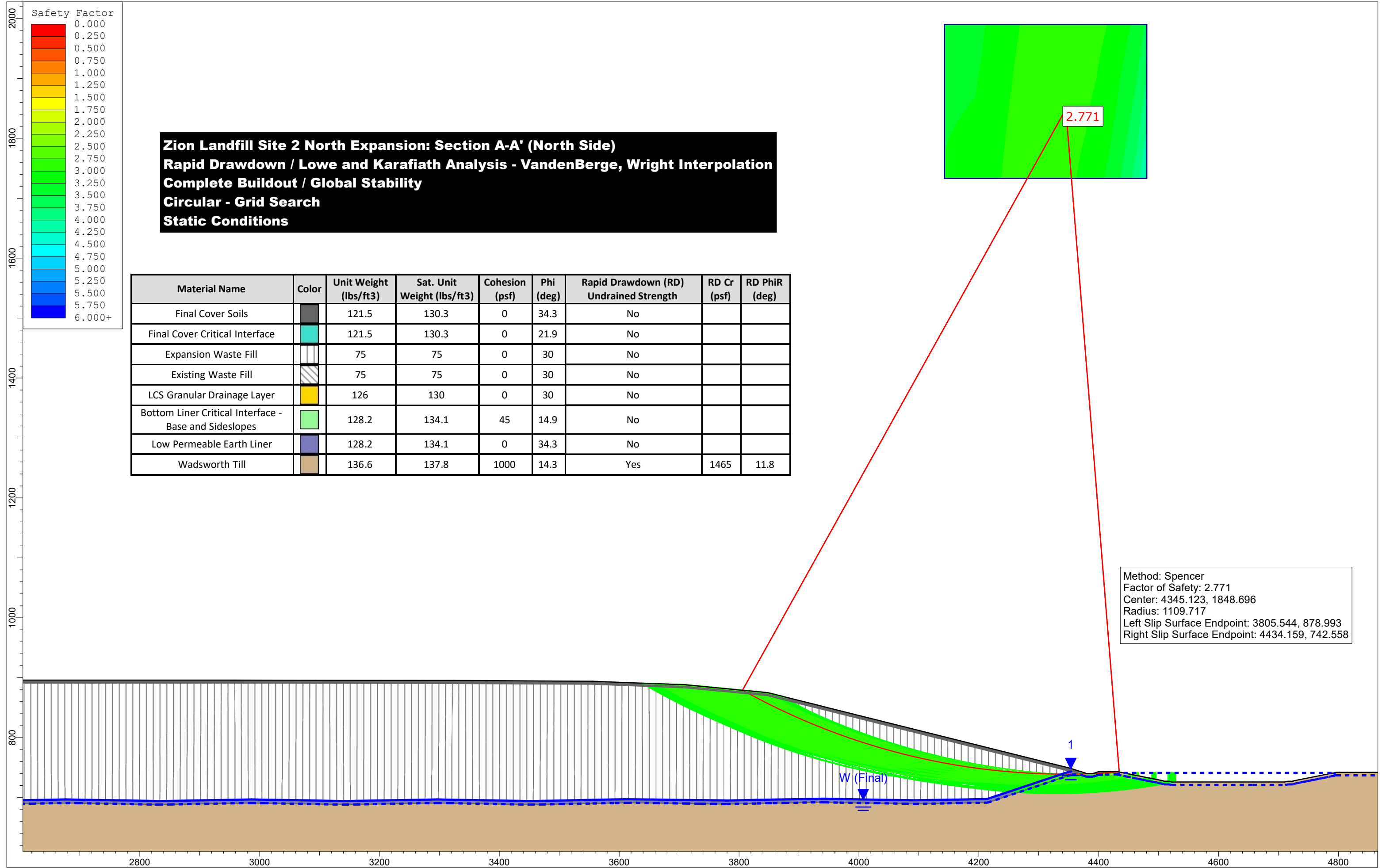
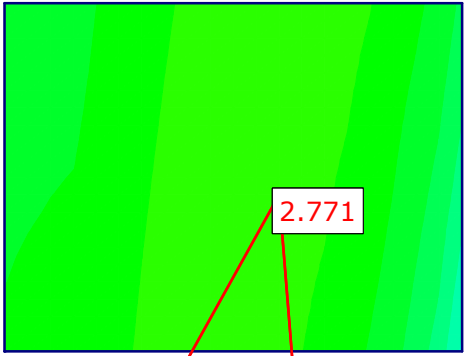
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Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Lowe and Karafiath Analysis - VandenBerge, Wright Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Spencer
 Factor of Safety: 2.771
 Center: 4345.123, 1848.696
 Radius: 1109.717
 Left Slip Surface Endpoint: 3805.544, 878.993
 Right Slip Surface Endpoint: 4434.159, 742.558

Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:52.346s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Lowe and Karafiath (1960)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	2.782260
Center:		4345.123, 1848.696
Radius:		1109.717
Left Slip Surface Endpoint:		3805.544, 878.993
Right Slip Surface Endpoint:		4434.159, 742.558
Resisting Moment:		1.08473e+09 lb-ft
Driving Moment:		3.89873e+08 lb-ft
Total Slice Area:		18354.6 ft2
Surface Horizontal Width:		628.615 ft
Surface Average Height:		29.1985 ft

Method: janbu corrected

	FS	2.814670
Center:		4345.123, 1848.696
Radius:		1109.717
Left Slip Surface Endpoint:		3805.544, 878.993
Right Slip Surface Endpoint:		4434.159, 742.558
Resisting Horizontal Force:		977161 lb
Driving Horizontal Force:		347167 lb
Total Slice Area:		18354.6 ft2
Surface Horizontal Width:		628.615 ft
Surface Average Height:		29.1985 ft

Method: spencer

FS	2.771350
Center:	4345.123, 1848.696
Radius:	1109.717
Left Slip Surface Endpoint:	3805.544, 878.993
Right Slip Surface Endpoint:	4434.159, 742.558
Resisting Moment:	1.08048e+09 lb-ft
Driving Moment:	3.89873e+08 lb-ft
Resisting Horizontal Force:	943351 lb
Driving Horizontal Force:	340394 lb
Total Slice Area:	18354.6 ft ²
Surface Horizontal Width:	628.615 ft
Surface Average Height:	29.1985 ft

Method: gl/morgenstern-price

FS	2.777760
Center:	4345.123, 1848.696
Radius:	1109.717
Left Slip Surface Endpoint:	3805.544, 878.993
Right Slip Surface Endpoint:	4434.159, 742.558
Resisting Moment:	1.08297e+09 lb-ft
Driving Moment:	3.89873e+08 lb-ft
Resisting Horizontal Force:	945523 lb
Driving Horizontal Force:	340391 lb
Total Slice Area:	18354.6 ft ²
Surface Horizontal Width:	628.615 ft
Surface Average Height:	29.1985 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4422
Number of Invalid Surfaces:	429

Error Codes

Error Code -110 reported for 66 surfaces
 Error Code -118 reported for 363 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4422
Number of Invalid Surfaces:	429

Error Codes

Error Code -110 reported for 66 surfaces
 Error Code -118 reported for 363 surfaces

Method: spencer

Number of Valid Surfaces:	4422
Number of Invalid Surfaces:	429

Error Codes

Error Code -110 reported for 66 surfaces
 Error Code -118 reported for 363 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4422
 Number of Invalid Surfaces: 429

Error Codes

Error Code -110 reported for 66 surfaces
 Error Code -118 reported for 363 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

- 110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.
- 118 = Surface does not pass through the search focus

Entity Information

◆ L&K - Van

Shared Entities

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1389.43, 910.122
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	1454.15, 894.087
	1456.17, 893.588
	1456.17, 893.587
	1457.93, 893.15
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	1858.87, 893.15
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	2181.65, 893.15
	2195.59, 893.15
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	2475.13, 893.15
	2486.56, 893.15
	2956.54, 893.15
	3093.33, 892.905
	3324.03, 892.493
	3501.9, 891.358
	3555.84, 891.15
	3608.19, 889.152
	3660.59, 887.152
	3711.06, 885.155
	3712.05, 885.059
	3731.64, 883.163
	3735.81, 882.765
	3742.15, 882.163
	3742.45, 882.134
	3742.79, 882.102
	3753.01, 881.129
	3763.16, 880.163
	3774.12, 879.12
	3784.18, 878.163
	3795.56, 877.08
	3805.2, 876.163
	3817.03, 875.037
	3826.22, 874.163
	3838.25, 873.038
	3847.41, 872.183
	3861.79, 868.588
	3887.2, 862.235
	3907.15, 857.247
	3927.2, 852.235
	3942.59, 848.389
	4034.21, 825.483
	4146.59, 797.389
	4307.21, 757.235
	4332.82, 750.816
	4346.52, 747.389
	4353.25, 745.706
	4353.34, 745.686
	4361.26, 743.112
Material Boundary	4447.57, 739.873
	4456.92, 738
	4461.97, 736.994

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	

608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
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675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
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724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
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920.654, 678.998
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921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
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932.042, 679.457
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968.611, 687.32

Water Table

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983.137, 690.678
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	-9e-16, 723.896 0.809461, 723.908 1.15168, 723.913 1.45658, 723.917 1.93345, 723.924 4.30816, 723.96 12.6281, 724.084 24.2881, 724.257 113.381, 725.584 150.125, 726.131 251.754, 727.644 262.684, 727.807 264.857, 727.839 267.412, 727.877 267.74, 727.882 267.83, 727.883 268.093, 727.885 268.368, 727.882 269.061, 727.877 272.624, 727.847 281.529, 727.773 284.508, 727.748 284.959, 727.743 286.557, 727.708 289.734, 727.637 295.936, 727.498 301.316, 727.378 309.904, 727.186 313.768, 727.1 314.73, 727 317.794, 727 322.428, 727 327.892, 727 334.438, 727 352.838, 735.003 356.385, 735.013	

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1500.34, 687

Drawdown Line

DRAWDOWN LINE

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4075.88, 689.669
4092.79, 689.38
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4441.29, 736.032
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4500.96, 724.097
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4506.46, 722.809
4508.48, 722
4513.9, 722
4516.95, 722
4519.28, 721.534
4521.95, 721
4656.84, 721
4685.73, 721

	<p>4711.04, 721 4713.3, 721.452 4716.04, 722 4718.38, 722 4724.51, 722 4726.29, 722.711 4727.48, 723.187 4728.79, 723.45 4732.03, 724.097 4732.91, 724.274 4734.53, 724.597 4735.41, 724.773 4737.03, 725.097 4740.55, 725.801 4747.03, 727.097 4748.79, 727.448 4752.03, 728.097 4753.78, 728.448 4757.03, 729.097 4760.53, 729.797 4763.5, 730.39 4767.03, 731.097 4767.91, 731.272 4769.54, 731.598 4771.42, 731.978 4774.43, 732.582 4776.5, 733 4776.53, 733 4784.64, 734.61 4786.11, 734.902 4786.67, 735.014 4787.93, 735.263 4795.37, 736.738 4795.62, 736.787 4796.96, 737.054 5009.05, 737.054</p>	
	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

515.347, 716.4
519.551, 715
521.35, 714.401
525.559, 713
529.162, 711.807
537.749, 709
539.827, 708.301
543.591, 707
545.694, 706.3
549.599, 705
551.706, 704.298
555.606, 703
560.534, 701.359
567.615, 699
570.071, 698.182
573.623, 697
577.692, 695.645
579.631, 695
583.706, 693.643
583.706, 693.643
585.639, 693
594.551, 690.033
597.655, 689
599.571, 688.362
603.662, 687
608.129, 685.513
609.67, 685
618.233, 685
628.127, 685.241
628.175, 685.242
700.514, 687
768.454, 688.654
835.534, 687
874.622, 687
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885.837, 687
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914.512, 685.056
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1013.62, 703

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1036.48, 706.144
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1692.86, 691
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1874.47, 689
1875.79, 689.44
1880.49, 691
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1881.86, 691.439

Piezoline

1886.54, 693
1891.23, 694.563
1892.54, 695
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1893.84, 695.436
1898.54, 697
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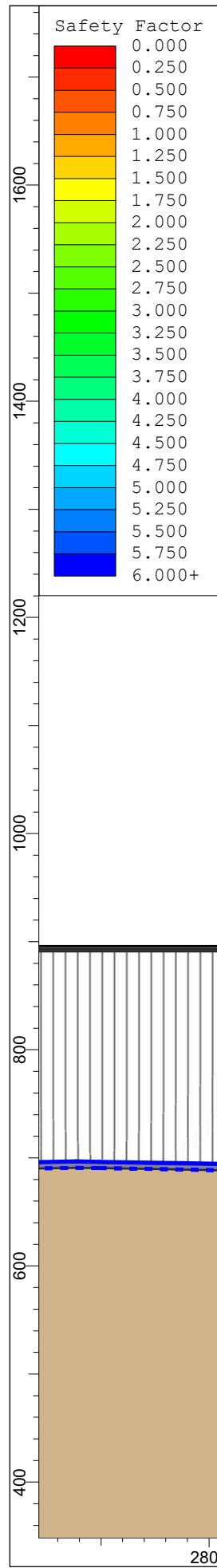
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SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION

**RAPID DRAWDOWN GLOBAL STABILITY
LOWE AND KARAFIATH ANALYSIS
DUNCAN, WRIGHT AND WONG INTERPOLATION**

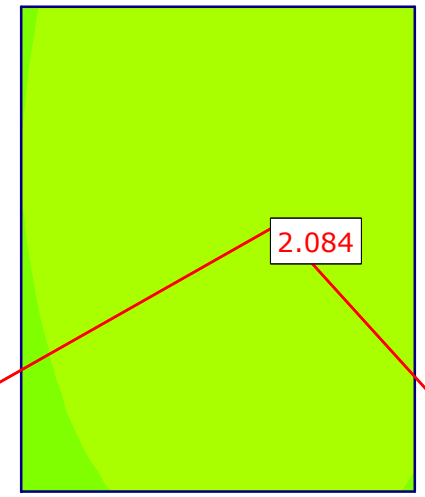
**COMPLETE BUILDOUT LANDFORM
CIRCULAR ANALYSIS
(ROTATIONAL SLOPE FAILURE)**



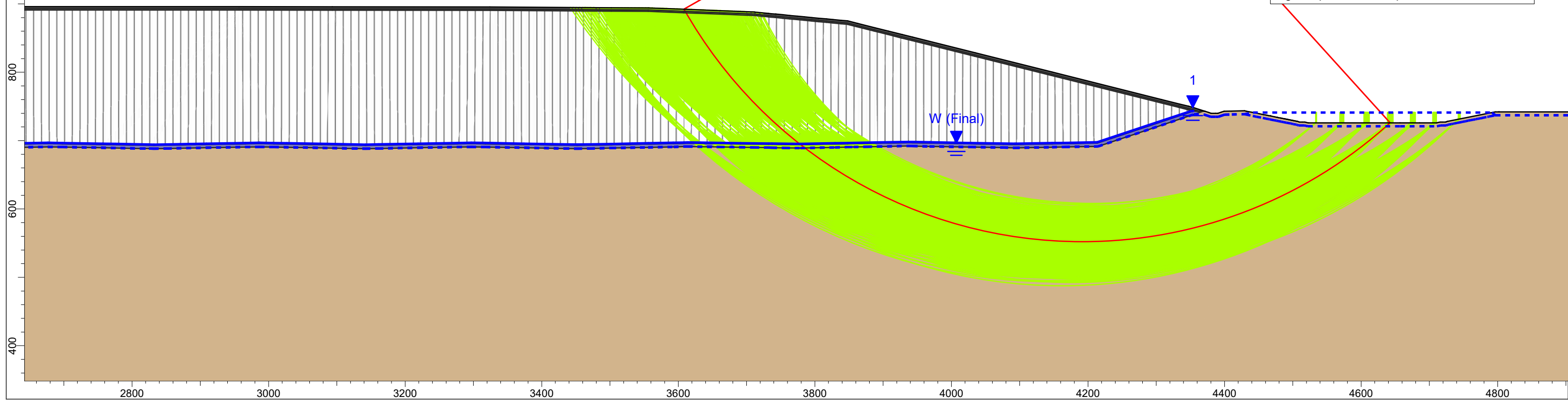
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Lowe and Krafiath Analysis - Duncan, Wright and Wong Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Seismic Conditions

0.0461

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils	[Dark Grey]	121.5	130.3	0	34.3	No		
Final Cover Critical Interface	[Cyan]	121.5	130.3	0	21.9	No		
Expansion Waste Fill	[White with vertical lines]	75	75	0	30	No		
Existing Waste Fill	[White with diagonal lines]	75	75	0	30	No		
LCS Granular Drainage Layer	[Yellow]	126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes	[Light Green]	128.2	134.1	45	14.9	No		
Low Permeable Earth Liner	[Dark Blue]	128.2	134.1	0	34.3	No		
Wadsworth Till	[Brown]	136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Janbu corrected
 Factor of Safety: 2.084
 Center: 4191.680, 1222.990
 Radius: 670.937
 Left Slip Surface Endpoint: 3608.063, 892.009
 Right Slip Surface Endpoint: 4642.410, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:51.498s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Lowe and Karafiath (1960)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb

Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	2.176140
Center:	4206.199, 1384.008	
Radius:	791.103	
Left Slip Surface Endpoint:	3586.032, 892.850	
Right Slip Surface Endpoint:	4645.367, 726.000	
Resisting Moment:	3.97539e+09 lb-ft	
Driving Moment:	1.82681e+09 lb-ft	
Total Slice Area:	155518 ft2	
Surface Horizontal Width:	1059.34 ft	
Surface Average Height:	146.807 ft	

Method: janbu corrected

	FS	2.084050
Center:	4191.680, 1222.990	
Radius:	670.937	
Left Slip Surface Endpoint:	3608.063, 892.009	
Right Slip Surface Endpoint:	4642.410, 726.000	
Resisting Horizontal Force:	5.43033e+06 lb	
Driving Horizontal Force:	2.60566e+06 lb	
Total Slice Area:	183710 ft2	
Surface Horizontal Width:	1034.35 ft	
Surface Average Height:	177.61 ft	

Method: spencer

FS	2.162260
Center:	4206.199, 1384.008
Radius:	809.494
Left Slip Surface Endpoint:	3562.038, 893.766
Right Slip Surface Endpoint:	4677.693, 726.000
Resisting Moment:	4.4931e+09 lb-ft
Driving Moment:	2.07797e+09 lb-ft
Resisting Horizontal Force:	5.10144e+06 lb
Driving Horizontal Force:	2.35931e+06 lb
Total Slice Area:	178308 ft ²
Surface Horizontal Width:	1115.65 ft
Surface Average Height:	159.823 ft

Method: gle/morgenstern-price

FS	2.162260
Center:	4220.718, 1366.117
Radius:	768.610
Left Slip Surface Endpoint:	3615.990, 891.707
Right Slip Surface Endpoint:	4646.173, 726.000
Resisting Moment:	3.63711e+09 lb-ft
Driving Moment:	1.68209e+09 lb-ft
Resisting Horizontal Force:	4.36218e+06 lb
Driving Horizontal Force:	2.01742e+06 lb
Total Slice Area:	145661 ft ²
Surface Horizontal Width:	1030.18 ft
Surface Average Height:	141.394 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4830
Number of Invalid Surfaces:	21

Error Codes

Error Code -103 reported for 21 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4830
Number of Invalid Surfaces:	21

Error Codes

Error Code -103 reported for 21 surfaces

Method: spencer

Number of Valid Surfaces:	4830
Number of Invalid Surfaces:	21

Error Codes

Error Code -103 reported for 21 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4830

Number of Invalid Surfaces: 21

Error Codes

Error Code -103 reported for 21 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

Entity Information

◆ L&K - DWW**Shared Entities**

Type	Coordinates (x,y)
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	4796.47, 742.054
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	4724.43, 727.354
	4723.55, 727
	4718.38, 727
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	4710.54, 726
	4685.73, 726
	4656.84, 726
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	4520.26, 726.437
	4517.44, 727
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	4509.44, 727
	4508.31, 727.451
	4506.94, 728
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	4033.69, 823.397
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	4306.68, 755.149
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	4346, 745.303
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	4354.12, 743.273
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	450.157, 745.94
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	454.153, 746.94
	458.147, 747.94
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	466.139, 749.94
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	474.149, 751.941
	474.848, 752.114

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514.14, 761.94
514.848, 762.119
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602.147, 783.941
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898.137, 857.94
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Material Boundary

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713.234, 811.867
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882.101, 854.086
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884.146, 854.597
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898.101, 858.086
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Material Boundary


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912.746, 861.747
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944.562, 869.701
945.181, 869.856
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1071.45, 901.423

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1162.12, 916.911
1174.41, 918.139
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3735.81, 882.765
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3784.18, 878.163
3795.56, 877.08
3805.2, 876.163
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4332.82, 750.816
4346.52, 747.389
4353.25, 745.706
4353.34, 745.686
4361.26, 743.112

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	

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618.307, 679
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642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
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700.659, 681.002
711.292, 681.261
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724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
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888.824, 680.784
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914.27, 679.058
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918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
940.841, 680.61
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948.634, 682.055
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968.611, 687.32

Water Table

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992.201, 692.586
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1029.95, 699.071
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1068.84, 701.86
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1077.4, 701.408
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1093.76, 700.535
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1119.27, 697.117
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1151.08, 690.719
1156.36, 689.409
1157.27, 689.184
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1180.77, 687.159
1199.14, 687
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Drawdown Line

DRAWDOWN LINE

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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79 507.535, 719 509.344, 718.398 513.543, 717</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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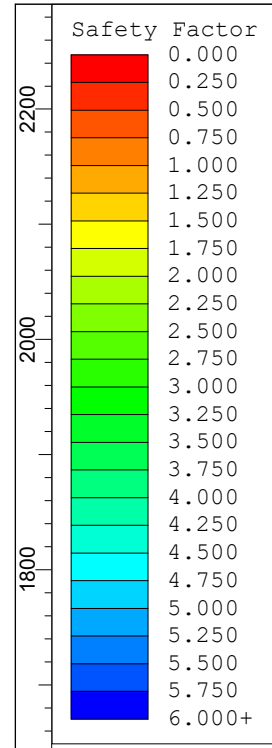
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Piezoline

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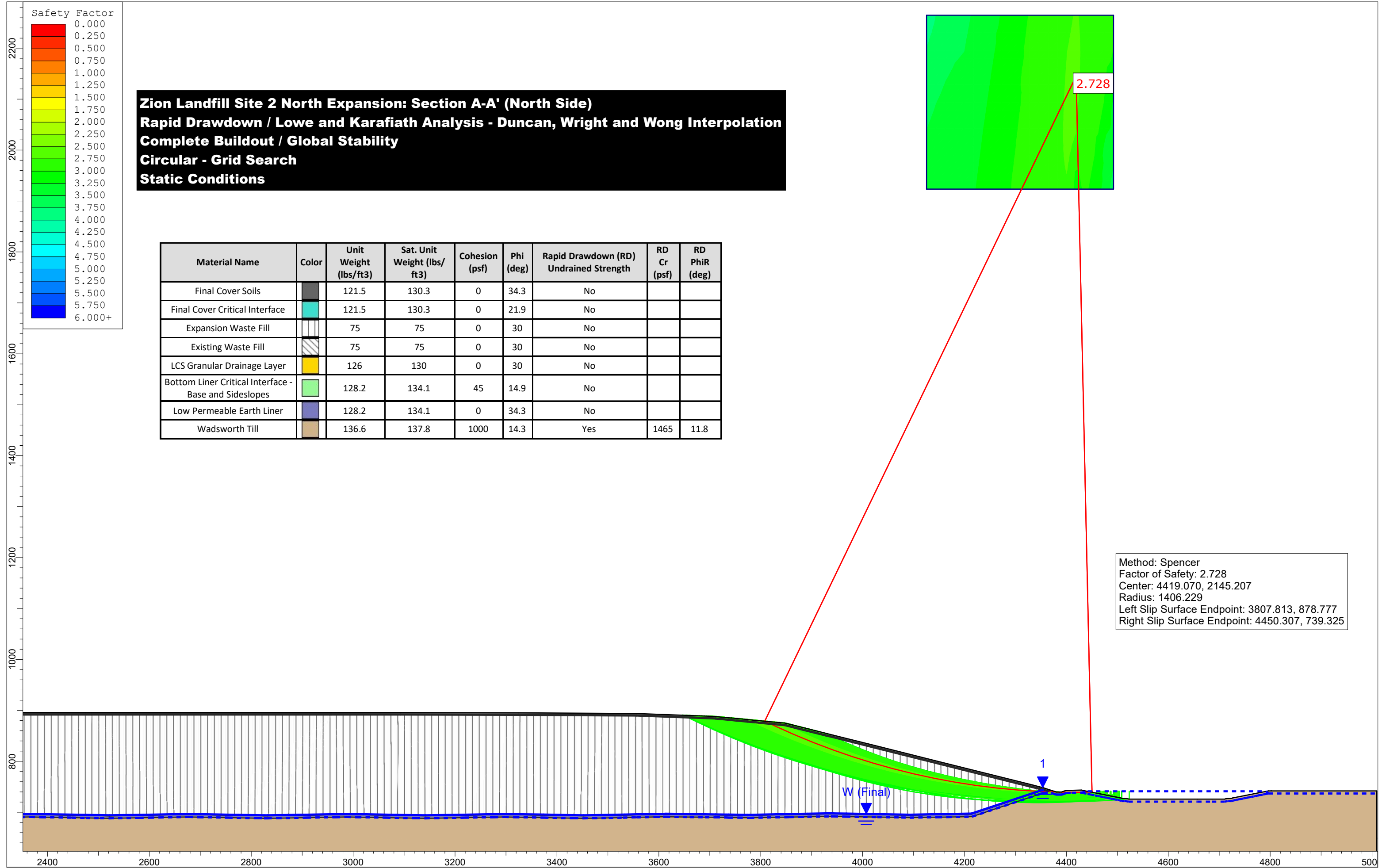
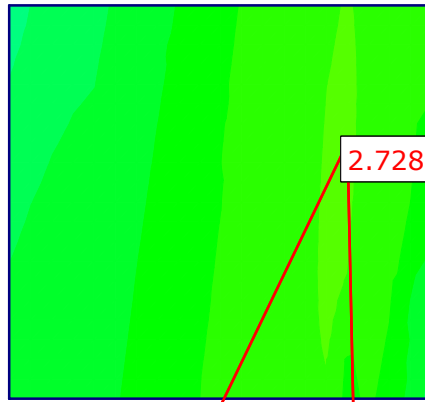
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Focus Search Window	4434.16, 722.071 4762.52, 722.071 4762.52, 752 4434.16, 742.558	



Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Lowe and Karafiath Analysis - Duncan, Wright and Wong Interpolation
Complete Buildout / Global Stability
Circular - Grid Search
Static Conditions

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Spencer
 Factor of Safety: 2.728
 Center: 4419.070, 2145.207
 Radius: 1406.229
 Left Slip Surface Endpoint: 3807.813, 878.777
 Right Slip Surface Endpoint: 4450.307, 739.325

Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:48.633s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Lowe and Karafiath (1960)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0

Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

FS	2.736430
Center:	4419.070, 2162.269
Radius:	1423.276
Left Slip Surface Endpoint:	3803.003, 879.235
Right Slip Surface Endpoint:	4450.260, 739.334
Resisting Moment:	1.15196e+09 lb-ft
Driving Moment:	4.20973e+08 lb-ft
Total Slice Area:	15149.8 ft2
Surface Horizontal Width:	647.257 ft
Surface Average Height:	23.4062 ft

Method: janbu corrected

FS	2.769900
Center:	4419.070, 2145.207
Radius:	1406.229
Left Slip Surface Endpoint:	3807.813, 878.777
Right Slip Surface Endpoint:	4450.307, 739.325
Resisting Horizontal Force:	790919 lb
Driving Horizontal Force:	285541 lb
Total Slice Area:	14800 ft2
Surface Horizontal Width:	642.493 ft
Surface Average Height:	23.0352 ft

Method: spencer

FS	2.727560
Center:	4419.070, 2145.207
Radius:	1406.229
Left Slip Surface Endpoint:	3807.813, 878.777
Right Slip Surface Endpoint:	4450.307, 739.325
Resisting Moment:	1.11277e+09 lb-ft
Driving Moment:	4.07972e+08 lb-ft
Resisting Horizontal Force:	768557 lb
Driving Horizontal Force:	281775 lb
Total Slice Area:	14800 ft ²
Surface Horizontal Width:	642.493 ft
Surface Average Height:	23.0352 ft

Method: gl/morgenstern-price

FS	2.732350
Center:	4419.070, 2145.207
Radius:	1406.229
Left Slip Surface Endpoint:	3807.813, 878.777
Right Slip Surface Endpoint:	4450.307, 739.325
Resisting Moment:	1.11472e+09 lb-ft
Driving Moment:	4.07972e+08 lb-ft
Resisting Horizontal Force:	769737 lb
Driving Horizontal Force:	281712 lb
Total Slice Area:	14800 ft ²
Surface Horizontal Width:	642.493 ft
Surface Average Height:	23.0352 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4532
Number of Invalid Surfaces:	319

Error Codes

Error Code -110 reported for 86 surfaces
 Error Code -118 reported for 233 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4532
Number of Invalid Surfaces:	319

Error Codes

Error Code -110 reported for 86 surfaces
 Error Code -118 reported for 233 surfaces

Method: spencer

Number of Valid Surfaces:	4532
Number of Invalid Surfaces:	319

Error Codes

Error Code -110 reported for 86 surfaces
 Error Code -118 reported for 233 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4532
 Number of Invalid Surfaces: 319

Error Codes

Error Code -110 reported for 86 surfaces
 Error Code -118 reported for 233 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

-118 = Surface does not pass through the search focus

Entity Information

◆ L&K - DWW

Shared Entities

Type	Coordinates (x,y)
	4801.68, 742.054
	4796.47, 742.054
	4726.05, 728
	4724.43, 727.354
	4723.55, 727
	4718.38, 727
	4715.55, 727
	4712.32, 726.355
	4710.54, 726
	4685.73, 726
	4656.84, 726
	4522.44, 726
	4520.26, 726.437
	4517.44, 727
	4513.9, 727
	4509.44, 727
	4508.31, 727.451
	4506.94, 728
	4486.94, 732
	4484.34, 732.521
	4481.94, 733
	4481.87, 733
	4479.43, 733.489
	4476.94, 734
	4476.94, 734
	4473.66, 734.657
	4466.94, 736
	4466.94, 736
	4466.94, 736
	4464.44, 736.5
	4461.97, 736.994
	4459.97, 737.393
	4456.92, 738

4451.39, 739.108
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4431.95, 743
4429.16, 743.558
4410.47, 743.103
4399.15, 742.903
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4390.15, 739.897
4386.69, 739.881
4380.14, 739.897
4377.03, 740.918
4371.14, 742.896
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4354.12, 748.427
4347.22, 750.154
4333.51, 753.581
4307.9, 760
4147.28, 800.154
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3927.9, 855
3907.85, 860.012
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3838.52, 875.876
3826.49, 877
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3753.28, 883.966
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3742.72, 884.971
3742.42, 885
3736.08, 885.603
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3711.25, 888
3660.7, 890
3608.3, 892
3555.9, 894
3501.92, 894.208
3324.04, 895.343
3093.34, 895.755
2956.54, 896
2486.56, 896
2475.13, 896
2234.57, 896
2195.59, 896
2181.65, 896
2141.81, 896
2127.47, 896
1858.87, 896
1616.33, 896
1495.01, 896

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1446.76, 898.853
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1430.62, 902.853
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External Boundary

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768.454, 687.654
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874.622, 686
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914.512, 684.056
916.783, 684
918.713, 683.952
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1013.62, 702
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1807.13, 688
1816.05, 688
1826.74, 688
1846.47, 687.169
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1855.02, 684.278

Material Boundary

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1856.55, 684
1858.87, 684
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1866.61, 685.507
1867.43, 685.773
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1868.4, 686.102
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1893.8, 694.436
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1899.84, 696.434
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1911.78, 700.431
1911.83, 700.431
1916.54, 702
1917.78, 702.43
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1923.77, 704.428
1923.82, 704.428
1928.54, 706
1929.77, 706.426
1929.82, 706.426
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1953.8, 714.42
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2013.76, 734.405
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2019.73, 736.403
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	3141.93, 693.125
	3160.85, 693.454
	3218.92, 694.461
	3276.17, 695.453
	3296.93, 695.814
	3317.63, 695.456
	3374.97, 694.464
	3433.1, 693.458
	3451.93, 693.132
	3470.7, 693.458
	3501.92, 693.999
	3529.71, 694.545
	3555.9, 695.059
	3593.08, 695.794
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	1456.13, 893.441
	1457.91, 893
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	1616.33, 893
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	2141.81, 893
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	3555.83, 891
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	3712.04, 884.91
	3731.62, 883.014
	3735.8, 882.616
	3742.13, 882.013
	3742.44, 881.984
	3742.77, 881.952
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	454.117, 747.086
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626.101, 790.086
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713.234, 811.867
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738.101, 818.086
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746.101, 820.086
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898.101, 858.086
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Material Boundary


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915.521, 862.441
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923.528, 864.443
930.101, 866.086
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938.101, 868.086
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945.181, 869.856
945.941, 870.046
946.101, 870.086
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951.569, 871.453
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960.237, 873.62
962.101, 874.086
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1002.93, 884.294
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1031.94, 891.544
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1036.44, 892.671
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1066.1, 900.086
1071.43, 901.419
1071.45, 901.423

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1076.99, 902.808
1077.01, 902.812
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1098.1, 908.086
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1106.1, 910.086
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1131.8, 913.879
1134.41, 914.139
1142.12, 914.911
1154.41, 916.139
1162.12, 916.911
1174.41, 918.139
1192.53, 919.952
1194.41, 920.139
1197.66, 920.465
1201.22, 920.776
1212.16, 921.718
1219.87, 922.15
1223.74, 922.15
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1247.67, 922.15
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1271.48, 920.143
1275.91, 919.79
1278.63, 919.622
1298.92, 918.144
1308.26, 917.4
1313.97, 917.047
1326.37, 916.144
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1389.43, 910.122
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1414.79, 903.84
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1428.47, 900.45
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1446.69, 895.936
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1456.17, 893.587
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1495.01, 893.15
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3712.05, 885.059
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3735.81, 882.765
3742.15, 882.163
3742.45, 882.134
3742.79, 882.102
3753.01, 881.129
3763.16, 880.163
3774.12, 879.12
3784.18, 878.163
3795.56, 877.08
3805.2, 876.163
3817.03, 875.037
3826.22, 874.163
3838.25, 873.038
3847.41, 872.183
3861.79, 868.588
3887.2, 862.235
3907.15, 857.247
3927.2, 852.235
3942.59, 848.389
4034.21, 825.483
4146.59, 797.389
4307.21, 757.235
4332.82, 750.816
4346.52, 747.389
4353.25, 745.706
4353.34, 745.686
4361.26, 743.112

Material Boundary	4486.94, 732 4491.68, 731.051 4496.94, 730 4498.1, 729.768 4499.44, 729.5 4500.79, 729.23 4501.94, 729 4504.67, 728.454 4506.94, 728
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Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:  Wadsworth Till
	0.809461, 723.908	
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	
	439.628, 735.289	
	488.131, 719.138	
	522.779, 707.602	

608.698, 679
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629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
721.531, 681.51
724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
760.228, 682.452
768.453, 682.652
775.361, 682.481
785.608, 682.229
792.858, 682.05
834.445, 681.025
836.108, 681
839.229, 681
842.442, 681
846.403, 681
851.249, 681
853.768, 681
858.565, 681
861.768, 681
864.838, 681
868.722, 681
876.123, 681
879.684, 681
882.824, 681
885.634, 681
888.824, 680.784
905.654, 679.642
913.684, 679.098
914.27, 679.058
916.598, 679.003
918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
926.99, 679.234
928.274, 679.291
929.213, 679.332
930.561, 679.392
932.042, 679.457
934.813, 679.981
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968.611, 687.32

Water Table

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992.201, 692.586
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	-9e-16, 723.896 0.809461, 723.908 1.15168, 723.913 1.45658, 723.917 1.93345, 723.924 4.30816, 723.96 12.6281, 724.084 24.2881, 724.257 113.381, 725.584 150.125, 726.131 251.754, 727.644 262.684, 727.807 264.857, 727.839 267.412, 727.877 267.74, 727.882 267.83, 727.883 268.093, 727.885 268.368, 727.882 269.061, 727.877 272.624, 727.847 281.529, 727.773 284.508, 727.748 284.959, 727.743 286.557, 727.708 289.734, 727.637 295.936, 727.498 301.316, 727.378 309.904, 727.186 313.768, 727.1 314.73, 727 317.794, 727 322.428, 727 327.892, 727 334.438, 727 352.838, 735.003 356.385, 735.013	

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Drawdown Line

DRAWDOWN LINE

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1692.76, 685
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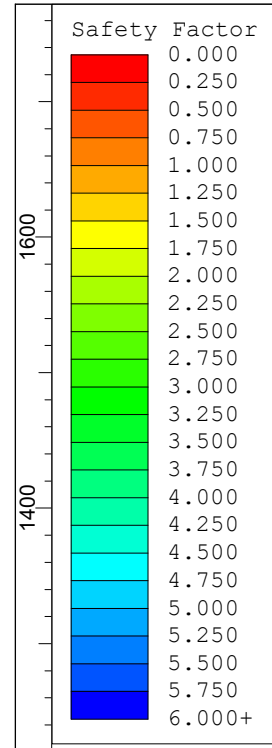
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SLOPE STABILITY
SECTION **A-A'** – HORIZONTAL EXPANSION

**RAPID DRAWDOWN GLOBAL STABILITY
ARMY CORPS OF ENGINEERS ANALYSIS**

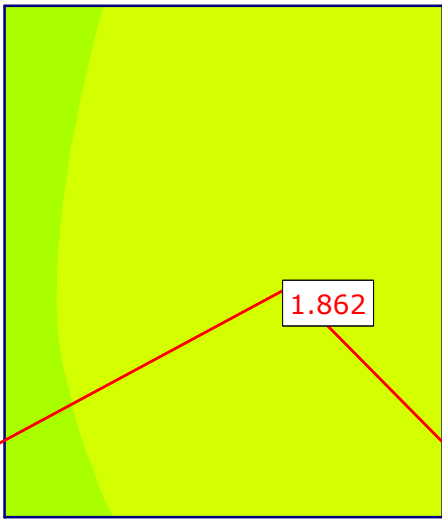
**COMPLETE BUILDOUT LANDFORM
CIRCULAR ANALYSIS
(ROTATIONAL SLOPE FAILURE)**



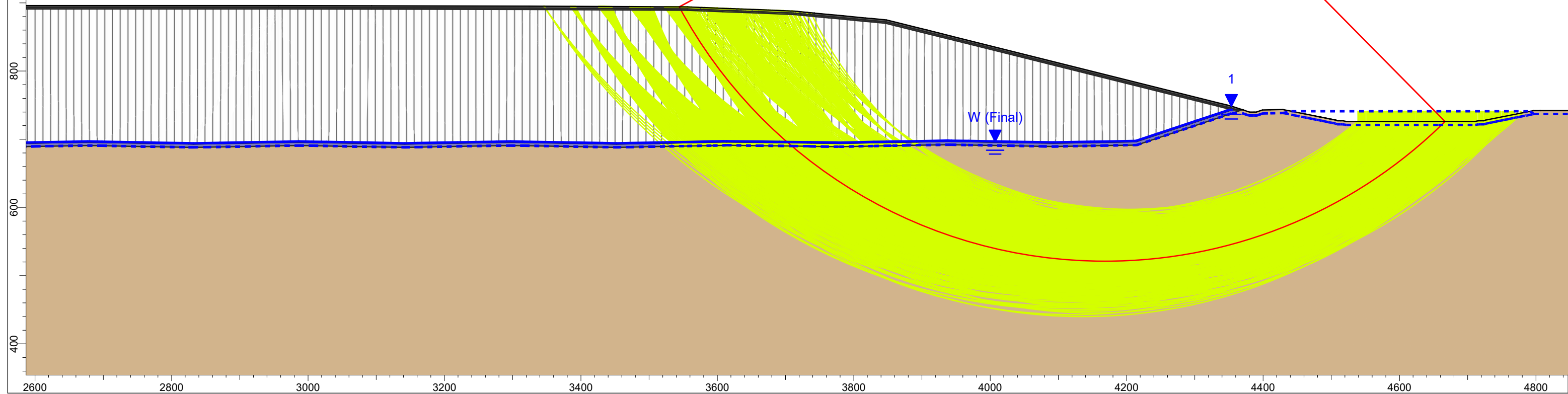
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Army Corps of Engineers Analysis
Complete Buildout / Global Stability
Circular - Grid Search
Seismic Conditions

0.0461

Material Name	Color	Unit Weight (lbs/ft3)	Sat. Unit Weight (lbs/ft3)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8



Method: Janbu corrected
 Factor of Safety: 1.862
 Center: 4168.264, 1230.247
 Radius: 709.014
 Left Slip Surface Endpoint: 3544.028, 894.046
 Right Slip Surface Endpoint: 4666.697, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:54.451s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Army Corp. Eng. 2 Stage (1970)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth:	Not Defined
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading


Advanced seismic analysis:	No
Staged pseudostatic analysis:	No
Seismic Load Coefficient (Horizontal):	0.0461

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb

Unsaturated Unit Weight [lbs/ft3]	121.5
Saturated Unit Weight [lbs/ft3]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	75
Saturated Unit Weight [lbs/ft3]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0


LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	126
Saturated Unit Weight [lbs/ft3]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	128.2
Saturated Unit Weight [lbs/ft3]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

	FS	1.946360
Center:	4184.420, 1437.806	
Radius:	892.478	
Left Slip Surface Endpoint:	3476.469, 894.371	
Right Slip Surface Endpoint:	4724.747, 727.480	
Resisting Moment:	5.41796e+09 lb-ft	
Driving Moment:	2.78364e+09 lb-ft	
Total Slice Area:	228821 ft2	
Surface Horizontal Width:	1248.28 ft	
Surface Average Height:	183.31 ft	

Method: janbu corrected

	FS	1.861990
Center:	4168.264, 1230.247	
Radius:	709.014	
Left Slip Surface Endpoint:	3544.028, 894.046	
Right Slip Surface Endpoint:	4666.697, 726.000	
Resisting Horizontal Force:	5.70822e+06 lb	
Driving Horizontal Force:	3.06566e+06 lb	
Total Slice Area:	228088 ft2	
Surface Horizontal Width:	1122.67 ft	
Surface Average Height:	203.166 ft	

Method: spencer

FS	1.936550
Center:	4200.577, 1400.068
Radius:	840.687
Left Slip Surface Endpoint:	3529.194, 894.103
Right Slip Surface Endpoint:	4702.958, 726.000
Resisting Moment:	4.59285e+09 lb-ft
Driving Moment:	2.37167e+09 lb-ft
Resisting Horizontal Force:	5.00113e+06 lb
Driving Horizontal Force:	2.5825e+06 lb
Total Slice Area:	201231 ft ²
Surface Horizontal Width:	1173.76 ft
Surface Average Height:	171.441 ft

Method: gle/morgenstern-price

FS	1.935160
Center:	4200.577, 1400.068
Radius:	840.687
Left Slip Surface Endpoint:	3529.194, 894.103
Right Slip Surface Endpoint:	4702.958, 726.000
Resisting Moment:	4.58955e+09 lb-ft
Driving Moment:	2.37167e+09 lb-ft
Resisting Horizontal Force:	5.0066e+06 lb
Driving Horizontal Force:	2.58717e+06 lb
Total Slice Area:	201231 ft ²
Surface Horizontal Width:	1173.76 ft
Surface Average Height:	171.441 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces: 4645
 Number of Invalid Surfaces: 206

Error Codes

Error Code -103 reported for 123 surfaces
 Error Code -110 reported for 83 surfaces

Method: janbu corrected

Number of Valid Surfaces: 4645
 Number of Invalid Surfaces: 206

Error Codes

Error Code -103 reported for 123 surfaces
 Error Code -110 reported for 83 surfaces

Method: spencer

Number of Valid Surfaces: 4645
 Number of Invalid Surfaces: 206

Error Codes

Error Code -103 reported for 123 surfaces
 Error Code -110 reported for 83 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4645
 Number of Invalid Surfaces: 206

Error Codes

Error Code -103 reported for 123 surfaces
 Error Code -110 reported for 83 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-110 = The water table or a piezoline does not span the slip region for a given slip surface, when Water Surfaces is specified as the method of pore pressure calculation. If this error occurs, check that the water table or piezoline(s) span the appropriate soil cells.

Entity Information

◆ Army

Shared Entities

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1071.45, 901.423

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1098.1, 908.086
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1106.1, 910.086
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3742.15, 882.163
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3763.16, 880.163
3774.12, 879.12
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3927.2, 852.235
3942.59, 848.389
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4307.21, 757.235
4332.82, 750.816
4346.52, 747.389
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4353.34, 745.686
4361.26, 743.112

Material Boundary	4481.94, 733
	4481.94, 733
	4484.34, 732.521
	4486.94, 732
	4491.68, 731.051
	4496.94, 730
	4498.1, 729.768
	4499.44, 729.5
	4500.79, 729.23
	4501.94, 729
	4504.67, 728.454
	4506.94, 728

Scenario-based Entities

Type	Coordinates (x,y)	Seismic
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	

439.628, 735.289
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629.067, 679.262
642.777, 679.595
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660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
711.292, 681.261
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724.458, 681.581
731.21, 681.745
742.989, 682.032
746.542, 682.118
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768.453, 682.652
775.361, 682.481
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839.229, 681
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888.824, 680.784
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918.712, 678.951
920.654, 678.998
921.208, 679.012
921.899, 679.03
923.072, 679.061
924.878, 679.141
925.393, 679.164
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929.213, 679.332
930.561, 679.392
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Water Table

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	<p>435.797, 740.289 443.602, 740.289 447.47, 739 450.033, 738.146 453.47, 737 454.639, 736.61 456.913, 735.853 459.474, 735 462.023, 734.151 465.481, 733 465.482, 733 470.568, 731.307 477.496, 729 480.031, 728.156 483.505, 727 486.035, 726.158 489.513, 725 493.701, 723.607 495.524, 723 499.16, 721.79</p>	<p>Assigned to:</p> <ul style="list-style-type: none">  LCS Granular Drainage Layer  Bottom Liner Critical Interface - Base and Sideslopes  Low Permeable Earth Liner

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545.694, 706.3
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609.67, 685
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628.175, 685.242
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885.837, 687
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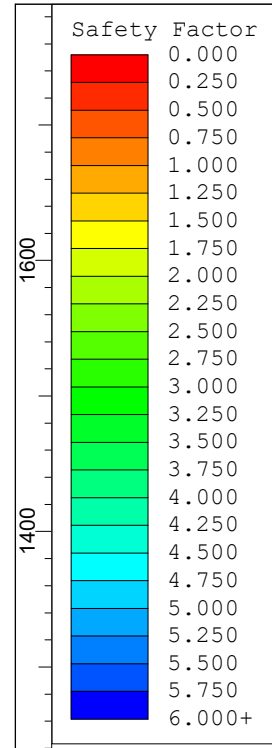
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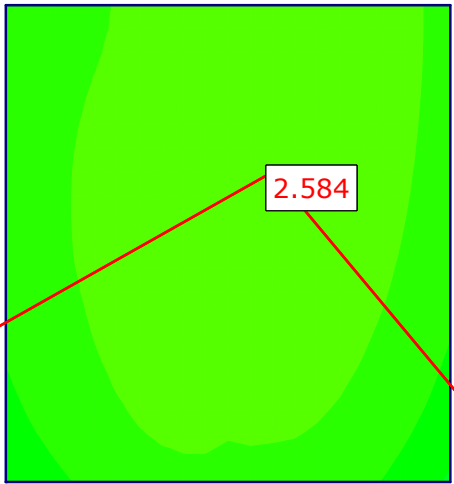
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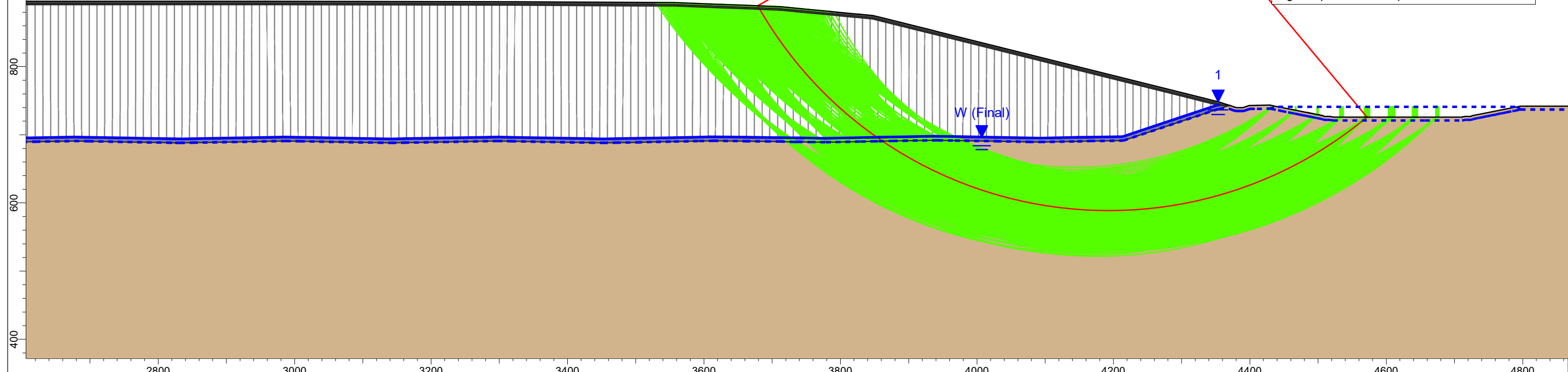
Zion Landfill Site 2 North Expansion: Section A-A' (North Side)
Rapid Drawdown / Army Corps of Engineers Analysis
Complete Buildout / Global Stability
Circular - Grid Search
Static Conditions

Material Name	Color	Unit Weight (lbs/ft ³)	Sat. Unit Weight (lbs/ft ³)	Cohesion (psf)	Phi (deg)	Rapid Drawdown (RD) Undrained Strength	RD Cr (psf)	RD PhiR (deg)
Final Cover Soils		121.5	130.3	0	34.3	No		
Final Cover Critical Interface		121.5	130.3	0	21.9	No		
Expansion Waste Fill		75	75	0	30	No		
Existing Waste Fill		75	75	0	30	No		
LCS Granular Drainage Layer		126	130	0	30	No		
Bottom Liner Critical Interface - Base and Sideslopes		128.2	134.1	45	14.9	No		
Low Permeable Earth Liner		128.2	134.1	0	34.3	No		
Wadsworth Till		136.6	137.8	1000	14.3	Yes	1465	11.8

1600
1400
1200
1000
800
600
400



Method: Janbu corrected
 Factor of Safety: 2.584
 Center: 4193.141, 1179.681
 Radius: 590.846
 Left Slip Surface Endpoint: 3678.579, 889.293
 Right Slip Surface Endpoint: 4571.654, 726.000



Slide Analysis Information

A-A_final_north_LT

Project Summary

File Name:	A-A_final_north_LT.slmd
Slide Modeler Version:	9.006
Compute Time:	00h:00m:48.916s
Project Title:	SLIDE - An Interactive Slope Stability Program
Date Created:	1/30/2020, 11:59:15 AM

General Settings

Units of Measurement:	Imperial Units
Time Units:	days
Permeability Units:	feet/second
Data Output:	Standard
Failure Direction:	Left to Right

Analysis Options

Slices Type:	Vertical
Analysis Methods Used	
	Bishop simplified
	GLE/Morgenstern-Price with interslice force function (Half Sine)
	Janbu corrected
	Spencer
Number of slices:	50
Tolerance:	0.005
Maximum number of iterations:	75
Check $m\alpha < 0.2$:	Yes
Create Interslice boundaries at intersections with water tables and piezos:	Yes
Initial trial value of FS:	1
Steffensen Iteration:	Yes

Groundwater Analysis

Groundwater Method:	Water Surfaces
Pore Fluid Unit Weight [lbs/ft ³]:	62.4
Use negative pore pressure cutoff:	Yes
Maximum negative pore pressure [psf]:	0
Advanced Groundwater Method:	Rapid Drawdown
Rapid Drawdown Method:	Army Corp. Eng. 2 Stage (1970)

Random Numbers

Pseudo-random Seed:	10116
Random Number Generation Method:	Park and Miller v.3

Surface Options


Surface Type:	Circular
Search Method:	Grid Search
Radius Increment:	10
Composite Surfaces:	Disabled
Reverse Curvature:	Invalid Surfaces
Minimum Elevation:	Not Defined
Minimum Depth [ft]:	6
Minimum Area:	Not Defined
Minimum Weight:	Not Defined

Seismic Loading

Advanced seismic analysis:	No
Staged pseudostatic analysis:	No

Materials

Final Cover Soils


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5
Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	None
Ru Value	0

Final Cover Critical Interface


Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	121.5

Saturated Unit Weight [lbs/ft ³]	130.3
Cohesion [psf]	0
Friction Angle [deg]	21.9
Water Surface	None
Ru Value	0


Expansion Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

Existing Waste Fill

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	75
Saturated Unit Weight [lbs/ft ³]	75
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	None
Ru Value	0

LCS Granular Drainage Layer

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	126
Saturated Unit Weight [lbs/ft ³]	130
Cohesion [psf]	0
Friction Angle [deg]	30
Water Surface	Piezometric Line 1
Hu Value	1


Bottom Liner Critical Interface - Base and Sideslopes

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	45
Friction Angle [deg]	14.9
Water Surface	Piezometric Line 1
Hu Value	1

Low Permeable Earth Liner

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft ³]	128.2
Saturated Unit Weight [lbs/ft ³]	134.1
Cohesion [psf]	0
Friction Angle [deg]	34.3
Water Surface	Piezometric Line 1
Hu Value	1

Wadsworth Till

Color	
Strength Type	Mohr-Coulomb
Unsaturated Unit Weight [lbs/ft3]	136.6
Saturated Unit Weight [lbs/ft3]	137.8
Cohesion [psf]	1000
Friction Angle [deg]	14.3
Water Surface	Water Table
Hu Value	1
Rapid Drawdown Undrained Behaviour	Yes
RD Shear Strength Envelope Properties	CR: 1465PhiR: 11.8

Global Minimums

Method: bishop simplified

FS	2.710510
Center:	4193.141, 1302.816
Radius:	690.297
Left Slip Surface Endpoint:	3639.275, 890.818
Right Slip Surface Endpoint:	4572.341, 726.000
Resisting Moment:	2.69843e+09 lb-ft
Driving Moment:	9.95543e+08 lb-ft
Total Slice Area:	125458 ft2
Surface Horizontal Width:	933.066 ft
Surface Average Height:	134.458 ft

Method: janbu corrected

FS	2.584430
Center:	4193.141, 1179.681
Radius:	590.846
Left Slip Surface Endpoint:	3678.579, 889.293
Right Slip Surface Endpoint:	4571.654, 726.000
Resisting Horizontal Force:	3.84059e+06 lb
Driving Horizontal Force:	1.48605e+06 lb
Total Slice Area:	134572 ft2
Surface Horizontal Width:	893.075 ft
Surface Average Height:	150.684 ft

Method: spencer

	FS	2.685010
Center:		4193.141, 1302.816
Radius:		690.297
Left Slip Surface Endpoint:		3639.275, 890.818
Right Slip Surface Endpoint:		4572.341, 726.000
Resisting Moment:		2.67304e+09 lb-ft
Driving Moment:		9.95543e+08 lb-ft
Resisting Horizontal Force:		3.53824e+06 lb
Driving Horizontal Force:		1.31778e+06 lb
Total Slice Area:		125458 ft ²
Surface Horizontal Width:		933.066 ft
Surface Average Height:		134.458 ft

Method: gle/morgenstern-price

	FS	2.687780
Center:		4209.540, 1302.816
Radius:		681.949
Left Slip Surface Endpoint:		3666.922, 889.754
Right Slip Surface Endpoint:		4573.323, 726.000
Resisting Moment:		2.46897e+09 lb-ft
Driving Moment:		9.18592e+08 lb-ft
Resisting Horizontal Force:		3.31284e+06 lb
Driving Horizontal Force:		1.23256e+06 lb
Total Slice Area:		114203 ft ²
Surface Horizontal Width:		906.401 ft
Surface Average Height:		125.996 ft

Valid and Invalid Surfaces

Method: bishop simplified

Number of Valid Surfaces:	4774
Number of Invalid Surfaces:	77

Error Codes

Error Code -103 reported for 77 surfaces

Method: janbu corrected

Number of Valid Surfaces:	4768
Number of Invalid Surfaces:	83

Error Codes

Error Code -103 reported for 77 surfaces

Error Code -108 reported for 1 surface

Error Code -200 reported for 5 surfaces

Method: spencer

Number of Valid Surfaces: 4766

Number of Invalid Surfaces: 85

Error Codes

Error Code -103 reported for 77 surfaces

Error Code -108 reported for 3 surfaces

Error Code -200 reported for 5 surfaces

Method: gle/morgenstern-price

Number of Valid Surfaces: 4767

Number of Invalid Surfaces: 84

Error Codes

Error Code -103 reported for 77 surfaces

Error Code -108 reported for 2 surfaces

Error Code -200 reported for 5 surfaces

Error Code Descriptions

The following errors were encountered during the computation:

-103 = Two surface / slope intersections, but one or more surface / nonslope external polygon intersections lie between them. This usually occurs when the slip surface extends past the bottom of the soil region, but may also occur on a benched slope model with two sets of Slope Limits.

-108 = Total driving moment or total driving force < 0.1. This is to limit the calculation of extremely high safety factors if the driving force is very small (0.1 is an arbitrary number).

-200 = Factor of Safety <= min iteration value. Could mean 0 Normal/Shear resistance along part of the slip surface

Entity Information**◆ Army****Shared Entities**

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	4685.73, 726
	4656.84, 726
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	4517.44, 727
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	4506.94, 728
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1905.79, 699.433
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Material Boundary	1457.67, 891 1495.01, 891 1616.33, 891 1858.87, 891 2127.47, 891 2141.81, 891 2181.65, 891 2195.59, 891 2234.57, 891 2475.13, 891 2486.56, 891 2956.54, 891 3093.33, 890.755 3324.02, 890.343 3501.89, 889.208 3555.79, 889 3608.11, 887.004 3660.51, 885.004 3710.91, 883.01 3711.85, 882.919 3731.43, 881.023 3735.61, 880.625 3741.94, 880.022 3742.25, 879.993 3742.58, 879.961 3752.81, 878.988 3762.96, 878.022 3773.92, 876.98 3783.98, 876.022 3795.35, 874.94 3804.99, 874.022 3816.83, 872.896 3826.02, 872.022 3838.05, 870.897 3847.04, 870.057 3861.26, 866.502 3886.68, 860.149 3906.63, 855.161 3926.68, 850.149 3942.07, 846.303 4033.69, 823.397 4146.07, 795.303 4306.68, 755.149 4332.29, 748.731 4346, 745.303 4352.73, 743.621 4354.12, 743.273
	426.87, 740.119 442.157, 743.94 449.793, 745.849 450.157, 745.94 450.483, 746.022 454.153, 746.94 458.147, 747.94 461.473, 748.773 466.139, 749.94 473.455, 751.767 474.149, 751.941 474.848, 752.114

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525.743, 764.846
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538.115, 767.936
538.372, 768
538.376, 768
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593.377, 781.753
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674.997, 802.153
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755.079, 822.176
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811.847, 836.368
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833.077, 841.675
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835.239, 842.216
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849.204, 845.707
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851.109, 846.183
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859.153, 848.194
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867.198, 850.205
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884.182, 854.452
890.137, 855.94
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898.137, 857.94
904.884, 859.627
906.137, 859.94
912.782, 861.602
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915.557, 862.295

Material Boundary

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923.565, 864.297
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936.197, 867.455
938.137, 867.94
944.598, 869.556
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	450.121, 746.086
	450.446, 746.167
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	461.436, 748.919

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634.901, 792.286
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645.858, 795.025
650.112, 796.086
654.377, 797.152
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702.685, 809.229
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713.234, 811.867
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714.999, 812.308
715.007, 812.31
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726.775, 815.254
730.101, 816.086
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738.101, 818.086
742.842, 819.271
746.101, 820.086
753.167, 821.852
753.175, 821.854
754.101, 822.086
755.035, 822.319
755.043, 822.321
762.101, 824.086
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770.101, 826.086
777.704, 827.987
777.707, 827.987
778.101, 828.086
778.761, 828.251
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842.101, 844.086
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866.101, 850.086
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874.101, 852.086
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882.101, 854.086
883.256, 854.375
884.146, 854.597
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898.101, 858.086
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Material Boundary


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912.746, 861.747
914.101, 862.086
915.521, 862.441
922.101, 864.086
923.528, 864.443
930.101, 866.086
936.16, 867.601
938.101, 868.086
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945.181, 869.856
945.941, 870.046
946.101, 870.086
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950.891, 871.283
951.569, 871.453
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1071.45, 901.423

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1110.71, 911.237
1110.72, 911.241
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1162.12, 916.911
1174.41, 918.139
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1389.43, 910.122
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1405.72, 906.087
1405.93, 906.034
1413.79, 904.087
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1414.79, 903.84
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1456.17, 893.587
1457.93, 893.15
1495.01, 893.15
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2956.54, 893.15
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3712.05, 885.059
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3735.81, 882.765
3742.15, 882.163
3742.45, 882.134
3742.79, 882.102
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3763.16, 880.163
3774.12, 879.12
3784.18, 878.163
3795.56, 877.08
3805.2, 876.163
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3907.15, 857.247
3927.2, 852.235
3942.59, 848.389
4034.21, 825.483
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4307.21, 757.235
4332.82, 750.816
4346.52, 747.389
4353.25, 745.706
4353.34, 745.686
4361.26, 743.112

Material Boundary	4481.94, 733
	4481.94, 733
	4484.34, 732.521
	4486.94, 732
	4491.68, 731.051
	4496.94, 730
	4498.1, 729.768
	4499.44, 729.5
	4500.79, 729.23
	4501.94, 729
	4504.67, 728.454
	4506.94, 728

Scenario-based Entities

Type	Coordinates (x,y)	Static
	-9e-16, 723.896	Assigned to:
	0.809461, 723.908	 Wadsworth Till
	1.15168, 723.913	
	1.45658, 723.917	
	1.93345, 723.924	
	4.30816, 723.96	
	12.6281, 724.084	
	24.2881, 724.257	
	113.381, 725.584	
	150.125, 726.131	
	251.754, 727.644	
	262.684, 727.807	
	264.857, 727.839	
	267.412, 727.877	
	267.74, 727.882	
	267.83, 727.883	
	268.093, 727.885	
	268.368, 727.882	
	269.061, 727.877	
	272.624, 727.847	
	281.529, 727.773	
	284.508, 727.748	
	284.959, 727.743	
	286.557, 727.708	
	289.734, 727.637	
	295.936, 727.498	
	301.316, 727.378	
	309.904, 727.186	
	313.768, 727.1	
	314.73, 727	
	317.794, 727	
	322.428, 727	
	327.892, 727	
	334.438, 727	
	352.838, 735.003	
	356.385, 735.013	
	366.425, 735.2	
	372.719, 735.318	
	379.181, 735.415	
	391.383, 735.573	
	399.078, 735.005	
	407.323, 734.789	
	412.029, 734.347	

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488.131, 719.138
522.779, 707.602
608.698, 679
618.307, 679
629.067, 679.262
642.777, 679.595
647.559, 679.711
660.405, 680.023
675.999, 680.402
689.108, 680.721
700.659, 681.002
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Focus Search Window	4434.16, 725.451 4762.52, 725.451 4762.52, 752 4434.16, 742.558	

J.2-E Summary of Shear Strength Values



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: SUMMARY OF ACCEPTABLE SHEAR STRENGTH VALUES

Problem Statement

Summarize the acceptable peak shear strength values for interface friction angles of the liner/leachate collection system and final cover components for the landfill based on previous calculations in this appendix.

Given

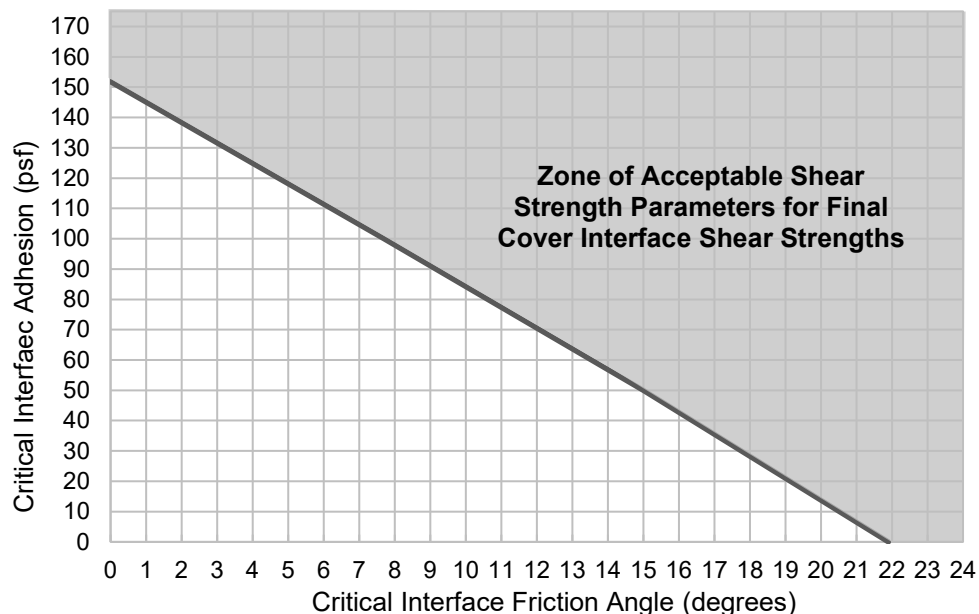
- Appendix J.2-A:** Shear Strength Evaluation of Final Cover
- Appendix J.2-B:** Shear Strength Evaluation of Liner / Leachate Collection System Prior to Waste Placement
- Appendix J.2-C:** Shear Strength Evaluation of Liner / Leachate Collection System After Waste Placement (Global Stability Analyses)
- Appendix J.2-D:** Shear Strength Evaluation of Wadsworth Till During Rapid Drawdown of Detention Basin (Global Stability Analysis)

Results

Final Cover

Interfaces of the final cover system shall be evaluated as described in the CQA Plan to ensure that the interface peak shear strength friction angle and adhesion values are within the acceptable range shown below. These interfaces include the following:

- Protective soil cover to geocomposite;
- Geocomposite to textured geomembrane; or
- Textured geomembrane to low permeable soil layer.





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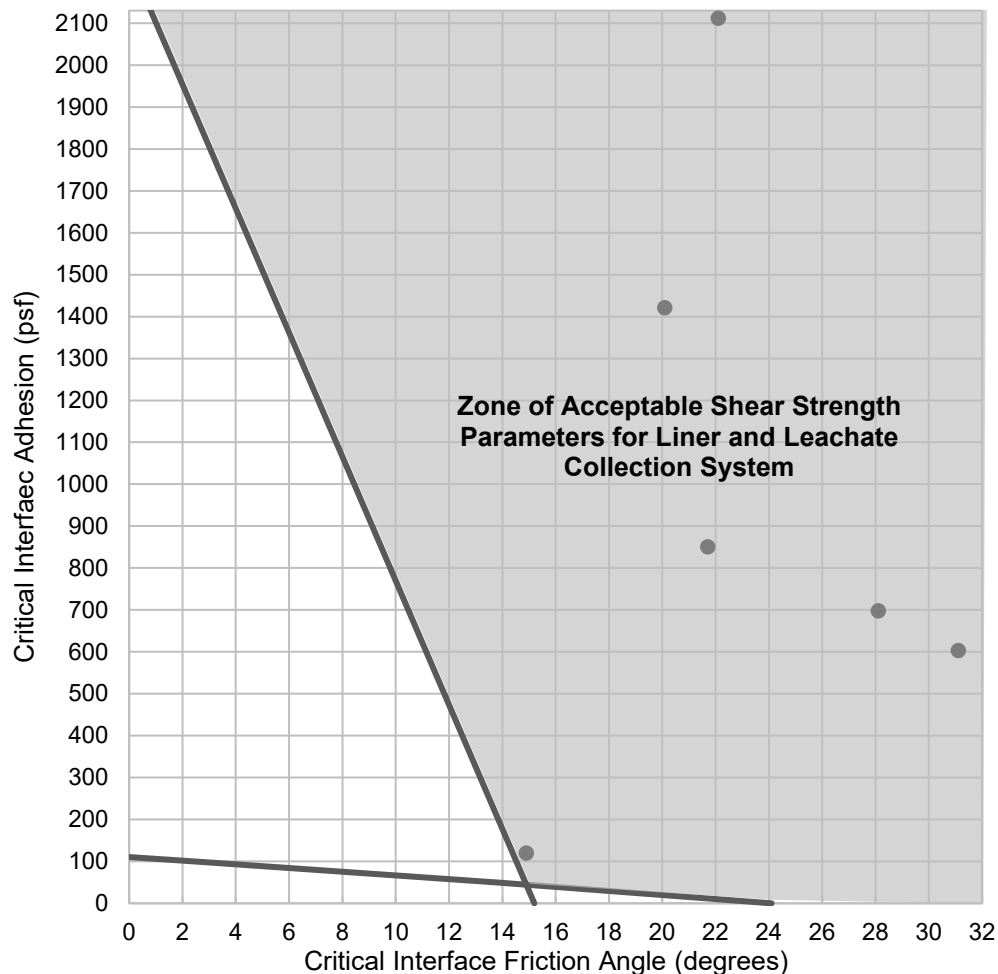
Date: 05/2022

TITLE: SUMMARY OF ACCEPTABLE SHEAR STRENGTH VALUES
Liner and Leachate Collection System

Interfaces of the liner and leachate collection shall be evaluated as described in the CQA Plan to ensure that the interface peak shear strength friction angle and adhesion values are within the acceptable range shown below. These interfaces include the following:

- Geotextile to granular drainage layer;
- Geotextile to textured geomembrane; or
- Textured geomembrane to low permeable earth liner.

Laboratory reported peak shear strengths for the bottom liner critical interfaces in Cells 9 and 10 of Zion Landfill are shown as points below (see lab test results also in **Appendix J.1**). It should be noted that the interface results from the constructed bottom liner fall within the acceptable window developed for the Site 2 North Expansion.



J.3 – Foundation Evaluations

- J.3-A Hydrostatic Uplift
- J.3-B Foundation Settlement
- J.3-C Bearing Capacity

J.3-A Hydrostatic Uplift



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TITLE: STABILITY AGAINST HYDROSTATIC UPLIFT

Problem Statement

Determine the stability against hydrostatic uplift of the excavation during construction activities by estimating the greatest anticipated hydrostatic uplift forces that may act on the bottom of excavation.

Given

- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this application.
- Excavation grades included in the design drawings contained in this application.
- A minimum factor of safety of 1.0 will be maintained against hydrostatic uplift by maintaining the potentiometric

Assumptions

- Potentiometric levels for the Wadsworth Till are assumed to be 5-feet below the existing ground surface and once the liner system is constructed it is assumed to be in contact with the bottom of the low permeability earth liner layer along the base and sideslopes. This represents the worst-case scenario for groundwater.
- The approximate maximum excavation depth will occur in the eastern end of Cell 11. At this location:

Existing ground level* (avg)	= El. 743 feet MSL
Assumed Potentiometric level	= El. 738 feet MSL
Top of leachate collection system (min)	= El. 689 feet MSL
Top of liner (min)	= El. 688 feet MSL
Liner thickness	= 5 feet
Bottom of excavation	= El. 683 feet MSL

* Note: The existing ground level is the approximate average existing ground level in the proposed horizontal expansion. The top of the leachate collection system is the minimum elevation of the leachate collection system in the proposed horizontal expansion.

- The following material properties are assumed:

γ_{water} (γ_w)	= 62.4 pcf
γ_{LCS}	= 130.0 pcf
γ_{liner}	= 134.1 pcf
γ_{waste}	= 75 pcf



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TITLE: STABILITY AGAINST HYDROSTATIC UPLIFT

- ❑ Because of the backfill/liner is considered an engineered, very low permeable layer (relatively impermeable); the hydrostatic uplift forces are assumed to act on the base of the backfill/liner at the bottom of the excavation.
- ❑ The hydrostatic uplift is calculated by multiplying the difference of the excavation elevation and potentiometric elevation by the unit weight of water.
- ❑ The factor of safety (FS) against hydrostatic uplift after excavation is assumed equal to 1.2 and is calculated as:

$$FS = \frac{\text{counter pressure}}{\text{hydrostatic uplift}}$$

Calculations

The factor of safety against hydrostatic uplift is calculated as follows:

$$\text{Hydrostatic uplift} = (El. 743 - El. 683)\gamma_w = (60 \text{ feet})(62.4 \text{ pcf}) = 3,744 \text{ psf}$$

$$\begin{aligned} \text{Counter pressure} &= H_{\text{Waste}}\gamma_{\text{Waste}} + H_{\text{LCS}}\gamma_{\text{LCS}} + H_{\text{Liner}}\gamma_{\text{Liner}} \\ &= H_{\text{Waste}}(75 \text{ pcf}) + (1 \text{ ft})(130.0 \text{ pcf}) + (5 \text{ ft})(134.1 \text{ pcf}) \\ &= 75 \times H_{\text{Waste}} + 800.5 \end{aligned}$$

$$FS = \frac{\text{counter pressure}}{\text{hydrostatic uplift}} = \frac{800.5 + 75 \times H_{\text{Waste}}}{3,744} = 1.2$$

$$H_{\text{Waste}} = 49.2 \text{ feet}$$

Results

Based on the worst anticipated conditions at the site and conservative assumptions, the liner system will be stable with respect to hydrostatic uplift once waste is placed to an initial height of approximately 49.2 feet in the horizontal expansion. Before the waste reaches this height, stability will be achieved by dewatering of the till using the gradient control system. It is recognized that this calculation conservatively represents the worst-case scenario at the site.

J.3-B Foundation Settlement



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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF LEACHATE COLLECTION SYSTEM

Problem Statement

Differential settlement is a measure of the difference in settlement that occurs between different locations based on location-specific loads. Differential settlement has the potential to impact landfills when the variation of settlement is greater than the design slopes of design features. This calculation determines the maximum settlement that is anticipated to occur at multiple locations within the underlying foundation to ensure that the leachate collection system will maintain a positive slope.

Note: A differential settlement of the waste mass is considered in a subsequent calculation to determine whether the final cover will maintain positive drainage after settlement occurs.

Given

- Drawings of the “Final Landform Grades” and “Excavation Grades” contained in this Application.
- Section 2.2, Hydrogeologic Report, in this Application.
- Appendix J.1** “Summary of Geotechnical Parameters” contained in this Application.
- Microsoft Excel settlement calculation spreadsheets (refer to attached pages).
- Yee, K., Menard Geosystems Sdn Bhd, Lumpur, K., Ugrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect, Master Builders Journal, September, 1999 (Refer to attached pages).
- Personal Communication, Research Reference Notes, Craig H. Benson, University of Wisconsin-Madison, 2009.
- Budhu, Muni, Foundations and Earth Retaining Structures (refer to attached pages).



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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF LEACHATE COLLECTION SYSTEM

Background

Consolidation of materials that will lead to settlement is divided into two categories:

1. Primary Consolidation Settlement

Occurs in saturated cohesive soils occurs due to the expulsion of water in response to an increase in effective stress. Primary settlement is determined using the following equation for normally consolidated soils.

$$S_P = \left((C_c) \left(\frac{H_o}{1 + e_o} \right) \log \left(\frac{\sigma'_{vf}}{\sigma'_{vo}} \right) \right)$$

Where,

- S_p = Primary Settlement, feet
- C_c = Compression Index
- H = Thickness of the layer, feet
- e_o = Initial void ratio = $n/(1-n)$
- σ'_{vf} = Final vertical effective stress, psf
- σ'_{vo} = Initial vertical effective stress, psf

2. Secondary Consolidation Settlement

Occurs only in saturated cohesive soils and is the result of the plastic adjustment of soil fabrics. Secondary consolidation is calculated using the following equation.

$$S_S = \left(\left(\frac{C_\alpha}{1 + e_p} \right) H \log \left(\frac{T_2}{T_1} \right) \right)$$

Where:

- S_S = Secondary settlement, feet
- C_α = Secondary compression index
- H = Thickness of Layer, feet
- e_p = Void Ratio at end of primary consolidation
= e_o (to be conservative)
- T_1 = Time at start of secondary compression, 30 years
- T_2 = Time at end of observation period, 60 years

Note: The Wadsworth Till soils have an overconsolidation ratio (OCR) ranging from 0.82 to 1.52 (average value of 1.11, see Appendix J.1) and are therefore normally consolidated to slightly overconsolidated. To be conservative, it is assumed in this calculation that the Wadsworth Till will be normally consolidated.



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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF LEACHATE COLLECTION SYSTEM

Assumptions and Calculations – Liner System/Foundation Settlement

The leachate collection system (LCS) grades will settle as the low permeable earth liner and underlying foundation soils settle due to the weight of the overlying landfill. Differential settlement has been evaluated to ensure that the LCS will continue to drain after compression of the underlying compressible soils occurs. The compressible layers beneath the LCS include the low permeable earth liner and the Wadsworth Till.

The minimum slopes of the LCS is along the pipe runs. Therefore, the maximum potential differential settlement was evaluated along the pipe runs considered the most likely to be impacted by differential settlement in the horizontal and vertical expansions. **Table 1** provides the elevations and thicknesses of layers prior to landfill development. **Table 2** provides the thicknesses of the relevant landfill system layers after landfill development.

Vertical Expansion Locations Analyzed for Liner System / Foundation Settlement

The LCS pipes in the permitted landfill were analyzed to ensure that they will continue to perform as intended after additional loading from the vertical expansion. For the purpose of this evaluation, the right LCS pipe in Cell 7 (1.0% slope down pipe run and 33.33% slope in the LCS sump) and the left LCS pipe in Cell 9 (slope 0.5%) were analyzed. Both of these pipes underlie the vertical expansion area. Points S1, S2, and S3 are used to analyze the Cell 7 LCS pipe and points S4 and S5 are used to analyze the Cell 9 LCS pipe.

The base liner system underlying the vertical expansion has already been constructed. This calculation conservatively evaluates differential settlement at the vertical expansion analysis points due to the weight of the landfill only, rather than evaluating the weight of the landfill less the initial overburden loading. The result of this approach conservatively over-estimates settlement at these locations.

The LCS pipe in Cell 7 was analyzed as it contains the maximum waste thickness overlying a pipe in the vertical expansion area (Point S1). Point S1 is also located in the sump area. Point S2 represents the transition from the sump area (slope 33.33%) to the highest point in the LCS pipe (slope 1%) under the vertical expansion (Point S3). Point S3 is also the maximum till thickness under this pipe.

The LCS pipe in Cell 9 was analyzed because it is the lowest slope of any LCS pipe in either the horizontal or vertical expansion (0.5%). Point S4 represents the maximum waste thickness to overly this specific pipe. Point S5 is the highest point in the LCS pipe that is still under the vertical expansion.



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The horizontal distance (used to determine differential settlement between the points are as follows:

- S1 to S2: 39 ft
- S1 to S3: 366 ft
- S2 to S3: 327 ft
- S4 to S5: 141 ft

Horizontal Expansion Locations Analyzed for Liner System / Foundation Settlement

The leachate pipe that is considered the most likely to be impacted by differential settlement in the horizontal expansion area is the pipe running through the center of Cell 11, for the following reasons:

1. The maximum waste thickness and elevation in the horizontal expansion is located over the LCS pipe in this Cell, which provides the greatest loading that may drive differential settlement.
2. The Wadsworth Till is thicker and more consistently thick under Cell 11 than in any other Cell.
3. The leachate collection pipe exhibits a shallow slope of 1.0%, which provides the potential impact due to differential settlement.

Point S6 represents the lowest point in the LCS pipe for Cell 11. The maximum waste thickness and elevation in the horizontal expansion overlies the LCS pipe in Cell 11 and is located at Point S8. The maximum till thickness beneath the LCS pipe is approximately 40.5 feet and is represented as Point S10. Points S7 and S9 represent the transition points from the final cover plateau to the sideslopes immediately above the Cell 11 pipe.

The horizontal distance (used to determine differential settlement between the points are as follows:

- S6 to S7: 345 ft
- S6 to S8: 464 ft
- S6 to S9: 581 ft
- S6 to S10: 979 ft
- S7 to S8: 119 ft
- S7 to S9: 236 ft
- S7 and S10: 635 ft
- S8 and S9: 117 ft
- S8 to S10: 516 ft
- S9 to S10: 399 ft



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Table 1 – Layer Thicknesses for Analysis by Point Liner System / Foundation Settlement					
Location / Point	Existing Ground Elevation (ft MSL)	Base of Excavation Elevation (ft (MSL)	Bottom of Wadsworth Till Elevation (ft MSL)	Thickness of Wadsworth Till to be Removed (ft)	Thickness of Wadsworth Till to Remain (ft)*
Vertical Expansion Area					
S1	748.0	675.3	652.8	0	22.5
S2	748.0	683.0	652.7	0	30.3
S3	766.6	687.0	652.6	0	34.4
S4	748.6	677.4	652.6	0	24.8
S5	747.2	678.4	652.3	0	26.1
Horizontal Expansion Area					
S6	738.2	683.5	649.1	54.7	34.4
S7	744.2	686.7	652.8	57.5	33.9
S8	744.8	687.8	654.0	57.0	33.8
S9	741.2	689.2	654.2	52.0	35.0
S10	745.3	693.4	652.9	51.9	40.5

* Note: Wadsworth Till will not be removed from the vertical expansion area due to the liner already being constructed. The upper five feet of Wadsworth Till to be removed from the horizontal expansion area is assumed to be unsaturated. The remaining thickness is assumed to be saturated.

Table 2 – Layer Thicknesses for Analysis by Point Liner System / Foundation Settlement					
Location / Point	Final Cover Thickness (ft)	Waste Thickness (ft)	LCS Drainage Layer Thickness (ft)	Low Permeable Earth Liner Thickness (ft)	Saturated Wadsworth Till Thickness (ft)
Vertical Expansion Area					
S1	5	206.0	1	5	22.5
S2	5	198.4	1	5	30.3
S3	5	193.7	1	5	34.4
S4	5	170.2	1	5	24.8
S5	5	168.1	1	5	26.1
Horizontal Expansion Area					
S6	5	103.8	1	5	34.4
S7	5	187.2	1	5	33.9
S8	5	198.0	1	5	33.8
S9	5	185.1	1	5	35.0
S10	5	81.4	1	5	40.5



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Initial Conditions

For each point, the thickness of the Wadsworth Till (separated by saturated and unsaturated thicknesses) is considered to allow a comparison of pre-development and post-development stress concentrations for each compressible unit. The bottom liner under the vertical expansion has already been constructed, so no excavation will be required. The groundwater level under the vertical expansion is conservatively assumed to be in contact with the base of the liner. A groundwater elevation of 5 feet below ground surface is conservatively assumed at each location in the horizontal expansion for initial conditions. Unit weights of each material is based on summaries provided in **Appendix J.1**.

The equations presented on the previous pages were used to estimate the foundation settlement at each point:

Initial Effective Stress. The initial effective stress of the in-situ materials is the average effective stress prior to excavation and waste placement. The initial effective stress for the low permeable earth liner was calculated as its self-weight. Please see the attached spreadsheets for calculations. The effective stress values for initial conditions are summarized in **Table 3** below.

Final Effective Stress. The final effective stress is the effective stress following final cover placement. The final effective stress is calculated at the center of the low permeable earth liner. Please see attached spreadsheets for calculations. The effective stress values for final conditions are summarized in **Table 3**.

As previously noted, the base liner system underlying the vertical expansion has already been constructed. This calculation conservatively evaluates differential settlement at the vertical expansion analysis points due to the weight of the landfill only, rather than evaluating the weight of the landfill less the initial overburden loading. The result of this approach conservatively over-estimates settlement at these locations.



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Table 3 – Layer Stresses by Point Liner System / Foundation Settlement				
Location / Point	Layer Description	Unit Thickness (ft)	Initial Mid-Layer Stress (psf)	Final Mid-Layer Stress (psf)
Vertical Expansion Area				
S1	Earth Liner	5	335.3	16,566.8
	Saturated Till (To Remain in Place)	22.5	848.3	17,750.3
S2	Earth Liner	5	335.3	15,996.8
	Saturated Till (To Remain in Place)	30.3	1,142.3	17,474.3
S3	Earth Liner	5	335.3	15,644.3
	Saturated Till (To Remain in Place)	34.4	1,296.9	17,276.4
S4	Earth Liner	5	335.3	13,881.8
	Saturated Till (To Remain in Place)	24.8	935.0	15,152.0
S5	Earth Liner	5	335.3	13,724.3
	Saturated Till (To Remain in Place)	26.1	984.0	15,043.5
Horizontal Expansion Area				
S6	Earth Liner	5	335.3	8,901.8
	Saturated Till (To Remain in Place)	34.4	5,733.3	10,533.9
S7	Earth Liner	5	335.3	15,156.8
	Saturated Till (To Remain in Place)	33.9	5,925.5	16,770.0
S8	Earth Liner	5	335.3	15,966.8
	Saturated Till (To Remain in Place)	33.8	5,884.1	17,576.3
S9	Earth Liner	5	335.3	14,999.3
	Saturated Till (To Remain in Place)	35.0	5,552.3	16,654.0
S10	Earth Liner	5	335.3	7,221.8
	Saturated Till (To Remain in Place)	40.5	5,752.1	9,083.9



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Primary and Secondary Consolidation Settlement

Table 4 below summarizes the results of calculations completed in spreadsheets that are attached at the end of this section.

Table 4 – Settlement by Point Liner System / Foundation Settlement					
Location / Point	Layer Description	Primary Settlement (ft)	Secondary Settlement (ft)	Total Settlement (ft)	Total Settlement at Point (ft)
Vertical Expansion Area					
S1	Earth Liner	0.5	0.003	0.5	2.2
	Saturated Till	1.7	0.015	1.7	
S2	Earth Liner	0.5	0.003	0.5	2.5
	Saturated Till	2.0	0.020	2.0	
S3	Earth Liner	0.5	0.003	0.5	2.7
	Saturated Till	2.2	0.023	2.2	
S4	Earth Liner	0.5	0.003	0.5	2.1
	Saturated Till	1.7	0.017	1.7	
S5	Earth Liner	0.5	0.003	0.5	2.2
	Saturated Till	1.7	0.018	1.7	
Horizontal Expansion Area					
S6	Earth Liner	0.4	0.003	0.4	0.9
	Saturated Till	0.5	0.023	0.5	
S7	Earth Liner	0.5	0.003	0.5	1.3
	Saturated Till	0.9	0.023	0.9	
S8	Earth Liner	0.5	0.003	0.5	1.4
	Saturated Till	0.9	0.023	0.9	
S9	Earth Liner	0.5	0.003	0.5	1.4
	Saturated Till	0.9	0.024	1.0	
S10	Earth Liner	0.4	0.003	0.4	0.9
	Saturated Till	0.4	0.027	0.5	



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Differential Settlement

The differential settlement between points are calculated as follows:

$$S_{\text{diff}} = \frac{|S_{S1} - S_{S2}|}{\text{Distance}_{S1 \text{ to } S2}} \times 100\%$$

Table 5 – Differential Settlement Between Points Liner System / Foundation Settlement							
Point 1	Point 2	Settlement 1 (ft)	Settlement 2 (ft)	Distance Between Points (ft)	Differential Settlement (%)	Initial Pipe Slope (%)	Pipe Slope After Settlement (%)
Vertical Expansion Area							
S1	S2	2.2	2.5	39	0.89	33.33	32.44
S1	S3	2.2	2.7	366	0.14	1.00	0.86
S2	S3	2.5	2.7	327	0.05	1.00	0.95
S4	S5	2.1	2.2	141	0.04	0.50	0.46
Horizontal Expansion Area							
S6	S7	0.9	1.3	345	0.12	1.00	0.88
S6	S8	0.9	1.4	464	0.10	1.00	0.90
S6	S9	0.9	1.4	581	0.09	1.00	0.91
S6	S10	0.9	0.9	979	0.01	1.00	0.99
S7	S8	1.3	1.4	119	0.04	1.00	0.96
S7	S9	1.3	1.4	236	0.03	1.00	0.97
S7	S10	1.3	0.9	635	0.08	1.00	0.92
S8	S9	1.4	1.4	117	0.02	1.00	0.98
S8	S10	1.4	0.9	516	0.10	1.00	0.90
S9	S10	1.4	0.9	399	0.14	1.00	0.86



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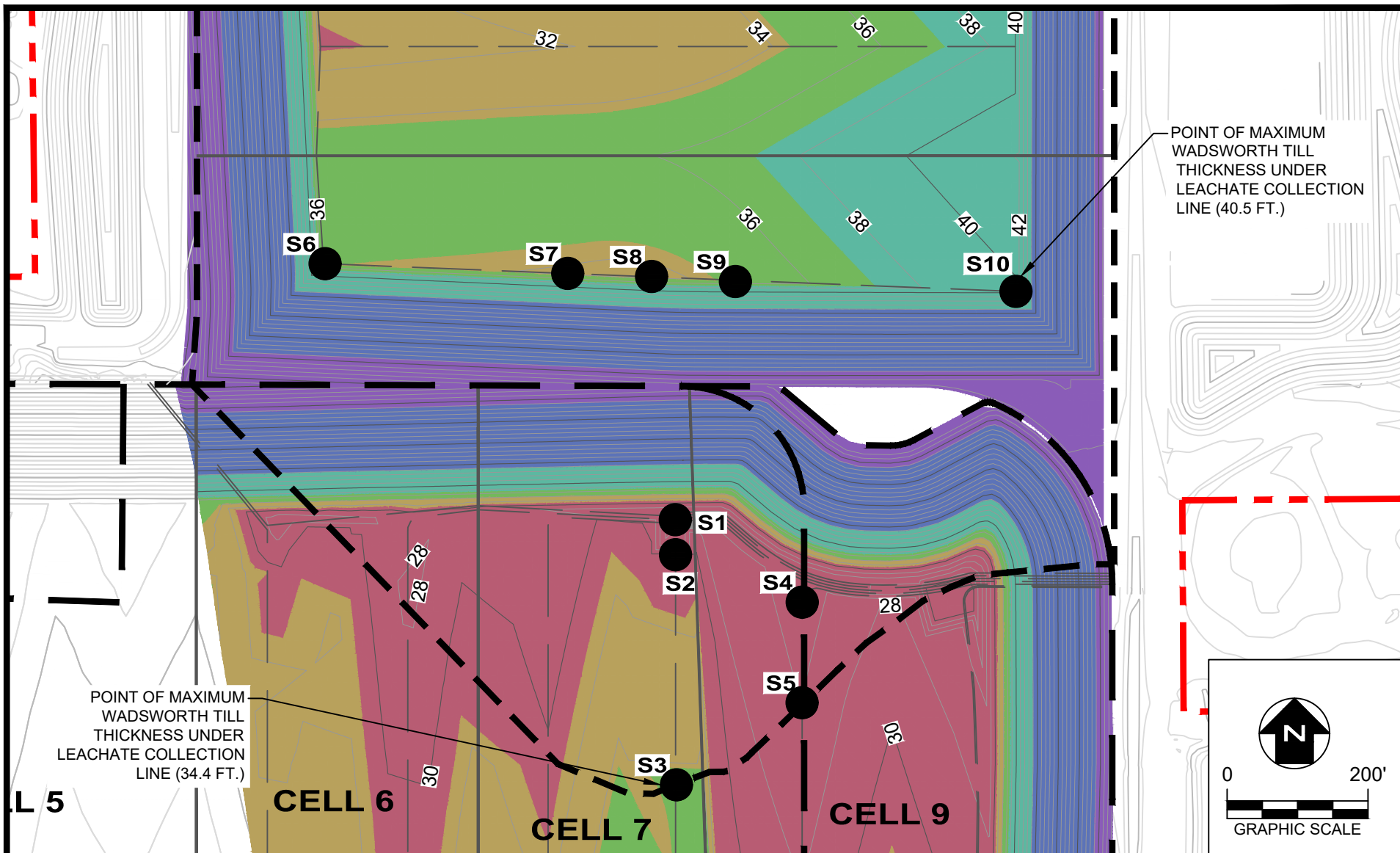
TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF LEACHATE COLLECTION SYSTEM

Summary of Results

The leachate collection system pipe slope in Cell 7 of the vertical expansion was designed with a nominal slope of 1.0% and 33.33% in the sump area. The maximum differential settlement along the LCS pipe is 0.14% and in the sump area it is 0.89%. Therefore, the LCS pipe slope is anticipated to be reduced to 0.86% and in the sump area 32.44%. Both slopes are acceptable, as the pipe will remain free-draining.

The leachate collection system pipe slope in Cell 9 of the vertical expansion was designed with a nominal slope of 0.5%. The maximum differential settlement along this line is anticipated to be 0.04%. Therefore, the pipe slope is anticipated to be reduced to 0.46%. This slope is acceptable, as the pipe will remain free-draining.

The leachate collection system pipe slope in Cell 11 of the horizontal expansion is designed with a nominal slope of 1.0%. The maximum differential settlement along this line is anticipated to be 0.14%, located between points S9 and S10. Therefore, the pipe slope is anticipated to be reduced to 0.86%. This slope is acceptable, as the pipe will remain free-draining.



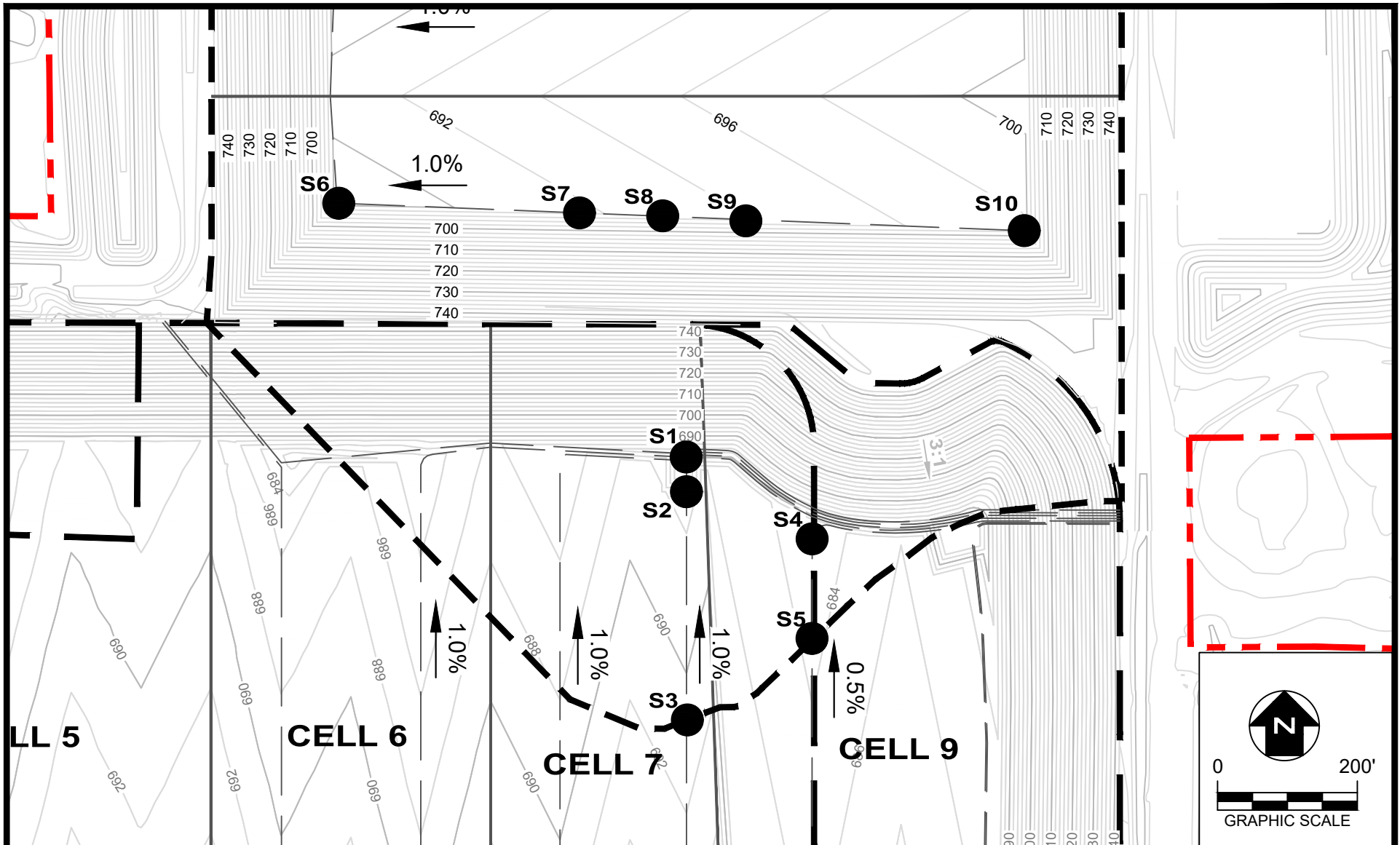
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**FIGURE 1
LOCATION OF FOUNDATION SETTLEMENT ANALYSIS POINTS
THICKNESS OF WADSWORTH TILL TO REMAIN IN PLACE**

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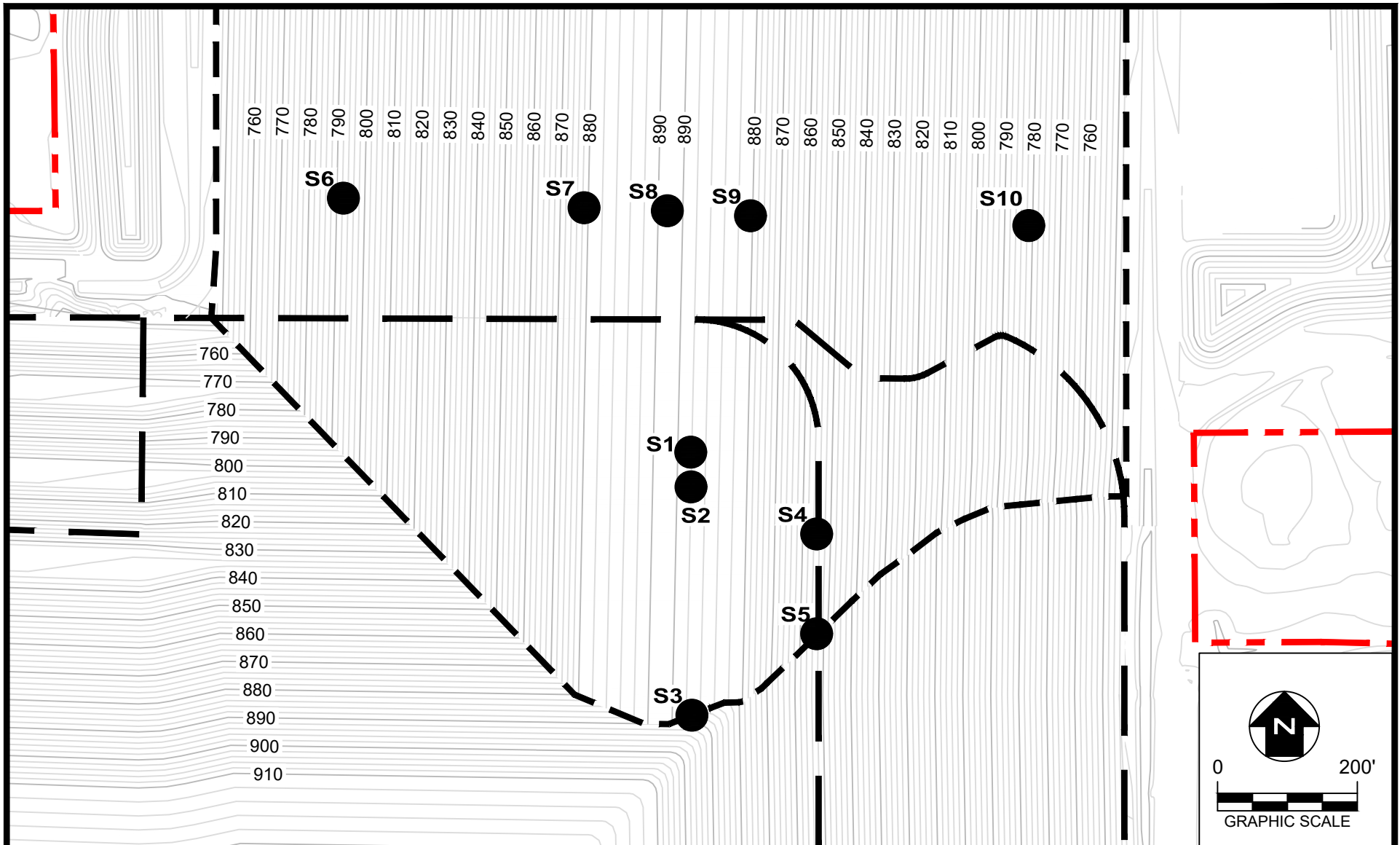
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**FIGURE 2
LOCATION OF FOUNDATION SETTLEMENT ANALYSIS POINTS
TOP OF LINER**

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**FIGURE 3
LOCATION OF FOUNDATION SETTLEMENT ANALYSIS POINTS
TOP OF WASTE**

DRAWN BY:	ORC	APPROVED BY:	RDS	PROJ. NO.:	003211	DATE:	FEBRUARY 2020
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Settlement Points for Leachate Collection Layer						
Initial Stresses by Geologic Layer						
Location / Point	Layer Description	Unit Thickness (ft)	Unit Weight (pcf)	Bottom Layer Stress (psf) (Individual)	Mid Layer Stress (psf) (Individual)	Mid Layer Stress (psf) (Individual + Overlying)
VERTICAL EXPANSION						
S1	Unsaturated Till (To Be Excavated)	0.0	137.8	0.0	0.0	0.0
	Saturated Till (To Be Excavated)	0.0	75.4	0.0	0.0	0.0
	Saturated Till (To Remain in Place)	22.5	75.4	1,696.5	848.3	848.3
S2	Unsaturated Till (To Be Excavated)	0.0	137.8	0.0	0.0	0.0
	Saturated Till (To Be Excavated)	0.0	75.4	0.0	0.0	0.0
	Saturated Till (To Remain in Place)	30.3	75.4	2,284.6	1,142.3	1,142.3
S3	Unsaturated Till (To Be Excavated)	0.0	137.8	0.0	0.0	0.0
	Saturated Till (To Be Excavated)	0.0	75.4	0.0	0.0	0.0
	Saturated Till (To Remain in Place)	34.4	75.4	2,593.8	1,296.9	1,296.9
S4	Unsaturated Till (To Be Excavated)	0.0	137.8	0.0	0.0	0.0
	Saturated Till (To Be Excavated)	0.0	75.4	0.0	0.0	0.0
	Saturated Till (To Remain in Place)	24.8	75.4	1,869.9	935.0	935.0
S5	Unsaturated Till (To Be Excavated)	0.0	137.8	0.0	0.0	0.0
	Saturated Till (To Be Excavated)	0.0	75.4	0.0	0.0	0.0
	Saturated Till (To Remain in Place)	26.1	75.4	1,967.9	984.0	984.0
HORIZONTAL EXPANSION						
S6	Unsaturated Till (To Be Excavated)	5.0	137.8	689.00	344.50	344.50
	Saturated Till (To Be Excavated)	49.7	75.4	3,747.4	1,873.7	2,562.7
	Saturated Till (To Remain in Place)	34.4	75.4	2,593.8	1,296.9	5,733.3
S7	Unsaturated Till (To Be Excavated)	5.0	137.8	689.00	344.50	344.50
	Saturated Till (To Be Excavated)	52.5	75.4	3,958.5	1,979.3	2,668.3
	Saturated Till (To Remain in Place)	33.9	75.4	2,556.1	1,278.0	5,925.5
S8	Unsaturated Till (To Be Excavated)	5.0	137.8	689.00	344.50	344.50
	Saturated Till (To Be Excavated)	52.0	75.4	3,920.8	1,960.4	2,649.4
	Saturated Till (To Remain in Place)	33.8	75.4	2,548.5	1,274.3	5,884.1
S9	Unsaturated Till (To Be Excavated)	5.0	137.8	689.00	344.50	344.50
	Saturated Till (To Be Excavated)	47.0	75.4	3,543.8	1,771.9	2,460.9
	Saturated Till (To Remain in Place)	35.0	75.4	2,639.0	1,319.5	5,552.3
S10	Unsaturated Till (To Be Excavated)	5.0	137.8	689.00	344.50	344.50
	Saturated Till (To Be Excavated)	46.9	75.4	3,536.3	1,768.1	2,457.1
	Saturated Till (To Remain in Place)	40.5	75.4	3,053.7	1,526.9	5,752.1

Settlement Points for Leachate Collection Layer						
Final Stresses by Geologic Layer						
Location / Point	Layer Description	Unit Thickness (ft)	Unit Weight (pcf)	Bottom Layer Stress (psf) (Individual)	Mid Layer Stress (psf) (Individual)	Mid Layer Stress (psf) (Individual + Overlying)
VERTICAL EXPANSION						
S1	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	206.0	75.0	15,450.0	7,725.0	8,376.5
	LCS Drainage Layer	1.0	130.0	130.0	65.0	16,166.5
	Earth Liner	5.0	134.1	670.5	335.3	16,566.8
	Saturated Till (To Remain in Place)	22.5	75.4	1,696.5	848.3	17,750.3
	Final Cover	5.0	130.3	651.5	325.8	325.8
S2	Waste	198.4	75.0	14,880.0	7,440.0	8,091.5
	LCS Drainage Layer	1.0	130.0	130.0	65.0	15,596.5
	Earth Liner	5.0	134.1	670.5	335.3	15,996.8
	Saturated Till (To Remain in Place)	30.3	75.4	2,284.6	1,142.3	17,474.3
	Final Cover	5.0	130.3	651.5	325.8	325.8
S3	Waste	193.7	75.0	14,527.5	7,263.8	7,915.3
	LCS Drainage Layer	1.0	130.0	130.0	65.0	15,244.0
	Earth Liner	5.0	134.1	670.5	335.3	15,644.3
	Saturated Till (To Remain in Place)	34.4	75.4	2,593.8	1,296.9	17,276.4
	Final Cover	5.0	130.3	651.5	325.8	325.8
S4	Waste	170.2	75.0	12,765.0	6,382.5	7,034.0
	LCS Drainage Layer	1.0	130.0	130.0	65.0	13,481.5
	Earth Liner	5.0	134.1	670.5	335.3	13,881.8
	Saturated Till (To Remain in Place)	24.8	75.4	1,869.9	935.0	15,152.0
	Final Cover	5.0	130.3	651.5	325.8	325.8
S5	Waste	168.1	75.0	12,607.5	6,303.8	6,955.3
	LCS Drainage Layer	1.0	130.0	130.0	65.0	13,324.0
	Earth Liner	5.0	134.1	670.5	335.3	13,724.3
	Saturated Till (To Remain in Place)	26.1	75.4	1,967.9	984.0	15,043.5
	Final Cover	5.0	130.3	651.5	325.8	325.8
HORIZONTAL EXPANSION						
S6	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	103.8	75.0	7,785.0	3,892.5	4,544.0
	LCS Drainage Layer	1.0	130.0	130.0	65.0	8,501.5
	Earth Liner	5.0	134.1	670.5	335.3	8,901.8
	Saturated Till (To Remain in Place)	34.4	75.4	2,593.8	1,296.9	10,533.9
S7	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	187.2	75.0	14,040.0	7,020.0	7,671.5
	LCS Drainage Layer	1.0	130.0	130.0	65.0	14,756.5
	Earth Liner	5.0	134.1	670.5	335.3	15,156.8
	Saturated Till (To Remain in Place)	33.9	75.4	2,556.1	1,278.0	16,770.0
S8	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	198.0	75.0	14,850.0	7,425.0	8,076.5
	LCS Drainage Layer	1.0	130.0	130.0	65.0	15,566.5
	Earth Liner	5.0	134.1	670.5	335.3	15,966.8
	Saturated Till (To Remain in Place)	33.8	75.4	2,548.5	1,274.3	17,576.3
S9	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	185.1	75.0	13,882.5	6,941.3	7,592.8
	LCS Drainage Layer	1.0	130.0	130.0	65.0	14,599.0
	Earth Liner	5.0	134.1	670.5	335.3	14,999.3
	Saturated Till (To Remain in Place)	35.0	75.4	2,639.0	1,319.5	16,654.0
S10	Final Cover	5.0	130.3	651.5	325.8	325.8
	Waste	81.4	75.0	6,105.0	3,052.5	3,704.0
	LCS Drainage Layer	1.0	130.0	130.0	65.0	8,621.5
	Earth Liner	5.0	134.1	670.5	335.3	7,221.8
	Saturated Till (To Remain in Place)	40.5	75.4	3,053.7	1,526.9	9,083.9

Settlement Points for Leachate Collection Layer							
Calculated Settlement Values by Layer and Point Location							
Location / Point	Layer Description	Unit Thickness (ft)	Initial Mid-Layer Stress (psf)	Final Mid-Layer Stress (psf)	Primary Settlement (ft)	Secondary Settlement (ft)	Total Settlement at Point (ft)
VERTICAL EXPANSION							
S1	Earth Liner	5.0	335.3	16,566.8	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	22.5	848.3	17,750.3	1.7	0.015	1.7
S2	Earth Liner	5.0	335.3	15,996.8	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	30.3	1,142.3	17,474.3	2.0	0.020	2.0
S3	Earth Liner	5.0	335.3	15,644.3	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	34.4	1,296.9	17,276.4	2.2	0.023	2.2
S4	Earth Liner	5.0	335.3	13,881.8	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	24.8	935.0	15,152.0	1.7	0.017	1.7
S5	Earth Liner	5.0	335.3	13,724.3	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	26.1	984.0	15,043.5	1.7	0.018	1.7
HORIZONTAL EXPANSION							
S6	Earth Liner	5.0	335.3	8,901.8	0.4	0.003	0.4
	Saturated Till (To Remain in Place)	34.4	5,733.3	10,533.9	0.5	0.023	0.5
S7	Earth Liner	5.0	335.3	15,156.8	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	33.9	5,925.5	16,770.0	0.9	0.023	0.9
S8	Earth Liner	5.0	335.3	15,966.8	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	33.8	5,884.1	17,576.3	0.9	0.023	0.9
S9	Earth Liner	5.0	335.3	14,999.3	0.5	0.003	0.5
	Saturated Till (To Remain in Place)	35.0	5,552.3	16,654.0	0.9	0.024	1.0
S10	Earth Liner	5.0	335.3	7,221.8	0.4	0.003	0.4
	Saturated Till (To Remain in Place)	40.5	5,752.1	9,083.9	0.4	0.027	0.5

Differential Settlement for Leachate Collection Layer							
Point 1	Point 2	Settlement 1 (ft)	Settlement 2 (ft)	Distance Between Points (ft)	Differential Settlement (%)	Initial Pipe Slope (%)	Final Pipe Slope (%)
VERTICAL EXPANSION							
S1	S2	2.2	2.5	39	0.89	33.33	32.44
S1	S3	2.2	2.7	366	0.14	1.00	0.86
S2	S3	2.5	2.7	327	0.05	1.00	0.95
S4	S5	2.1	2.2	141	0.04	0.50	0.46
HORIZONTAL EXPANSION							
S6	S7	0.9	1.3	345	0.12	1.00	0.88
S6	S8	0.9	1.4	464	0.10	1.00	0.90
S6	S9	0.9	1.4	581	0.08	1.00	0.92
S6	S10	0.9	0.9	979	0.01	1.00	0.99
S7	S8	1.3	1.4	119	0.04	1.00	0.96
S7	S9	1.3	1.4	236	0.03	1.00	0.97
S7	S10	1.3	0.9	635	0.08	1.00	0.92
S8	S9	1.4	1.4	117	0.02	1.00	0.98
S8	S10	1.4	0.9	516	0.10	1.00	0.90
S9	S10	1.4	0.9	399	0.14	1.00	0.86

Assumed Consolidation Parameters:	
Void Ratio	0.432
Compression Index	0.08
Secondary Compression Index	0.0032

UPGRADING OF EXISTING LANDFILLS BY DYNAMIC CONSOLIDATION A GEOTECHNICAL ASPECT

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ABSTRACT: In recent years, the scarcity of land space available for new urban development has prompted a renewed interest from local authorities in the end use of various landfills or in the extension of the life of existing landfills. Rehabilitation of closed landfills for urban developments has received considerable interest. Likewise, the extension of landfill life to allow for more waste storage is also receiving equal attention. In both cases, ground improvement is required.

Dynamic consolidation (also known as dynamic compaction) is a ground improvement technique. The process involves dropping heavy weights (15ton - 20tons) on to the surface of the fill from a considerable height (15m - 20m) following a selected grid pattern. These high-energy impacts produce sufficient compaction effort to reduce void space, increase density and reduce long-term settlement of the fill. By increasing the density, it increases the storage capacity of the landfill. Beside, it also increases the bearing capacity. Reducing the long-term settlement, roads, parking bays and lighter structures can be designed on shallow foundations on closed landfills.

In this paper, the subject of settlement of waste fills is addressed. A case study concerning a housing development over a landfill is also presented.

1.0 INTRODUCTION

Landfilling is one of the most economic and feasible means of disposing municipal solid waste in Malaysia and other countries in Southeast Asia. In the past, the disposal of waste fills was carried out by uncontrolled dumping into ex-mining ponds and low-lying areas close to housing estates. With increasing scarcity of land in urban areas, it is increasingly difficult to find new landfill sites for future dumping. This has prompted the local authorities and privatized companies (operators of landfill) to find solution to extend the life of the landfill to allow for more waste storage.

Typical landfills may occupy an area ranging from several acres to hundreds of acres. Settlement estimation is a topic of concern. From the operator's viewpoint, landfill capacity will be increased if most settlement occurs during the stage of filling. Unfortunately, the landfill settlement continues over an extended period of time with a final settlement that can be as large as 30%-40% of the initial fill height (H.I.Ling, et.al. 1998). Hence, it is imperative that a solution is needed to increase the rate of settlement to recover the additional space.

Dynamic consolidation is a good method of compacting refuse and waste fill. This technique

involves dropping heavy weights (15 – 20 tons) on to the surface of the fill from a height of 10 to 20m following a selected grid pattern. The high-energy impacts produce shock waves that propagate to great depths (figure 1). As a result, the density of the waste fill is increased and hence, the storage capacity of the landfill is also increased.

With the increase in the density of the waste fill, the overall bearing capacity is improved. The long-term settlement is reduced and hence, the differential settlement is also reduced which is important for the integrity of the cover system when the landfill is closed. In the past such landfills have been considered suitable only for green areas. With the increasing scarcity of land in urban areas, it is making it necessary to build structures above such fills. Charles et.al. (1981) report several case histories of construction on old refuse tips, which include construction of a 2-storey hospital, roads and highways. Welsh (1983) cites a roadway site with 6m to 12m of waste fills. Menard (1984) cites a case for a warehouse designed with floor loads of 20 kN/m² and spread footing with 145 kN/m² with 6m to 17m of refuse waste. There are many other recorded and published case studies on such developments (e.g. Aziz & Mohd. Raihan (1992), Downie & Treharne (1979), Faisal, K.Yee & Varaksin (1997), Fryman & Baker (1987),

Lewis & Langer (1994), Mappleback & Fraser (1993), Steinberg & Lukas (1984), etc.).

In this paper, the subject of settlement of waste fills and rehabilitation of landfill for housing development is presented. Only the geotechnical aspect is covered. The related environmental issue has been intentionally left out due to space constraint.

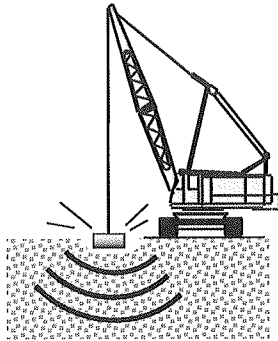


Figure 1

2.0 COMPOSITION OF LANDFILL

Most landfills are heterogeneous and they exhibit anisotropic material properties that are difficult to characterize. Typically, a landfill consists of food and garden wastes, paper products, plastics and rubber, textiles, wood, ashes and the soils used as cover material. Table 1 shows the various components of waste fills with their range of unit weights. The unit weight and void ratio vary with the types of waste, composition, depth, method of compaction and the rate of decomposition, among other factors. The rate of decomposition is further complicated by several factors including the effects of time, temperature and environmental conditions. In short, it is a combination of all of the problems of soft clay, uncompacted fill, organic consolidation and decomposition and even collapse of cavities and erosion of soil into cavities. It is as heterogeneous as the modern industrial-urban complex that produces it. Hence, its composition varies from community to community and from nation to nation. Thus, the waste properties can be considered as site-specific.

Two different forms of landfill can be defined. The uncontrolled dump is of random composition, dumped loosely from trucks,

accumulated without control or compaction, and sometimes covered with a thin layer of soil when it reached its capacity (see figure 2a). At the other end, it is the well-managed sanitary landfill. The materials are spread in layers and compacted by bulldozers and compactors. In some cases, certain wastes such as tires are segregated from others (see figure 2b). Most of the old landfills are the uncontrolled dumps. Until recently, through privatization scheme the landfill operation follows the engineered landfill scheme. Thus, it is expected that developments over old landfills will require more engineering effort.

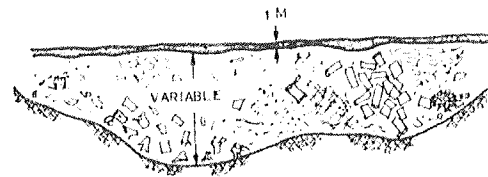


Figure 2(a)

Uncontrolled Landfill (No controlled placement and no compaction)

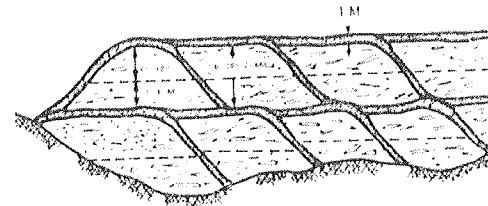


Figure 2(b)

Controlled Sanitary Landfill (Spread and compacted in layers of 2-3m thick; encapsulated with soil in cells of 2-6m thick)

3.0 SETTLEMENT CHARACTERISTICS

Settlement is the major problem with landfills. Sowers (1972) cites a case of a small shopping center built over a landfill. The buildings are on piles driven through the waste fills. The building walls and roof have remained intact. However, floor slabs supported directly on the fill surface have settled as much as 75mm. The floor slabs were connected to the pile-supported exterior grade beams, but was not connected to the interior columns. As a result, the floor drapes downward from the exterior walls toward the interior of the building. Small interior partitions resting directly on the floor have cracked badly and doorframes have been wrecked out of shape.

Table 1 (source: After Tchobanoglous et. al. 1977)

Waste Component	Uncompacted Unit Weight (kN/m ³)	Water Content	Ratio of Compacted to Uncompacted Unit Weight	
			Normal Compaction	Well Compacted
Food waste	1.3 – 4.7	50 – 80	2.9	3.0
Paper / paper board	0.3 – 1.3	4 – 10	4.5	6.2
Plastics	0.3 – 1.3	1 – 4	6.7	10
Textiles	0.3 – 0.9	6 – 15	5.6	6.7
Rubber and leather	0.9 – 2.5	1 – 12	3.3	3.3
Yard waste	0.6 – 2.2	30 – 80	4	5
Wood	1.3 – 3.1	15 – 40	3.3	3.3
Glass	1.6 – 4.7	1 – 4	1.7	2.5
Metals	0.5 – 11.0	2 – 6	4.3	5.3
Ash, brick, dirt	3.1 – 9.4	6 – 12	1.2	1.3

Furthermore, settlement has increased since then, probably due to a change in the moisture environment from leaking sewers in the fill.

There are two possible approaches to the assessment of settlement:

- (a) Extrapolation of monitored data obtained specifically for the given fill
 - 1) By graphical method
 - 2) By analytical method
- (b) Estimation from existing published data on similar type of fills
 - 1) By graphical method
 - 2) By analytical method

Method (a) is the most reliable but requires time for monitoring. This method relies on the approximately linear relationship between settlement and logarithm of time elapsed since placement of waste fill. Method (b) relies on published data for other fills of similar type, and gives approximate answers quickly. However, the results are less dependable since the published data are rarely likely to apply exactly to a specific given fill. Preliminary estimates obtained by method (b) should be checked by monitoring. We shall address the different categories of settlement as follow:

3.1 Settlement Under Self-Weight

One of the contributing factors to the overall settlement is caused by the self-weight of the fill. The time-settlement relationship under self-weight is analogous to the secondary compression of soils after a short period of pseudo-primary settlement, typically, 1 to 4 months long. Measurements taken from past records indicate a coefficient of secondary compression ranging from 0.1 to 0.4 (NAVFAC, 1983). Thus, settlement of the waste fills under its self-weight after completion of filling can be estimated by equation (1) below.

$$(\Delta H)_{sw} = H C_{\alpha} \log (t_2 / t_1) \dots\dots\dots (1)$$

where

- (ΔH)_{sw} = self-weight settlement at time t₂ (m)
- H = thickness of waste fill (m)
- t₁ = time pseudo-primary settlement to occur after completion of fill (years)
- t₂ = time after completion of fill (years)
- C_α = coefficient of secondary compression

Table 2 below suggests typical self-weight settlements. According to Leach & Goodger (1991), a good compaction can reduce the self-weight settlement potential by between 50% and 75%.

Typical unit weights for municipal waste are summarized in Table 3.

Table 4 below shows the unit weights obtained from various landfills sites.

3.2 Settlement Under External Loads

The time-settlement behavior of an old waste fills under an applied load is analogous to the behavior of peat. As load is placed large primary (mechanical) settlements occur rapidly with little or no pore pressure build up. This is followed by secondary compression, which occurs over a long period of time.

The relation of the imposed stress to settlement can be expressed as follow:

$$(\Delta H)_p = H C_r \log (\{ \sigma'_o + \Delta \sigma' \} / \sigma'_o) \dots\dots\dots (2)$$

where

- (ΔH)_p = primary (or mechanical) settlement (m)
- H = thickness of waste fill (m)
- e_o = initial void ratio

Table 2 (Source: Leach & Goodger (1991) – CIRIA Special Publication 78)

Material	Potential Self-Weight Settlement (expressed as % of depth of fill)
Well-compacted, well-graded sand and gravel	0.5
Well-compacted shale and rockfill	0.5
Medium-compacted rockfill	1
Well-compacted clay	0.5
Lightly compacted clay	1.5
Lightly compacted clay placed in deep layers	1 – 2
Nominally compacted opencast backfill	1.2
Uncompacted sand	3.5
Uncompacted (pumped) clay	12
Well-compacted mixed refuse (waste fill)	30
Well-controlled domestic refuse (waste fill) placed in layers and well compacted	10

Table 3

Description	Average Total Unit Weight γ_T (kN/m ³)	Source
Sanitary Landfill		Tchobanoglous et.al. (1977)
• Poor compaction	2.8 – 4.7	
• Moderate to good compaction	4.7 – 7.1	
• Good to excellent compaction	7.1 – 9.4	
• Baled waste	5.5 – 10.5	
• Shredded and compacted	6.4 – 10.5	
• In situ density	5.5 – 6.9	
Active landfill with leachate mound	6.6	
Household Trash Can	1.1	NAVFAC (1983)
Delivery Truck	2.4	
Sanitary Landfill (a) Not shredded		
• Poor compaction	3.1	
• Good compaction	6.3	
• Best compaction	9.4	
Shredded	8.6	
Sanitary Landfill		NSWMA (1985)
• In a landfill	6.9 – 7.5	
After degradation and settlement	9.9 – 11.0	

Table 4

Landfill Sites	Waste Density (kN/m ³)
Old Klang Road, Kuala Lumpur	7.0
Kelana Jaya, Kuala Lumpur	6.0
Merrylands, Sydney ¹	9.4
Thornleigh, Sydney ¹	8.4
Lucas Heights, Sydney ¹	11.3
Albany, New York ²	7 – 16
Fayetteville, Arkansas ³	4.8
Richmond, California ⁴	7.2

Note: 1 – data obtained from Hausmann et.al (1993)

2 – data obtained from Gifford et.al. (1992)

Note: This is a construction and demolition debris landfill

3 – data obtained from Welsh (1983)

4 – data obtained from Sharma et.al. (1989)

σ'_o = effective overburden pressure (kN/m²)
 $\Delta\sigma'$ = effective imposed stress (kN/m²)
 C_r = compression ratio (= $C_c / (1 + e_0)$)
 C_c = compression index

NAVFAC (1983) reports that the primary compression ratio (C_r) ranges from 0.1 to 0.4. Sowers (1972) reports that the compression index (C_c) is related to the initial void ratio as shown in figure 3. The relation can be expressed as follow:

For fills low in organic matters $C_c = 0.15e_0$
 For fills high in organic matters $C_c = 0.55e_0$

It is interesting to note that the maximum C_c for peat is about one-third greater than the maximum observed for waste fills.

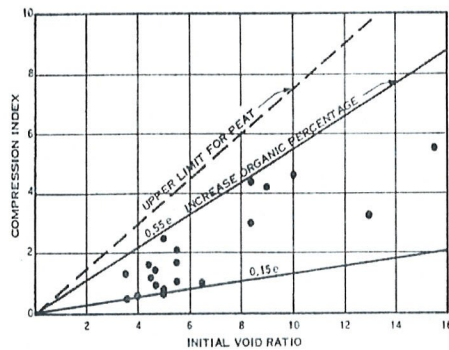


Figure 3

Environmental conditions as well as the composition of the waste fills determine the amount of long-term settlement. This long-term settlement is a combination of mechanical secondary compression, physico-chemical action, and bio-chemical decay. When there is no drastic change in the environment the settlement-log time relationship is more or less linear, similar to secondary compression of soils. The settlement can be expressed by the same equation (1) above. NAVFAC(1983) reported the coefficient of secondary compression (C_α) ranged from 0.02 to 0.07. These values are for fills, which have undergone decomposition for about 10-15 years. Higher compressibility is usually associated with high organic content. It is also true for advanced degree of decomposition.

Sowers (1972) introduces a factor “ α ” for the long-term settlement. He suggested “ α ” as a function of the initial void ratio (e_0). This “ α ” value is high if the organic content subject to decay is large and the environment is favorable (i.e. warm and moist, with fluctuating water table that pumps fresh air into the fill). This value is low for more inert materials and under non-favorable environments. Nonetheless, for any given void ratio there is a large range of values for “ α ” (see figure 4). The relation can be expressed as follow:

For favorable condition to decay $\alpha = 0.03e_0$
 For unfavorable condition to decay $\alpha = 0.09e_0$

This “ α ” value can be translated to the classical C_α by dividing “ α ” by $\{1 + e_0\}$ i.e. $C_\alpha = \alpha / \{1 + e_0\}$.

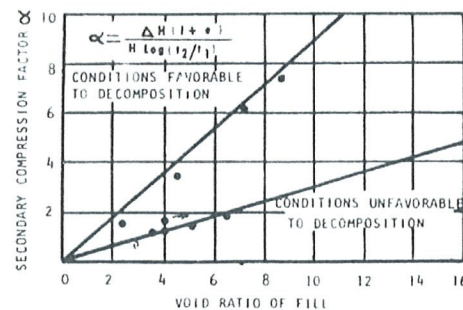


Figure 4

Other calculation methods include the use of a rheological model as presented in the Gibson and Lo theory or the power creep law. The power creep law provides a better representation of the field measured settlement data than the rheological model. However, the rheological model has parameters that can be assigned physical meaning and reflect the effects of certain refuse placement conditions. The details are not presented in this paper.

4.0 DIFFERENTIAL SETTLEMENT & DESIGN MEASURES

There are too many uncertainties for accurate prediction of differential settlement on waste fills. In this case, recourse should be made to the generally accepted rule in engineering practice that, in uniform ground, differential movement will not exceed 75% of the total overall

NOTES ON PRIMARY/INITIAL COMPRESSION OF MSW

Settlement of MSW generally occurs in three phases: initial compression, primary compression, and secondary compression. Initial and primary compression occur rapidly due to the open structure and high hydraulic conductivity of MSW (10^{-3} to 10^{-5} cm/s) and are difficult to separate. Consequently, initial and primary compression usually are grouped together. Secondary compression occurs over longer time frames and is caused by mechanical creep and biological decay. From the perspective of operations and maintenance planning after closure, secondary compression is the most important settlement process. Sowers (1973) indicates that primary compression is normally complete within 30 d and Sharma and De (2007) indicate that 3-4 months is typically assumed in practice. Bjarngard and Edgers (1990) analyzed 24 case histories and reported that primary compression was complete within the first two days following loading.

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FOUNDATIONS
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Layer 2

$$E_u \text{ at top of layer} = 10,000 \text{ kPa, } E_u \text{ at bottom of layer} = 30,000 \text{ kPa}$$

$$(E_u)_{\text{avg}} = \frac{10,000 + 30,000}{2} = 20,000 \text{ kPa}$$

Step 3: Find weighted harmonic mean E_u .

$$E_u = \frac{2(6000) + 1(20,000)}{3} = 10,667 \text{ kPa}$$

Step 4: Find shape parameter $\frac{A_b}{L_1^2}$

$$A_b = (3 \times 4) + (8 \times 6) + (3 \times 4) = 72 \text{ m}^2, \quad \frac{A_b}{L_1^2} = \frac{72}{12^2} = 0.5$$

Step 5: Find shape, embedment, and wall factors.

$$\mu_s = 0.45(0.5)^{-0.38} = 0.59, \quad \mu_{\text{emb}} = 1 - 0.08 \frac{4}{10} \left(1 + \frac{4}{3} \times 0.5\right) = 0.94$$

$$A_w = \text{perimeter} \times \text{depth} = (3 + 4 + 3 + 4 + 4 + 3 + 4 + 5 + 6 + 8) \times 4 = 176 \text{ m}$$

$$\frac{A_w}{A_b} = \frac{172}{72} = 2.44, \quad \mu_{\text{wall}} = 1 - 0.16(2.44)^{0.54} = 0.74$$

Step 6: Calculate immediate settlement.

$$\rho_e = \frac{2Q_a}{E_u L_1} (1 - \nu_u^2) \mu_s \mu_{\text{emb}} \mu_{\text{wall}} = \frac{2 \times 5000}{10667 \times 12} (1 - 0.45^2) \times 0.59 \times 0.94 \times 0.74 = 0.026 \text{ m} = 26 \text{ mm.}$$

7.7.2 Primary Consolidation Settlement

Primary consolidation settlement for fine-grained soils is normally estimated from parameters deduced from the one-dimensional consolidation test. In using the parameters from the one-dimensional consolidation test, you are assuming (1) vertical uniform strain only (lateral strain is zero), (2) no settlement occurs from shear stresses, (3) saturated soil, (4) the initial excess porewater pressure is equal to the change in applied stress at the instant the load is applied, and (5) excess porewater pressures are dissipated only vertically. In practice, lateral strains are significant except when the ratio of the layer thickness to lateral dimension of the loaded area is small (approaches zero). Shear stresses also cause settlement and excess porewater pressures can dissipate not only in the vertical but in any direction.

The initial excess porewater pressure is equal to the change in vertical stress at the instant the vertical stress is applied is possible only if the lateral stresses are equal to the vertical stresses. If the lateral strains are zero, then under undrained conditions (at the instant the load is applied), the vertical settlement is zero.

Normally Consolidated Clays The primary consolidation settlement for normally consolidated soils ($\text{OCR} = 1$) occurs solely along the compression line (NCL) and is given as

$$\rho_{pc} = H \frac{\Delta e}{1 + e_0} = \frac{H}{1 + e_0} C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}; \quad \text{OCR} = 1 \quad (7.49)$$

where H is the thickness of the soil layer, $\Delta e = C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}$, C_c is the compression index, e_0 ($e_0 = wG_s$) is the initial void load ratio, $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma_z$, σ'_{z_0} is the preconsolidation vertical effective stress, and $\Delta\sigma_z$ is the increase in vertical stress at the center of the layer.

Overconsolidated Clays If the soil were overconsolidated, we would have to consider two cases depending on the magnitude of $\Delta\sigma_z$ (Fig. 7.16). In the first case (Fig. 7.16a), the increase in $\Delta\sigma_z$ is such that $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma_z$ is less than the preconsolidation vertical effective stress, σ'_{zc} . In this case, consolidation occurs along the unloading/reloading line (URL), and

$$\rho_{pc} = \frac{H}{1 + e_0} C_r \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}; \quad \sigma'_{\text{fin}} < \sigma'_{zc} \quad (7.50)$$

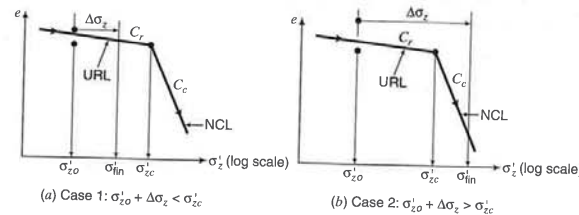


FIGURE 7.16 Two cases to consider for calculating primary consolidation settlement of overconsolidated soils.

In case 2 (Fig. 7.16b), the increase in $\Delta\sigma_z$ is such that $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma_z$ is greater than σ'_{zc} . In this case, we have to consider two components of settlement—one along the URL and the other along the NCL. The primary consolidation for this case is

$$\rho_{pc} = \frac{H}{1 + e_0} \left(C_r \log \frac{\sigma'_{zc}}{\sigma'_{z_0}} + C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{zc}} \right); \quad \sigma'_{\text{fin}} > \sigma'_{zc} \quad (7.51)$$

or

$$\rho_{pc} = \frac{H}{1 + e_0} \left\{ C_r \log(\text{OCR}) + C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{zc}} \right\}; \quad \sigma'_{\text{fin}} > \sigma'_{zc} \quad (7.52)$$

Primary Consolidation Settlement Using m_v You can also calculate the primary consolidation settlement using the modulus of compressibility, m_v . However, unlike C_c , which is constant, m_v varies with vertical effective stress levels. You should compute an average value of m_v over the stress range σ'_{z_0}

J.3-C Bearing Capacity



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj. #: 631020105

Calculated By: ORC

Date: 05/2022

Checked By: DAM

Date: 05/2022

TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION

Problem Statement

Determine the factor of safety against bearing capacity failure of the landfill foundation. 35 Ill. Admin. Code Section 811.304 (c) requires that “the solid waste disposal unit shall be designed to achieve a safety factor against bearing capacity failure of at least: 2.0 under static conditions and 1.5 under seismic loadings.

Given

- Coduto, Donald P, *Foundation Design Principles and Practices*, Second Edition (refer to the attached pages).
- Caterpillar Product Information, 836K, Landfill Compactor (refer to attached pages).
- Appendix J.1** - “Summary of Geotechnical Parameters” contained in this Application.
- Hydrogeologic Report contained in this Application.
- Landfill design specifications for layer types, thicknesses, and material properties (please refer to **Appendix J.1**).
- Design grades for the mass liner grades and final landform shown on respective plan drawings contained in this Application.

Assumptions*Scenarios Analyzed:*

The following four scenarios were analyzed for the proposed Landfill Expansion:

1. Low Permeable Earth Liner bearing capacity for the final landform at the point of maximum waste thickness (long term conditions). The long term analysis is conservatively based on consolidated undrained tri-axial shear strength test results for effective stress conditions.
2. Low Permeable Earth Liner bearing capacity under vehicle loading (short term, undrained conditions).
3. Wadsworth Till foundation bearing capacity for the final landform at the point of maximum waste thickness (long term conditions). The long term analysis is conservatively based on consolidated undrained tri-axial shear strength test results for total stress conditions.
4. Wadsworth Till foundation bearing capacity under vehicle loading (short term, undrained conditions).



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Landfill and Foundation Material Properties:

- The following saturated unit weights were conservatively assumed in the bearing capacity calculations for the Final Cover Soils, Leachate Collection System (LCS) Granular Drainage Layer and Waste Fill (please refer to **Appendix J.1** for detailed discussion):

- Final Cover Soils, $\gamma_{\text{sat}} = 130.3$ pcf
- LCS Granular Drainage Layer, $\gamma_{\text{sat}} = 130$ pcf
- Waste Fill, $\gamma_{\text{sat}} = 75$ pcf

- Low Permeable Earth Liner. The Low Permeable Earth Liner lies directly on top of the Wadsworth Till Foundation and beneath the LCS Granular Drainage Layer. The following material properties were conservatively assumed in the bearing capacity calculations (please refer to **Appendix J.1** for detailed discussion).

Unit Weights:

- Total unit weight, $\gamma_{\text{total}} = 128.2$ pcf
- Saturated unit weight, $\gamma_{\text{sat}} = 134.1$ pcf

Short Term / Undrained Conditions:

- Friction angle, $\phi' = 11.8^\circ$
- Cohesion, $c' = 1,465$ psf

Long Term / Drained Conditions:

- Friction angle, $\phi' = 34.3^\circ$
- Cohesion, $c' = 0$ psf

- Wadsworth Till Foundation. The Wadsworth Till beneath the base liner varies in thickness across the site. Beneath the proposed final landform 10H:1V plateau (which varies in elevation between 885 feet MSL and 896 feet MSL) - where the largest waste column thicknesses occur - the thickness of Wadsworth Till will range in thickness from approximately 29 to 87 feet in the horizontal expansion area, and from approximately 23 to 84 feet in the vertical expansion area. The thickness of Wadsworth Till beneath the maximum waste column of 206-feet (vertical expansion) is approximately 26.4 feet. For the purpose of this calculation it was assumed to be approximately 27-feet thick. The following material properties for the Wadsworth Till Foundation were based on laboratory test results (please refer to **Appendix J.1** for data source):

Unit Weights:

- Total unit weight, $\gamma_{\text{total}} = 136.6$ pcf
- Saturated unit weight, $\gamma_{\text{sat}} = 137.8$ pcf

Short Term / Undrained Conditions:

- Friction angle, $\phi' = 11.8^\circ$



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION

- Cohesion, $c' = 1,465$ psf

Long Term / Undrained Conditions:

- Friction angle, $\phi' = 14.3^\circ$
- Cohesion, $c' = 1,000$ psf

- The maximum waste thickness of **206 feet** occurs in Cell 7 of the vertical expansion area, just south of the horizontal expansion waste boundary. The maximum waste thickness of **198 feet** occurs in the horizontal expansion area in Cell 11. The elevations and thicknesses of the landfill layers corresponding to the proposed expansion maximum waste column are summarized in the table below.

Summary of Average Thickness of Landfill Layers		
Layer	Top Elevation (ft. MSL)	Thickness (ft.)
Final Cover System	896	5
Waste	891	206
LCS Granular Drainage Layer	685	1
Low Permeable Earth Liner	684	5
Wadsworth Till (Foundation Soil)	679	27
Shallow Drift Aquifer	652	-
Total =		244

- Landfill Cell Dimensions. The length and width of the landfill cell coinciding with the maximum waste column in the vertical expansion (Cell 7) location is approximately **940 feet (L) by 307 feet (B)**, respectively.
- Potentiometric Surface. The groundwater potentiometric surface is assumed to be 5 feet below the existing ground surface and in contact with the base of the landfill liner system. Therefore the Wadsworth Till underlying the landfill is assumed to be below the groundwater potentiometric surface (saturated conditions).
- Horizontal Acceleration. The maximum horizontal acceleration is approximately 0.0461g (refer to **Appendix J.1**).



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION*Bearing Capacity Equation for Static Conditions:*

- Karl Terzaghi's bearing capacity equation for square footings is used to calculate bearing capacity of landfill foundation for static conditions. The bearing capacity factor values for N_c , N_q , and N_γ are selected from Table 6.1 of Coduto (refer to attached pages). Due to the size and depth of the landfill, the equation is overly conservative for landfills.

$$q_{ult} = 1.3c'N_c + \sigma'_{zD}N_q + 0.4\gamma'BN_\gamma$$

Where,

q_{ult}	= Ultimate bearing capacity, psf
c, c'	= Soil cohesion (total and effective), psf
σ'_{zD}	= Vertical effective stress, psf
γ'	= Effective unit weight of soil, pcf
B	= Width of foundation, feet
N_c, N_q, N_γ	= Non-dimensional factors, functions of ϕ
ϕ, ϕ'	= Soil friction angle (total, effective), degrees

For Low Permeable Earth Liner – Short Term Loading Conditions:

$$\phi' = 11.8^\circ \approx 12^\circ \rightarrow N_c = 10.8, N_q = 3.3, N_\gamma = 1.4$$

For Low Permeable Earth Liner – Long Term Loading Conditions:

$$\phi' = 34.3^\circ \approx 34^\circ \rightarrow N_c = 52.6, N_q = 36.5, N_\gamma = 39.6$$

For Wadsworth Till Foundation – Short Term Loading Conditions:

$$\phi' = 11.8^\circ \approx 12^\circ \rightarrow N_c = 10.8, N_q = 3.3, N_\gamma = 1.4$$

For Wadsworth Till Foundation – Long Term Loading Conditions:

$$\phi' = 14.3^\circ \approx 14^\circ \rightarrow N_c = 12.1, N_q = 4.0, N_\gamma = 1.9$$



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION

Bearing Capacity Equation for Seismic Conditions:

- For seismic conditions, the total stress on the landfill foundation is equal to the effective overburden stress and the increase in stress due to overturning moment, σ_m , which is calculated using the following equation.

$$\sigma_m = \frac{Mc}{I}$$

$$M = B \times L \times H \times \gamma \left(\frac{H}{2} \right) \text{ (Horiz. Accel.)}$$

$$I = \frac{LB^3}{12}$$

Where,

M = Overturning moment
 c = Distance from center
 I = Moment of Inertia
 B = Width of foundation
 L = Length of foundation
 H = Height of landfill

Factor of Safety:

- For both static and seismic conditions, the factor of safety, FS, is calculated using the following equation:

$$FS = \left(\frac{q_{ult}}{\sigma'_v} \right)$$

Calculations

Calculate ultimate bearing capacity, q_{ult} on the Low Permeable Earth Liner and the Wadsworth Till Foundation. The vertical effective stress (σ'_{zd}) was conservatively assumed equal to zero.

Low Permeable Earth Liner – Short Term Loading Conditions:

($\phi' = 11.8^\circ$, $c' = 1,465$ psf)

$q_{ult} = 1.3c'N_c + \sigma'_{zd}N_q + 0.4\gamma'BN_\gamma$

$q_{ult} = (1.3)(1,465 \text{ psf})(10.8) + (0.0 \text{ psf})(3.3) + (0.4)(134.1-62.4 \text{ pcf})(307 \text{ ft})(1.4)$

$q_{ult} = 32,895$ psf



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION**Low Permeable Earth Liner – Long Term Loading Conditions:**

$$\phi' = 34.3^\circ, c' = 0 \text{ psf}$$

$$q_{ult} = 1.3c'N_c + \sigma'_{zd}N_q + 0.4\gamma'BN_\gamma$$

$$q_{ult} = (1.3)(0 \text{ psf})(52.6) + (0.0 \text{ psf})(36.5) + (0.4)(134.1-62.4 \text{ pcf})(307 \text{ ft})(39.6)$$

$$q_{ult} = 348,668 \text{ psf}$$

Wadsworth Till Foundation – Short Term Loading Conditions:

$$\phi' = 11.8^\circ, c' = 1,465 \text{ psf}$$

$$q_{ult} = 1.3c'N_c + \sigma'_{zd}N_q + 0.4\gamma'BN_\gamma$$

$$q_{ult} = (1.3)(1,465 \text{ psf})(10.8) + (0.0 \text{ psf})(3.3) + (0.4)(137.8-62.4 \text{ pcf})(307 \text{ ft})(1.4)$$

$$q_{ult} = 33,531 \text{ psf}$$

Wadsworth Till Foundation – Long Term Loading Conditions:

$$\phi' = 14.3^\circ, c' = 1,000 \text{ psf}$$

$$q_{ult} = 1.3c'N_c + \sigma'_{zd}N_q + 0.4\gamma'BN_\gamma$$

$$q_{ult} = (1.3)(1,000 \text{ psf})(12.1) + (0.0 \text{ psf})(4.0) + (0.4)(137.8-62.4 \text{ pcf})(307 \text{ ft})(1.9)$$

$$q_{ult} = 33,322 \text{ psf}$$

Bearing Capacity under Final Landform Loading:

Calculate the effective overburden stress (σ'_v) due to waste and soil load for the worst case final conditions.

Low Permeable Earth Liner:

Effective Overburden Stress, σ'_v, on the Low Permeable Earth Liner From Final Landform			
Layer	Thickness, t (ft)	Density, γ (pcf)	$\sigma'_v = (t) \times (\gamma)$ (psf)
Final Cover System	5	130.3	651.5
Waste	206	75.0	15,450
LCS Granular Drainage Layer	1	130.0	130
Total Thickness =	212	$\Sigma(\sigma'_v) =$	16,231.5
Weighted Average $\gamma'_v = 76.6$			



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION
Wadsworth Till Foundation:

Effective Overburden Stress, σ'_v , on the Wadsworth Till Foundation From Final Landform			
Layer	Thickness, t (ft)	Density, γ (pcf)	$\sigma'_v = (t) \times (\gamma)$ (psf)
Final Cover System	5	130.3	651.5
Waste	206	75.0	15,450
LCS Granular Drainage Layer	1	130.0	130
Low Permeable Earth Liner	5	134.1	670.5
Total Thickness = 217		$\Sigma(\sigma'_v) =$	16,902
Weighted Average $\gamma'_v = 77.9$			

Static Conditions

Factor of Safety, FS, against bearing capacity failure at final landform height under long term static conditions (conservatively based on consolidated undrained tri-axial shear strength test results):

$$\text{Low Permeable Earth Liner: } FS = \frac{q_{ult}}{\sigma'_v} = \frac{348,668 \text{ psf}}{16,231.5 \text{ psf}} = 21.5$$

$$\text{Wadsworth Till Foundation: } FS = \frac{q_{ult}}{\sigma'_v} = \frac{33,322 \text{ psf}}{16,902 \text{ psf}} = 2.0$$

Seismic Conditions

First, calculate the increase in stress due to the overturning moment, σ_M .

Low Permeable Earth Liner:

$$M = BLH\gamma_{\text{weighted}} \left(\frac{H}{2} \right) (\text{Horiz. Accel.})$$

$$M = (307 \text{ ft})(940 \text{ ft})(206 \text{ ft})(76.6 \text{ pcf}) \left(\frac{206 \text{ ft}}{2} \right) (0.0461) = 2.16 \times 10^{10} \text{ lb-ft}$$



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$$c = \frac{B}{2} = \frac{307 \text{ ft}}{2} = 153.5 \text{ ft}$$

$$I = \frac{LB^3}{12} = \frac{(940 \text{ ft})(307 \text{ ft})^3}{12} = 2.27 \times 10^9 \text{ ft}^4$$

$$\sigma_m = \frac{Mc}{I} = \frac{(2.16 \times 10^{10} \text{ lb-ft})(153.5 \text{ ft})}{2.27 \times 10^9 \text{ lb-ft}} = 1,461 \text{ psf}$$

Wadsworth Till Foundation:

$$M = BLHY_{\text{weighted}} \left(\frac{H}{2} \right) (\text{Horiz. Accel.})$$

$$M = (307 \text{ ft})(940 \text{ ft})(206 \text{ ft})(77.9 \text{ pcf}) \left(\frac{206 \text{ ft}}{2} \right) (0.0461) = 2.20 \times 10^{10} \text{ lb-ft}$$

$$c = \frac{B}{2} = \frac{307 \text{ ft}}{2} = 153.5 \text{ ft}$$

$$I = \frac{LB^3}{12} = \frac{(940 \text{ ft})(307 \text{ ft})^3}{12} = 2.27 \times 10^9 \text{ ft}^4$$

$$\sigma_m = \frac{Mc}{I} = \frac{(2.20 \times 10^{10} \text{ lb-ft})(153.5 \text{ ft})}{2.27 \times 10^9 \text{ lb-ft}} = 1,488 \text{ psf}$$

Factor of Safety, FS, against bearing capacity failure, at final landform height, under long term, seismic conditions (conservatively based on consolidated undrained tri-axial shear strength test results):

$$\text{Low Permeable Earth Liner: } FS = \frac{q_{\text{ult}}}{\sigma'_v + \sigma_m} = \frac{348,668 \text{ psf}}{16,231.5 \text{ psf} + 1,461 \text{ psf}} = 19.7$$

$$\text{Wadsworth Till Foundation: } FS = \frac{q_{\text{ult}}}{\sigma'_v + \sigma_m} = \frac{33,322 \text{ psf}}{16,902 \text{ psf} + 1,488 \text{ psf}} = 1.8$$



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TITLE: BEARING CAPACITY OF LANDFILL FOUNDATIONBearing Capacity under Vehicle Loading

Calculate the effective overburden stress (σ'_v) due to the placement of the leachate collection system, Low Permeable Earth Liner and loading by a vehicle (compactor). Conservatively assume that the vehicle load does not attenuate with depth.

Assume loading by CAT 836K compactor (Caterpillar Product Information)

$$\text{Weight of the Vehicle } (W_{eq}) = 123,319 \text{ lb}$$

$$\text{Contact Pressure } (P) = \frac{123,319 \text{ lb}}{(4 \text{ drums} \times \text{Area}_{\text{contact}})}$$

$$P = \frac{123,319 \text{ lb}}{(4 \text{ drums} \times (4.67 \text{ ft} \times \frac{1}{3} \times 5.83 \text{ ft}))}$$

$$P = 3,397 \text{ psf}$$

Effective Overburden Stress, σ'_v, on Low Permeable Earth Liner from Vehicle Load			
Layer	Thickness, t (ft)	Density, γ (pcf)	$\sigma'_v = (t) \times (\gamma)$ (psf)
Vehicle Load	-	3,397	3,397
LCS Granular Drainage Layer	1	130	130
Total Thickness =	1	$\Sigma (\sigma'_v) =$	3,527

Effective Overburden Stress, σ'_v, on Wadsworth Till Foundation from Vehicle Load			
Layer	Thickness, t (ft)	Density, γ (pcf)	$\sigma'_v = (t) \times (\gamma)$ (psf)
Vehicle Load	-	3,397	3,397
LCS Granular Drainage Layer	1	130	130
Low Permeable Earth Liner	5	134.1	670.5
Total Thickness =	6	$\Sigma (\sigma'_v) =$	4,197.5



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj. #: 631020105

Calculated By: ORC

Date: 05/2022

Checked By: DAM

Date: 05/2022

TITLE: BEARING CAPACITY OF LANDFILL FOUNDATION

Factor of Safety against bearing capacity failure, vehicle loading, FS:

$$\text{Low Permeable Earth Liner: } FS = \frac{q_{ult}}{\sigma'_v} = \frac{32,895 \text{ psf}}{3,527 \text{ psf}} = 9.3$$

$$\text{Wadsworth Till Foundation: } FS = \frac{q_{ult}}{\sigma'_v} = \frac{33,531 \text{ psf}}{4,197.5 \text{ psf}} = 8.0$$

Results

The proposed design will achieve a minimum factor of safety against bearing capacity failure of 2.0 under static conditions and 1.5 under seismic conditions in accordance with 35 Ill. Admin. Code Section 811.304 (c). A summary of the determined factors of safety against bearing capacity failure of the landfill foundation layers are presented in the following table.

Factors of Safety Against Bearing Capacity Failure			
Conditions	Calculated FS	Required FS	Compliance
Landfill Expansion Design – Low Permeable Earth Liner (Max. Waste Height of 206 feet):			
Static	21.5	2.0	✓
Seismic	19.7	1.5	✓
Vehicle Loading	9.3	2.0	✓
Landfill Expansion Design – Wadsworth Till Foundation (Max. Waste Height of 206 feet):			
Static	2.0	2.0	✓
Seismic	1.8	1.5	✓
Vehicle Loading	8.0	2.0	✓

Published Technical References

SECOND EDITION
**FOUNDATION
DESIGN**

Principles and Practices



DONALD P. CODUTO

proper scaling factors. However, the advent of centrifuge model tests has partially overcome this problem.

Limit equilibrium analyses are the dominant way to assess bearing capacity of shallow foundations. These analyses define the shape of the failure surface, as shown in Figure 6.1, then evaluate the stresses and strengths along this surface. These methods of analysis have their roots in Prandtl's studies of the punching resistance of metals (Prandtl, 1920). He considered the ability of very thick masses of metal (i.e., not sheet metal) to resist concentrated loads. Limit equilibrium analyses usually include empirical factors developed from model tests.

Occasionally, geotechnical engineers perform more detailed bearing capacity analyses using numerical methods, such as the finite element method (FEM). These analyses are more complex, and are justified only on very critical and unusual projects.

We will consider only limit equilibrium methods of bearing capacity analyses, because these methods are used on the overwhelming majority of projects.

Simple Bearing Capacity Formula

The limit equilibrium method can be illustrated by considering the continuous footing shown in Figure 6.4. Let us assume this footing experiences a bearing capacity failure, and that this failure occurs along a circular shear surface as shown. We will further assume the soil is an undrained clay with a shear strength s_u . Finally, we will neglect the shear strength between the ground surface and a depth D , which is conservative. Thus, the soil in this zone is considered to be only a surcharge load that produces a vertical total stress of $\sigma_{zD} = \gamma D$ at a depth D .

The objective of this derivation is to obtain a formula for the *ultimate bearing capacity*, q_{ult} , which is the bearing pressure required to cause a bearing capacity failure. By

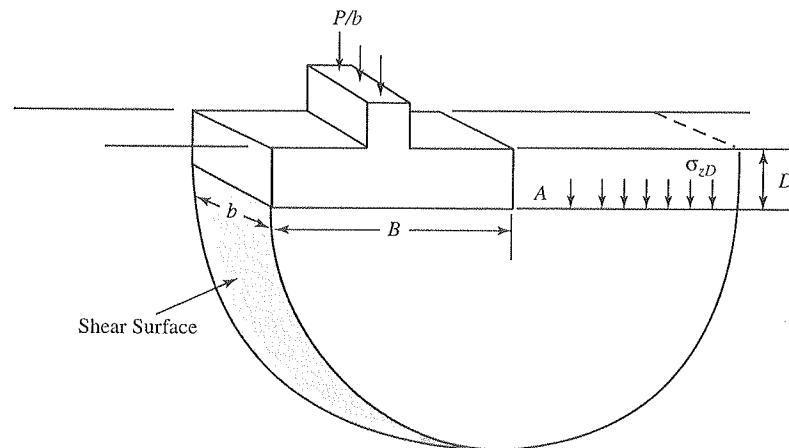


Figure 6.4 Bearing capacity analysis along a circular failure surface.

considering a slice of the foundation of length b and taking moments about Point A, we obtain the following:

$$M_A = (q_{ult} Bb)(B/2) - (s_u \pi Bb)(B) - \sigma_{zD} Bb(B/2) \quad (6.1)$$

$$q_{ult} = 2 \pi s_u + \sigma_{zD} \quad (6.2)$$

It is convenient to define a new parameter, called a *bearing capacity factor*, N_c , and rewrite Equation 6.2 as:

$$q_{ult} = N_c s_u + \sigma_{zD} \quad (6.3)$$

Equation 6.3 is known as a *bearing capacity formula*, and could be used to evaluate the bearing capacity of a proposed foundation. According to this derivation, $N_c = 2\pi = 6.28$.

This simplified formula has only limited applicability in practice because it considers only continuous footings and undrained soil conditions ($\phi = 0$), and it assumes the foundation rotates as the bearing capacity failure occurs. However, this simple derivation illustrates the general methodology required to develop more comprehensive bearing capacity formulas.

Terzaghi's Bearing Capacity Formulas

Various limit equilibrium methods of computing bearing capacity of soils were advanced in the first half of the twentieth century, but the first one to achieve widespread acceptance was that of Terzaghi (1943). His method includes the following assumptions:

- The depth of the foundation is less than or equal to its width ($D \leq B$).
- The bottom of the foundation is sufficiently rough that no sliding occurs between the foundation and the soil.
- The soil beneath the foundation is a homogeneous semi-infinite mass (i.e., the soil extends for a great distance below the foundation and the soil properties are uniform throughout).
- The shear strength of the soil is described by the formula $s = c' + \sigma' \tan \phi'$.
- The general shear mode of failure governs.
- No consolidation of the soil occurs (i.e., settlement of the foundation is due only to the shearing and lateral movement of the soil).
- The foundation is very rigid in comparison to the soil.
- The soil between the ground surface and a depth D has no shear strength, and serves only as a surcharge load.
- The applied load is compressive and applied vertically to the centroid of the foundation and no applied moment loads are present.

Terzaghi considered three zones in the soil, as shown in Figure 6.5. Immediately beneath the foundation is a *wedge zone* that remains intact and moves downward with the

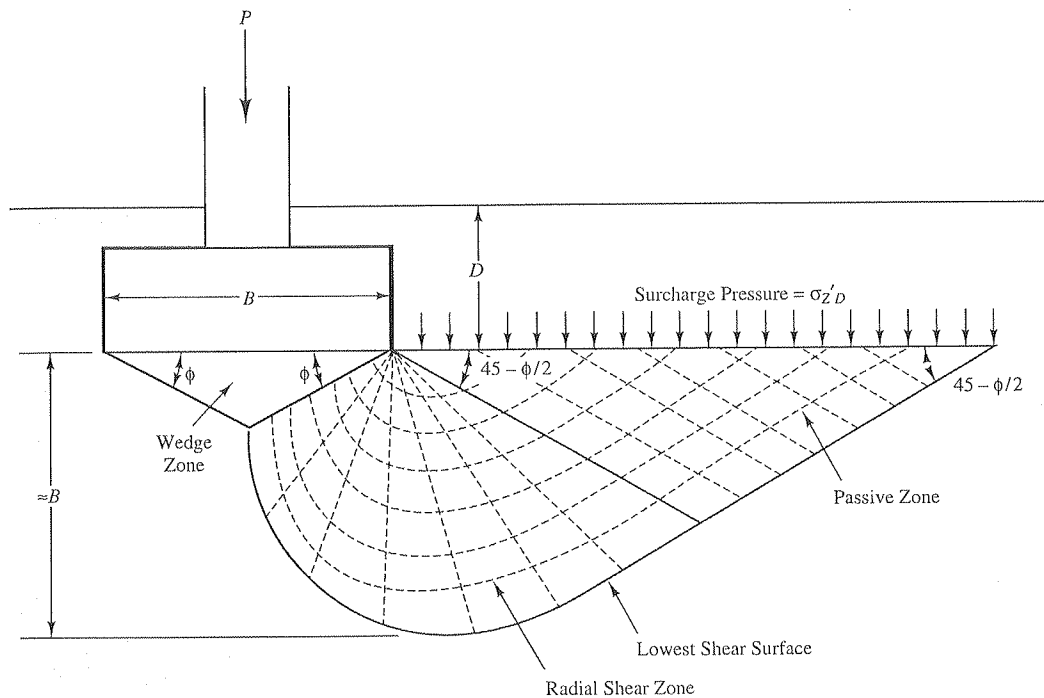


Figure 6.5 Geometry of failure surface for Terzaghi's bearing capacity formulas.

foundation. Next, a *radial shear zone* extends from each side of the wedge, where he took the shape of the shear planes to be logarithmic spirals. Finally, the outer portion is the *linear shear zone* in which the soil shears along planar surfaces.

Since Terzaghi neglected the shear strength of soils between the ground surface and a depth D , the shear surface stops at this depth and the overlying soil has been replaced with the surcharge pressure σ'_{zD} . This approach is conservative, and is part of the reason for limiting the method to relatively shallow foundations ($D \leq B$).

Terzaghi developed his theory for continuous foundations (i.e., those with a very large L/B ratio). This is the simplest case because it is a two-dimensional problem. He then extended it to square and round foundations by adding empirical coefficients obtained from model tests and produced the following bearing capacity formulas:

For square foundations:

$$q_{ult} = 1.3 c'N_c + \sigma'_{zD}N_q + 0.4\gamma'BN_\gamma \quad (6.4)$$

For continuous foundations:

$$q_{ult} = c'N_c + \sigma'_{zD}N_q + 0.5\gamma'BN_\gamma \quad (6.5)$$

For circular foundations:

$$q_{ult} = 1.3 c' N_c + \sigma'_{zD} N_q + 0.3 \gamma' B N_\gamma \quad (6.6)$$

Where:

q_{ult} = ultimate bearing capacity

c' = effective cohesion for soil beneath foundation

ϕ' = effective friction angle for soil beneath foundation

σ'_{zD} = vertical effective stress at depth D below the ground surface
($\sigma'_{zD} = \gamma D$ if depth to groundwater table is greater than D)

γ' = effective unit weight of the soil ($\gamma = \gamma'$ if groundwater table is very deep; see discussion later in this chapter for shallow groundwater conditions)

D = depth of foundation below ground surface

B = width (or diameter) of foundation

N_c, N_q, N_γ = Terzaghi's bearing capacity factors = $f(\phi')$ (See Table 6.1 or Equations 6.7–6.12.)

Because of the shape of the failure surface, the values of c' and ϕ' only need to represent the soil between the bottom of the footing and a depth B below the bottom. The soils between the ground surface and a depth D are treated simply as overburden.

Terzaghi's formulas are presented in terms of effective stresses. However, they also may be used in a total stress analyses by substituting c_T , ϕ_T , and σ_D for c' , ϕ' , and σ'_D . If saturated undrained conditions exist, we may conduct a total stress analysis with the shear strength defined as $c_T = s_u$ and $\phi_T = 0$. In this case, $N_c = 5.7$, $N_q = 1.0$, and $N_\gamma = 0.0$.

The Terzaghi bearing capacity factors are:

$$N_q = \frac{a_\theta^2}{2 \cos^2(45 + \phi'/2)} \quad (6.7)$$

$$a_\theta = e^{\pi(0.75 - \phi'/360)\tan\phi'} \quad (6.8)$$

$$N_c = 5.7 \quad \text{for } \phi' = 0 \quad (6.9)$$

$$N_c = \frac{N_q - 1}{\tan\phi'} \quad \text{for } \phi' > 0 \quad (6.10)$$

$$N_\gamma = \frac{\tan\phi'}{2} \left(\frac{K_{p\gamma}}{\cos^2\phi'} - 1 \right) \quad (6.11)$$

These bearing capacity factors are also presented in tabular form in Table 6.1. Notice that Terzaghi's N_c of 5.7 is smaller than the value of 6.28 derived from the simple bearing capacity analysis. This difference the result of using a circular failure surface in the simple method and a more complex geometry in Terzaghi's method.

TABLE 6.1 BEARING CAPACITY FACTORS

ϕ' (deg)	Terzaghi (for use in Equations 6.4–6.6)			Vesic (for use in Equation 6.13)		
	N_c	N_q	N_γ	N_c	N_q	N_γ
0	5.7	1.0	0.0	5.1	1.0	0.0
1	6.0	1.1	0.1	5.4	1.1	0.1
2	6.3	1.2	0.1	5.6	1.2	0.2
3	6.6	1.3	0.2	5.9	1.3	0.2
4	7.0	1.5	0.3	6.2	1.4	0.3
5	7.3	1.6	0.4	6.5	1.6	0.4
6	7.7	1.8	0.5	6.8	1.7	0.6
7	8.2	2.0	0.6	7.2	1.9	0.7
8	8.6	2.2	0.7	7.5	2.1	0.9
9	9.1	2.4	0.9	7.9	2.3	1.0
10	9.6	2.7	1.0	8.3	2.5	1.2
11	10.2	3.0	1.2	8.8	2.7	1.4
12	10.8	3.3	1.4	9.3	3.0	1.7
13	11.4	3.6	1.6	9.8	3.3	2.0
14	12.1	4.0	1.9	10.4	3.6	2.3
15	12.9	4.4	2.2	11.0	3.9	2.6
16	13.7	4.9	2.5	11.6	4.3	3.1
17	14.6	5.5	2.9	12.3	4.8	3.5
18	15.5	6.0	3.3	13.1	5.3	4.1
19	16.6	6.7	3.8	13.9	5.8	4.7
20	17.7	7.4	4.4	14.8	6.4	5.4
21	18.9	8.3	5.1	15.8	7.1	6.2
22	20.3	9.2	5.9	16.9	7.8	7.1
23	21.7	10.2	6.8	18.0	8.7	8.2
24	23.4	11.4	7.9	19.3	9.6	9.4
25	25.1	12.7	9.2	20.7	10.7	10.9
26	27.1	14.2	10.7	22.3	11.9	12.5
27	29.2	15.9	12.5	23.9	13.2	14.5
28	31.6	17.8	14.6	25.8	14.7	16.7
29	34.2	20.0	17.1	27.9	16.4	19.3
30	37.2	22.5	20.1	30.1	18.4	22.4
31	40.4	25.3	23.7	32.7	20.6	26.0
32	44.0	28.5	28.0	35.5	23.2	30.2
33	48.1	32.2	33.3	38.6	26.1	35.2
34	52.6	36.5	39.6	42.2	29.4	41.1
35	57.8	41.4	47.3	46.1	33.3	48.0
36	63.5	47.2	56.7	50.6	37.8	56.3
37	70.1	53.8	68.1	55.6	42.9	66.2
38	77.5	61.5	82.3	61.4	48.9	78.0
39	86.0	70.6	99.8	67.9	56.0	92.2
40	95.7	81.3	121.5	75.3	64.2	109.4
41	106.8	93.8	148.5	83.9	73.9	130.2

836K

Landfill Compactor



Engine

Engine Model	Cat® C18 ACERT™	
Emissions	Tier 4 Final/Stage IV	
Gross (SAE J1349)	419 kW	562 hp

Operating Specifications

Maximum Operating Weight –	55 927 kg	123,319 lb
Multiple Blade and Wheel Offerings		

836K Landfill Compactor Specifications

Engine

Engine Model	C18 ACERT	
Emissions	U.S. EPA Tier 4 Final and EU Stage IV	
Rated Power (Lab)	414 kW	555 hp
Rated Power (Net ISO 14396)	412 kW	553 hp
Gross (SAE J1349)	419 kW	562 hp
Net Power – SAE J1349		
Direct Drive – Gross Power	370 kW	496 hp
Direct Drive – Torque Rise	52%	
Converter Drive – Gross Power	370 kW	496 hp
Converter Drive – Torque Rise	52%	
Maximum Gross Torque @ 1,300 rpm	3085 N·m	2,275 lbf-ft
Maximum Altitude without Derating	2286 m	7,500 ft
Bore	145 mm	5.71 in
Stroke	183 mm	7.2 in
Displacement	18.1 L	1,104.5 in ³
High Idle Speed	2,120 rpm	
Low Idle Speed	750 rpm	

Operating Specifications

Operating Weight with Full Tank Capacities and U-blade	55 927 kg	123,319 lb
--	-----------	------------

Transmission

Transmission Type	Planetary – Powershift – ECPC	
Travel Speeds		
Forward – Converter 1st	6.2 km/h	3.9 mph
Forward – Lockup 1st	6.5 km/h	4 mph
Forward – Converter 2nd	10.9 km/h	6.8 mph
Forward – Lockup 2nd	11.7 km/h	7.3 mph
Reverse – Converter 1st	6.5 km/h	4 mph
Reverse – Lockup 1st	6.9 km/h	4.3 mph
Reverse – Converter 2nd	10.4 km/h	6.5 mph
Reverse – Lockup 2nd	12.3 km/h	7.6 mph

Hydraulic System

Hydraulic System	Flow Sharing Implement	
Maximum Supply Pressure	32 000 kPa	4,640 psi
Main Relief Pressure	24 100 kPa	3,495 psi
Pump Flow at 2,006 rpm	250 L/min	66 gal/min
Steering System	Double Acting – End Mounted	
Bore	127 mm	5 in
Stroke	740 mm	29.1 in
Vehicle Articulation Angle	86°	
Lift System	Double Acting Cylinder	
Bore	137.9 mm	5.5 in
Stroke	1021 mm	40.2 in

Service Refill Capacities

Fuel Tank	793 L	209 gal
Cooling System	107 L	28 gal
Crankcase	60 L	16 gal
Diesel Engine Fluid Tank	32.8 L	9 gal
Transmission	120 L	32 gal
Differentials and Final Drives – Front	186 L	49 gal
Differentials and Final Drives – Rear	190 L	50 gal
Hydraulic System (tank only)	240 L	63 gal

- All non-road Tier 4 Final/Stage IV, and Japan (MLIT) Step 4 diesel engines are required to use:
 - Ultra Low Sulfur Diesel (ULSD) fuels containing 15 ppm (mg/kg) sulfur or less. Biodiesel blends up to B20 are acceptable when blended with 15 ppm (mg/kg) sulfur or less ULSD and when the biodiesel feedstock meets ASTM D7467 specifications.
 - Cat DEO-ULS™ or oils that meet the Cat ECF-3, API CJ-4, and ACEA E9 specifications are required.

Axles

Front	Planetary – Fixed
Rear	Planetary – Oscillating
Oscillation Angle	13°

Brakes

Control System	Full Hydraulic Split Circuit
Parking Brake	Spring Applied, Hydraulic Released

Cab

	Standard	Suppression
Interior Sound Level	72 dB(A)	71 dB(A)
Exterior Sound Level	111 dB(A)	109 dB(A)

Hydraulic System – Steering

Steering System – Circuit	Steering Double Acting – End Mounted	
Steering System – Pump	Piston – Variable Displacement	
Maximum Flow @ × rpm	52 L/min @ 2,006 rpm	
Steering Pressure Limited	24 100 kPa	3,495 psi
Total Steering Angle	86 degrees	

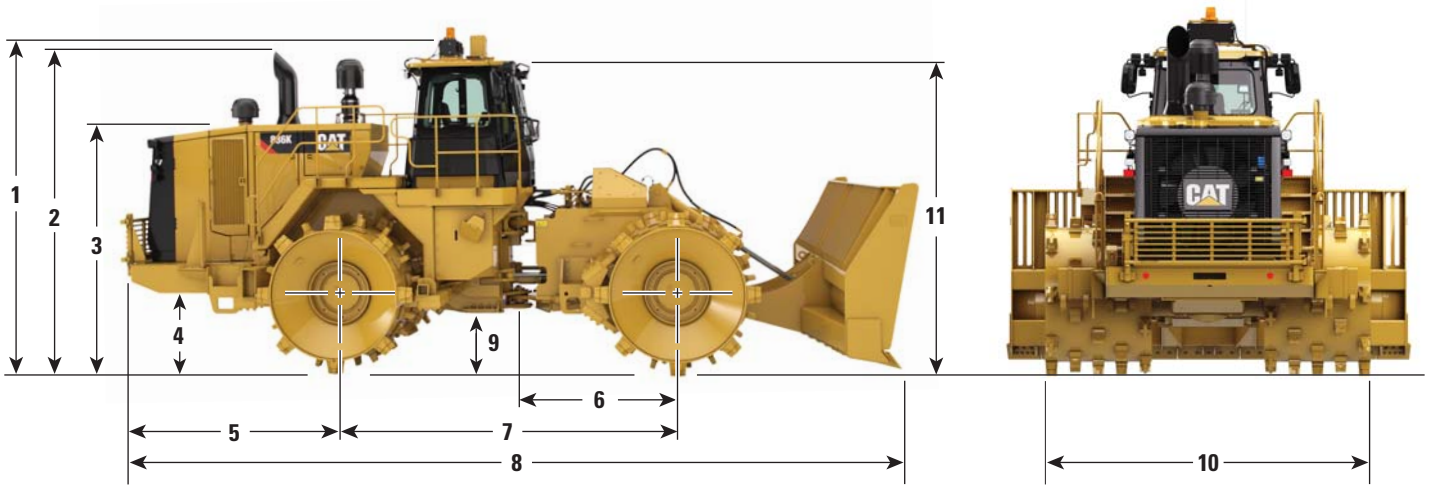
Wheels and Tips

Drum Width	1400 mm	4 ft 8 in
Drum Diameter	1770 mm	5 ft 10 in
Diameter with Tips	2125 mm	7 ft 0 in
Tips per Wheel	40	

836K Landfill Compactor Specifications

Dimensions

All dimensions are approximate.



1 Height to Top of Cab with A/C	4655 mm	15 ft 3 in
2 Height to Top of Exhaust Pipe	4608 mm	15 ft 1 in
3 Height to Top of Hood	3421 mm	11 ft 3 in
4 Ground Clearance to Bumper	1029 mm	3 ft 5 in
5 Center Line of Rear Axle to Edge of Counterweight	3187 mm	10 ft 5 in
6 Hitch to Center Line of Front Axle	2275 mm	7 ft 6 in
7 Wheelbase	4550 mm	14 ft 11 in
8 Length with Blade on Ground (straight blade)	10 182 mm	33 ft 5 in
9 Ground Clearance	632 mm	2 ft 1 in
10 Width over Wheels	4280 mm	14 ft 1 in
11 Height to ROPS/Canopy	4284 mm	14 ft 1 in
Height to Top of Cab with Strobe	4845 mm	15 ft 11 in
Turning Radius – Inside of Wheels	3635 mm	11 ft 11 in

Blade Selection

	Straight Blade		Semi U-blade		U-blade	
Width – Moldboard Length	4990 mm	16 ft 4 in	5238 mm	17 ft 2 in	5172 mm	17 ft
Width Over End Bits	5193 mm	17 ft	5311 mm	17 ft 5 in	5258 mm	17 ft 3 in
Height with Cutting Edge and Screen	2236 mm	7 ft 4 in	2215 mm	7 ft 3 in	2210 mm	7 ft 3 in
Height with Cutting Edge, No Screen	1217 mm	4 ft	1253 mm	4 ft 1 in	1255 mm	4 ft 1 in
Maximum Depth of Cut	364 mm	1 ft 2 in	362 mm	1 ft 2 in	934 mm	3 ft 1 in
Maximum Lift above Ground	1730 mm	5 ft 8 in	1735 mm	5 ft 8 in	1198 mm	3 ft 11 in
Cutting Edges, Reversible						
Length, Each End Section (3 edges)	1408.2 mm	4 ft 7 in	816.6 mm	2 ft 8 in	2 @ 779.1 mm and 1 @ 856 mm	2 @ 2 ft 7 in and 1 @ 2 ft 10 in
Length, Each End Section (2 edges)	NA		988 mm	3 ft 3 in	1094.4 mm	3 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
End Bits (2), Self-sharpening						
Length, Each	472 mm	1 ft 7 in	472 mm	1 ft 7 in	472 mm	1 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
Capacity, Rated	19.3 m ³	25.9 yd ³	22.4 m ³	29.3 yd ³	9.74 m ³	13 yd ³
Turning Diameter, Outside Corner of Blade at 43° ART	8737 mm	28 ft 8 in	8823 mm	28 ft 11 in	8795 mm	28 ft 10 in
Overall Machine Length	10 182 mm	33 ft 5 in	10 379 mm	34 ft 1 in	10 272 mm	33 ft 8 in

J.4 – Liner/LCS Evaluations

- J.4-A Anchor Trench
- J.4-B Wheel Loading on Geomembrane
- J.4-C Puncture Resistance of Geosynthetics

J.4-A Anchor Trench



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: ANCHOR TRENCH / RUNOUT

Problem Statement:

The geosynthetics to be used as part of the Expansion design provide sufficient friction angles that they are anticipated to hold themselves in place after installation. However, anchor trenches are proposed to be used along the perimeter of the waste boundary to bury the edge of geosynthetic materials. This is completed in order to protect the edges and provide protection from wind uplift.

In the unlikely event that a geosynthetic is subjected to sufficient force to pull it from its installed location, it is important that the anchor trench is designed to allow the geosynthetic material to be pulled out prior to tearing or damaging the geosynthetic. This calculation determines the maximum acceptable anchor trench dimensions that will ensure a “pull-out” condition.

Given:

- Coduto, D.P., *Foundation Design, Principles and Practices*. Prentice Hall, Second Edition (refer to attached pages).
- Koerner, R.M., *Designing with Geosynthetics*. Prentice Hall, Fifth Edition (refer to attached pages).
- Sharma, Hari and Lewis Sangeeta, *Waste containment Systems, Waste Stabilization, and Landfills Design and Evaluation*, John Wiley and Sons, Inc., 1994 (refer to attached pages).
- Poly-Flex (Manufacturer) product information for 60-mil textured HDPE geomembrane (refer to attached pages).
- Appendix J.1 “Summary of Geotechnical Design Parameters” contained in this Application.
- Design detail of liner termination, including slope of geomembrane liner runout.
- Cross sectional diagrams of liner termination (refer to attached pages).

Assumptions

- Anchor trenches will line the perimeter of the landfill in order to protect the geosynthetic edges and prevent wind uplift.
- Holding capacity (resistance) to pull-out should not exceed the ultimate material strength of the geomembrane. The geomembrane will tear if a force greater than the ultimate strength is applied to the geomembrane. The holding capacity should be slightly less than the ultimate tensile strength to be anchored (see attached Sharma and Lewis reference).



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- The equations for anchor trenches developed by Koerner (refer to attached pages) were modified to account for the 3H:1V slope of the geomembrane liner runout (refer to attached sketches).

$$\Sigma F_x = 0$$

$$(T_{allow}) (\cos \beta) = F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P$$

Therefore:

$$(T_{allow}) = \frac{F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P}{\cos \beta}$$

Where,

$$F_{U\sigma} = (\sigma_n) (\tan \delta) (L_{RO}) = 0 \text{ (negligible)}$$

$$F_{L\sigma} = (\sigma_n) (\tan \delta) (L_{RO})$$

$$F_{LT} = (T_{allow}) (\sin \beta) (\tan \delta)$$

$$P_a = \left[\left(\frac{\gamma_{AT} * d_{AT}^2 * K_a}{2} \right) - (2c * d_{AT} \sqrt{K_a}) + \left(\frac{2c^2}{\gamma_{AT}} \right) \right]$$

$$K_a = \left(\tan^2 \left(45^\circ - \frac{\phi}{2} \right) \right)$$

$$P_p = \left[\left(\frac{\gamma_{AT} * d_{AT}^2 * K_p}{2} \right) + (2c * d_{AT} \sqrt{K_p}) \right]$$

$$K_p = \left(\tan^2 \left(45^\circ + \frac{\phi}{2} \right) \right)$$

Note: Equations for active and passive earth pressures were modified to account for soil cohesion (see attached pages). Variables used above are defined on the following page.

Note: Variables used above are defined on the next page.
Where,

T_{allow}	= Allowable force in geomembrane
β	= Side slope angle (degrees)
$F_{U\sigma}$	= Shear force above geomembrane due to cover soil (note, that for thin cover soils tensile cracking will occur and this value will be negligible)
$F_{L\sigma}$	= Shear force below geomembrane due to cover soil
F_{LT}	= Shear force below geomembrane due to vertical component of T_{allow}
P_A	= Active earth pressure against the backfill side of the anchor trench



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TITLE: ANCHOR TRENCH / RUNOUT

P_P	=	Passive earth pressure against the in-situ side of the anchor trench
K_A	=	Coefficient of active earth pressure
K_P	=	Coefficient of passive earth pressure
σ_n	=	Applied normal stress from cover soil = $(d_{CS})(\gamma_{CS})$
d_{CS}	=	Depth of cover soil overlying runout for the “no anchor trench scenario”
d_{CS-AT}	=	Depth of cover soil overlying runout for the “with anchor trench scenario”
γ_{CS}	=	Unit weight of cover soil
δ	=	Angle of shearing resistance (interface friction angle) between geomembrane and soil
L_{RO}	=	Length of geomembrane runout
d_{AT}	=	Depth of anchor trench
γ_{AT}	=	Unit weight of backfill soil in anchor trench
ϕ	=	Angle of shearing resistance (friction angle) of backfill soil
c	=	Cohesion of backfill soil

- Assumed geomembrane yield strength (allowable geomembrane stress), $T_{allow} = 126 \text{ lb/in} = 1,512 \text{ lb/ft}$ (see product information).
- Interface friction angle (δ) for earth liner soil to textured geomembrane = 14.9 degrees (refer to **Appendix J.2-C** and **J.2-D**).
- Backfill and cover soils to be recompacted cohesive soil for which the unit weight (γ_{CS}) was conservatively estimated to be 134.1 pcf (saturated unit weight).
- Assumed a 2.0-foot thick cover soil over runout, for which $\gamma_{CS} = 134.1 \text{ pcf}$.
- Assumed the offset distance of the anchor trench (horizontal runout, L_{RO}) to be 2-feet from the top of slope, for which backfill $\gamma_{AT} = 134.1 \text{ pcf}$.
- The depth of anchor trench (d_{AT}) was solved for using the equations provided on page 2 (unit weight of backfill soil: $\gamma_{AT} = 134.1 \text{ pcf}$).
- Assumed undrained conditions: friction angle of backfill soil (ϕ) = 11.8 degrees and a cohesion value of backfill soil (c) = 1,465 psf, to be conservative.
- Liner side slope angle, $\beta = 18.43 \text{ degrees}$.



Client: Zion Landfill, Inc.

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TITLE: ANCHOR TRENCH / RUNOUT**Calculations**

Determine the maximum depth of the anchor trench (d_{AT}) by trial and error.

$$(T_{actual}) = \frac{F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P}{\cos(\beta)}$$

$$F_{U\sigma} = 0 \text{ (negligible)}$$

$$\begin{aligned} F_{L\sigma} &= (\sigma_n)(\tan\delta)(L_{RO}) = (2.0 \text{ ft} \times 134.1 \text{ pcf})(\tan 14.9^\circ)(2 \text{ ft}) \\ &= 142.7 \text{ lb / ft} \end{aligned}$$

$$\begin{aligned} F_{LT} &= (T_{allow})(\sin\beta)(\tan\delta) = (1,512 \text{ lb/ft})(\sin 18.43^\circ)(\tan 14.9^\circ) \\ &= 127.2 \text{ lb / ft} \end{aligned}$$

$$P_a = \left[\left(\frac{Y_{AT} \times d_{AT}^2 \times K_a}{2} \right) - (2c \times d_{AT} \sqrt{K_a}) + \left(\frac{2c^2}{Y_{AT}} \right) \right]$$

$$K_a = \left(\tan^2 \left(45^\circ - \frac{\phi}{2} \right) \right) = \left(\tan^2 \left(45^\circ - \frac{11.8^\circ}{2} \right) \right) = 0.7$$

$$\begin{aligned} P_a &= \left[\left(\frac{134.1 \text{ pcf} \times (d_{AT})^2 \times 0.7}{2} \right) - (2 \times 1,465 \text{ psf} \times d_{AT} \sqrt{0.7}) + \left(\frac{2(1,465 \text{ psf})^2}{134.1 \text{ pcf}} \right) \right] \\ &= [46.9d_{AT}^2 - 2,451d_{AT} + 32,009] \text{ lb / ft} \end{aligned}$$

$$P_p = \left[\left(\frac{Y_{AT} \times d_{AT}^2 \times K_p}{2} \right) + (2c \times d_{AT} \sqrt{K_p}) \right]$$

$$K_p = \left(\tan^2 \left(45^\circ + \frac{\phi}{2} \right) \right) = \left(\tan^2 \left(45^\circ + \frac{11.8^\circ}{2} \right) \right) = 1.5$$



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$$P_P = \left[\left(\frac{134.1 \text{ pcf} \times (d_{AT})^2 \times 1.5}{2} \right) + \left(2 \times 1,465 \text{ psf} \times d_{AT} \sqrt{1.5} \right) \right]$$

$$= [100.6d_{AT}^2 + 3,589d_{AT}] \text{ lb / ft}$$

$$(T_{\text{actual}}) = \frac{F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P}{\cos(\beta)}$$

$$= \left(\frac{0 + 142.7 + 127.2}{\cos(18.43)} \right) \text{ lb/ft} - \left(\frac{46.9d_{AT}^2 - 2,451d_{AT} + 32,009}{\cos(18.43)} \right) \text{ lb/ft} + \left(\frac{100.6d_{AT}^2 + 3,589d_{AT}}{\cos(18.43)} \right) \text{ lb/ft}$$

$$(T_{\text{actual}}) = \left(\frac{53.7d_{AT}^2 + 6,040d_{AT} - 31,739.1}{\cos(18.43)} \right) \text{ lb/ft}$$

Set T_{actual} to 1,512 lb/ft and solve for $d_{AT} \rightarrow d_{AT} = 5.2 \text{ ft}$.

Results

Based on the strength properties of geomembranes, the depth of the anchor trench should not exceed 5.2-feet in order to ensure that the geomembrane will not be damaged in the unlikely event that it is subjected to significant forces. The proposed design depth for each anchor trench is 2.0-feet. Therefore, the proposed anchor trench design is considered appropriate.



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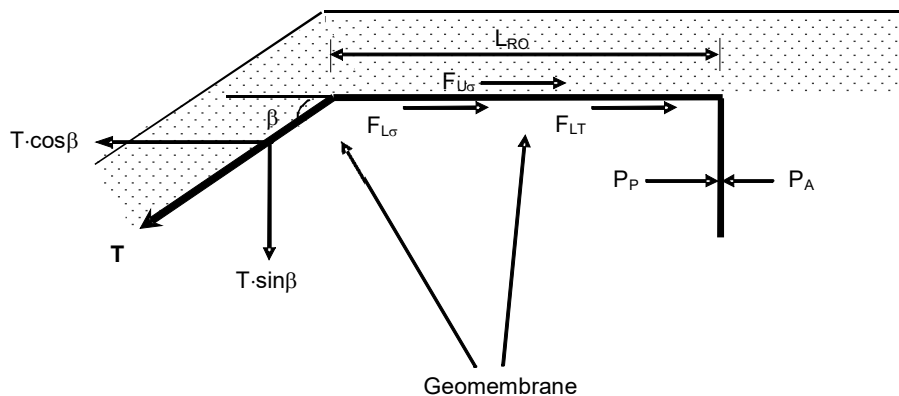
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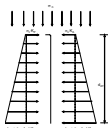
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Published Technical References

SECOND EDITION
**FOUNDATION
DESIGN**

Principles and Practices



DONALD P. CODUTO

23.3 Lateral Earth Pressures in Soils with $C \geq 0$ and $\Phi \geq 0$

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- 23.3 A basement is to be built using 2.5-m tall masonry walls. These walls will be backfilled with a silty sand that has $c' = 0$, $\phi' = 35^\circ$, and $\gamma = 19.7 \text{ kN/m}^3$. Assuming the at-rest conditions will exist and using an overconsolidation ratio of 2, compute the normal force per meter acting on the back of this wall. Also, draw a pressure diagram and indicate the lateral earth pressure acting at the bottom of the wall.
- 23.4 A 10-ft tall concrete wall with a vertical back is to be backfilled with a silty sand that has a unit weight of 122 lb/ft^3 , an effective cohesion of 0, and an effective friction angle of 32° . The ground behind the wall will be level. Using Rankine's method, compute the normal force per foot acting on the back of the wall. Assume the wall moves sufficiently to develop the active condition in the soil.
- 23.5 The wall described in Problem 23.4 has a foundation that extends from the ground surface to a depth of 2 ft. As the wall moves slightly away from the backfill soils to create the active condition, the footing moves into the soils below the wall, creating the passive condition as shown in Figure 23.9. Compute the ultimate passive pressure acting on the front of the foundation.
- 23.6 A 12-ft tall concrete wall with a vertical back is to be backfilled with a clean sand that has a unit weight of 126 lb/ft^3 , an effective cohesion of 0, and an effective friction angle of 36° . The ground behind the wall will be inclined at a slope of 2 horizontal to 1 vertical. Using Rankine's method, compute the normal and shear forces per foot acting on the back of the wall. Assume the wall moves sufficiently to develop the active condition in the soil.
- 23.7 Repeat Problem 23.6 using Coulomb's method.

23.3 LATERAL EARTH PRESSURES IN SOILS WITH $c \geq 0$ AND $\phi \geq 0$

Rankine did not address lateral earth pressures in soils with cohesion ($c \geq 0$ and $\phi \geq 0$) and Coulomb did not address passive pressures. However, later investigators developed complete formulas for cohesive soils. Bell (1915) was among those who contributed.

Theoretical Behavior

Soils with cohesion can stand vertically to a height of no more than the *critical height*, H_c :

$$H_c = \frac{2c}{\gamma \sqrt{K_a}} \quad (23.20)$$

In other words, if $H < H_c$ the earth will stand vertically without a wall. In practice we would apply some factor of safety to H_c (perhaps 1.5 to 2) before deciding not to build a wall. An engineer also would want to consider the potential for surface erosion and other modes of failure.

If $H > H_c$, the theoretical pressure distribution is as shown in Figure 23.14.

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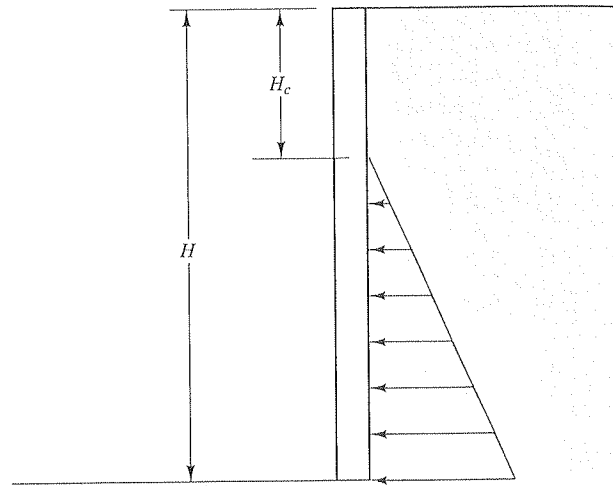


Figure 23.14 Theoretical active pressure distribution in soils with cohesion ($c \geq 0$, $\phi \geq 0$).

Equations 23.5 and 23.6 then become:

$$P_a/b = \left(\frac{\gamma H^2 K_a}{2} - 2cH\sqrt{K_a} + \frac{2c^2}{\gamma} \right) \cos\beta \geq 0 \quad (23.21)$$

$$V_a/b = \left(\frac{\gamma H^2 K_a}{2} - 2cH\sqrt{K_a} + \frac{2c^2}{\gamma} \right) \sin\beta \geq 0 \quad (23.22)$$

These formulas often are incorrectly stated without the $2c^2/\gamma$ term. This term must be present to account for the lack of tensile forces between the wall and the soil at depths shallower than H_c .

The Rankine equations for passive conditions in soils with cohesion are as follows:

$$P_p/b = \left(\frac{\gamma H^2 K_p}{2} + 2cH\sqrt{K_p} \right) \cos\beta \quad (23.23)$$

$$V_p/b = \left(\frac{\gamma H^2 K_p}{2} + 2cH\sqrt{K_p} \right) \sin\beta \quad (23.24)$$

The theoretical shape of the passive pressure distribution is shown in Figure 23.15.

Actual Behavior

The theoretical behavior may be approximately correct for sandy soils that develop cohesive strength from cementing agents, such as calcium carbonate or iron oxide. However, it is not a good indicator of lateral earth pressures in clays, and may produce unconservative

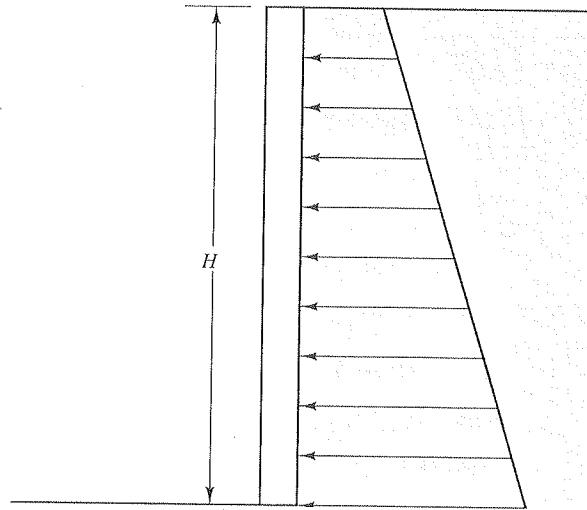


Figure 23.15 Theoretical distribution of passive earth pressure in soils with cohesion ($c \geq 0$, $\phi \geq 0$).

designs. This discrepancy occurs because the earth pressure theories do not consider the following aspects of clay behavior:

Creep

When the shear stresses in a clayey soil are a large percentage of the shear strength, a phenomenon known as *creep* can occur. The soil slowly shears and never reaches complete equilibrium unless the shear stresses become sufficiently small. When this occurs behind a retaining wall, the failure wedge slowly moves toward the wall, so it is impossible to maintain the active condition (which demands high shear stresses) for long periods. Therefore, design cantilever walls backfilled with clay using something higher than the active pressure.

Expansiveness

Another potentially more significant problem is that clayey soils may be expansive (see Chapter 19). An expanding backfill places very large loads on the wall, far greater than the computed active or at-rest pressure. The exact magnitude of lateral pressures caused by expansive soils is difficult to predict, but the passive pressure would be an upper bound.

The cyclic expansion and contraction cycles in expansive soils further aggravate this situation and could cause the following scenario:

1. The soil expands and the retaining wall moves outward.
2. The soil shrinks and moves slightly downhill, thus remaining in intimate contact with the wall. Cracks form.

3. Loose debris falls into the cracks, preventing them from closing.
4. The soil expands again and moves the wall farther out.

This process could continue indefinitely, moving the wall much farther than would a single cycle of expansion.

A slightly cohesive soil, such as an SC, would probably not pose any serious problems, but highly cohesive soils, such as a CH, could produce large movements or even failure.

Many of the preventive measures described in Chapter 19 are also appropriate for reducing the potential for expansion behind a retaining wall.

Poor Drainage

Clays have a low hydraulic conductivity, and thus obstruct drainage. Therefore, groundwater may become trapped behind the wall, producing hydrostatic pressures.

Design Guidelines

The best procedure is to avoid backfilling any wall with clay, especially in regions with adverse climates. It is often possible to bring in other soils from on-site or off-site to use for backfill material. However, if clay is used, the design must reflect their presence. Jennings (1973) described the South African practice of designing such walls using at-rest pressures with a K_0 of 0.8 to 1.0 and making them as flexible as possible. Terzaghi and Peck's design values, described in Section 23.5, also provide lateral earth pressures for walls supporting clay.

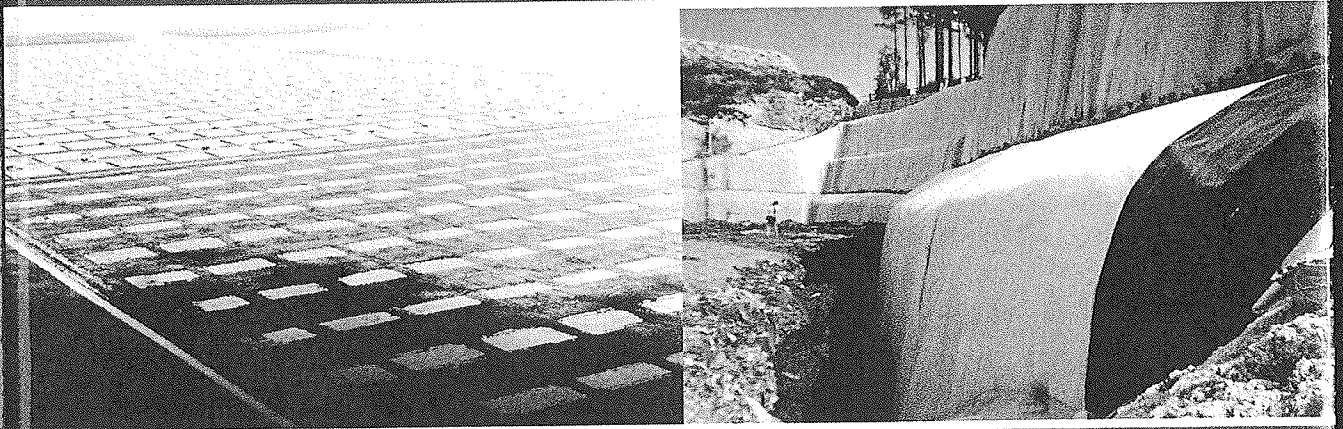
23.4 EQUIVALENT FLUID METHOD

As discussed earlier, the theoretical distribution of lateral earth pressure acting on a wall backfilled with a cohesionless soil is triangular. This is the same shape as the pressure distribution that would be imposed if the wall was backfilled with a fluid instead of soil. Furthermore, if this fluid had the proper unit weight, the magnitude of the lateral pressure also would be equal to that from the soil. Engineers often use this similarity when expressing lateral earth pressures for design purposes. Instead of quoting K values, we can describe the lateral earth pressure using the *equivalent fluid density*, G_h , which is the unit weight of a fictitious fluid that would impose the same horizontal pressures on the wall as the soil. The vertical component of the lateral earth pressure also can be expressed using a similar value, G_v . Thus, the normal and shear forces acting on the wall may then be expressed as:

$$P_a/b = \frac{G_h H^2}{2} \quad (23.25)$$

DESIGNING WITH GEOSYNTHETICS

FIFTH EDITION



ROBERT M. KOERNER

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$$\begin{aligned}
 c &= (N_A \tan \delta + C_a) \sin \beta \sin \left(\frac{\omega + \beta}{2} \right) \tan \phi \\
 &= (370 \tan 22 + 0) \sin 18.4 \sin \left(\frac{16 + 18.4}{2} \right) \tan 30 \\
 &= 8.07 \text{ kN/m} \\
 \text{FS} &= \frac{-b + \sqrt{b^2 - 4ac}}{2a} \\
 &= \frac{62.8 + \sqrt{(-62.8)^2 - 4(37.2)(8.07)}}{2(37.2)} \\
 \text{FS} &= 1.55 \text{ (vs. 1.25 for the constant thickness cross section)}
 \end{aligned}$$

Example 5.12 has also been extended to a set of design curves, as seen in Figure 5.26b. The anticipated trends are again noted, as is the agreement with the worked out example. Clearly, this type of stabilizing solution can be used if space at the toe of the slope is available. Often it is not or it occupies valuable air space and then geosynthetic reinforcement as discussed in Chapter 3 is the alternative solution.

5.3.6 Runout and Anchor Trench Design

As shown in Figure 5.18 and the profile sections of geomembrane-lined reservoirs, the liner coming up from the bottom of the excavation, covers the side slopes, and then runs over the top a short distance. It often terminates vertically down into an anchor trench. This anchor trench is typically dug by a small backhoe or trenching machine; the liner is draped over the edge, and then the trench is backfilled with the same soil that was there originally. The backfilled soil should be compacted in layers as the backfilling proceeds. Although concrete has been used as an anchorage block, it is rarely justified, at least on the basis of calculations, as will be seen in this section.

Regarding design, two separate cases will be analyzed: one with geomembrane runout only and no anchor trench at all (as is often used with canal liners), and the other as described above, with both runout and anchor trench considerations (as with reservoirs and landfills). Figure 5.27 defines the first situation, together with the forces and stresses involved. Note that the cover soil applies normal stress due to its weight but does not contribute frictional resistance above the geomembrane. This is due to the fact that the soil moves along with the geomembrane as it deforms and undoubtedly cracks, thereby losing its integrity.

From Figure 5.27, the following horizontal force summation results, which leads to the appropriate design equation:

$$\begin{aligned}
 \Sigma F_x &= 0 \\
 T_{\text{allow}} \cos \beta &= F_{U\sigma} + F_{L\sigma} + F_{LT} \\
 &= \sigma_n \tan \delta_U (L_{RO}) + \sigma_n \tan \delta_L (L_{RO}) + 0.5 \left(\frac{2T_{\text{allow}} \sin \beta}{L_{RO}} \right) (L_{RO}) \tan \delta_L \\
 L_{RO} &= \frac{T_{\text{allow}} (\cos \beta - \sin \beta \tan \delta_L)}{\sigma_n (\tan \delta_U + \tan \delta_L)} \tag{5.25}
 \end{aligned}$$

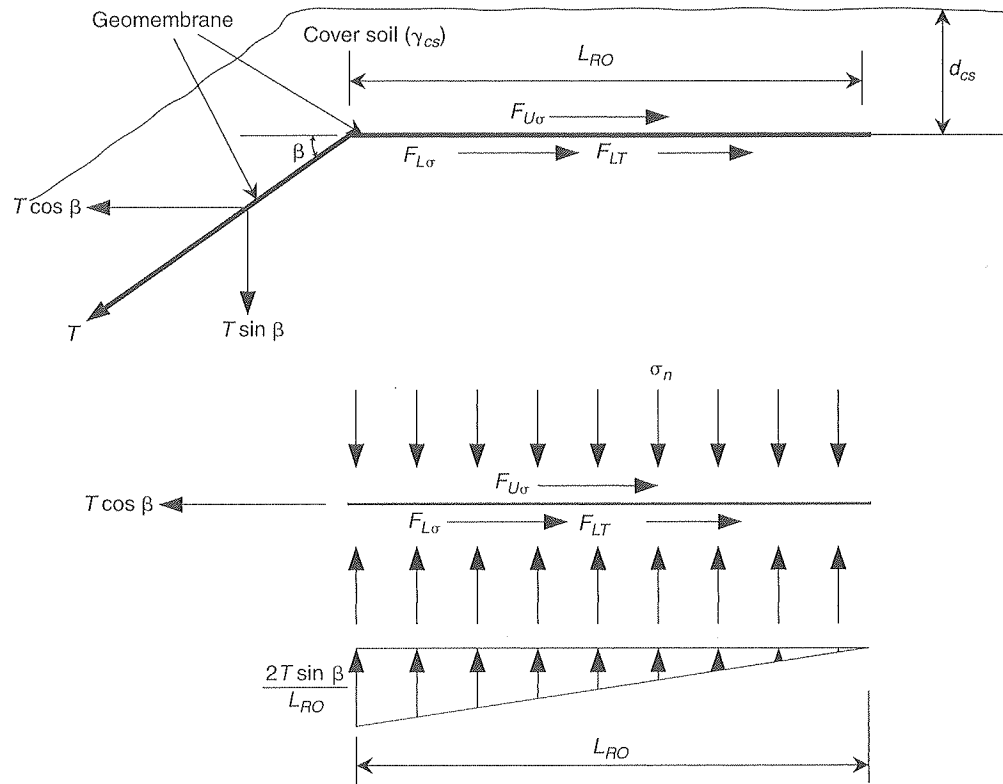


Figure 5.27 Cross section of geomembrane runout section and related stresses and forces involved.

where

T_{allow} = allowable force in geomembrane = $\sigma_{\text{allow}} t$, where

σ_{allow} = allowable stress in geomembrane, and

t = thickness of geomembrane;

β = side slope angle;

$F_{U\sigma}$ = shear force above geomembrane due to cover soil (note that for thin cover soils tensile cracking will occur and this value will be negligible);

$F_{L\sigma}$ = shear force below geomembrane due to cover soil;

F_{LT} = shear force below geomembrane due to vertical component of T_{allow} ;

σ_n = applied normal stress from cover soil;

δ = angle of shearing resistance between geomembrane and adjacent material (i.e., soil or geotextile); and

L_{RO} = length of geomembrane runout.

Example 5.13 illustrates the use of the concept and equations just developed.

(5.25)

Example 5.13

Consider a 1.0 mm thick LLDPE geomembrane with a mobilized allowable stress of 7000 kPa, which is on a 3(H) to 1(V) side slope. Determine the required runout length to resist this stress without use of a vertical anchor trench. In this analysis use 300 mm of cover soil weighing 16.5 kN/m³ and a friction angle of 30° with the geomembrane.

Solution: From the design equations just presented,

$$\begin{aligned} T_{\text{allow}} &= \sigma_{\text{allow}} t \\ &= (7000)(0.001) \\ T_{\text{allow}} &= 7.0 \text{ kN/m} \end{aligned}$$

and

$$\begin{aligned} L_{RO} &= \frac{T_{\text{allow}}(\cos \beta - \sin \beta \tan \delta_L)}{\sigma_n(\tan \delta_U + \tan \delta_L)} \\ &= \frac{(7.0)[\cos 18.4 - (\sin 18.4)(\tan 30)]}{(16.5)(0.30)[\tan 0 + \tan 30]} \\ &= \frac{5.37}{2.86} \\ L_{RO} &= 1.9 \text{ m} \end{aligned}$$

Note that this value is strongly dependent on the value of mobilized allowable stress used in the analysis. To mobilize the failure strength of the geomembrane would require a longer runout length or embedment in an anchor trench. This, however, might not be desirable. Pullout without geomembrane failure might be a preferable phenomenon. It is a site-specific situation left up to the designer.

The situation with an anchor trench at the end of the runout section is illustrated in Figure 5.28. The configuration requires some important assumptions regarding the state of stress within the anchor trench and its resistance mechanism. In order to provide lateral resistance, the vertical portion within the anchor trench has lateral forces acting upon it. More specifically, an active earth pressure (P_A) is tending to destabilize the situation, whereas a passive earth pressure (P_P) is tending to resist pullout. As will be shown, this passive earth pressure is very effective in providing a resisting force (see Holtz and Kovacs [48]). Using the free-body diagram of Figure 5.28,

$$\begin{aligned} \Sigma F_x &= 0 \\ T_{\text{allow}} \cos \beta &= F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P \end{aligned} \quad (5.26)$$

where

- T_{allow} = allowable force in geomembrane = $\sigma_{\text{allow}} t$, where
- σ_{allow} = allowable stress in geomembrane, and
- t = thickness of geomembrane;
- β = side slope angle;
- $F_{U\sigma}$ = shear force above geomembrane due to cover soil (note that for thin cover soils, tensile cracking will occur, and this value will be negligible);

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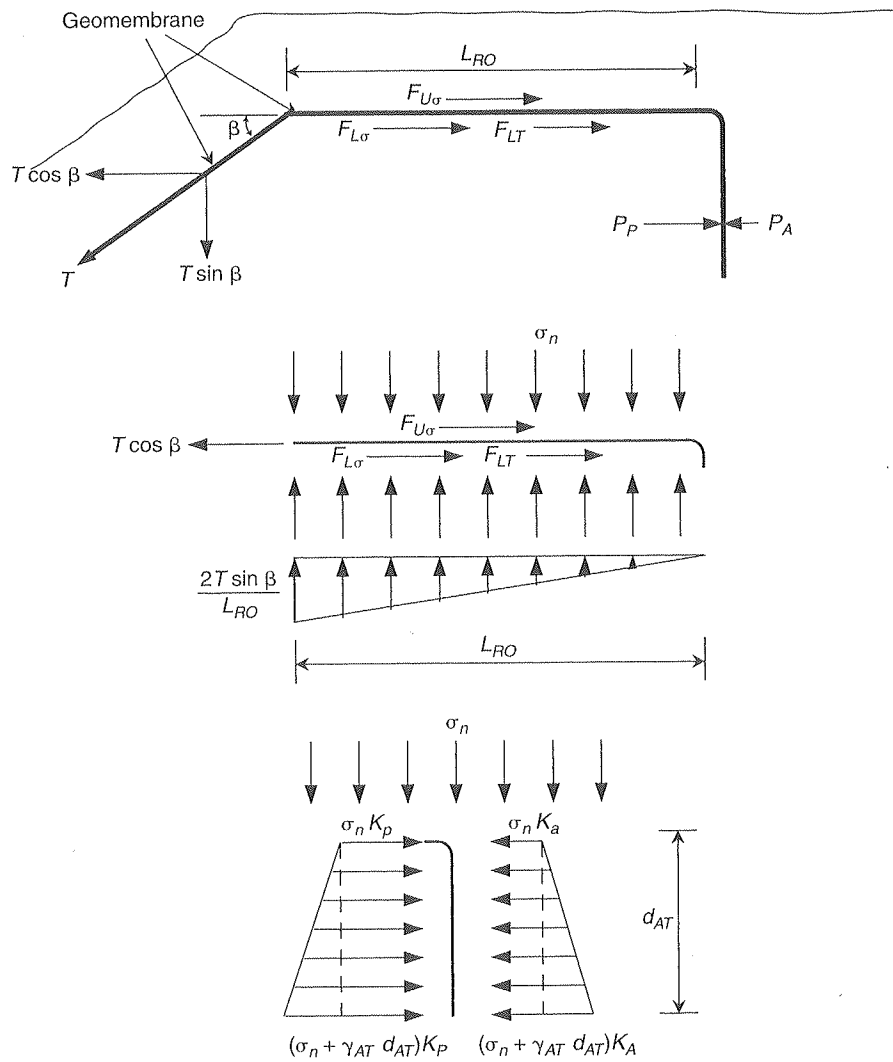


Figure 5.28 Cross section of geomembrane runout section with anchor trench and related stresses and forces involved.

- $F_{L\sigma}$ = shear force below geomembrane due to cover soil
- F_{LT} = shear force below geomembrane due to vertical component of T_{allow} ;
- P_A = active earth pressure against the backfill side of the anchor trench; and
- P_P = passive earth pressure against the in-situ side of the anchor trench.

The values of $F_{U\sigma}$, $F_{L\sigma}$, and F_{LT} have been defined previously. The values of P_A and P_P require the use of lateral earth pressure theory.

$$P_A = \frac{1}{2}(\gamma_{AT}d_{AT})K_A d_{AT} + (\sigma_n)K_A d_{AT}$$

$$P_A = (0.5\gamma_{AT}d_{AT} + \sigma_n)K_A d_{AT} \quad (5.27)$$

$$P_P = (0.5\gamma_{AT}d_{AT} + \sigma_n)K_P d_{AT} \quad (5.28)$$

where

γ_{AT} = unit weight of soil in anchor trench,

d_{AT} = depth of the anchor trench,

σ_n = applied normal stress from cover soil,

K_A = coefficient of active earth pressure = $\tan^2(45 - \phi/2)$,

K_P = coefficient of passive earth pressure = $\tan^2(45 + \phi/2)$, and

ϕ = angle of shearing resistance of respective soil.

This situation results in one equation with two unknowns; thus a choice of either L_{RO} or d_{AT} is necessary to calculate the other. As with the previous situation, the factor of safety is placed on the geomembrane force T , which is used as an allowable value, T_{allow} . Example 5.14 illustrates the procedure.

Example 5.14

Consider a 1.5 mm thick HDPE geomembrane extending out of a facility as shown in Figure 5.28. What depth anchor trench is needed if the runout distance is limited to 1.0 m? In the solution, use a geomembrane allowable stress of 16,000 kPa on a 3(H) to 1(V) side slope. Cover soil at 16.5 kN/m³ and 300 mm thick is placed over the geomembrane runout and anchor trench (this is also the unit weight of the anchor trench soil). The friction angle of the geomembrane to the soil is 30° (although assume 0° for the top of the geomembrane under a soil-cracking assumption) and the soil itself is 35°. Also, develop a design chart for this example assuming that the runout length is not limited to 1.0 m.

Solution: Using the previously developed design equations based on Figure 5.28,

$$\begin{aligned} T_{\text{allow}} &= \sigma_{\text{allow}} t \\ &= 16000(0.0015) \\ &= 24.0 \text{ kN/m} \end{aligned}$$

and

$$\begin{aligned} F_{U\sigma} &= \sigma_n \tan \delta_U(L_{RO}) \\ &= (0.3)(16.5) \tan 0(L_{RO}) \\ &= 0 \end{aligned}$$

$$\begin{aligned} F_{L\sigma} &= \sigma_n \tan \delta_L(L_{RO}) \\ &= (0.3)(16.5) \tan 30(L_{RO}) \\ &= 2.86L_{RO} \end{aligned}$$

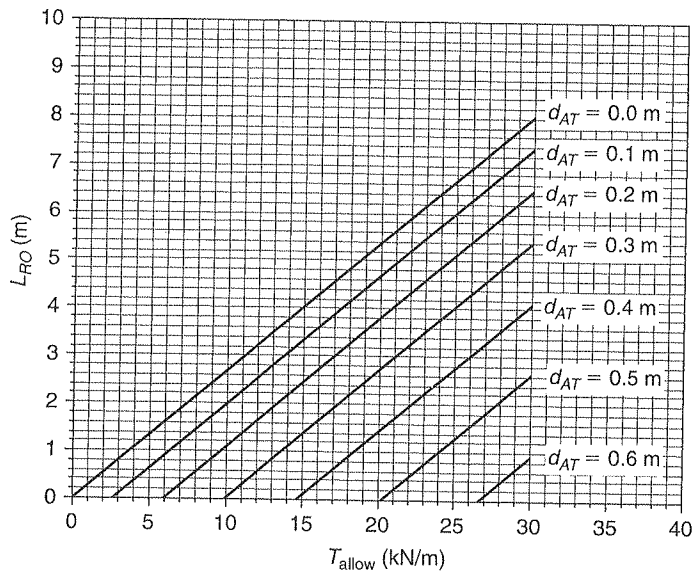
$$\begin{aligned} F_{LT} &= T_{\text{allow}} \sin \beta \tan \delta_L \\ &= (24.0) \sin 18.4 \tan 30 \\ &= 4.37 \text{ kN/m} \end{aligned}$$

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$$\begin{aligned}
 P_A &= (0.5\gamma_{AT}d_{AT} + \sigma_n)K_A d_{AT} \\
 &= [(0.5)(16.5)d_{AT} + (0.3)(16.5)] \tan^2(45 - 35/2)d_{AT} \\
 &= [8.25d_{AT} + 4.95](0.271)d_{AT} \\
 &= 2.24d_{AT}^2 + 1.34d_{AT}
 \end{aligned}$$

$$\begin{aligned}
 P_P &= (0.5\gamma_{AT}d_{AT} + \sigma_n)K_P d_{AT} \\
 &= [(0.5)(16.5)d_{AT} + (0.3)(16.5)] \tan^2(45 + 35/2)d_{AT} \\
 &= [8.25d_{AT} + 4.95](3.69)d_{AT} \\
 &= 30.4d_{AT}^2 + 18.3d_{AT}
 \end{aligned}$$

This is substituted into the general force equation (5.26) to arrive at the solution in terms of the two variables L_{RO} and d_{AT} :

$$\begin{aligned}
 T_{allow} \cos \beta &= F_{U\sigma} + F_{L\sigma} + F_{LT} - P_A + P_P \\
 (24.0) \cos 18.4 &= 0 + 2.86 L_{RO} + 4.37 - 2.24d_{AT}^2 \\
 &\quad - 1.34d_{AT} + 30.4d_{AT}^2 + 18.3d_{AT} \\
 18.4 &= 2.86L_{RO} + 17.0d_{AT} + 28.2d_{AT}^2
 \end{aligned}$$

Since $L_{RO} = 1.0$ m, the equation can be solved for the unknown d_{AT} .

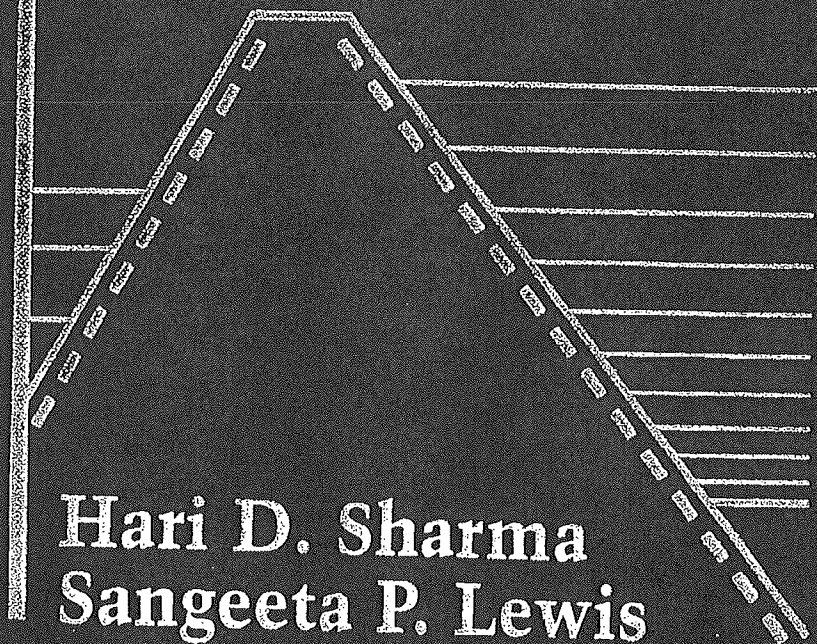
$$d_{AT} = 0.50 \text{ m}$$

Using this formulation, we can develop a *design chart* for a wide range of geomembranes and thicknesses as characterized by different values of T_{allow} . For the specific conditions of Example 5.14, we obtain

$$\begin{aligned}
 \beta &= 18.4^\circ, \text{ which is } 3(H) \text{ to } 1(V) \\
 \sigma_n &= d_{cs}\gamma_{cs} \\
 &= (0.30)(16.5) \\
 &= 4.95 \text{ kN/m}^2
 \end{aligned}$$

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Hari D. Sharma
Sangeeta P. Lewis

since creases in the geomembrane caused by sharp corners may lead to environmental stress cracking.

8.3.3.6 Placement of Soils over Geomembranes. As discussed in Section 8.3.3.2, soil should be "floated" over geomembranes such that a minimum 12 inches of this material exists between the construction equipment and the geomembrane at all times. This minimizes the possibility of geomembrane puncture and impact damage since the effective stress exerted by the construction equipment is reduced and the soil is not dumped on top of the geomembrane.

Soil placement over polyethylene geomembranes should occur in the early morning when there is adequate lighting and the geomembrane is contracted. By midday, wrinkles often develop in polyethylene geomembranes, making soil placement difficult. On days where the temperature exceeds 100°F, the wrinkles can be as large as 1 to 2 feet high. Even in the morning, 6-inch-high wrinkles can easily develop. If it cannot be avoided, soils may be placed over geomembrane wrinkles by placing the soil directly on top of the wrinkle such that it forms two smaller wrinkles. By continuously placing soil directly above the wrinkle, the wrinkle will eventually work itself out. Therefore, if possible, the geomembrane should not be permanently anchored until the soil overlying the geomembrane has been placed. In no situation should the geomembrane wrinkle be allowed to fold over under the weight of the overlying soil. These folds will crease the geomembrane and provide a preferential location for stress cracking and eventual leakage.

Placement of soils over geomembranes on slopes should occur from the bottom of slope upward. This will minimize the stresses on the geomembrane from construction equipment. Soils should be placed over geomembranes as soon as possible following geomembrane installation. This prevents UV degradation of the geomembrane and damage from ongoing construction activities, and also provides for good contact between the geomembrane and underlying material.

8.3.4 Structural Details

8.3.4.1 Anchorage. Anchor trenches are used at the top of side-slope liners to hold installed geosynthetics in place against applied loads and to prevent potential tears caused by wind intrusion beneath the geosynthetics. As shown in Figure 8.19, anchor trenches can generally be classified as flat, rectangular, or V-shaped. Selection of the appropriate anchor trench configuration for any particular site depends on the required holding capacity, access considerations, dimensional constraints, and available construction equipment. Often, a contractor may request that the anchor trench configuration be modified based on the equipment available. All such modifications should be checked and approved by the designer.

The holding capacity of anchor trenches is developed by the applied normal load of the soil placed above the geosynthetics, which creates frictional resistance between the geosynthetics and the underlying soil; there is minimal friction resistance developed between the upper soil and the geosynthetic since the soil above the

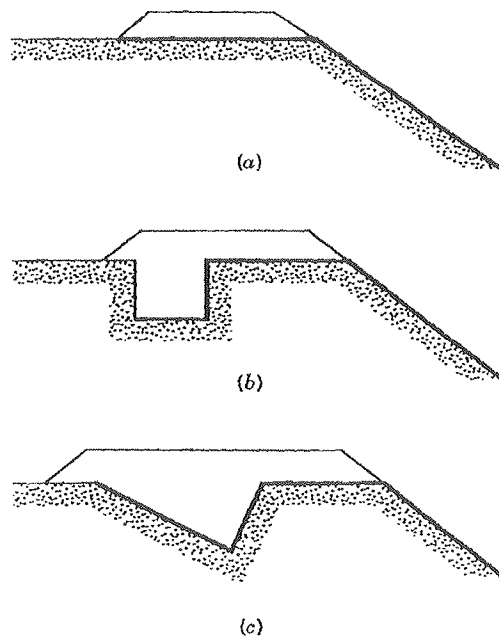


Figure 8.19 Typical anchor trench configurations: (a) flat anchor; and (b) rectangular anchor; and (c) V-shaped anchor.

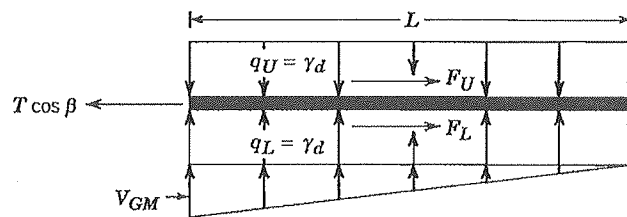
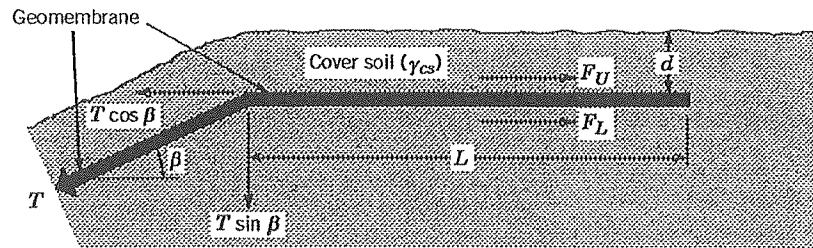
geosynthetic is likely to move with the geosynthetic. The soil depth, type of soil or other material underlying the geosynthetics, and geosynthetic anchorage length are therefore the key factors in developing the required anchor trench holding capacity.

The easiest anchor trench configuration to analyze is the flat anchor. The free-body diagram for the flat anchor and the development of equation (8.14) for anchorage length is shown in Figure 8.20.

$$L = \frac{T \cos \beta - T \sin \beta \tan \delta_L}{\gamma d \tan \delta_L} \quad (8.14)$$

There is no ideal solution for rectangular or V trenches. Koerner (1990) recommends that the problem be solved using imaginary, frictionless pulleys, as shown in Figure 8.21.

The anchor trench should be designed to resist pullout loads (T) caused by the self-weight of the geosynthetics. For geomembranes that may be exposed to severe temperature and wind loading conditions, stresses caused by these forces should also be evaluated. Ideally, the anchor trench should be designed to allow the geosynthetics to pull out slightly rather than cause tearing of the geosynthetics. The reasoning for this is that even if complete pullout occurred, it would usually be easier to replace pulled-out materials than to repair torn geosynthetics. The maxi-



$$F_U = q_U \tan \delta_U(L) \text{ (neglected since cover soil moves with geomembrane)}$$

$$F_L = q_L + 0.5 v_{GM} \tan \delta_L(L)$$

$$= \left[q_U + 0.5 \left(\frac{2 T \sin \beta}{L} \right) \right] \tan \delta_L(L)$$

$$T \cos \beta = q_L \tan \delta_L(L) + T \sin \beta \tan \delta_L$$

$$L = \frac{T \cos \beta - T \sin \beta \tan \delta_L}{\gamma_d \tan \delta_L}$$

Where: V_{GM} = vertical force due to geomembrane

F_U = friction force above geomembrane

F_L = friction force below geomembrane

q_U = stress above geomembrane due to cover soil weight

q_L = stress below geomembrane due to cover soil weight

T = tensile force in geomembrane

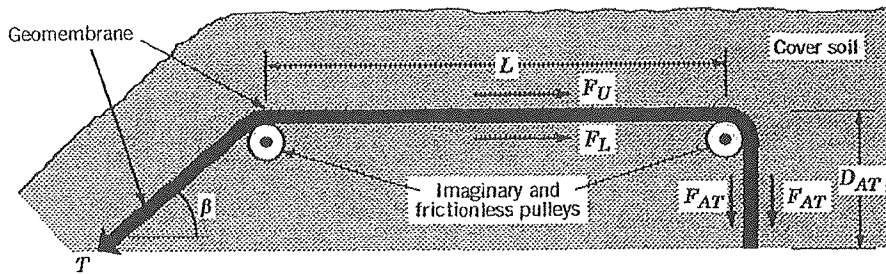
β = slope angle

d =

= unit weight of cover soil

δ = interface friction angle

Figure 8.20 Design of a flat anchor. (From Koerner, 1990.)



$$T = F_U + F_L + 2F_{AT}$$

Where: T = tensile stress in geomembrane

F_U = friction force above geomembrane
(assumed to be negligible since cover soil likely moves with geomembrane)

$$F_L = q \tan \delta(L)$$

q = surcharge pressure = γd

d = depth of cover soil

γ = unit weight of cover soil

δ = interface friction angle

L = runout length

$$F_{AT} = (\sigma_h \text{ ave}) \tan \delta (d_{AT})$$

σ_h = average horizontal stress in anchor trench

$$= k_o \sigma_v$$

$$\sigma_v = \gamma \text{ Have}$$

Have = average depth of anchor trench (requires an estimate)

$$k_o = 1 - \sin \phi$$

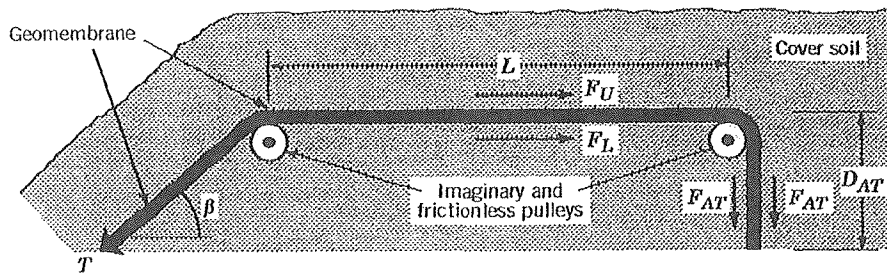
ϕ = angle of shearing resistance of backfill soil

d_{AT} = depth of anchor trench (unknown)

Figure 8.21 Design of a rectangular and V anchor trenches. (From Koerner, 1990.)

imum holding capacity of the anchor trench should therefore be slightly less than the ultimate tensile strength of the geosynthetic to be anchored, irrespective of the applied loads. If the applied loads are greater than the tensile strength of the geosynthetics, measures should be taken to reduce the applied loads or higher-strength geosynthetics should be used.

If soil materials are placed above side-slope geosynthetics, the load caused by soil, seepage forces, and construction equipment should be assessed. Often, a high-strength reinforcing geotextile or geogrid is required to hold the soil on the slopes. Druschel and Underwood (1993) used a force equilibrium method to assess the required anchorage force for these high-strength materials. The free-body and force vector diagram for this method are illustrated in Figures 8.22 and 8.23, respectively. As shown, the items⁴ to be evaluated include the toe buttress resistance, soil



$$T = F_U + F_L + 2F_{AT}$$

Where: T = tensile stress in geomembrane

F_U = friction force above geomembrane
(assumed to be negligible since cover soil likely moves with geomembrane)

$$F_L = q \tan \delta(L)$$

q = surcharge pressure = γd

d = depth of cover soil

γ = unit weight of cover soil

δ = interface friction angle

L = runout length

$$F_{AT} = (\sigma_h \text{ ave}) \tan \delta (d_{AT})$$

σ_h = average horizontal stress in anchor trench

$$= k_o \sigma_v$$

$$\sigma_v = \gamma \text{ Have}$$

Have = average depth of anchor trench (requires an estimate)

$$k_o = 1 - \sin \phi$$

ϕ = angle of shearing resistance of backfill soil

d_{AT} = depth of anchor trench (unknown)

Figure 8.21 Design of a rectangular and V anchor trenches. (From Koerner, 1990.)

imum holding capacity of the anchor trench should therefore be slightly less than the ultimate tensile strength of the geosynthetic to be anchored, irrespective of the applied loads. If the applied loads are greater than the tensile strength of the geosynthetics, measures should be taken to reduce the applied loads or higher-strength geosynthetics should be used.

If soil materials are placed above side-slope geosynthetics, the load caused by soil, seepage forces, and construction equipment should be assessed. Often, a high-strength reinforcing geotextile or geogrid is required to hold the soil on the slopes. Druschel and Underwood (1993) used a force equilibrium method to assess the required anchorage force for these high-strength materials. The free-body and force vector diagram for this method are illustrated in Figures 8.22 and 8.23, respectively. As shown, the items⁴ to be evaluated include the toe buttress resistance, soil

TEXTURED HDPE GEOMEMBRANE

ENGLISH UNITS



Minimum Average Values

Property	Test Method	40 mil	60 mil	80 mil	100 mil
Thickness, mils	ASTM D 5994				
minimum average		38	57	76	95
lowest individual of 8 of 10 readings		36	54	72	90
lowest individual of 10 readings		34	51	68	85
Asperity Height ¹ , mils	ASTM D 7466	10	10	10	10
Sheet Density, g/cc	ASTM D 1505/D 792	0.940	0.940	0.940	0.940
Tensile Properties²	ASTM D 6693				
1. Yield Strength, lb/in		84	126	168	210
2. Break Strength, lb/in		60	90	120	150
3. Yield Elongation, %		12	12	12	12
4. Break Elongation, %		100	100	100	100
Tear Resistance, lb	ASTM D 1004	28	42	56	70
Puncture Resistance, lb	ASTM D 4833	60	90	120	150
Stress Crack Resistance ³ , hrs	ASTM D 5397 (App.)	300	300	300	300
Carbon Black Content ⁴ , %	ASTM D 1603	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596	--Note 5--			
Oxidative Induction Time (OIT)					
Standard OIT, minutes	ASTM D 3895	100	100	100	100
Oven Aging at 85°C	ASTM D 5721				
High Pressure OIT - % retained after 90 days	ASTM D 5885	80	80	80	80
UV Resistance ⁶	ASTM D 7238				
High Pressure OIT ⁷ - % retained after 1600 hrs	ASTM D 5885	50	50	50	50
Roll Dimensions					
1. Width (feet):		23	23	23	23
2. Length (feet)		750	500	375	300
3. Area (square feet):		17,250	11,500	8,625	6,900
4. Gross weight (pounds, approx.)		3,500	3,500	3,470	3,470

- 1 Of 10 readings; 8 must be ≥ 7 mils and lowest individual reading must be ≥ 5 mils.
- 2 Machine direction (MD) and cross machine direction (XMD) average values should be on the basis of 5 test specimens each direction. Yield elongation is calculated using a gauge length of 1.3 inches; Break elongation is calculated using a gauge length of 2.0 inches.
- 3 The yield stress used to calculate the applied load for the SP-NCTL test should be the mean value via MQC testing.
- 4 Other methods such as ASTM D 4218 or microwave methods are acceptable if an appropriate correlation can be established.
- 5 Carbon black dispersion for 10 different views: Nine in Categories 1 and 2 with one allowed in Category 3.
- 6 The condition of the test should be 20 hr. UV cycle at 75°C followed by 4 hr. condensation at 60°C.
- 7 UV resistance is based on percent retained value regardless of the original HP-OIT value.

These data are provided for informational purposes only and are not intended as a warranty or guarantee. Poly-America, L.P. assumes no responsibility in connection with the use of these data. Suitability for a particular use shall be determined by and is the sole responsibility of the end user. These values are subject to change without notice. REV. 03/14

J.4-B Wheel Loading on Geomembrane



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj. #: 631020105

Calculated By: ORC

Date: 05/2022

Checked By: DAM

Date: 05/2022

TITLE: WHEEL LOADING ON GEOMEMBRANE**Problem Statement**

Calculate the wheel loading of construction equipment acting on the geomembrane.

Given

- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this application.
- Caterpillar Product Information, 836K, Landfill Compactor (refer to attached pages).
- Product information for 60-mil HDPE geomembrane (refer to attached page).

Assumptions

- Total weight of compactor is 123,319 lb (reference Caterpillar).
- Wheel load assumed to act on 1.0 foot of leachate drainage layer material plus 5 feet of initial waste placement, for a total height of 6.0 feet above the geomembrane liner.
- Unit weight of leachate drainage layer material assumed to be 130 pcf.
- Density of waste assumed to be 75 pcf.
- Allowable stress for the 60 mil HDPE geomembrane assumed to be 2,200 psi (reference PolyFlex specifications).

Calculations

Calculate the wheel loading of the construction equipment, conservatively assuming that the vehicle load does not attenuate with depth:

$$\text{Contact Pressure (P)} = \frac{\text{Weight of the Vehicle}}{(\text{No. of Drums})(\text{Contact Area of Drums})}$$

$$P = \frac{123,319 \text{ lb}}{(4 \text{ Drums} \times \text{Contact Area of Drums})} = \frac{123,319 \text{ lb}}{(4 \text{ Drums} \times ((4.67 \text{ ft}) \times (1/3) \times (5.83 \text{ ft})))}$$

$$P = 3,397 \text{ psf} = 23.6 \text{ psi}$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj. #: 631020105

Calculated By: ORC

Date: 05/2022

Checked By: DAM

Date: 05/2022

TITLE: WHEEL LOADING ON GEOMEMBRANE

Assume dynamic load is 1.5 times static load:

$$1.5 (23.6 \text{ psi}) = 35.4 \text{ psi}$$

Calculate load due to overlying materials:

$$\text{Waste: } (5 \text{ ft})(75 \text{ pcf}) = 375 \text{ psf} = 2.6 \text{ psi}$$

$$\text{Drainage Layer: } (1.0 \text{ ft})(130 \text{ pcf}) = 130 \text{ psf} = 0.9 \text{ psi}$$

Total all loads on geomembrane liner:

$$\sigma_{\text{total}} = 35.4 \text{ psi} + 2.6 \text{ psi} + 0.9 \text{ psi} = 38.9 \text{ psi}$$

Calculate Factor of Safety:

$$FS = \frac{\sigma_{\text{allow}}}{\sigma_{\text{total}}} = \frac{2,200 \text{ psi}}{38.9 \text{ psi}} = 56.6$$

Results

The calculated Factor of Safety of 56.6 indicates that the geomembrane can withstand the wheel loading of the construction equipment.

Published Technical References

836K

Landfill Compactor



Engine

Engine Model	Cat® C18 ACERT™	
Emissions	Tier 4 Final/Stage IV	
Gross (SAE J1349)	419 kW	562 hp

Operating Specifications

Maximum Operating Weight –	55 927 kg	123,319 lb
Multiple Blade and Wheel Offerings		

836K Landfill Compactor Specifications

Engine

Engine Model	C18 ACERT	
Emissions	U.S. EPA Tier 4 Final and EU Stage IV	
Rated Power (Lab)	414 kW	555 hp
Rated Power (Net ISO 14396)	412 kW	553 hp
Gross (SAE J1349)	419 kW	562 hp
Net Power – SAE J1349		
Direct Drive – Gross Power	370 kW	496 hp
Direct Drive – Torque Rise	52%	
Converter Drive – Gross Power	370 kW	496 hp
Converter Drive – Torque Rise	52%	
Maximum Gross Torque @ 1,300 rpm	3085 N·m	2,275 lbf-ft
Maximum Altitude without Derating	2286 m	7,500 ft
Bore	145 mm	5.71 in
Stroke	183 mm	7.2 in
Displacement	18.1 L	1,104.5 in ³
High Idle Speed	2,120 rpm	
Low Idle Speed	750 rpm	

Operating Specifications

Operating Weight with Full Tank Capacities and U-blade	55 927 kg	123,319 lb
--	-----------	------------

Transmission

Transmission Type	Planetary – Powershift – ECPC	
Travel Speeds		
Forward – Converter 1st	6.2 km/h	3.9 mph
Forward – Lockup 1st	6.5 km/h	4 mph
Forward – Converter 2nd	10.9 km/h	6.8 mph
Forward – Lockup 2nd	11.7 km/h	7.3 mph
Reverse – Converter 1st	6.5 km/h	4 mph
Reverse – Lockup 1st	6.9 km/h	4.3 mph
Reverse – Converter 2nd	10.4 km/h	6.5 mph
Reverse – Lockup 2nd	12.3 km/h	7.6 mph

Hydraulic System

Hydraulic System	Flow Sharing Implement	
Maximum Supply Pressure	32 000 kPa	4,640 psi
Main Relief Pressure	24 100 kPa	3,495 psi
Pump Flow at 2,006 rpm	250 L/min	66 gal/min
Steering System	Double Acting – End Mounted	
Bore	127 mm	5 in
Stroke	740 mm	29.1 in
Vehicle Articulation Angle	86°	
Lift System	Double Acting Cylinder	
Bore	137.9 mm	5.5 in
Stroke	1021 mm	40.2 in

Service Refill Capacities

Fuel Tank	793 L	209 gal
Cooling System	107 L	28 gal
Crankcase	60 L	16 gal
Diesel Engine Fluid Tank	32.8 L	9 gal
Transmission	120 L	32 gal
Differentials and Final Drives – Front	186 L	49 gal
Differentials and Final Drives – Rear	190 L	50 gal
Hydraulic System (tank only)	240 L	63 gal

- All non-road Tier 4 Final/Stage IV, and Japan (MLIT) Step 4 diesel engines are required to use:
 - Ultra Low Sulfur Diesel (ULSD) fuels containing 15 ppm (mg/kg) sulfur or less. Biodiesel blends up to B20 are acceptable when blended with 15 ppm (mg/kg) sulfur or less ULSD and when the biodiesel feedstock meets ASTM D7467 specifications.
 - Cat DEO-ULS™ or oils that meet the Cat ECF-3, API CJ-4, and ACEA E9 specifications are required.

Axles

Front	Planetary – Fixed
Rear	Planetary – Oscillating
Oscillation Angle	13°

Brakes

Control System	Full Hydraulic Split Circuit
Parking Brake	Spring Applied, Hydraulic Released

Cab

	Standard	Suppression
Interior Sound Level	72 dB(A)	71 dB(A)
Exterior Sound Level	111 dB(A)	109 dB(A)

Hydraulic System – Steering

Steering System – Circuit	Steering Double Acting – End Mounted	
Steering System – Pump	Piston – Variable Displacement	
Maximum Flow @ × rpm	52 L/min @ 2,006 rpm	
Steering Pressure Limited	24 100 kPa	3,495 psi
Total Steering Angle	86 degrees	

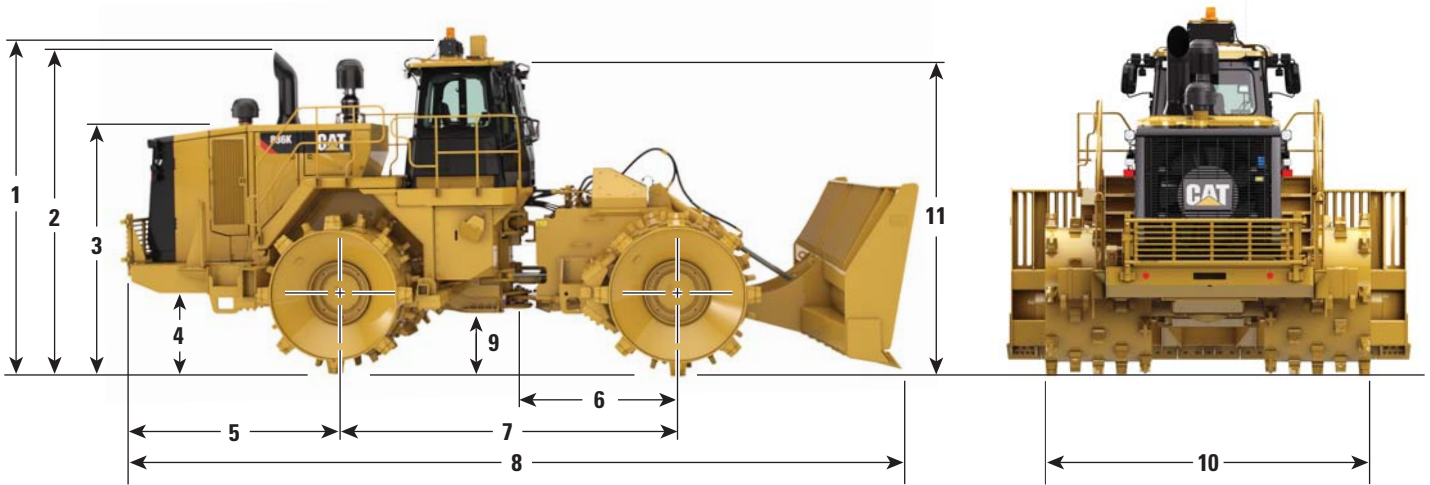
Wheels and Tips

Drum Width	1400 mm	4 ft 8 in
Drum Diameter	1770 mm	5 ft 10 in
Diameter with Tips	2125 mm	7 ft 0 in
Tips per Wheel	40	

836K Landfill Compactor Specifications

Dimensions

All dimensions are approximate.



1 Height to Top of Cab with A/C	4655 mm	15 ft 3 in
2 Height to Top of Exhaust Pipe	4608 mm	15 ft 1 in
3 Height to Top of Hood	3421 mm	11 ft 3 in
4 Ground Clearance to Bumper	1029 mm	3 ft 5 in
5 Center Line of Rear Axle to Edge of Counterweight	3187 mm	10 ft 5 in
6 Hitch to Center Line of Front Axle	2275 mm	7 ft 6 in
7 Wheelbase	4550 mm	14 ft 11 in
8 Length with Blade on Ground (straight blade)	10 182 mm	33 ft 5 in
9 Ground Clearance	632 mm	2 ft 1 in
10 Width over Wheels	4280 mm	14 ft 1 in
11 Height to ROPS/Canopy	4284 mm	14 ft 1 in
Height to Top of Cab with Strobe	4845 mm	15 ft 11 in
Turning Radius – Inside of Wheels	3635 mm	11 ft 11 in

Blade Selection

	Straight Blade		Semi U-blade		U-blade	
Width – Moldboard Length	4990 mm	16 ft 4 in	5238 mm	17 ft 2 in	5172 mm	17 ft
Width Over End Bits	5193 mm	17 ft	5311 mm	17 ft 5 in	5258 mm	17 ft 3 in
Height with Cutting Edge and Screen	2236 mm	7 ft 4 in	2215 mm	7 ft 3 in	2210 mm	7 ft 3 in
Height with Cutting Edge, No Screen	1217 mm	4 ft	1253 mm	4 ft 1 in	1255 mm	4 ft 1 in
Maximum Depth of Cut	364 mm	1 ft 2 in	362 mm	1 ft 2 in	934 mm	3 ft 1 in
Maximum Lift above Ground	1730 mm	5 ft 8 in	1735 mm	5 ft 8 in	1198 mm	3 ft 11 in
Cutting Edges, Reversible						
Length, Each End Section (3 edges)	1408.2 mm	4 ft 7 in	816.6 mm	2 ft 8 in	2 @ 779.1 mm and 1 @ 856 mm	2 @ 2 ft 7 in and 1 @ 2 ft 10 in
Length, Each End Section (2 edges)	NA		988 mm	3 ft 3 in	1094.4 mm	3 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
End Bits (2), Self-sharpening						
Length, Each	472 mm	1 ft 7 in	472 mm	1 ft 7 in	472 mm	1 ft 7 in
Width × Thickness	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in	254 mm × 25 mm	10 in × 1 in
Capacity, Rated	19.3 m ³	25.9 yd ³	22.4 m ³	29.3 yd ³	9.74 m ³	13 yd ³
Turning Diameter, Outside Corner of Blade at 43° ART	8737 mm	28 ft 8 in	8823 mm	28 ft 11 in	8795 mm	28 ft 10 in
Overall Machine Length	10 182 mm	33 ft 5 in	10 379 mm	34 ft 1 in	10 272 mm	33 ft 8 in

TEXTURED HDPE GEOMEMBRANE

ENGLISH UNITS



Minimum Average Values

Property	Test Method	40 mil	60 mil	80 mil	100 mil
Thickness, mils	ASTM D 5994				
minimum average		38	57	76	95
lowest individual of 8 of 10 readings		36	54	72	90
lowest individual of 10 readings		34	51	68	85
Asperity Height ¹ , mils	ASTM D 7466	10	10	10	10
Sheet Density, g/cc	ASTM D 1505/D 792	0.940	0.940	0.940	0.940
Tensile Properties²	ASTM D 6693				
1. Yield Strength, lb/in		84	126	168	210
2. Break Strength, lb/in		60	90	120	150
3. Yield Elongation, %		12	12	12	12
4. Break Elongation, %		100	100	100	100
Tear Resistance, lb	ASTM D 1004	28	42	56	70
Puncture Resistance, lb	ASTM D 4833	60	90	120	150
Stress Crack Resistance ³ , hrs	ASTM D 5397 (App.)	300	300	300	300
Carbon Black Content ⁴ , %	ASTM D 1603	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596	--Note 5--			
Oxidative Induction Time (OIT)					
Standard OIT, minutes	ASTM D 3895	100	100	100	100
Oven Aging at 85°C	ASTM D 5721				
High Pressure OIT - % retained after 90 days	ASTM D 5885	80	80	80	80
UV Resistance ⁶	ASTM D 7238				
High Pressure OIT ⁷ - % retained after 1600 hrs	ASTM D 5885	50	50	50	50
Roll Dimensions					
1. Width (feet):		23	23	23	23
2. Length (feet)		750	500	375	300
3. Area (square feet):		17,250	11,500	8,625	6,900
4. Gross weight (pounds, approx.)		3,500	3,500	3,470	3,470

- 1 Of 10 readings; 8 must be \geq 7 mils and lowest individual reading must be \geq 5 mils.
- 2 Machine direction (MD) and cross machine direction (XMD) average values should be on the basis of 5 test specimens each direction. Yield elongation is calculated using a gauge length of 1.3 inches; Break elongation is calculated using a gauge length of 2.0 inches.
- 3 The yield stress used to calculate the applied load for the SP-NCTL test should be the mean value via MQC testing.
- 4 Other methods such as ASTM D 4218 or microwave methods are acceptable if an appropriate correlation can be established.
- 5 Carbon black dispersion for 10 different views: Nine in Categories 1 and 2 with one allowed in Category 3.
- 6 The condition of the test should be 20 hr. UV cycle at 75°C followed by 4 hr. condensation at 60°C.
- 7 UV resistance is based on percent retained value regardless of the original HP-OIT value.

These data are provided for informational purposes only and are not intended as a warranty or guarantee. Poly-America, L.P. assumes no responsibility in connection with the use of these data. Suitability for a particular use shall be determined by and is the sole responsibility of the end user. These values are subject to change without notice. REV. 03/14

INHERENT PROPERTIES OF POLYETHYLENE LINERS

The properties listed in the table below are primarily inherent to the resin type used to produce the liner or are directly proportional to the thickness of the liner and less dependent on the manufacturing method. Therefore, these properties will not change from roll to roll or even lot to lot. Hence, they should not be included as part of routine quality control testing. The exception to this is Oxidative Induction Time. This test is a measurement of the amount of anti-oxidant added to the resin to produce the finished sheet. This test can function both as a performance test and a quality control test. As a quality control test it is desirable to run the test at high temperatures to keep the test duration short. This test is routinely run at the time of manufacture. As a performance test it is desirable to run the test at lower temperatures. Testing at lower temperatures cannot be done for quality control purposes.

The information given below is based on nominal values. Individual test results may vary from these values depending upon the reproducibility of the test.

NOMINAL PROPERTIES

TEST DESCRIPTION	TEST METHOD	UNITS	HDPE	LLDPE
Modulus of Elasticity	ASTM D 6693	lb/in ²	110,000	45,000
Secant Modulus	ASTM D 5323	lb/in ²	60,000	45,000
Volatile Loss	ASTM D 1203	%	0.1	0.1
Dimensional Stability	ASTM D 1204	%	+/- 0.5	+/- 1.0
Water Absorption (24 hr @ 23 °C)	ASTM D 570	% change	0.1	0.1
Coefficient of Linear Thermal Expansion	ASTM D 696	(cm/cm • °C)	1.2 x 10 ⁻⁴	1.4 x 10 ⁻⁴
Moisture Vapor Transmission Rate (100 °F and 100% relative humidity)	ASTM E 96	g/m ² ·day		
		100 mil	0.17	—
		80 mil	0.20	0.25
		60 mil	0.26	0.33
		40 mil	0.39	0.45
		30 mil	0.50	0.57
Low Temperature Brittleness	ASTM D 746	°F	<-112	<-112
Oxidative Induction Time	ASTM D 3895	minutes @ 200 °C	100	100
Multi-Axial Tension	ASTM D 5617	stress, lb/in ²	2200	1200
		strain, %	18	40+

J.4-C Puncture Resistance of Geosynthetics



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj.#: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: PUNCTURE RESISTANCE OF GEOSYNTHETICS

Problem Statement

Confirm that the geosynthetics used in the leachate collection system (LCS) are appropriately sized to resist puncture from the LCS aggregate materials that are in direct contact with them. These geosynthetics include both the geotextiles and geomembrane proposed to be used both across the floor, sideslopes and within the leachate collection trench areas.

Given

- A laboratory evaluation was conducted for Orchard Hills Landfill in 2019 when both Orchard Hills Landfill and Zion Landfill were under the common ownership of Advanced Disposal Services to replicate the puncture resistance of the in-place LCS geosynthetics at the Orchard Hills Landfill. LCS configuration evaluated in the laboratory evaluation is the same LCS configuration that was constructed in Cells 7 and 9 of Zion Landfill. This laboratory report is used to demonstrate the existing LCS in Cells 7 and 9 of Zion Landfill will not puncture. This approach is used because the existing LCS configuration in these cells is not described in the charts referenced in this calculation methodology (see attached). The results of the laboratory testing found that the existing leachate collection system did not puncture under the loading conditions that will result from the proposed vertical expansion. The report is attached to this calculation.
- Cell 6 underlies a small portion of the vertical expansion. The Cell 6 LCS was not analyzed for puncture as the addition of the vertical expansion will not increase the maximum waste thickness within Cell 6 and therefore the potential for puncture is not increased.
- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this application.
- Koerner, R.M., *Designing with Geosynthetics*. Prentice Hall, Fifth Edition (refer to attached pages).
- Koerner, R.M., *GRI White Paper #14, Modification to the “GRI-Method” for the RFCF-Factor Used in the Design of Geotextiles for Puncture Protection of Geomembranes*, November 24, 2008 (see attached pages).
- Narejo, D., Corcoran, G., *GSE® Geomembrane Protection Design Manual*, First Addition (see attached page).
- Product information for geosynthetics (refer to attached pages).



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj.#: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: PUNCTURE RESISTANCE OF GEOSYNTHETICS

Assumptions

- The relevant landfill system layers will include the following in the Horizontal Expansion area (listed from top to bottom):
 - Leachate Collection System / Liner System on Floor and Sideslopes:
 - 8-oz/yd² non-woven geotextile filter
 - 1-foot granular drainage layer (d < 1.0 inch)
 - 12-oz/yd² non-woven geotextile cushion (or equivalent)
 - 60-mil HDPE textured geomembrane
 - 5-foot low permeable earth liner
 - Main - Leachate Collection Pipe Trench / Liner System:
 - 8-oz/yd² non-woven geotextile filter
 - 24-inches coarse aggregate pipe bedding (D ≤ 1.5 inches)
 - 12-oz/yd² non-woven geotextile cushion (or equivalent)
 - 60-mil HDPE textured geomembrane
 - 5-foot low permeable earth liner
- A saturated density of 130.3 pcf (21 kN/m³) for the final cover soils and a saturated density of 130.0 (21 kN/m³) for the leachate collection system (LCS) drainage layer were assumed.
- Density of waste assumed to be 75 pcf (12 kN/m³).
- The maximum waste thickness for the horizontal component of the landfill expansion was determined to be **198 feet**. This waste thickness was used to calculate the maximum loading anticipated on the main LCS in the horizontal expansion. The corresponding elevations, layer thicknesses, and material unit weights used in these calculations are summarized in **Table J.4C-1**.



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Table J.4C-1 - Summary of Landfill Layers at Point of Maximum Waste Thickness			
Layer	Top Elevation (ft. MSL)	Thickness (ft)	Unit Weights (pcf)
Horizontal Expansion – Floor and Sideslopes			
Final Cover System	896	5	130.3
Waste	891	198	75
LCS Drainage Layer	693	1	130
Horizontal Expansion – LCS Pipe Trench			
Final Cover System	896	5	130.3
Waste	891	198	75
LCS Drainage Layer	693	1	130
Coarse Aggregate (pipe bedding)	692	2	130

- Puncture resistance for the geosynthetic materials based on project specifications are as follow:
- 8 oz/yd² Nonwoven Geotextile = 120 lb (see Note below)
 - 12 oz/yd² Nonwoven Geotextile = 190 lb (see Note below)
 - 60-mil HDPE Geomembrane = 90 lb

Note: The puncture resistance for the geotextiles may be met or exceeded by using multiple layers of thinner geotextiles, provided that a total of 8 oz/yd² or 12 oz/yd² is achieved (e.g. overlying three 4 oz/yd² geotextiles will provide equivalent or greater protection than a 12 oz/yd² geotextile).

- The particle size for the various leachate collection system drainage materials which will come into contact with the geotextiles are presented in **Table J.4C-2** below. The maximum LCS particle protrusion height (H) is also presented in **Table J.4C-2**. The maximum protrusion height was assumed equal to ½ of the maximum particle size (D) and is based on the GSE / N.D. Corcoran reference presented in the attached pages.

Table J.4C-2 - Leachate Collection System (LCS) Materials and Particle Size					
LCS Material	Location of LCS Material	Maximum LCS Particle Diameter "D"		Maximum LCS Particle Protrusion Height "H" (H = 1/2 x D)	
		inches	mm	inches	mm
LCS Drainage Layer	Landfill floor & sideslopes	1.0	12.7	0.25	6.35
Washed Gravel	LCS pipe collection trench	1.5	38.1	0.75	19.05



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- **Calculation No. 1:** The evaluation of LCS geotextiles for puncture resistance was performed for the area under the greatest waste column (maximum load) within the horizontal expansion. The following formula is to be used to evaluate whether the LCS geotextiles are appropriately sized to resist puncture from the materials that will come into direct contact with them (see first Koerner reference in attached pages):

$$FS = \frac{F_{\text{allowable}}}{F_{\text{actual}}}$$

$$F_{\text{actual}} = p' d_a^2 S_1 S_2 S_3$$

Where,

- p' = Pressure exerted on the geotextile (psi)
 d_a^2 = Average diameter of the puncturing aggregate/object (in)
 S_1 = Protrusion factor of the puncturing object
 S_2 = Scale factor to adjust for actual shape of puncturing object
 S_3 = Shape factor to adjust for actual shape of puncturing object

Simplify and rearrange equation to solve for the average particle size, or diameter “ d_a ”:

$$FS = \frac{F_{\text{allowable}}}{p' d_a^2 S_1 S_2 S_3}$$

$$d_a = \left(\frac{F_{\text{allowable}}}{(FS) p' S_1 S_2 S_3} \right)^{1/2}$$



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- **Calculation No. 2:** Adequate geotextile size is verified in Calculation No. 2, for the maximum load in the horizontal expansion. The following formula is to be used to determine the factor of safety against geomembrane puncture based on the level of protection provided by the overlying geotextile (see second Koerner reference in attached pages):

$$FS = \frac{P_{\text{allowable}}}{P_{\text{required}}}$$

$$P_{\text{allowable}} = \left(50 + 0.00045 \frac{M}{H^2} \right) \left[\frac{1}{MF_S \times MF_{PD} \times MF_A} \right] \left[\frac{1}{RF_{CR} \times RF_{CBD}} \right]$$

Where,

P_{required}	= required pressure due to the landfill
$P_{\text{allowable}}$	= allowable pressure
M	= geotextile mass per unit area
H	= protrusion height (see GSE reference in attached pages)
MF_S	= modification factor for protrusion shape
MF_{PD}	= modification factor for packing density
MF_A	= modification factor for arching in solids
RF_{CR}	= reduction factor for long-term creep
RF_{CBD}	= reduction factor for long-term chemical/biological degradation



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TITLE: PUNCTURE RESISTANCE OF GEOSYNTHETICS**Calculation No. 1 – Evaluate LCS Geotextiles for Sizing and Puncture Resistance in the Horizontal Expansion**

In the horizontal expansion LCS pipe trench and floor, the 8-oz/yd² geotextile will have the pressure of the overlying waste and final cover material on it and it will be in contact with the granular drainage layer below ($d < 1.0$ inches). The 12-oz/yd² geotextile (or equivalent) on the floor will be overlain by the granular drainage layer ($d < 1.0$ inches), the waste, and the final cover. The 12-oz/yd² geotextile (or equivalent) in the LCS pipe trench will be overlain by the coarse aggregate ($d \leq 1.5$ inches), the waste, and the final cover. Below are the calculations for the total pressure on each geotextile from the final landform:

8 – oz / yd² geotextile above granular drainage layer on floor and sideslopes:

$$[(5 \text{ ft})(130.3 \text{ pcf}) + (198 \text{ ft})(75 \text{ pcf})] = 15,501.5 \text{ psf} = 107.6 \text{ psi}$$

8 – oz / yd² geotextile above coarse aggregate within the LCS pipe trench:

$$[(5 \text{ ft})(130.3 \text{ pcf}) + (198 \text{ ft})(75 \text{ pcf})] = 15,501.5 \text{ psf} = 107.6 \text{ psi}$$

12 – oz / yd² geotextile (or equivalent) beneath granular drainage layer on floor or sideslopes:

$$[(5 \text{ ft})(130.3 \text{ pcf}) + (198 \text{ ft})(75 \text{ pcf}) + (1 \text{ ft})(130 \text{ pcf})] = 15,631.5 \text{ psf} = 108.6 \text{ psi}$$

12 – oz / yd² geotextile (or equivalent) beneath coarse aggregate in the LCS pipe trench:

$$[(5 \text{ ft})(130.3 \text{ pcf}) + (198 \text{ ft})(75 \text{ pcf}) + (2 \text{ ft})(130 \text{ pcf})] = 15,761.5 \text{ psf} = 109.5 \text{ psi}$$

The particle shape of the granular drainage layer materials will be sub-angular/sub-rounded to rounded. The following puncture factors will be used for the granular drainage layer (refer to Table 2.13 in the first Koerner reference, in attached pages):

- Protrusion Factor, $S_1 = 0.55$
- Scale Factor, $S_2 = 0.5$
- Shape Factor, $S_3 = 0.55$

The following puncture factors will be used for the coarse aggregate (refer to Table 2.13 in the first Koerner reference, in attached pages):

- Protrusion Factor, $S_1 = 0.7$
- Scale Factor, $S_2 = 0.6$
- Shape Factor, $S_3 = 0.6$

Assume a safety factor of 2.0, and calculate the average diameter (“ d_a ”) for materials to be placed in direct contact with the following geotextiles: 8 oz/yd² and 12 oz/yd² (or equivalent(s)).



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TITLE: PUNCTURE RESISTANCE OF GEOSYNTHETICS**8 – oz / yd² geotextile above granular drainage layer on floor and sideslopes:**

$$d_a = \left(\frac{F_{\text{allowable}}}{(FS)p'S_1S_2S_3} \right)^{1/2} = \left(\frac{120 \text{ lb}}{(1.5)(107.6 \text{ psi})(0.55)(0.5)(0.55)} \right)^{1/2} = 2.22 \text{ inches}$$

8 – oz / yd² geotextile above coarse aggregate within the LCS pipe trench:

$$d_a = \left(\frac{F_{\text{allowable}}}{(FS)p'S_1S_2S_3} \right)^{1/2} = \left(\frac{120 \text{ lb}}{(1.5)(107.6 \text{ psi})(0.7)(0.6)(0.6)} \right)^{1/2} = 1.72 \text{ inches}$$

12 – oz / yd² geotextile (or equivalent) beneath granular drainage layer of floor and sideslopes:

$$d_a = \left(\frac{F_{\text{allowable}}}{(FS)p'S_1S_2S_3} \right)^{1/2} = \left(\frac{190 \text{ lb}}{(1.5)(108.6 \text{ psi})(0.55)(0.5)(0.55)} \right)^{1/2} = 2.78 \text{ inches}$$

12 – oz / yd² geotextile (or equivalent) beneath coarse aggregate in LCS pipe trench:

$$d_a = \left(\frac{F_{\text{allowable}}}{(FS)p'S_1S_2S_3} \right)^{1/2} = \left(\frac{190 \text{ lb}}{(1.5)(109.5 \text{ psi})(0.7)(0.6)(0.6)} \right)^{1/2} = 2.14 \text{ inches}$$

Next, the maximum LCS material particle sizes (D) are compared to the calculated average diameters (d_a) for the various geotextiles to confirm they are all below the average diameter (d_a) and therefore appropriately sized. **Table J.4C-3** on the following page presents the results of this comparison.

Table J.4C-3 – Comparison of LCS Material Particle Size (D) to Calculated Average Diameter (d_a) – Horizontal Expansion					
LCS - Layer Material and Particle Size		Geotextile Location, Size, and Average Particle Size			
LCS Layer Material And Location	LCS Material Maximum Particle Size "D" (inches)	Geotextile Location w/ Respect to LCS Layer	Geotextile Size	Calculated Average Particle Size " d_a " (inches)	Is $D < d_a$?
Located on Floor and Sideslope:					
Granular Drainage Layer	< 1.0	Above	8 oz/yd ²	2.22	yes
		Below ¹	12 oz/yd ²	2.78	yes
Located in Main - LCS Pipe Trench:					
Coarse Aggregate	≤ 1.5	Above	8 oz/yd ²	1.72	yes
		Below ¹	12 oz/yd ²	2.14	yes
Notes:					
1. Denotes a geotextile that directly overlies the 60-mil textured geomembrane liner.					



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Calculation No. 2 - Verify Geotextile Size Will Safeguard Against Geomembrane Punctures in the Horizontal Expansion

The LCS pipe trench configuration for the proposed horizontal components of the Site 2 North Expansion of the Zion Landfill incorporates a 24-inches of coarse aggregate pipe bedding with a diameter less than or equal to 1.5 inches ($D \leq 1.5$), over a 12 oz/yd² geotextile which is overlying a 60-mil textured geomembrane.

A laboratory evaluation was conducted for Orchard Hills Landfill in 2019 when both Orchard Hills Landfill and Zion Landfill were under the common ownership of Advanced Disposal Services to replicate the puncture resistance of the in-place LCS geosynthetics at the Orchard Hills Landfill. The LCS configuration tested includes: a layer of coarse aggregate pipe bedding with a diameter less than 1.5 inches ($D < 1.5$), over a 12 oz/yd² geotextile which is overlying a 60-mil smooth geomembrane. This is the same configuration as what is proposed for the pipe trench of the horizontal component of the Site 2 North Expansion at Zion Landfill. Therefore, this laboratory report (see attached) is being used to demonstrate that the proposed LCS pipe trench configuration in the horizontal component of the Site 2 North Expansion at Zion Landfill will not puncture the 60-mil textured geomembrane. This approach is being used because the proposed configuration in the horizontal component of the Site 2 North Expansion is not described in the charts referenced in this calculation methodology (see attached). The results of the laboratory testing found that the existing leachate collection system at Orchard Hills did not puncture under a nearly identical LCS configuration and under loading conditions that are greater than what will result from the horizontal component of the Site 2 North Expansion at Zion Landfill. The laboratory report is attached to this calculation.

Conclusion

The geotextiles to be used in the proposed leachate collection system of the Site 2 North Expansion at Zion Landfill all demonstrate sufficient safety factors against puncture from the LCS granular drainage layer and coarse aggregate materials in the horizontal expansion area. Similarly, the geomembrane liners used in the LCS pipe trenches, within the horizontal expansion area provide sufficient safety factors against puncture at the point of maximum waste thickness. Therefore, the geosynthetics are appropriate for the proposed horizontal expansion design.

Published Technical References

Geomembrane Puncture Resistance Studies

July 30, 2019

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by

TRI Environmental, Inc.
9063 Bee Cave Road
Austin, Texas, USA 78733

NOTE: The testing reported herein is based upon industry practice as well as the test method(s) listed. The results reported herein do not apply to samples other than those tested. TRI neither accepts responsibility for nor makes claim as to the final use and purpose of the material(s). TRI observes and maintains client confidentiality. TRI limits the reproduction of this report, except in full, without prior approval of TRI.

Executive Summary

TRI is pleased to submit this final report for laboratory testing.

This report details bench-scale, large-scale, and accelerated testing performed to determine the affect of compressive loading on a geomembrane lined leachate collection trench system. Specifically, the puncture resistance of the HDPE geomembrane barrier in the system was of interest.

All testing was intended to evaluate geomembrane puncture resistance based on use of site specific materials utilized in previously constructed areas of the Orchard Hills landfill which is owned and operated by Advanced Disposal Services, Inc. The site specific materials used in testing are listed below.

Geosynthetic Test Material	Manufacturer
60 mil Smooth HDPE Geomembrane	AGRU America
12 OSY Nonwoven Cushion Geotextile	Skaps
HDPE Drainage Pipe	Advanced Drainage Systems
Clay	Site Specific
Leachate Collection Rock (Site specific aggregate used in existing constructed cells)	D < 1.5 inches
Granular Drainage Layer (Site specific aggregate used in existing constructed cells)	D < 1.0 inches

Table 1: Test Materials

The testing performed involved three distinct evaluations, each with a specific measurement objective as detailed below.

Test	Standard Reference	Objective
1) Bench-Scale Puncture Resistance	ASTM D5514	To evaluate the shorter term puncture resistance of the geomembrane using site specific materials (detailed in Appendix A)
<p>Description: TRI performed conventional puncture testing of the Orchard Hills geomembrane using site specific materials. The testing was performed in accordance with ASTM D5514, <i>Standard Test Method for Large-Scale Hydrostatic Puncture Testing of Geosynthetics</i>.</p>		

Test	Standard Reference	Objective
2) Large-Scale Puncture Resistance	ASTM D5514, using large-scale modeling of trench system	To evaluate the longer term puncture resistance of the geomembrane using site specific materials under approximated equilibrium conditions (detailed in Appendix B)
<p>Description: Large-scale puncture resistance testing was performed in modified accordance with ASTM D5514 by using a large out-door test box able to accommodate a more site-specific quantity of test materials.</p>		
3) Accelerated Bench-Scale Puncture Resistance	ASTM D7361, Modified using model of trench cross section	To evaluate the time-dependent loss of geomembrane thickness and potential for puncture under site specific profile stresses. (detailed in Appendix C)
<p>Description: In order to determine the time-dependent loss of geomembrane thickness and potential for puncture of the geomembrane in the Orchard Hills leachate collection trench, a series of accelerated compression creep tests were performed using ASTM D 7361, <i>Standard Test Method for Accelerated Compressive Creep of Geosynthetic Materials Based on Time-Temperature Superposition Using the Stepped Isothermal Method</i>. In simple terms, the stepped isotherm method (SIM) test is designed to simulate long periods of time during an accelerated test duration through the use of temperature increases.</p>		

Table 2: Tests and Measurement Objectives

Conclusion

The Orchard Hills geomembrane was evaluated for both short term and long term resistance to puncture. All evaluations performed resulted in an observation of no geomembrane puncture occurring.

All individual tests performed and their associated details and conclusions are detailed in the Appendices.



TRI is pleased to be of service to ADS and trusts the reporting herein is useful for evaluation of the puncture resistance of the geomembrane. Please contact me if you have any questions or if I may provide any additional information.

Sincerely,

A handwritten signature in black ink that reads "Sam R. Allen".

Sam Allen
Vice President

Appendix A: Bench-Scale Puncture Resistance Testing

Appendix B: Large-Scale Puncture Resistance Testing

Appendix C: Accelerated Bench-Scale Puncture Resistance and Geomembrane Compressive Creep Testing

Appendix A

Bench-Scale Puncture Resistance ASTM D5514

December 5, 2017

Tim Curry

Advanced Disposal Services, Inc. – Orchard Hills Landfill

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CC: Mark Sieracke <msieracke@wcgrp.com>
Jay Warzinski <jay.warzinski@advanceddisposal.com>
Chris Peters <Chris.Peters@Cornerstoneeg.com>

Subject: Bench-Scale Hydrostatic Puncture Resistance

Project: Orchard Hills Landfill

Log #: 32368

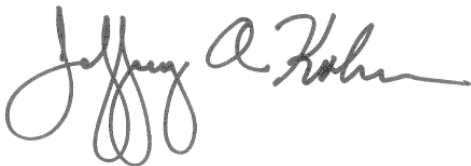
Test #: 1

Dear Mr. Curry,

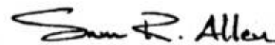
TRI is pleased to submit hydrostatic puncture testing results for Orchard Hills Landfill. This report documents the laboratory testing conducted by TRI Environmental (TRI) for a hydrostatic testing conducted from Friday, November 3rd through Monday, November 20th. in accordance ASTM D5514, Standard Test Method for Large Scale Hydrostatic Puncture Testing of Geosynthetics.

We appreciate the opportunity to work with you on this project and look forward to providing additional services in the future. Please contact us if you have any questions, comments, or requests.

Sincerely,



Jeffrey A. Kuhn, Ph.D., P.E.
Director of the Geotechnical Laboratory



Sam Allen
Vice President

SAMPLE RECEIPT, HANDLING, AND STORAGE

Samples of site-specific aggregate and clay were provided to TRI in a shipment received on September 20th, 2017. The standard disposal of samples occurs thirty day following final reporting. Otherwise, direction should be provided to TRI as to alternative arrangements for the disposal or return of samples.

1. TEST SETUP

Apparatus

A 24 inch diameter pressure vessel rated for a maximum pressure of 500 psi was used for testing. An air-activated hydraulic pump is used in combination with a servo-mechanical air regulator to apply a fixed ramp rate.

Configuration

The profile of material used in the testing program is presented in Figure 1. The first layer of material placed into the pressure vessel was site-specific clay that was processed, conditioned, and remolded to 90 percent of the maximum dry density and 3.5 percent wet of the optimum water content as determined by ASTM D1557 modified Proctor testing (see ASTM D1557 test results in Attachment to Appendix A). The clay was then covered with a 1 mil thick sheet of aluminum foil to assist in post-test deformation determination. Next, a 60 mil sheet of smooth HDPE liner was placed and was followed by a 12 oz/yd² nonwoven geotextile. Finally, the geotextile was overlain by 12 inches of leachate collection rock (see gradation test results in Attachment to Appendix A) which was tamped into place. The application of a normal stress to this profile was achieved through the use of a membrane overlain by pressurized water.

Component	Materials	Description/Placement
Upper Chamber	Pressurized Water	500 psi Max Pressure
	Stress Application Membrane	Test Apparatus
Lower Chamber	Leachate Collection Rock	Tamp in Place
	Skaps 12 oz/yd ² nonwoven geotextile	Roll #48413.7
	GSE 60 mil smooth HDPE geomembrane	Roll #102184955
	1 mil Aluminum Foil	Deformation Measurement (in addition to per-location thickness measurements)
	11 inches of compacted clay	90% & +3.5% of ASTM D1557 Maximum Density (w=14.9%, γd = 115.7 pcf)

Figure 1 – Test profile

2. TEST PROCEDURE

Material Conditioning/Processing

Images of the as-received clay and rock are presented below. Prior to testing, the clay was air-dried, broken down, and moisture-conditioned. The provided five five-gallon buckets of rock and eight five-gallon buckets of clay were sufficient for conducting this test method. No virgin material remains for additional testing. Images of the clay and rock samples are shown in Figure 2.



(a) Clay



(b) Rock

Figure 2 – Supplied Soils

Compression Test Initiation

Following placement of the testing profile, the vessel was pressurized at a rate of 1 psi/min up to a pressure of 130 psi. Due to compression/consolidation of the underlying materials, the stress application membrane ruptured at the edges of the vessel. Styrofoam and geotextile padding were added to the top of the profile, above the rock, and the vessel was re-pressurized at a rate of 1 psi/min up to a pressure of 130 psi. The target pressure of 130 psi was held for 10 days.

Post-test Examination / Breakdown

Following the 10 day hold period, the chamber was de-pressurized and the profile was exhumed for post-test examination. Photographs were obtained during exhumation and post-test evaluations of the geomembrane deformation, geomembrane thickness, and soil density were obtained.

3. POST-TEST MEASUREMENTS

Geomembrane Deformation

No geomembrane punctures were identified post-test. Post test high-to-low deformation measurements were taken across the geomembrane. A plot of the deformations over the surface area of the tested geomembrane is shown in Figure 3.

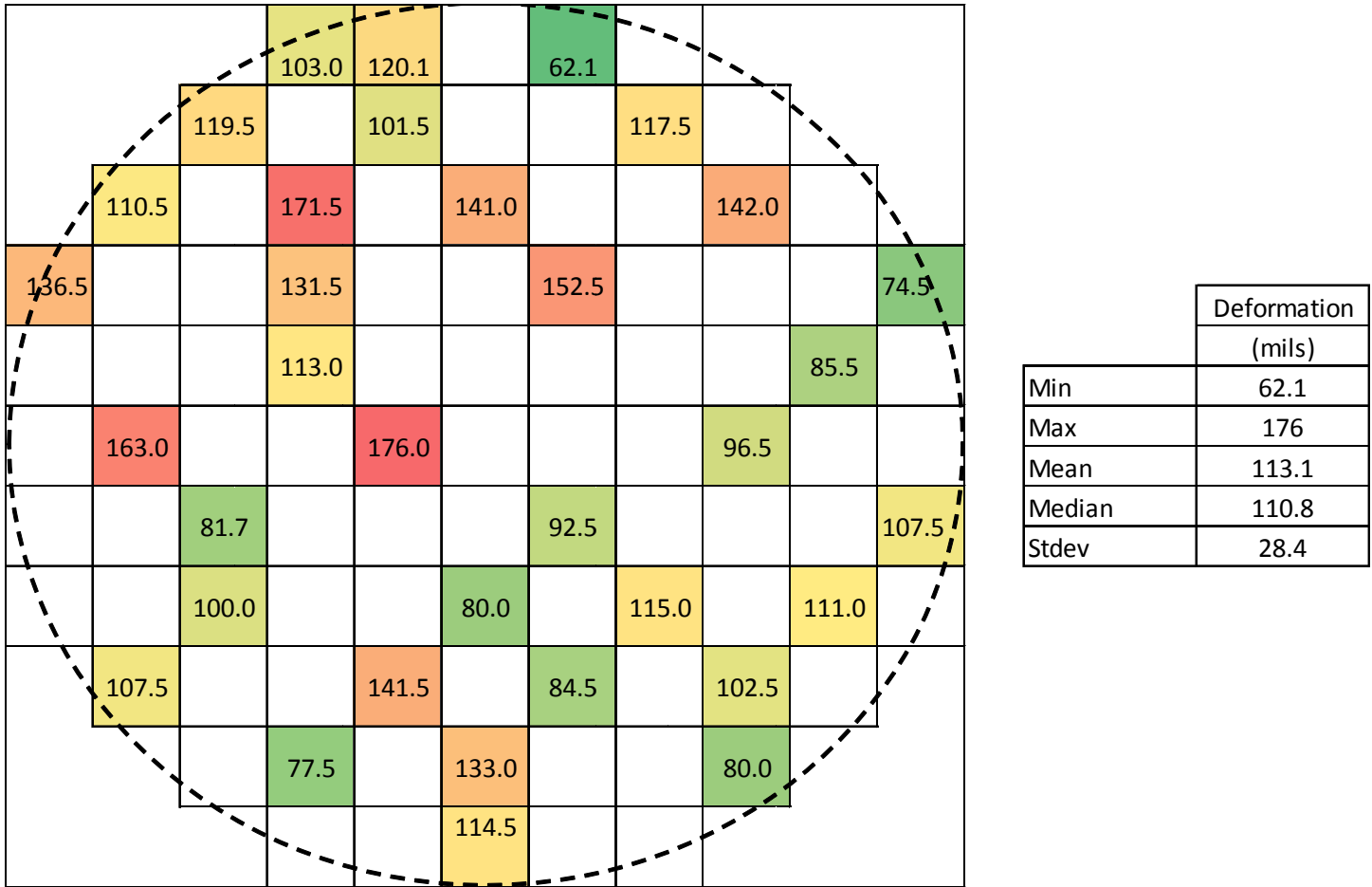


Figure 3 – Post-test Geomembrane Deformation Measurements

Aluminum foil (versus lead sheet or thicker aluminum flashing) with a thickness of 1 mil was used for the testing program in order to minimize interference with potential geomembrane deformation and/or puncture. The condition of the aluminum sheet following testing was such that its use for post-test deformation measurements could not be achieved. Instead, post-test geomembrane deformations were determined via direct measurements.

Geomembrane Thickness

The geomembrane was cut into 2 inch strips and the thickness was measured via single point observations of the minimal thickness per each 2 inch x 2 inch area. The resulting post-test geomembrane deformations are presented in Figure 4 along with pre-test measurements.

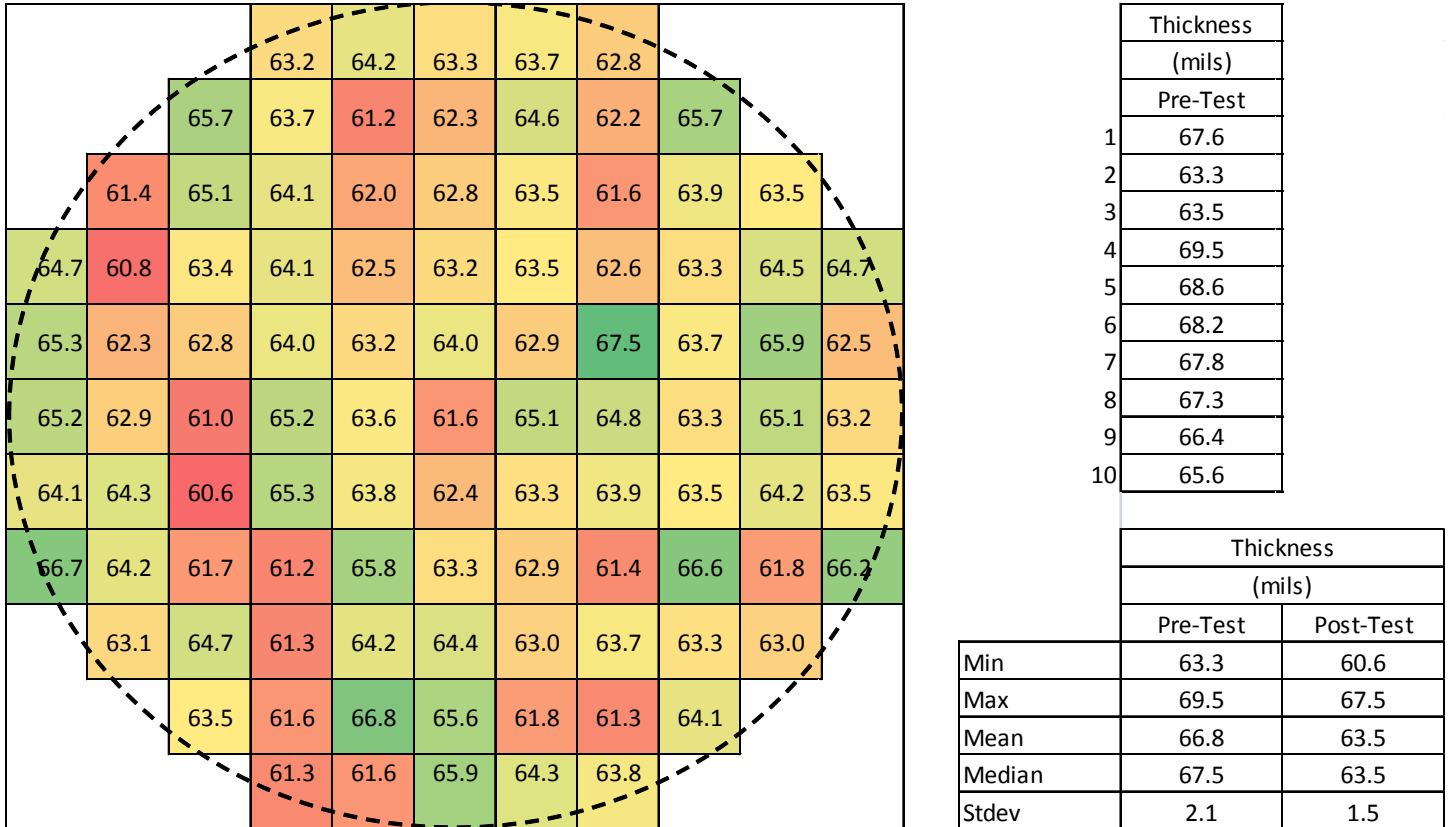


Figure 4 – Post-test Geomembrane Thickness Measurements

Clay Dry Density

A thin-walled sampling tube was pushed through the center of the compacted clay as a means of evaluating the post-test dry density of the compacted clay. A plot of the measured dry density is presented in Figure 5 with respect to the post-test clay height. The densification of material in the upper portion of the clay profile is notable.

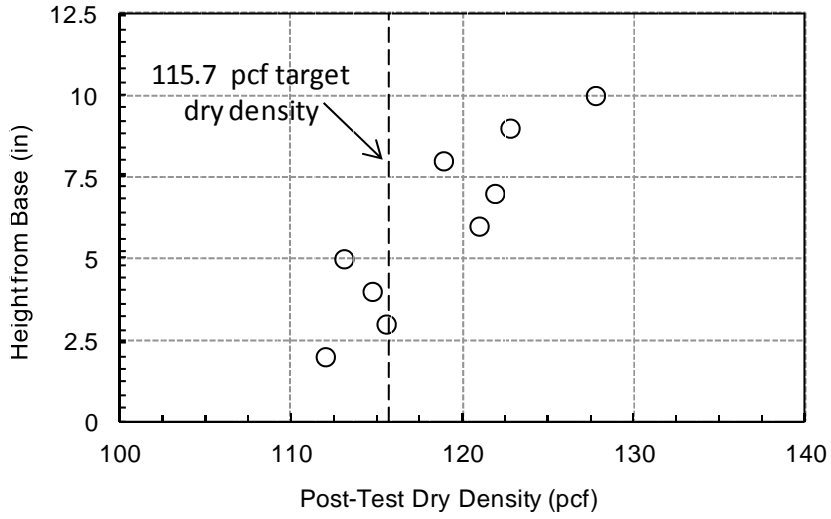


Figure 5 – Post-Test Dry Density Measurements

4. POST-TEST IMAGES

Photographs were obtained during exhumation of the testing profile. The following is a photo lot of the exhumation.



Rock: 12 in (Top)



Rock: 12 in - Fractured Particles



Rock: 10 in



Rock: 10 in Fractured particles adjacent to sidewall



Rock: 8 in



Rock: 6 in



Rock: 6 in



Rock: 6 in



Rock: 4 in



Rock: 2 in



Rock: 1 in



Geotextile



Geomembrane



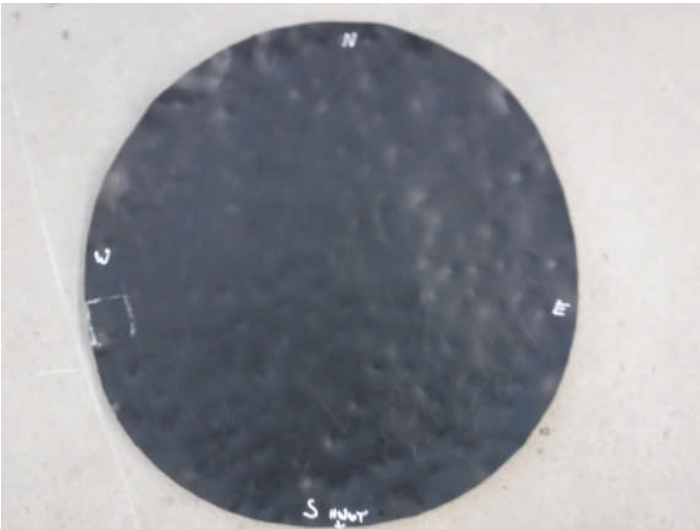
Aluminum foil



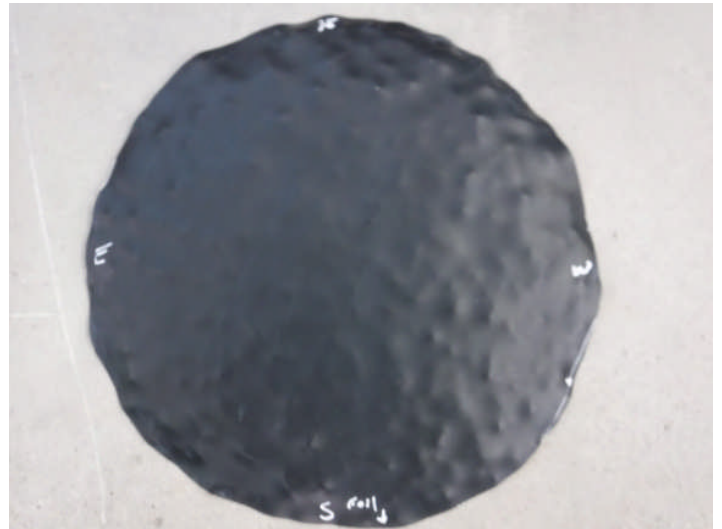
Clay sampling



Clay Sampling



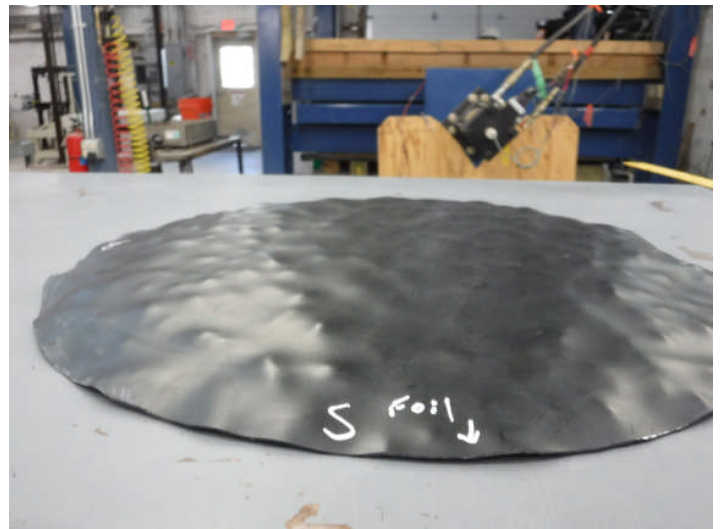
Geomembrane Top



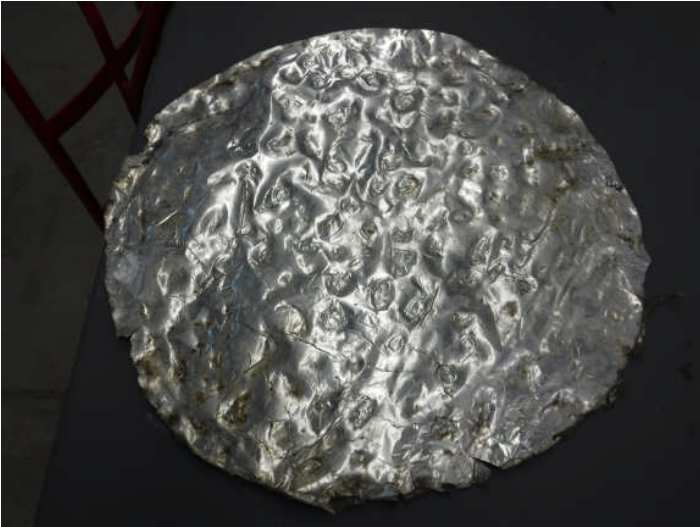
Geomembrane Bottom



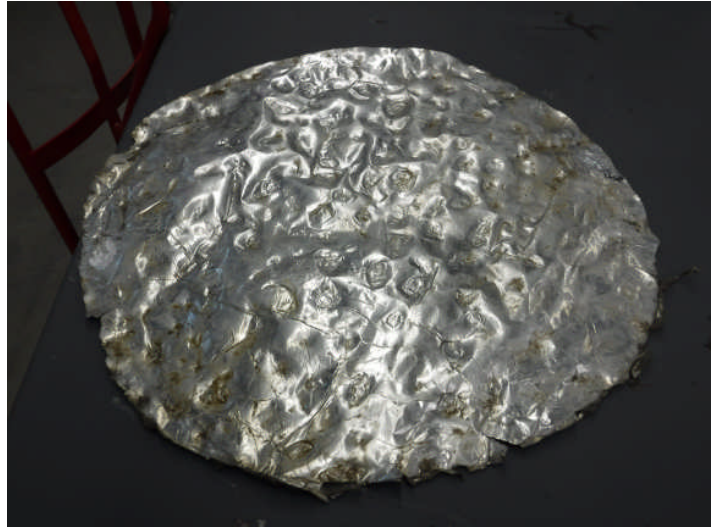
Geomembrane Top



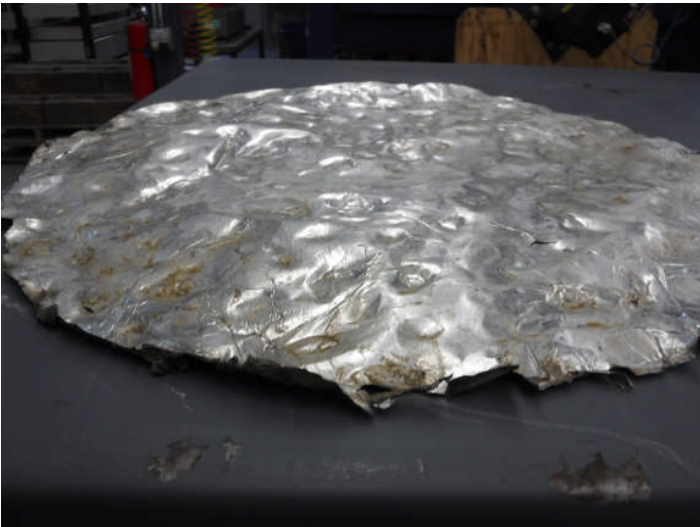
Geomembrane Bottom



Aluminum foil Top



Aluminum foil Bottom



Aluminum foil Top



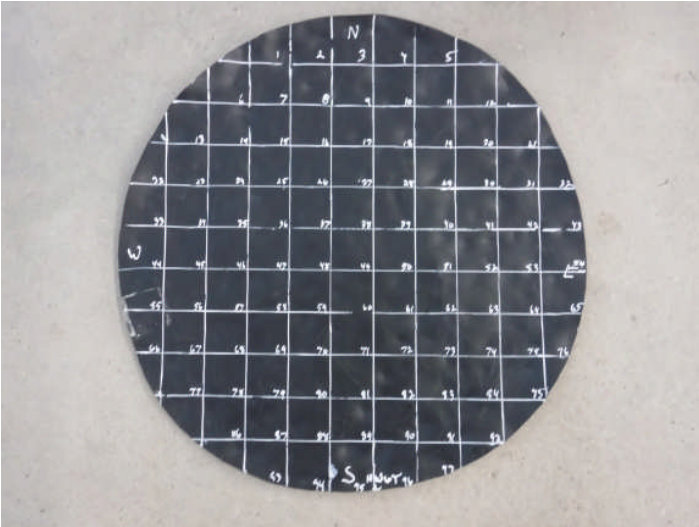
Aluminum foil Bottom



Aluminum foil



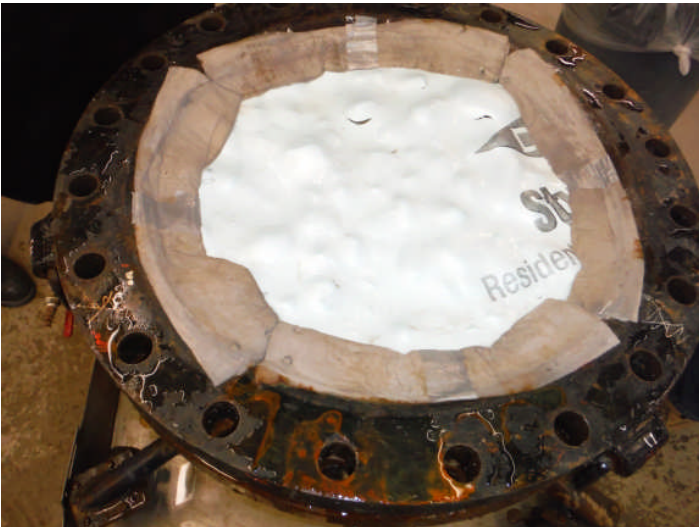
Aluminum foil



Geomembrane – 2 inch Square Deliniation



Styrofoam padding (Device)



Geotextile Padding (Device)



Stress Application Membrane (Device)

5. CONCLUDING REMARKS

A hydrostatic puncture test was performed on a profile consisting of leachate collection rock, nonwoven cushion geotextile, geomembrane and clay. The soils, clay and rock, were site-specific. The profile was subjected to a load of 130 psi for a maximum duration of 10 days. Photographs were obtained during exhumation and post-test measurements of the geomembrane deformation, geomembrane thickness, and soil density were performed. No geomembrane punctures were identified during post-test geomembrane inspection.



Attachments

A.1 Clay and Aggregate Characteristics

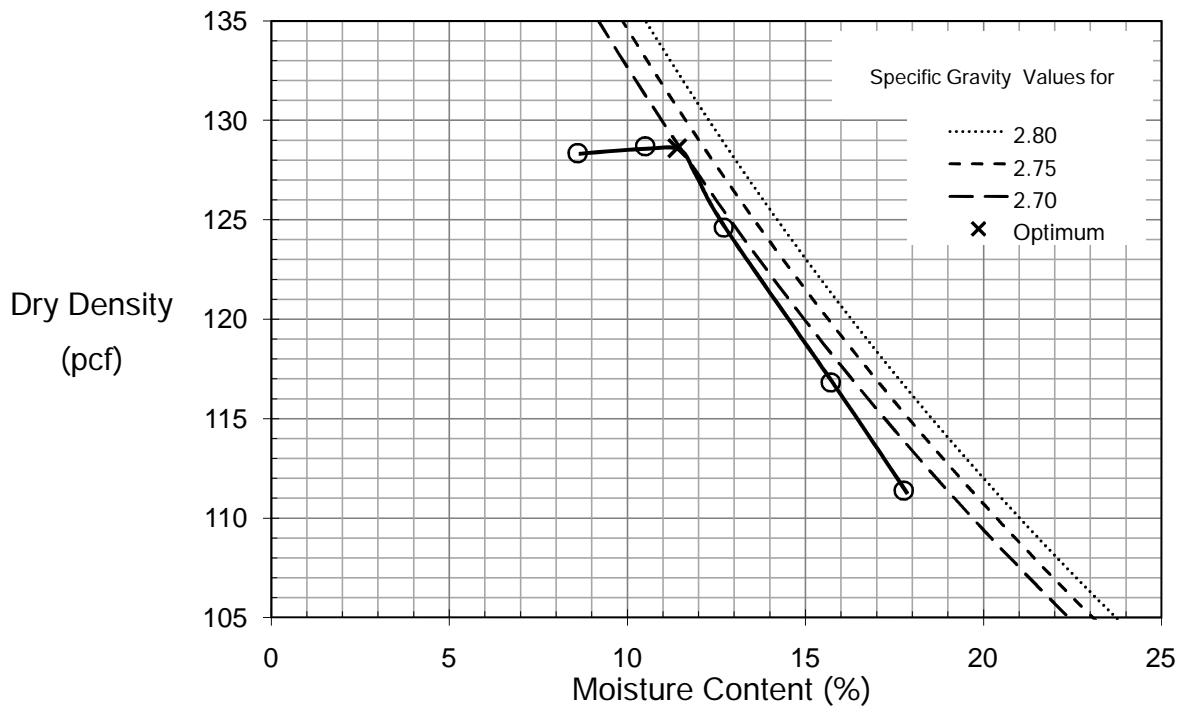
Laboratory Compaction Characteristics of Soil Using Modified Effort (ASTM D1557)

Client: Advances Disposal Services
 Project: Orchard Hills Landfill
 Sample ID: Clay

TRI Log #: 32368.2

Compaction Effort	-	Modified
Method	-	A
Rammer Type	-	Automatic
Maximum Dry Density	pcf	128.6
Optimum Water Content	%	11.4

Oversize Particle / "Rock" Correction (ASTM D4718)		
Oversized Particles	%	--
Maximum Dry Density	pcf	--
Optimum Water Content	%	--



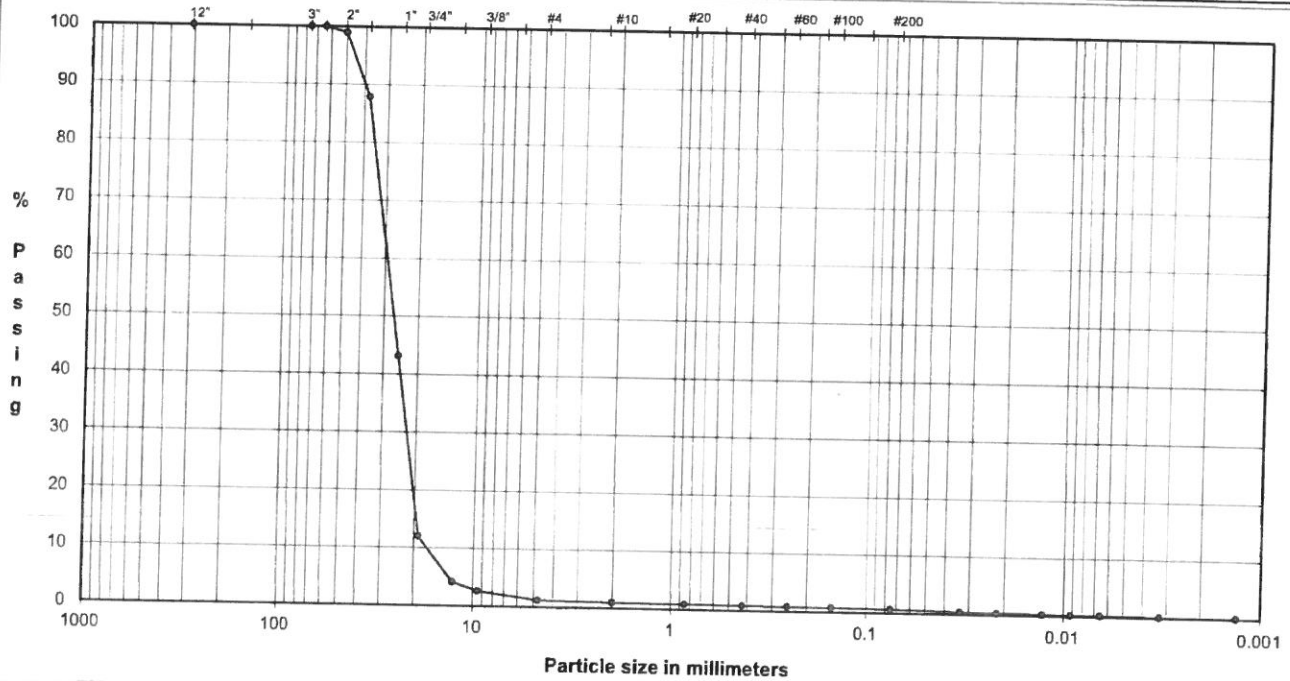
Jeffrey A. Kuhn, Ph.D, P.E., 7/17/2019

Quality Review / Date

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

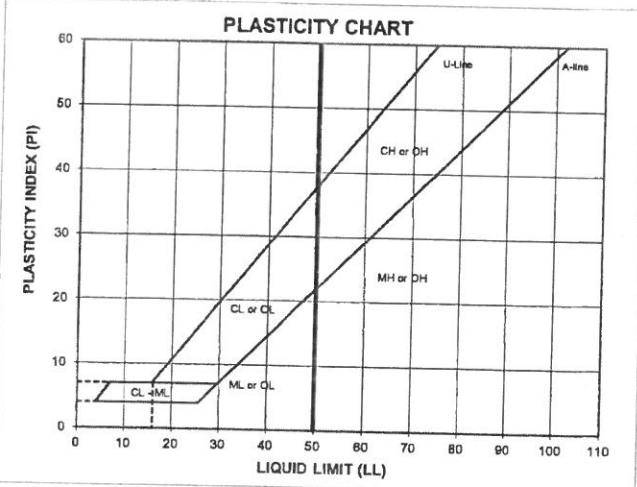
ASTM D421, D422, D4318

PROJECT NAME: **WEAVER/ORCHARD HILLS- PHASE X/IL**
 SAMPLE ID: **PB-1** Depth: **-**
 TYPE: **Bulk**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			FINES

U.S. Standard Sieves Sizes and Numbers	Particle Size (mm)	% Passing	Classification	Percentage
	12.0"	304.8	100.0	Cobbles
3.0"	75.0	100.0		
2.5"	63.5	100.0		
2.0"	50.0	98.9		
1.5"	37.5	87.9		
1.0"	25.0	43.2	Coarse Gravel	88.0
0.75"	19.0	12.0		
0.50"	12.7	4.1		
0.375"	9.5	2.6	Fine Gravel	10.8
#4	4.8	1.2		
#10	2.00	1.0	Coarse Sand	0.2
#20	0.85	0.9	Medium Sand	0.1
#40	0.43	0.9		
#60	0.25	0.8		
#100	0.15	0.8	Fine Sand	0.2
#200	0.075	0.7		



Hydrometer Analysis	(mm)	% Finer	Fines Silt or Clay	0.7
	0.033	0.5		
	0.022	0.4		
	0.013	0.3		
	0.0092	0.2		
	0.0065	0.2		
0.0033	0.2			
0.0013	0.1			

ATTERBERG LIMITS Method -B (Dry preparation)

M _c	LL	PL	PI	LI
-	-	-	-	-

LL (oven-dried)
 < 0.75 - ORGANIC (LOOI)

DESCRIPTION: **GRAVEL, fine to coarse, trace sand, trace fines; yellow, red, and gray.**
 USCS: **GP**

TECH: **WD**
 DATE: **7/11/16**
 CHECK: *[Signature]*
 REVIEW: *[Signature]*
 APPROVE: *[Signature]*

PARTICLE SIZE DISTRIBUTION & ATTERBERG LIMITS

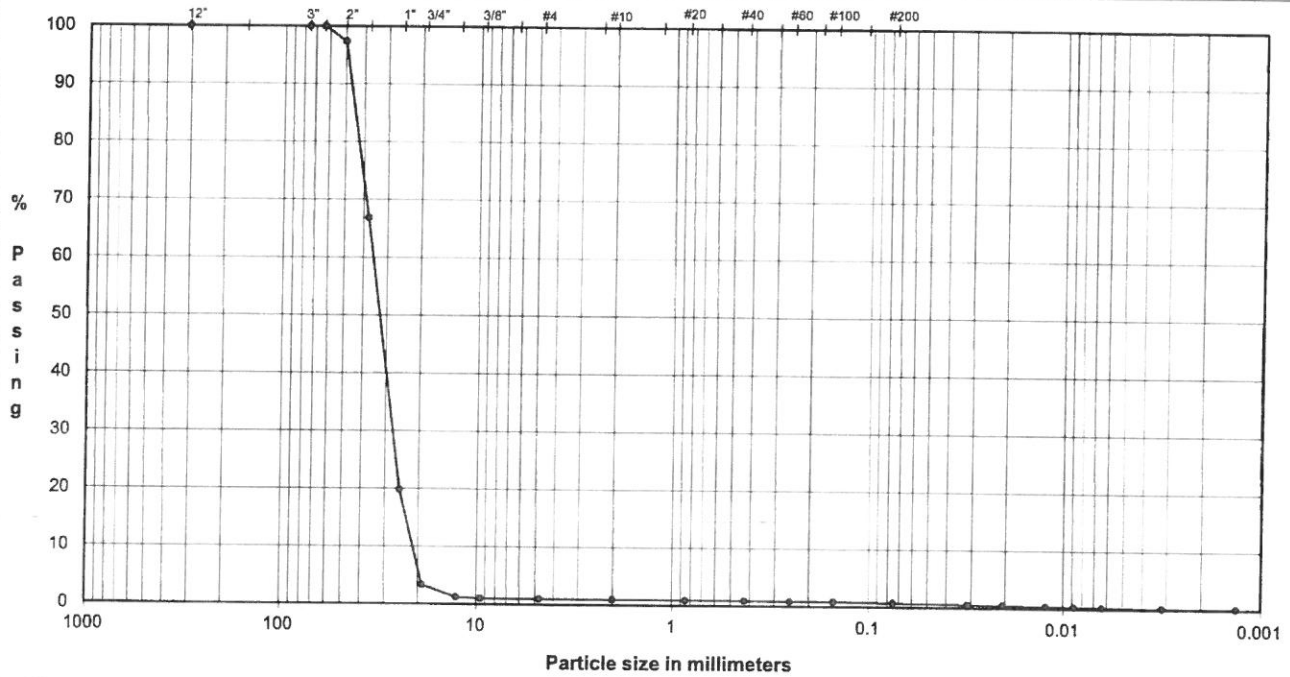
ASTM D421, D422, D4318

PROJECT NAME: **WEAVER/ORCHARD HILLS- PHASE X/IL**

SAMPLE ID: **PB-2**

Depth: -

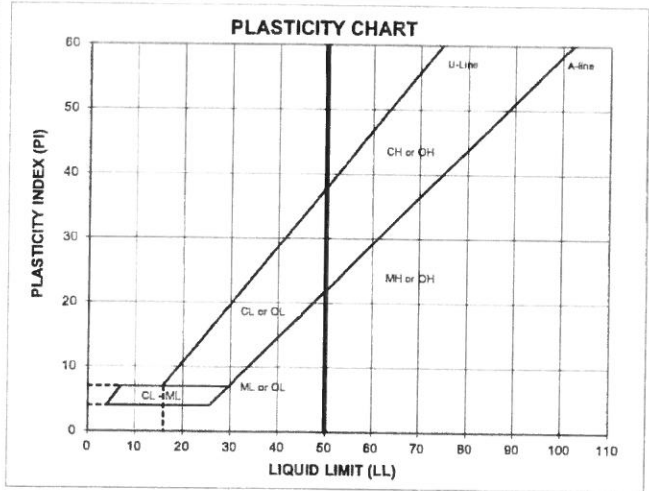
TYPE: **Bulk**



COBBLES	Coarse	Fine	Coarse	Medium	Fine	Silt or Clay
	GRAVEL		SAND			

U.S. Standard Sieves Sizes and Numbers

Particle Size (mm)	% Passing	Classification	Percentage
12.0"	304.8	100.0	
3.0"	75.0	100.0	
2.5"	63.5	100.0	
2.0"	50.0	97.3	
1.5"	37.5	66.9	
1.0"	25.0	19.8	
0.75"	19.0	3.3	Coarse Gravel
0.50"	12.7	1.2	
0.375"	9.5	1.0	Fine Gravel
#4	4.8	1.0	
#10	2.00	0.9	Coarse Sand
#20	0.85	0.9	Medium Sand
#40	0.43	0.9	
#60	0.25	0.9	Fine Sand
#100	0.15	0.8	
#200	0.075	0.8	0.1



Hydrometer Analysis

(mm)	% Finer	Fines Silt or Clay	0.8
0.031	0.6		
0.021	0.4		
0.012	0.4		
0.0088	0.3		
0.0063	0.2		
0.0032	0.2		
0.0013	0.2		

ATTERBERG LIMITS
Method -B (Dry preparation)

ML	LL	PL	PI	LI
-				

LL (oven-dried)
< 0.75 - ORGANIC (LO/OH)

DESCRIPTION: GRAVEL, fine to coarse, trace sand, trace fines; yellow, red, and gray.

USCS: GP

TECH	AT/TB
DATE	7/14/16
CHECK	30M
REVIEW	1654
APPROVE	

Particle Size Distribution Report ASTM D 422



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	4.3	92.4	0.9	0.3	0.4	1.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	95.7		
1/2"	55.9		
3/8"	19.4		
#4	3.3		
#8	2.5		
#16	2.3		
#30	2.2		
#50	2.0		
#100	1.9		
#200	1.7		

Soil Description

Poorly Graded GRAVEL

Atterberg Limits

PL= LL= PI=

Coefficients

D₉₀= 17.3634 D₈₅= 16.3462 D₆₀= 13.0978

D₅₀= 12.1575 D₃₀= 10.4657 D₁₅= 8.2952

D₁₀= 6.8790 C_u= 1.90 C_c= 1.22

Classification

USCS= GP AASHTO=

Remarks

Sample Received 5-25-2018

* (no specification provided)

Source of Sample: Drainage Layer
 Sample Number: DL-01

Date: 6-1-2018

<p>Weaver Consultants Group</p> <p style="text-align: center;">Granger, Indiana</p>	<p>Client: Advanced Disposal</p> <p>Project: Orchard Hills Landfill Phase X South</p> <p>Project No: 0306-303-16-02</p>
<p>Figure</p>	

Tested By: en Checked By: jjw

GRAIN SIZE DISTRIBUTION TEST DATA

6/1/2018

Client: Advanced Disposal
Project: Orchard Hills Landfill Phase X South
Project Number: 0306-303-16-02
Location: Drainage Layer
Sample Number: DL-01
Material Description: Poorly Graded GRAVEL
Date: 6-1-2018
USCS Classification: GP
Testing Remarks: Sample Received 5-25-2018
Tested by: en

Checked by: jjw

Sieve Test Data

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 6167.80
 Tare Wt. = 1205.60
 Minus #200 from wash = 1.5%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
6244.70	1205.60	1"	0.00	0.00	100.0
		3/4"	218.30	0.00	95.7
		1/2"	2002.90	0.00	55.9
		3/8"	1840.40	0.00	19.4
		#4	808.80	0.00	3.3
		#8	41.56	0.00	2.5
		#16	13.30	0.00	2.3
		#30	5.40	0.00	2.2
		#50	5.80	0.00	2.0
		#100	6.90	0.00	1.9
		#200	11.70	0.00	1.7

Fractional Components

Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	4.3	92.4	96.7	0.9	0.3	0.4	1.6			1.7

D ₅	D ₁₀	D ₁₅	D ₂₀	D ₃₀	D ₄₀	D ₅₀	D ₆₀	D ₈₀	D ₈₅	D ₉₀	D ₉₅
5.3448	6.8790	8.2952	9.5827	10.4657	11.2996	12.1575	13.0978	15.5213	16.3462	17.3634	18.7916

Fineness Modulus	C _u	C _c
6.71	1.90	1.22

Weaver Consultants Group

Particle Size Distribution Report ASTM D 422



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	11.1	86.8	0.2	0.2	0.1	1.6	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	88.9		
1/2"	40.2		
3/8"	11.4		
#4	2.1		
#8	1.9		
#16	1.8		
#30	1.8		
#50	1.7		
#100	1.6		
#200	1.6		

Soil Description

Poorly Graded GRAVEL

Atterberg Limits

PL= LL= PI=

Coefficients

D₉₀= 19.3312 D₈₅= 18.1948 D₆₀= 14.7430
D₅₀= 13.6894 D₃₀= 11.6696 D₁₅= 10.0032
D₁₀= 8.8751 C_u= 1.66 C_c= 1.04

Classification

USCS= GP AASHTO=

Remarks

* (no specification provided)

Source of Sample: Drainage Layer
Sample Number: DL-02

Date: 6-1-2018

<p>Weaver Consultants Group</p> <p style="text-align: center;">Granger, Indiana</p>	<p>Client: Advanced Disposal Project: Orchard Hills Landfill Phase X South</p> <p>Project No: 0306-303-16-02</p>
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Figure

Tested By: en Checked By: jiw

GRAIN SIZE DISTRIBUTION TEST DATA

6/1/2018

Client: Advanced Disposal
Project: Orchard Hills Landfill Phase X South
Project Number: 0306-303-16-02
Location: Drainage Layer
Sample Number: DL-02
Material Description: Poorly Graded GRAVEL
Date: 6-1-2018
USCS Classification: GP
Tested by: en

Checked by: jjw

Sieve Test Data

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 5879.30
 Tare Wt. = 1219.20
 Minus #200 from wash = 1.4%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
5945.10	1219.20	1"	0.00	0.00	100.0
		3/4"	523.70	0.00	88.9
		1/2"	2304.40	0.00	40.2
		3/8"	1361.30	0.00	11.4
		#4	437.00	0.00	2.1
		#8	8.10	0.00	1.9
		#16	4.60	0.00	1.8
		#30	3.10	0.00	1.8
		#50	4.00	0.00	1.7
		#100	5.00	0.00	1.6
		#200	1.00	0.00	1.6

Fractional Components

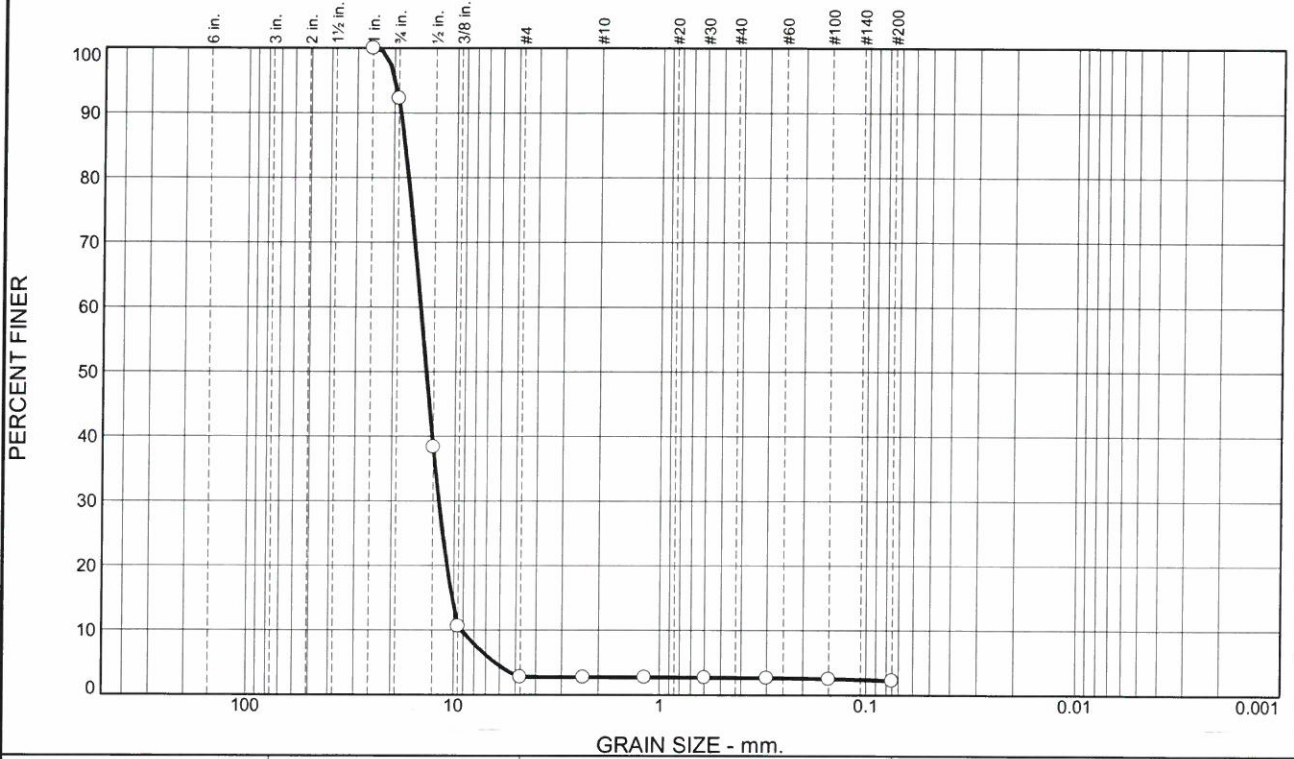
Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	11.1	86.8	97.9	0.2	0.2	0.1	0.5			1.6

D ₅	D ₁₀	D ₁₅	D ₂₀	D ₃₀	D ₄₀	D ₅₀	D ₆₀	D ₈₀	D ₈₅	D ₉₀	D ₉₅
6.4210	8.8751	10.0032	10.5943	11.6696	12.6842	13.6894	14.7430	17.3180	18.1948	19.3312	21.1382

Fineness Modulus	C _u	C _c
6.89	1.66	1.04

Weaver Consultants Group

Particle Size Distribution Report ASTM D 422



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	7.8	89.4	0.1	0.0	0.5	2.2	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	92.2		
1/2"	38.3		
3/8"	10.6		
#4	2.8		
#8	2.7		
#16	2.7		
#30	2.7		
#50	2.6		
#100	2.5		
#200	2.2		

Soil Description

Poorly Graded GRAVEL

Atterberg Limits

PL= LL= PI=

Coefficients

D₉₀= 18.5572 D₈₅= 17.6762 D₆₀= 14.7358
D₅₀= 13.7875 D₃₀= 11.8750 D₁₅= 10.1516
D₁₀= 9.1754 C_u= 1.61 C_c= 1.04

Classification

USCS= GP AASHTO=

Remarks

* (no specification provided)

Source of Sample: Drainage Layer
Sample Number: DI-03

Date: 6-1-2018

<p>Weaver Consultants Group</p> <p style="text-align: center;">Granger, Indiana</p>	<p>Client: Advanced Disposal Project: Orchard Hills Landfill Phase X South</p> <p>Project No: 0306-303-16-02</p>
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Tested By: pl Checked By: jjw

GRAIN SIZE DISTRIBUTION TEST DATA

6/1/2018

Client: Advanced Disposal
Project: Orchard Hills Landfill Phase X South
Project Number: 0306-303-16-02
Location: Drainage Layer
Sample Number: DI-03
Material Description: Poorly Graded GRAVEL
Date: 6-1-2018
USCS Classification: GP
Tested by: pl

Checked by: jjw

Sieve Test Data

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 6652.00
 Tare Wt. = 1198.50
 Minus #200 from wash = 2.1%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
6770.20	1198.50	1"	0.00	0.00	100.0
		3/4"	432.70	0.00	92.2
		1/2"	3002.50	0.00	38.3
		3/8"	1544.70	0.00	10.6
		#4	438.10	0.00	2.8
		#8	1.20	0.00	2.7
		#16	0.90	0.00	2.7
		#30	1.40	0.00	2.7
		#50	3.50	0.00	2.6
		#100	7.20	0.00	2.5
		#200	14.20	0.00	2.2

Fractional Components

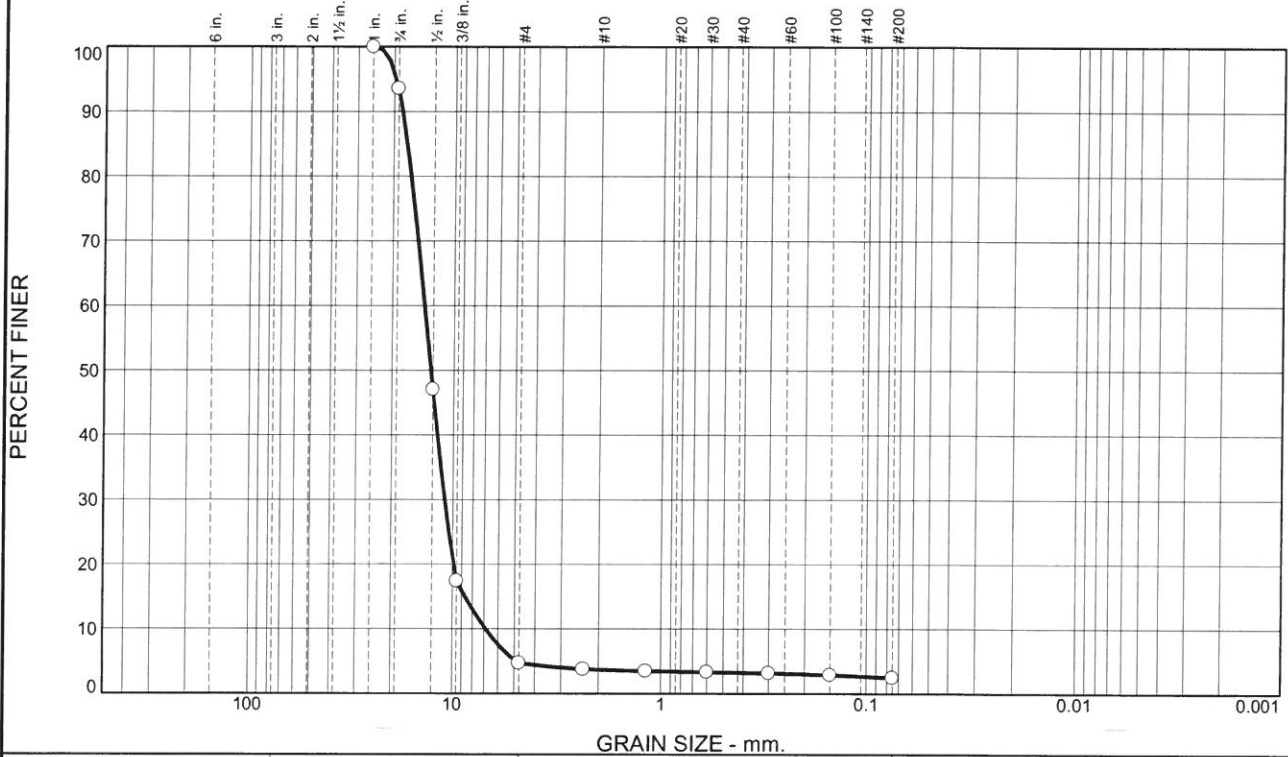
Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	7.8	89.4	97.2	0.1	0.0	0.5	0.6			2.2

D ₅	D ₁₀	D ₁₅	D ₂₀	D ₃₀	D ₄₀	D ₅₀	D ₆₀	D ₈₀	D ₈₅	D ₉₀	D ₉₅
6.2956	9.1754	10.1516	10.7774	11.8750	12.8569	13.7875	14.7358	16.9578	17.6762	18.5572	19.8404

Fineness Modulus	C _u	C _c
6.81	1.61	1.04

Weaver Consultants Group

Particle Size Distribution Report ASTM D 422



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	6.5	88.8	1.0	0.5	0.7	2.5	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1"	100.0		
3/4"	93.5		
1/2"	47.0		
3/8"	17.4		
#4	4.7		
#8	3.8		
#16	3.5		
#30	3.3		
#50	3.2		
#100	2.9		
#200	2.5		

Soil Description

Poorly Graded GRAVEL

Atterberg Limits

PL= LL= PI=

Coefficients

D₉₀= 18.1624 D₈₅= 17.2096 D₆₀= 14.0200

D₅₀= 12.9962 D₃₀= 10.9692 D₁₅= 8.6696

D₁₀= 6.8808 C_u= 2.04 C_c= 1.25

Classification

USCS= GP AASHTO=

Remarks

* (no specification provided)

Source of Sample: Drainage Layer
Sample Number: DL-04

Date: 6-1-2018

<p>Weaver Consultants Group</p> <p style="text-align: center;">Granger, Indiana</p>	<p>Client: Advanced Disposal Project: Orchard Hills Landfill Phase X South</p> <p>Project No: 0306-303-16-02</p>
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Figure

Tested By: en _____ Checked By: jiw _____

GRAIN SIZE DISTRIBUTION TEST DATA

6/1/2018

Client: Advanced Disposal
 Project: Orchard Hills Landfill Phase X South
 Project Number: 0306-303-16-02
 Location: Drainage Layer
 Sample Number: DL-04
 Material Description: Poorly Graded GRAVEL
 Date: 6-1-2018
 USCS Classification: GP
 Tested by: en

Checked by: jjw

Sieve Test Data

Post #200 Wash Test Weights (grams): Dry Sample and Tare = 5853.50
 Tare Wt. = 1182.20
 Minus #200 from wash = 2.1%

Dry Sample and Tare (grams)	Tare (grams)	Sieve Opening Size	Weight Retained (grams)	Sieve Weight (grams)	Percent Finer
5951.80	1182.20	1"	0.00	0.00	100.0
		3/4"	308.20	0.00	93.5
		1/2"	2217.80	0.00	47.0
		3/8"	1413.30	0.00	17.4
		#4	606.50	0.00	4.7
		#8	43.80	0.00	3.8
		#16	14.20	0.00	3.5
		#30	7.30	0.00	3.3
		#50	8.20	0.00	3.2
		#100	11.90	0.00	2.9
		#200	21.00	0.00	2.5

Fractional Components

Cobbles	Gravel			Sand				Fines		
	Coarse	Fine	Total	Coarse	Medium	Fine	Total	Silt	Clay	Total
0.0	6.5	88.8	95.3	1.0	0.5	0.7	2.2			2.5

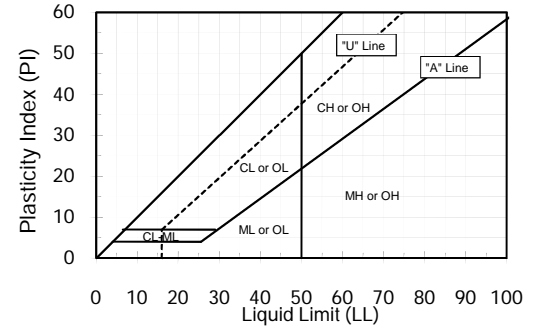
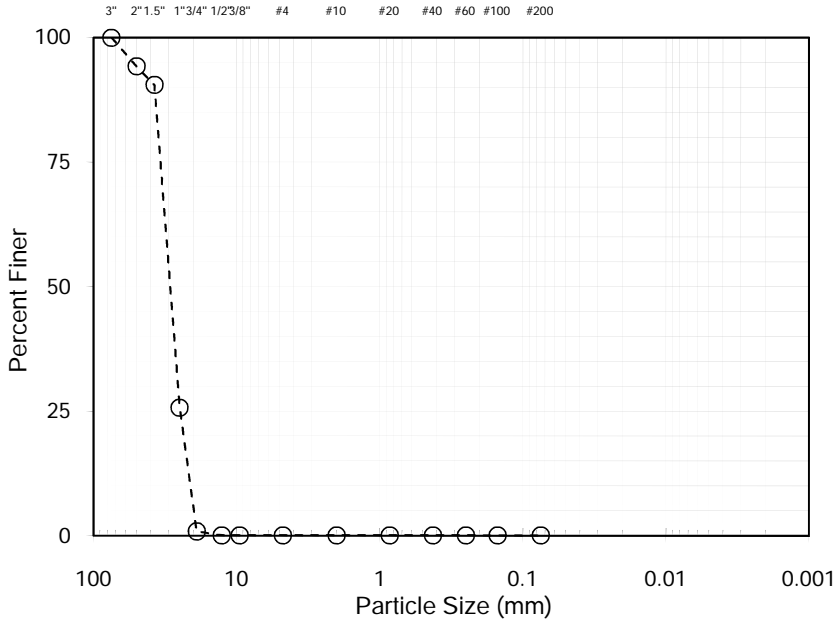
D ₅	D ₁₀	D ₁₅	D ₂₀	D ₃₀	D ₄₀	D ₅₀	D ₆₀	D ₈₀	D ₈₅	D ₉₀	D ₉₅
4.8957	6.8808	8.6696	9.8506	10.9692	11.9950	12.9962	14.0200	16.4294	17.2096	18.1624	19.5276

Fineness Modulus	C _u	C _c
6.68	2.04	1.25

Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Sample ID: DL-1

TRI Log #: 34773.2



Atterberg Limits	
(ASTM D4318, Method A : Multipoint, Air Dried)	
Liquid Limit	--
Plastic Limit	--
Plastic Index	--
(NL = No Liquid Limit, NP = No Plastic Limit)	

Mechanical Sieve		
(ASTM D422)		
Sieve Designation		Percent Passing
-	mm	
3 in.	76.2	100.0
2 in.	50.8	94.3
1.5 in.	38.1	90.5
1 in.	25.4	25.7
3/4 in.	19.0	0.9
1/2 in.	12.7	0.1
3/8 in.	9.51	0.1
No. 4	4.76	0.1
No. 10	2.00	0.1
No. 20	0.841	0.1
No. 40	0.420	0.1
No. 60	0.250	0.1
No. 100	0.149	0.1
No. 200	0.074	0.1

Hydrometer Analysis	
(ASTM D422)	
Particle Size	Percent Passing
mm	
--	--
--	--
--	--
--	--
--	--
--	--

Log-Linear Interpolation	
Particle Size	Percent Passing
mm	
0.005	--
0.002	--

D _x (mm), Log-Linear Interpolation			
10	30	50	60
21.01	25.68	29.10	30.98

Cu	Cc
1.47	1.01

USCS Classification (ASTM D2487)	
Poorly-graded gravel (GP)	
Moisture Content (%) (ASTM D2216)	
0.2	
Angularity (Percent by Mass*) (ASTM D2488)	
Angular	15.9
Subangular	31.1
Subrounded	46.2
Rounded	6.7

*110 C oven-dried material retained on a No. 4 Sieve
 Jeffrey A. Kuhn, Ph.D., P.E., 7/17/2019

Analysis & Quality Review/Date

Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services

TRI Log #: 34773.2

Project: Orchard Hills Landfill

Sample ID: DL-1

Image 1 of 5 (> No.4 Sieve)
Sub-Specimen for Angularity Evaluation



Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services
Project: Orchard Hills Landfill
Sample ID: DL-1

TRI Log #: 34773.2

Image 2 of 5 (> No.4 Sieve)
Angular



Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services
Project: Orchard Hills Landfill
Sample ID: DL-1

TRI Log #: 34773.2

Image 3 of 5 (> No.4 Sieve)
Subangular



Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services
Project: Orchard Hills Landfill
Sample ID: DL-1

TRI Log #: 34773.2

Image 4 of 5 (> No.4 Sieve)
Subrounded



Particle Size, Atterberg Limit, and USCS Analyses for Soils

Client: Advanced Disposal Services
Project: Orchard Hills Landfill
Sample ID: DL-1

TRI Log #: 34773.2

Image 5 of 5 (> No.4 Sieve)
Rounded



Appendix B

Large-Scale Puncture Resistance Testing ASTM D5514, Modified using Large-Scale Test Cell

26 February 2019

Timothy D. Curry

Advanced Disposal

232 Vance Road, Suite 208

Valley Park, Missouri 63088

Phone: 636 529 1974

tim.curry@advanceddisposal.com

RE: Large-Scale Puncture Resistance Testing

Dear Mr. Curry,

Large-scale puncture resistance testing was performed in modified accordance with ASTM D5514 by using a large out-door test box able to accommodate a more site-specific quantity of test materials.

TRI employed a large scale reinforced concrete test box with inside dimensions of 10 feet square by 8 feet tall. The box was used to construct and contain a model of the leachate collection trench used at Orchard Hills landfill.



Figure 1: TRI Accelerated-Loading Test Box

The trench was constructed of pipe, geomembrane, clay and rock materials acquired directly from Orchard Hills landfill via product rolls and container bulk bags. The cushion geotextile was acquired from Skaps, the manufacturing facility which supplies geotextiles for the Orchard Hills Landfill.

Figure 2 presents the construction plan followed. Importantly, plate load cells were used to monitor and verify the applied surcharge load. In addition, settlement plate gauges were used to monitor the consolidation of the constructed clay liner after loading.

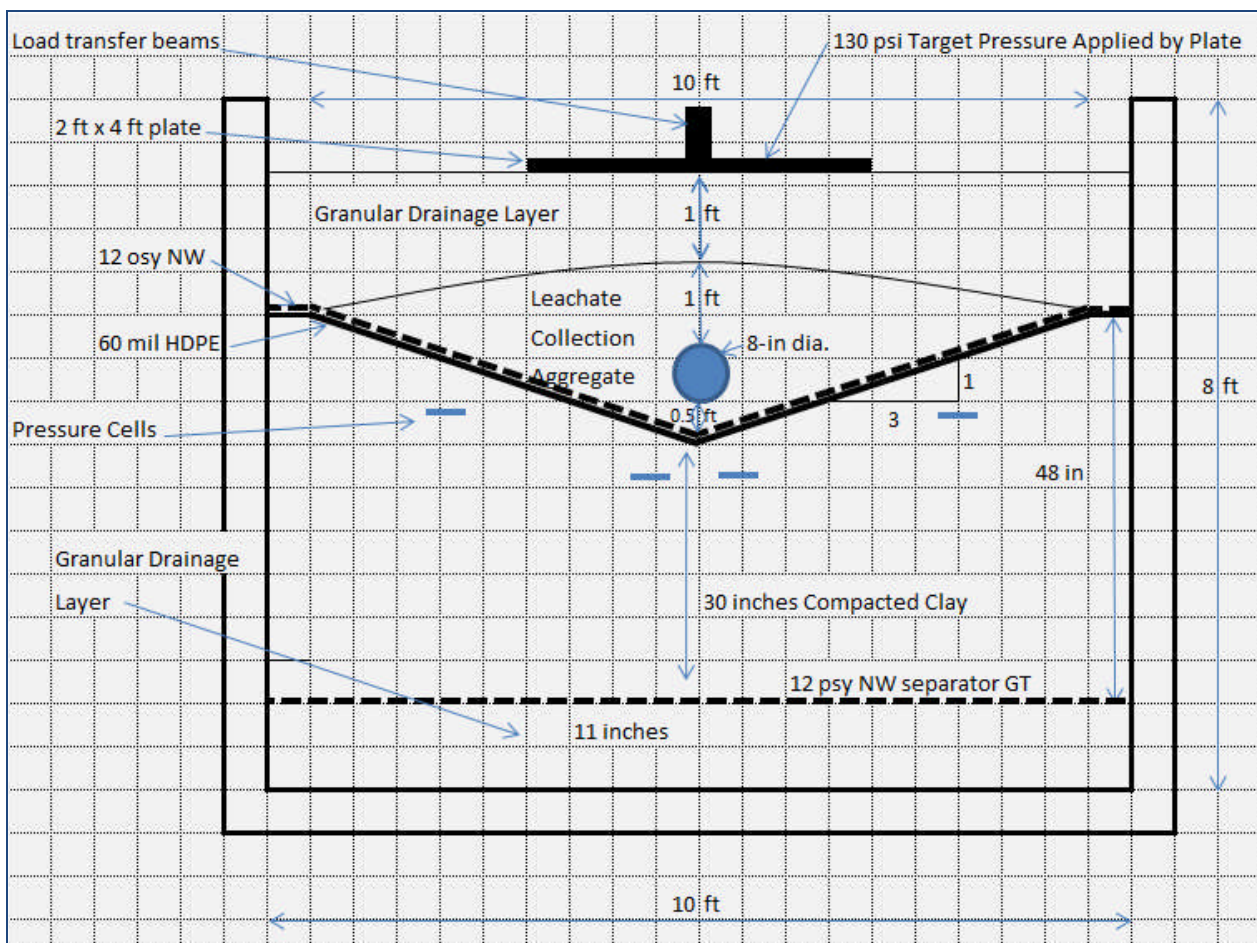


Figure 2: Cross Section of Test Profile

The test plan included the monitoring of the clay liner with plate consolidation meters for primary consolidation with a goal of determining when the test profile would approach an equilibrium thickness. That is, no other primary consolidation would take place as all non-clay materials would have already experienced ductile compressive deformation by the time the clay had consolidated.

The clay was tested for consolidation rate in accordance with ASTM D2435 (*Standard Test Methods for One-Dimensional Consolidation Properties of Soils Using Incremental Loading*) consolidation tests on a saturated clay. This test results are included as an attachment to this report. Table 1 outlines predicted consolidation times.

Average Degree of Consolidation (%)	T*	Clay Layer Thickness (ft)							
		0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0
		Height of Drainage (ft)							
		0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0
Time (days), $C_v = 31 \text{ ft}^2/\text{yr}$, Single-Sided Drainage									
50	0.20	0.6	2.3	5.3	9.4	15	21	29	38
60	0.29	0.9	3.4	7.7	14	21	31	42	55
70	0.41	1.2	4.8	11	19	30	43	58	76
80	0.56	1.7	6.6	15	26	41	60	81	110
90	0.83	2.4	9.7	22	39	61	88	120	160
95	1.12	3.3	13	30	53	82	120	160	210

*Trapezoidal "T" values after Hansen 1961.

Table 1: Time-Dependent Clay Consolidation Based on ASTM D2435 Testing

Based on a 30 inch clay liner thickness layer and laboratory testing, the clay sample was anticipated to require approximately 40-60 days to reach primary consolidation. The field test exposure plan called for real-time monitoring of the clay liner to determine the end-of-primary-consolidation. This would then initiate a post-consolidation waiting period of 30 days before consideration of load removal and subsequent geomembrane evaluation would begin.

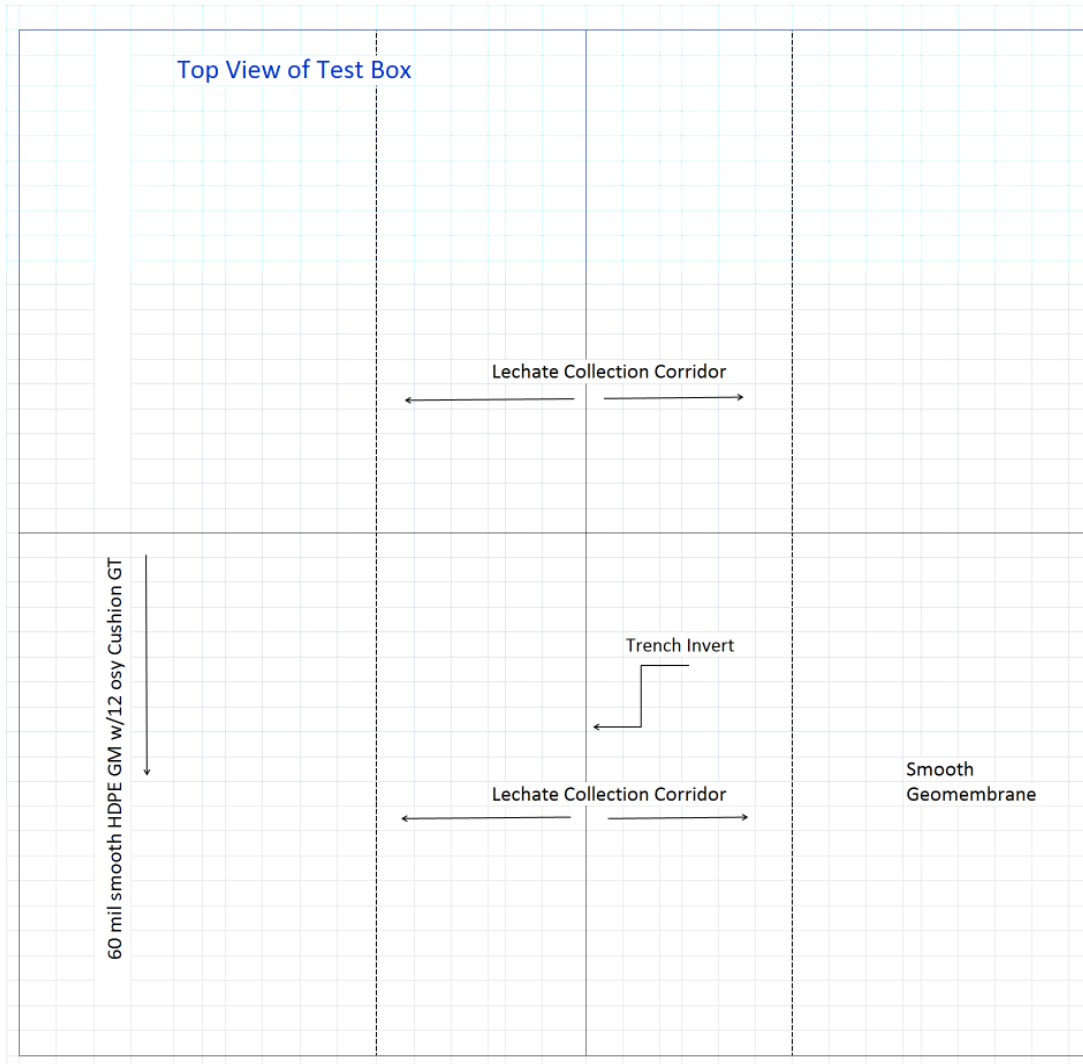


Figure 3: Test Plan (Top View)

The exposure plan for the geomembrane and cushion geotextile materials is presented in Figure 3. In addition, a 10 mil low-modulus aluminum alloy plate was included to record the localized geomembrane out-of-plane deformation under load. The geomembrane was also marked for any features that looked non-homogenous (gels, creases, scrapes). These same locations would be inspected carefully post-exposure.

Upon completion of the installation of all layers of the test profile, test materials and instrumentation, 1/6 of the total planned load was applied per 24 hour time period until a total surcharge load of 130 psi was achieved. Accordingly, the entire 130 psi compressive load was applied over 6 days. Continual monitoring of the clay liner was performed. Attachments to this report include photos of the construction of the test cell and post exposure inspection of the geomembrane.

TEST RESULTS:

Interpretation of the consolidation plate data was performed by TRI and APTIM Environmental & Infrastructure, LLC engineers to determine the conclusion of primary consolidation and initiation of a steady-state consolidation state. Figure 3 shows the consolidation plot of the compacted clay liner at the bottom of the Orchard Hills test box.

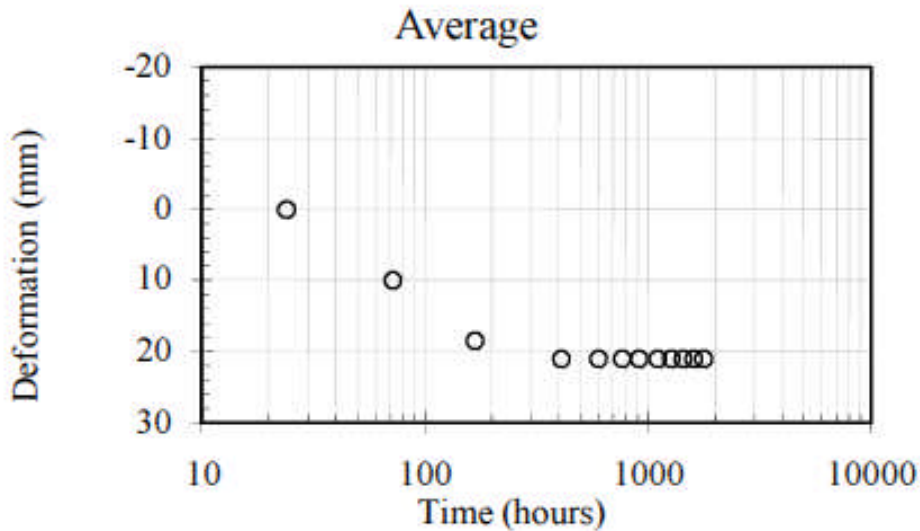


Figure 4: Consolidation of Orchard Hills Compacted Clay Liner

The first of six compressive loading blocks was loaded on July 23, 2018. The designed 130 psi surcharge load was achieved on July 30, 2018. This was maintained until December 3, 2018 at which time surcharge load was removed. The double lever loading system and overburden materials were then removed early December 4, 2018 leaving only the cushion geotextile, pipe and 1 inch of the leachate collection aggregate.

At 10:00 am on December 4, 2018, representatives of the client’s project team arrived to witness the geomembrane exhumation and geomembrane inspection. After removal of remaining aggregate and cushion geotextile, the geomembrane test panels were removed from the test box and laid to rest on the surrounding ground. Clean cotton cloths were used to wipe the geomembranes and remove the wet clay residuals concealing surface features.

After cleaning, the geomembranes were visually inspected by the project team with the following observations:

- i) The geomembrane did not show visible puncture.
- ii) Both the geomembrane panels showed out-of-plane deformation.
- iii) The very small defects (scrapes, scratches) noted on the as-installed geomembrane test panels did not demonstrate any unique features post exposure, that is, they did not correspond to increased out-of-plane deformation nor did they look thinner than adjacent locations.

Because not all punctures caused by aggregate are readily detected with the naked eye, after visual examination, TRI also performed a leak location test in accordance with ASTM D7953, *Standard Practice for Electrical Leak Location on Exposed Geomembranes Using the Arc Testing Method*. The principle of this electrical leak location method is to introduce a high voltage between a leak detection test probe and the conductive medium underneath the geomembrane, in this case, the moist ground. The geomembrane surface area is then swept with a test probe to locate points where the current completes the circuit through a leak. A visible electrical arc is formed when the current completes the circuit and the current flow is also converted into an alarm (audible, visual or other), which confirms leak detection and location. A test hole was made to confirm operating conditions, and the test confirmed that no ruptures, tears, pinholes or punctures resulted from exposure.

Finally, the three aluminum alloy plates that served to lock-in and preserve out-of-plane deformation under load, some of which would have been otherwise lost upon removal of loading, were scanned and mapped. This approach allows measurement of strain across a section of surface area of the geomembrane and provides a very accurate assessment of the maximum out-of-plane strain developed.

When the aluminum plate is mapped with a scanner, all points are assigned an x, y and z component based on their location (x and y) and their height (z). Because the monitoring sheet is initially flat prior to analysis, ie, $z=0$, the difference in height at each point (Δz) can be calculated post-analysis using the deformed height. To calculate the strain between any two points, the initial distance between them and the deformed distance between them is

determined. To calculate the initial distance, the distances between the central cell and each neighboring cell are determined as depicted below.

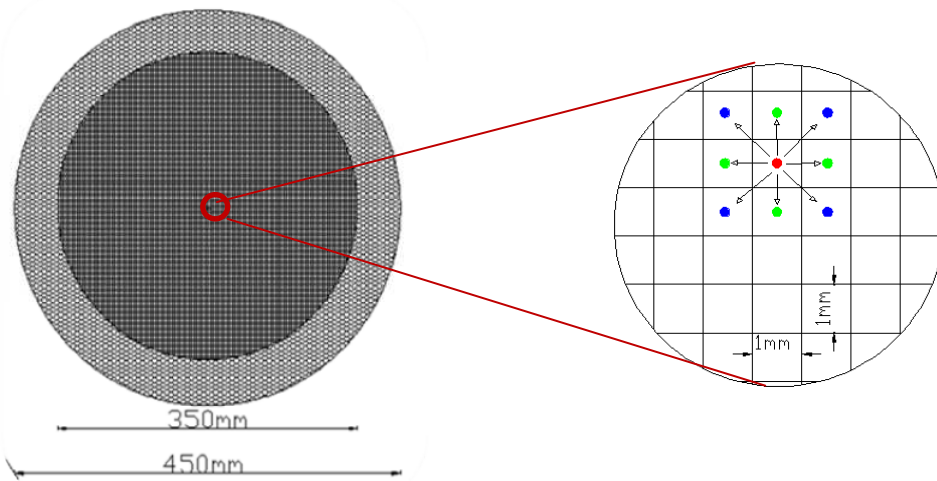
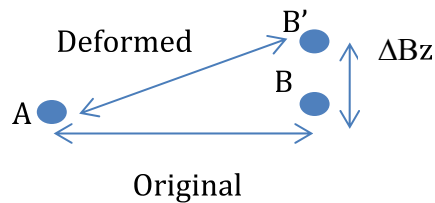


Figure 5: Determination of Out-of-Plane Strain

The distance between the red and green points is 1; however the distance between the red and blue points along a diagonal is $\sqrt{2}$. For every given location (except edges), a point will have 4 linear neighbors (green) and 4 diagonal neighbors (blue). To calculate the deformed distance between any two points, the Δz value of each point is determined using a laser measuring device.



To calculate the strain as a percentage of the original length;

$$\varepsilon (\%) = \left[\frac{\text{Deformed}}{\text{Original}} - 1 \right] \times 100$$

However, since the laser only calculates the Δz value and not the absolute length between any two points this is converted as follows;

$$\varepsilon (\%) = \left[\frac{\sqrt{\text{Original}^2 + \Delta Bz^2}}{\text{Original}} - 1 \right] \times 100$$

This is repeated for all 8 neighboring points. The connecting neighbor that has the highest strain value is recorded as the maximum value of strain for that cell while the other 7 points of information are discarded.

For this evaluation, a range of strain envelopes were assessed in 0.25% increments from 0.0% to 22.5%. The results presented represent the % strain that has occurred over the sample within the limits of the testing duration. The relative performance of the geotextile protection layer in conjunction with the leachate collection aggregate stone profile and deformation variances is provided below.

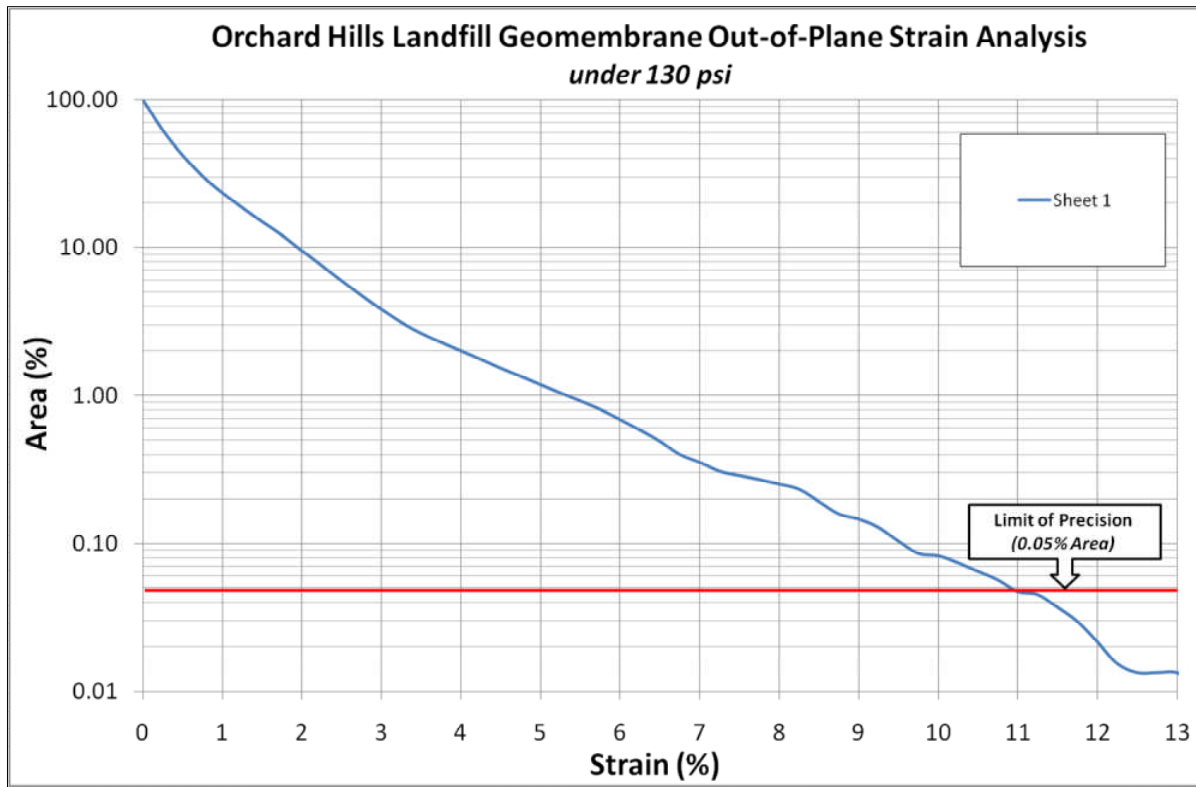
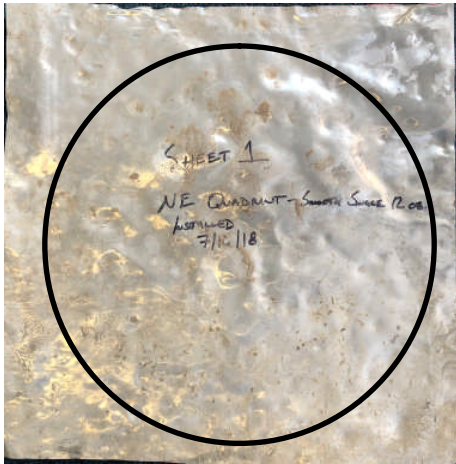
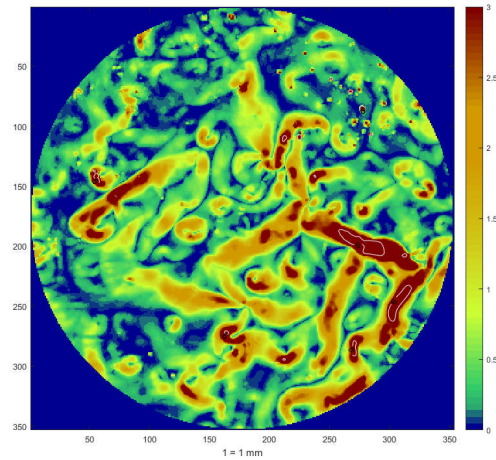


Figure 6: Smooth Geomembrane out-of-plane strain measurements



0.3mm Aluminium sheet



Strain Distribution

Sheet 1: beneath the cushion GT/smooth geomembrane

Figure 7: 3-dimensional mapping of out-of-plane strain measurements

Though the geomembrane test panels experienced out-of-plane strain, visual inspection did not indicate an associated high degree of, or significant, geomembrane thinning, even at the locations having the largest out-of-plane strain. The predicted geomembrane thinning, or thickness reduction over its service life, is covered under a separate report.

Attachments

B.1 One-Dimensional Consolidation Test Results
ASTM D2435, Method B

and

B.2 Photos of Test Cell Construction
Photos of Geomembrane Exumation



May 24, 2018

Tim Curry
Advanced Disposal Services, Inc. – Orchard Hills Landfill
8290 Highway 251
David Junction IL 61020
United States of America
Tim.curry@advanceddisposal.com

CC: Mark Sieracke <msieracke@wcgrp.com>
Jay Warzinski <jay.warzinski@advanceddisposal.com>
Chris Peters <Chris.Peters@Cornerstoneeg.com>

Subject: Laboratory Evaluation of the Clay Time Rate of Settlement
Project: Orchard Hills Landfill
Log #: 32368

Dear Mr. Curry,

TRI is pleased to submit the results of a laboratory evaluation of the time rate of settlement of the clay soil in support of field-scale puncture resistance testing for Orchard Hills Landfill. This report documents the laboratory testing conducted by TRI Environmental (TRI) and provides the expected time rate of settlement, magnitude of settlement, and proposed duration of the field test based on the results from laboratory testing and a baseline analysis with regards to the anticipated time-rate of settlement of the subject clay layer,

Based on the results of laboratory testing and a time rate of consolidation calculations for single sided drainage, TRI recommends a total compacted clay liner thickness of 2.5 feet in order that approximately ninety percent of settlement is achieved in a period of 60-88 days. A series of settlement plates will be utilized in the field installation in order to evaluate the time rate of settlement in the field.

We appreciate the opportunity to work with you on this project and look forward to providing additional services in the future. Please contact us if you have any questions, comments, or requests.

Sincerely,

A handwritten signature in black ink, appearing to read 'Jeffrey A. Kuhn'.

Jeffrey A. Kuhn, Ph.D., P.E.
Director of the Geotechnical Laboratory

A handwritten signature in black ink, appearing to read 'Sam R. Allen'.

Sam Allen
Vice President

1. INTRODUCTION/BACKGROUND

In preparation for the initiation of field-scale compression testing, an inquiry was placed by the design engineer with regards to the time rate of settlement of the compacted clay layer. Upon review of the 2003 permit modification response from STS Consultants, it was determined that laboratory-determined relationships between effective stress and settlement were utilized but that the coefficient of consolidation value utilized for time rate of settlement calculations did not originate from laboratory testing but rather from a reference source. In order that the time rate of settlement of the clay could be evaluated, TRI initiated one-dimensional consolidation testing of a specimen of remolded clay from the site. The testing procedure utilized and the measured consolidation properties are presented in this report. These properties are compared to those utilized by STS in their 2003 application and recommendations are made with respect to the duration of field-scale compression testing.

2. SAMPLE RECEIPT, HANDLING, AND STORAGE

Samples of site-specific aggregate and clay were provided to TRI in a shipment received on September 20th, 2017. The standard disposal of samples occurs thirty day following final reporting. TRI is currently holding samples until, at a minimum, the field-scale compression testing has been completed in order that any additional supporting geotechnical laboratory testing might be performed upon request.

3. TEST SETUP AND PROCEDURE

Material Conditioning/Processing

A 2.5 inch diameter consolidation cell was used in combination with a stepper-motor equipped load frame in order to perform one-dimensional consolidation testing in accordance with ASTM D2435 procedures. In preparation for remolding, the soil was air dried and passed through a No. 8 sieve to eliminate any over sized particles. The soil was then moisture conditioned, allowed to equilibrate, and then adjusted according to the target gravimetric moisture content based on an oven dried moisture content. The moisture conditioned site-specific clay was then remolded to 90 percent of the maximum dry density and 3.5 percent wet of the optimum water content as determined by ASTM D1557 modified Proctor testing. These are the same placement conditions utilized in Bench-Scale Hydrostatic Puncture testing as reported on 12/5/2017.

Consolidation Testing

Following remolding, the specimen was tested in accordance with ASTM D2435 test method B which utilized 24 hour increments between loads. The test was performed with a stepper-motor equipped load frame equipped with a load cell. Following placement of a seating load, the specimen was inundated and the height of the specimen was fixed. A swell

pressure of less than 100 psf was measured following inundation. The test was then advanced utilizing a loading increment ratio of two and an unload-reload cycle (ie. Rebound cycle, hysteresis loop) was performed at a stress of 16,000 psf utilizing a loading increment ratio of four (twice as large as that used in load application). A maximum applied stress of 64,000 psf was utilized for testing. Following application of the maximum applied stress, final unloading of the specimen was performed.

4. POST-TEST EVALUATION

Magnitude of Settlement

Tabular values and a plot of the void ratio versus the log of the effective stress are shown in Figure 4. The Casagrande Construction technique, includes utilizes the void ratio – effective stress, $e\text{-log}(\sigma')$ curve in semi-log space to calculate a preconsolidation pressure of 8,600 psf through a series of geometric constructions (see ASTM D2435 report for details). Other semi-log-linear constructions included the use of the reloading portion of the unload-reload cycle to determine a recompression index of 0.010 and the virgin compression portion of the $e\text{-log}(\sigma')$ curve was utilized to determine a recompression index of 0.079.

σ'_v (psf)	e (-)	Strain, ϵ (%)
Initial	0.448	0.0
250	0.413	2.4
500	0.405	2.9
1,000	0.392	3.8
2,000	0.380	4.7
4,000	0.368	5.5
8,000	0.353	6.5
16,000	0.336	7.7
8,000	0.337	7.7
4,000	0.340	7.5
8,000	0.338	7.6
16,000	0.334	7.9
32,000	0.315	9.2
64,000	0.291	10.8
16,000	0.294	10.6
4,000	0.305	9.9

Table 1 - Void Ratio vs. Effective Stress

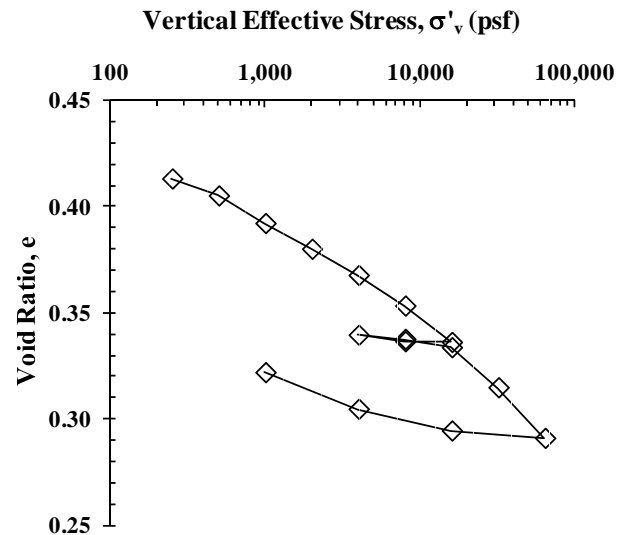


Figure 1 –Void Ratio vs. Effective Stress

Time Rate of Settlement

The time rate of settlement of the clay was evaluated via the square root of time and log-time methods. The measure relationship between the coefficient of consolidation and effective stress is presented in tabular and graphical form in Table 2 and Figure 2, respectively. The log-time method was only utilized for an effective stress of 32,000 psf where a clear inflection / t_{100} could be determined. The root-time method was used for normal stresses in which a clear root-time linear relationship was present. Time-settlement curves are available in the stand-alone separate consolidation report transmitted at the time of submission of this report.

σ'_v (psf)	C_v (ft ² /year)	
	Log Time	Root Time
Initial	-	-
250	-	-
500	-	-
1,000	-	68
2,000	-	40
4,000	-	36
8,000	-	18
16,000	-	26
8,000	-	-
4,000	-	-
8,000	-	13
16,000	-	31
32,000	15	24
64,000	-	29
16,000	-	-
4,000	-	-

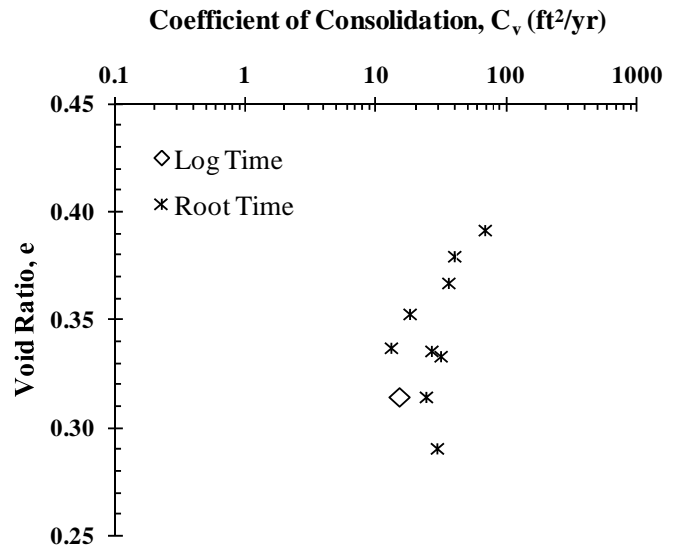


Table 2 - Coefficient of Consolidation vs. Effective Stress

Figure 2 – Coefficient of Consolidation vs. Effective Stress

Regarding Partially Saturated Soils

Please note that consolidation testing was performed on a specimen which was submerged during testing in accordance with ASTM D2435. The results of this testing will be used to estimate the time rate of consolidation for field testing but do not directly evaluate the time rate of consolidation of a partially saturated soil. This will be best achieved in the field through the use of field instrumentation (e.g. settlement plates).

5. COMPARISON WITH ASTM D5514C TESTING AND 2002 PERMIT DATA

ASTM D5514C Post-Test Dry Densities and Saturations

Bench-Scale Puncture Resistance testing, as reported on 12/5/2017, included the application of a 130 psi load for a time period of ten days. At the end of ten days, a thin-walled sampling tube was pushed through the center of the compacted clay as a means of evaluating the post-test dry density. A plot of the measured dry densities is presented in Figure 3. A semi-log interpolation of the results from one-dimensional consolidation tests results in a calculated void ratio of 0.330 which corresponds to a dry density of 129.0 pcf assuming a specific gravity 2.75. Despite consolidation testing being conducted on an inundated soil specimen and hydrostatic compression testing being conducted on a partially-saturated material / non-inundated, the post test dry densities indicate that the upper portion of the clay had reached a dry density of approximately 129 pcf. In order to further evaluate the post-test measurements from hydrostatic compression testing, the saturation of the material can be evaluated through assuming a specific gravity value. A plot of the calculated saturation of the post-test material is presented in Figure 4. Please note that two of the nine calculated values of saturation were greater than 100 percent given the assumed specific gravity value of 100 percent. Albeit the compression and consolidation of the field test profile is not being performed in a flooded environment, the planned applied stress is high enough to bring the clay liner to a state of approximate saturation.

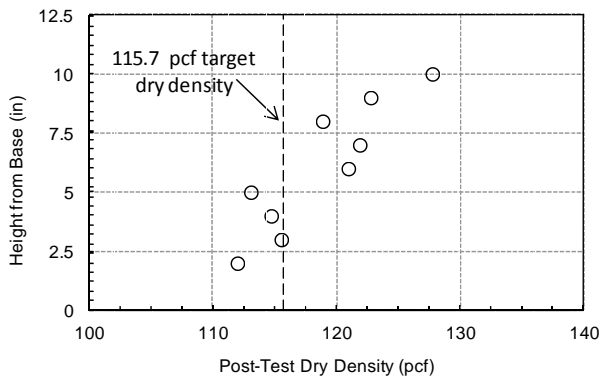


Figure 3 – Bench-Scale Puncture Resistance Testing:
Post-Test Dry Density

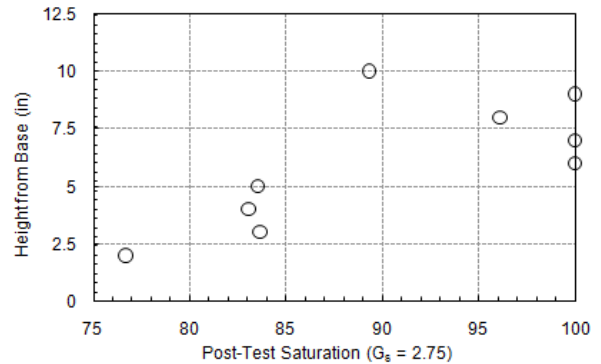


Figure 4 – Bench-Scale Puncture Resistance Testing:
Post-Test Saturation

Consolidation Testing

Tabular consolidation properties utilized by STS are compared to those determined from laboratory testing in Table 3. The laboratory determined values of the preconsolidation pressure, compression index, and recompression index were in good agreement between 2003 and current testing. The 2003 coefficient of consolidation utilized in time rate of settlement calculations, and obtained via a reference, was, however, approximately twice as large as the coefficient of consolidation as determined from laboratory testing under the 8,000 to 32,000 psf load increment and was comparable to the value obtained for the 500 to 1,000 psf load increment.

Table 3 – 2003 STS Values and 2018 Laboratory-Determined Values

	Preconsolidation Pressure	C _c	C _r	C _v
	-	-	-	ft ² /year
STS Consultants (2003)	≈ 10,400 to 12,400	0.07	0.014	55
TRI ASTM D2435 Testing (TRI Log #32368)	≈ 8,600	0.079	0.01	31

6. IMPLICATIONS ON AND RECOMMENDATIONS FOR FIELD TESTING

The calculated time requirements with single-sided drainage to achieve varying degrees of consolidation for different clay layer thickness are presented in Table 4 utilizing a coefficient of consolidation of 31 ft²/yr. The estimated time frame for 90 percent consolidation of a 2.5 thick section of liner with single-sided drainage is 61 days.

Table 4 – Time - Degree of Consolidation Dependency, Single-Sided Drainage

Average Degree of Consolidation (%)	T*	Clay Layer Thickness (ft)							
		0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0
		Height of Drainage (ft)							
		0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0
		Time (days), C _v = 31 ft ² /yr, Single-Sided Drainage							
50	0.20	0.6	2.3	5.3	9.4	15	21	29	38
60	0.29	0.9	3.4	7.7	14	21	31	42	55
70	0.41	1.2	4.8	11	19	30	43	58	76
80	0.56	1.7	6.6	15	26	41	60	81	110
90	0.83	2.4	9.7	22	39	61	88	120	160
95	1.12	3.3	13	30	53	82	120	160	210

*Trapezoidal "T" values after Hansen 1961.

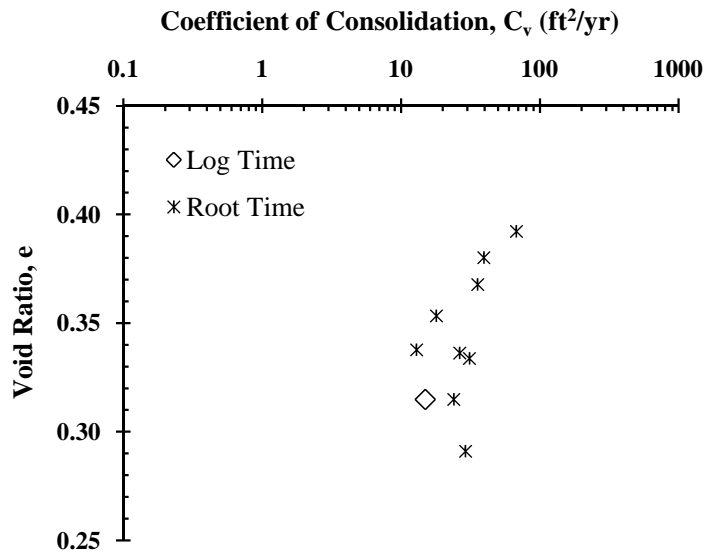
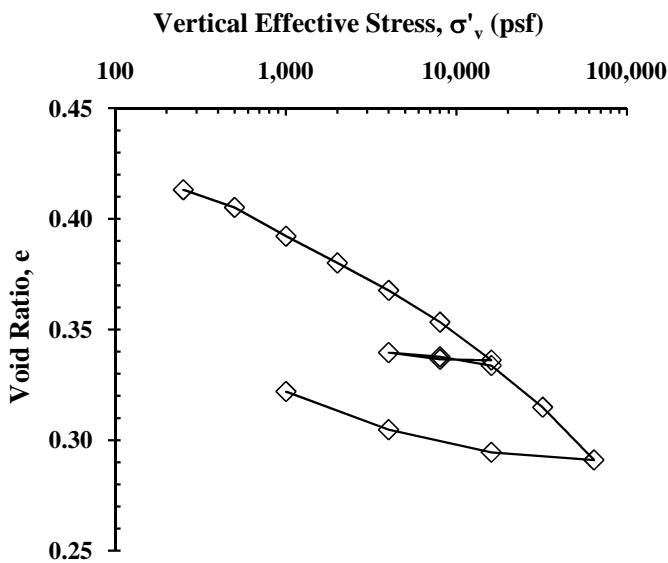
One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B

Soil Specimen Properties	
Initial Specimen Water Content (%)	14.8
Final Specimen Water Content (%)	13.2
Specimen Diameter (in)	2.495
Initial Specimen Height (in)	0.998
Final Specimen Height (in)	0.928
Final Differential Height (in)	0.069
Initial Dry Unit Weight, γ_o lb _f /ft ³	114.2
Final Dry Unit Weight, γ_f lb _f /ft ³	125.1
Initial Void Ratio, e_o	0.448
Final Void Ratio, e_f	0.322
Initial Degree of Saturation (%)	87.4
Preconsolidation Pressure (psf)	≈8600
Swell Pressure (psf), Maximum Measured	<100
Compression Index, C_c	0.079
Recompression Index, C_r	0.010

σ'_v (psf)	e	Strain, ϵ (%)	C_v (ft ² /year)	
			Log Time	Root Time
Initial	0.448	0.0	-	-
250	0.413	2.4	-	-
500	0.405	2.9	-	-
1,000	0.392	3.8	-	68
2,000	0.380	4.7	-	40
4,000	0.368	5.5	-	36
8,000	0.353	6.5	-	18
16,000	0.336	7.7	-	26
8,000	0.337	7.7	-	-
4,000	0.340	7.5	-	-
8,000	0.338	7.6	-	13
16,000	0.334	7.9	-	31
32,000	0.315	9.2	15	24
64,000	0.291	10.8	-	29
16,000	0.294	10.6	-	-
4,000	0.305	9.9	-	-



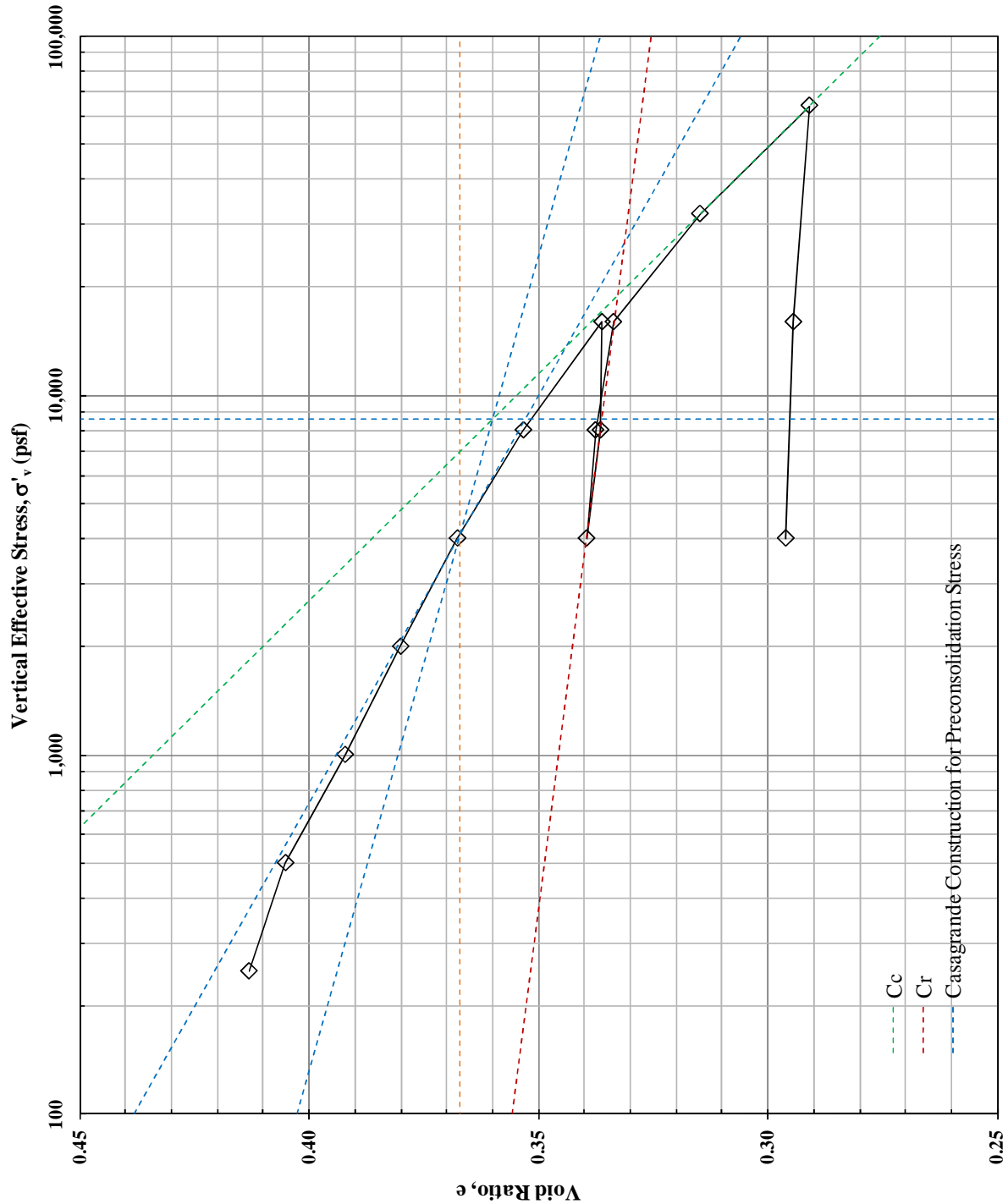
Note: The soil was air dried and passed through a No. 8 sieve to eliminate any over sized particles. The soil was moisture conditioned, allowed to equilibrate, and then adjusted according to the target gravimetric moisture content based on an oven dried moisture content. The specimen was then remolded into a known volume to achieve the target density. A specific gravity of 2.75 was assumed for weight-volume calculations. Calculations include machine deflections measured at each loading step. The preconsolidation pressure was determined using the Casagrande construction technique.

Jeffrey A. Kuhn, Ph.D., P.E., 5/24/2018
 Quality Review/Date

One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B

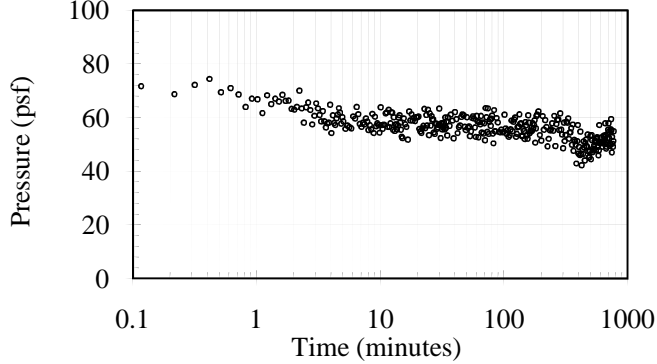


One-Dimensional Consolidation Properties of Soil

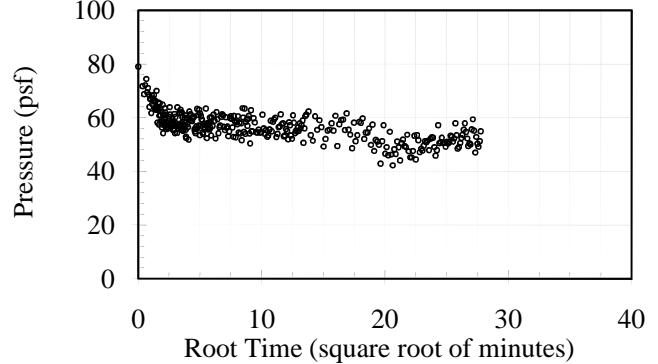
Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B

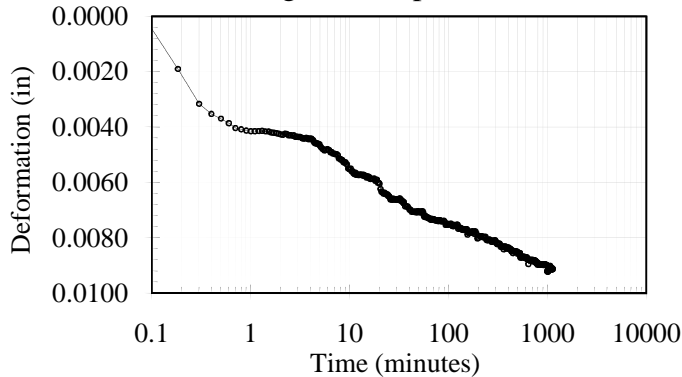
Stage 1: Swell Pressure Measurement



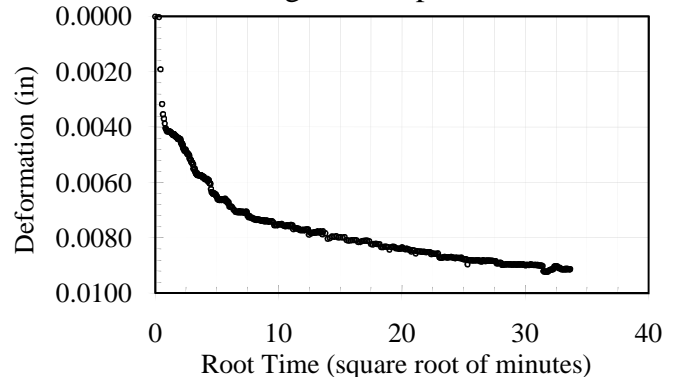
Stage 1: Swell Pressure Measurement



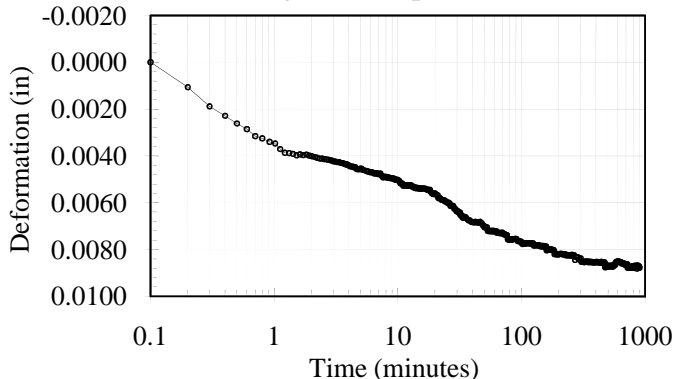
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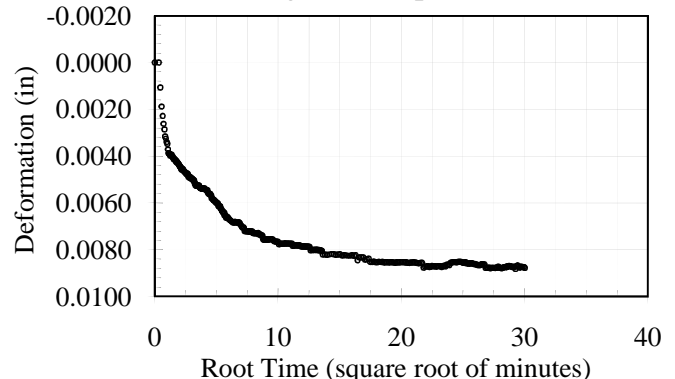
Stage 2: 250 psf



Stage 3: 500 psf



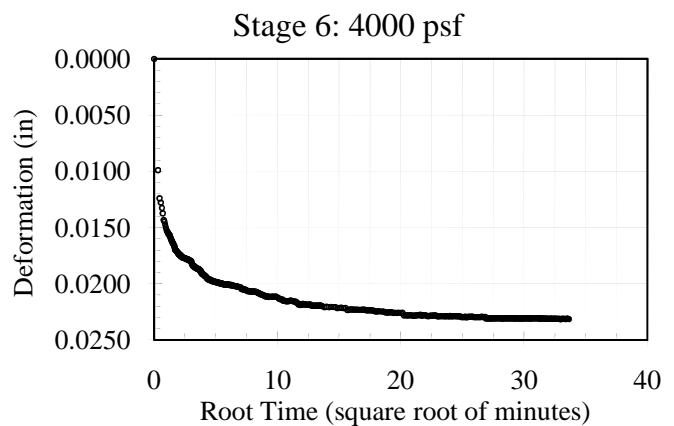
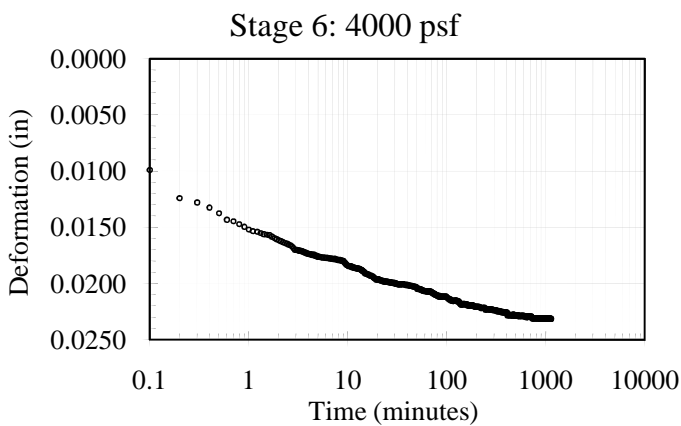
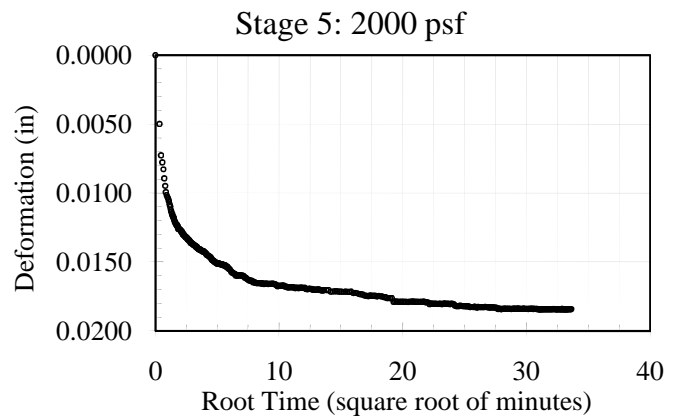
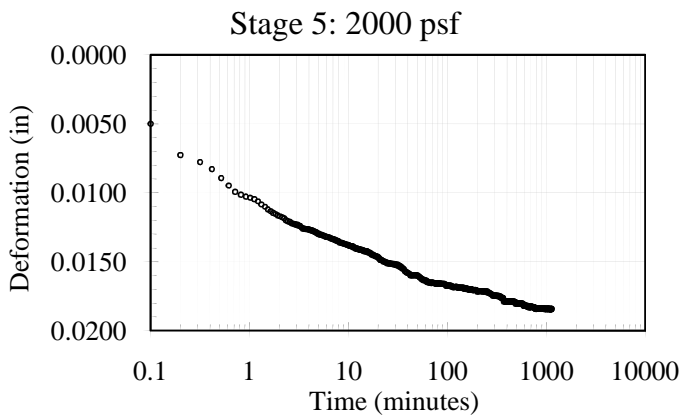
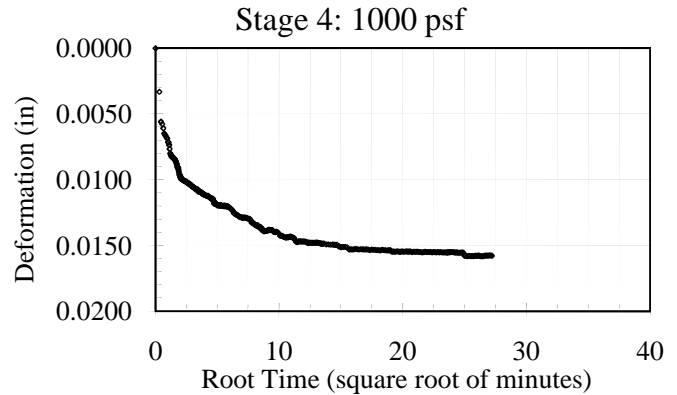
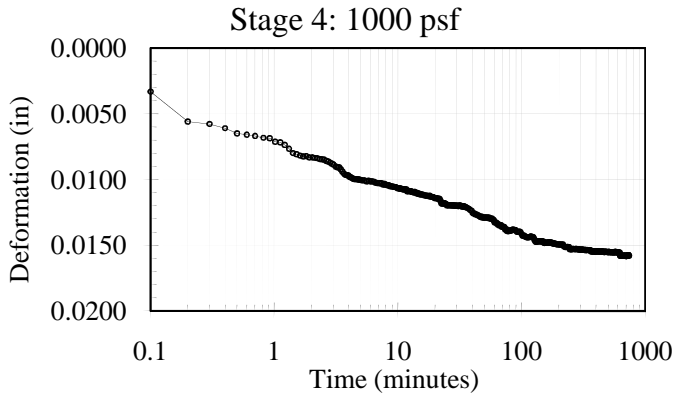
Stage 3: 500 psf



One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

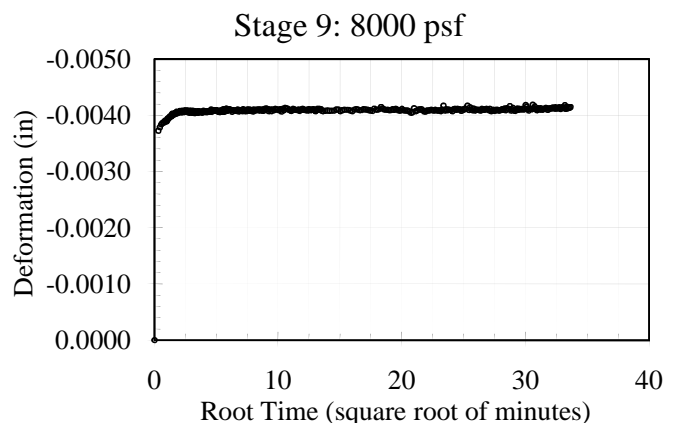
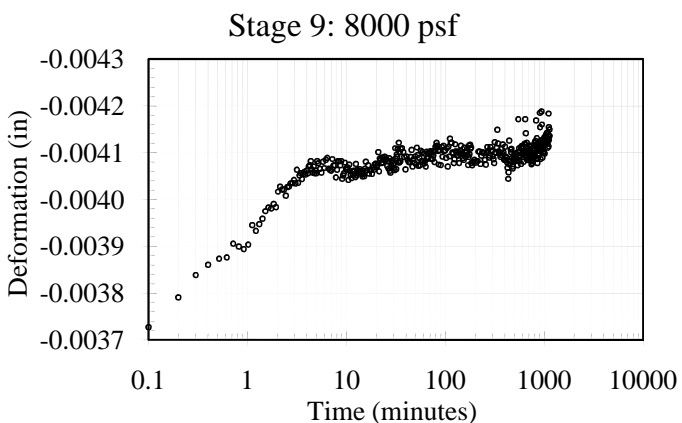
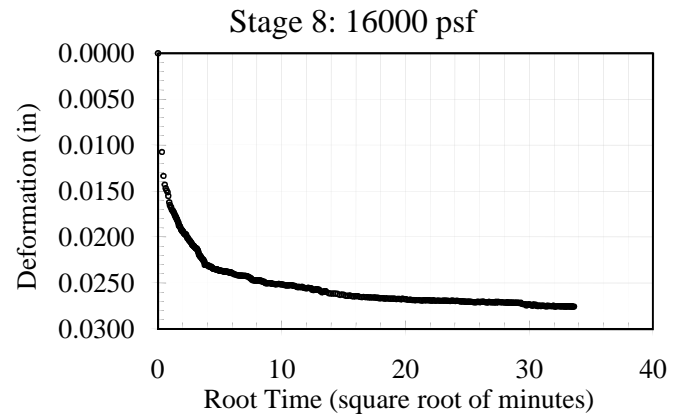
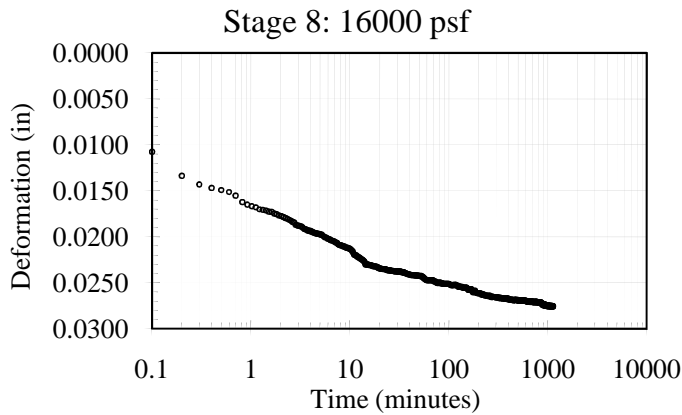
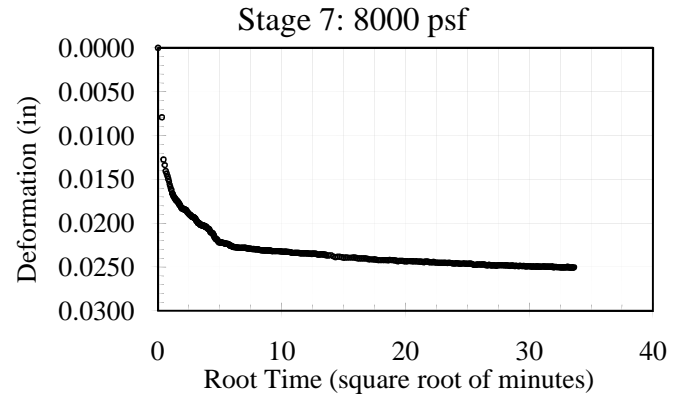
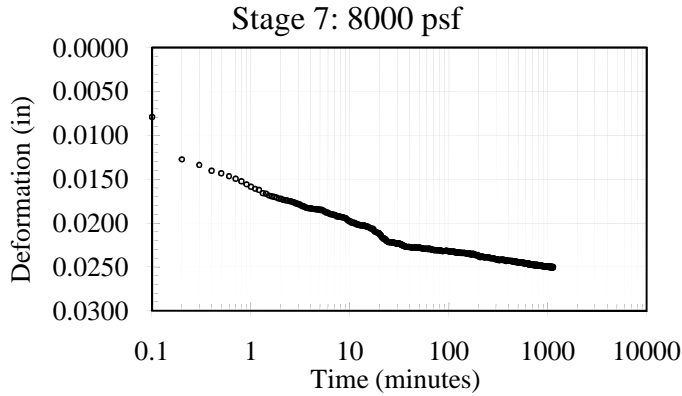
TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B



One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

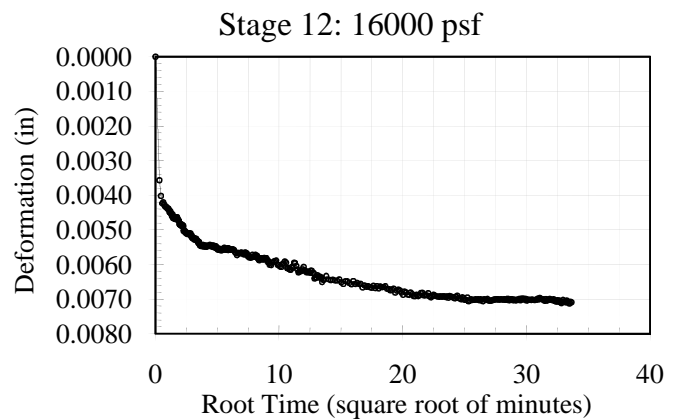
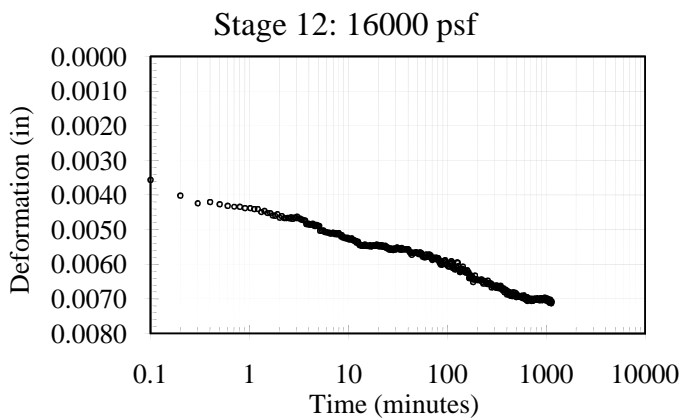
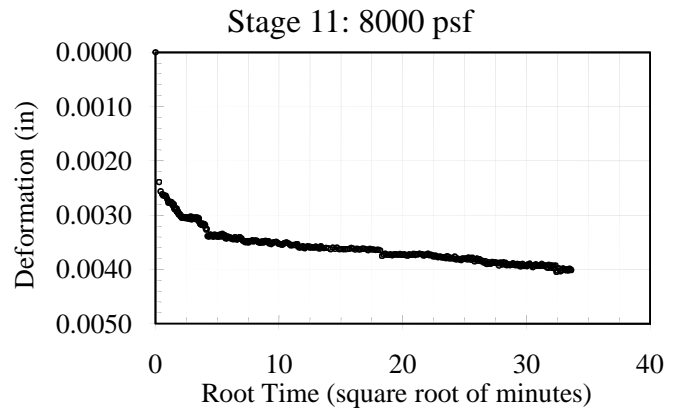
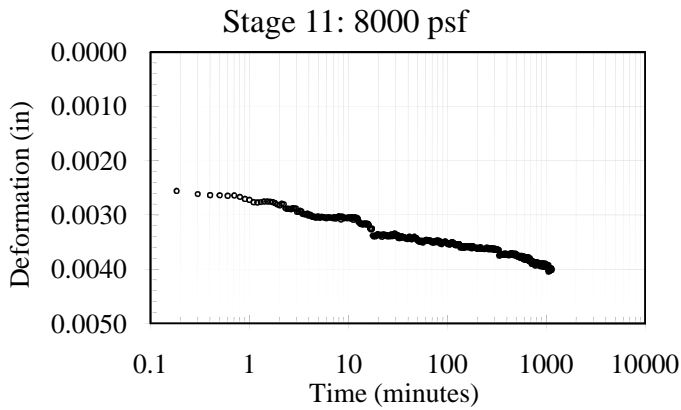
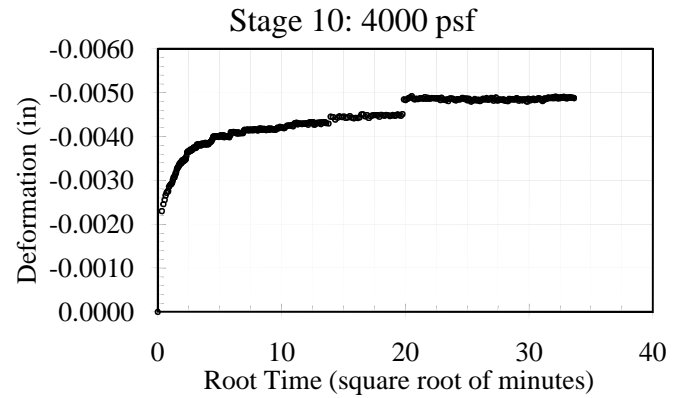
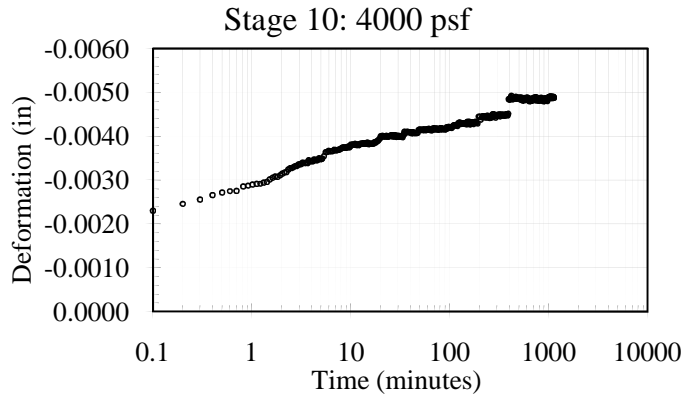
TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B



One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

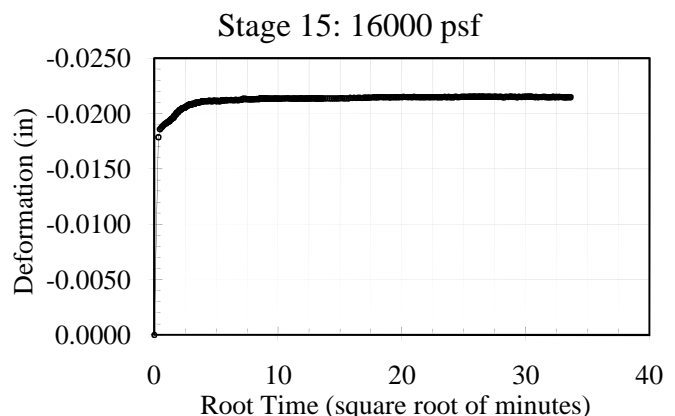
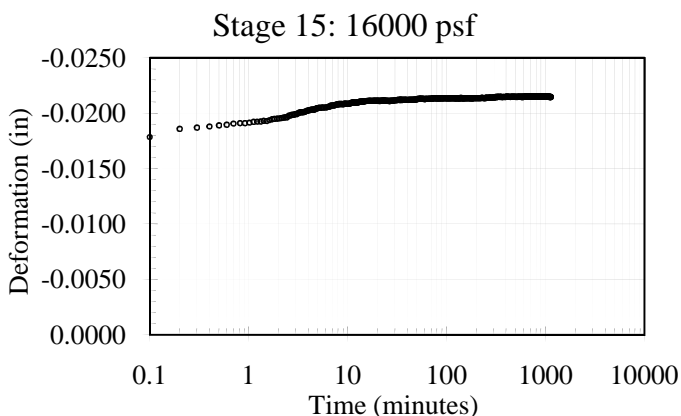
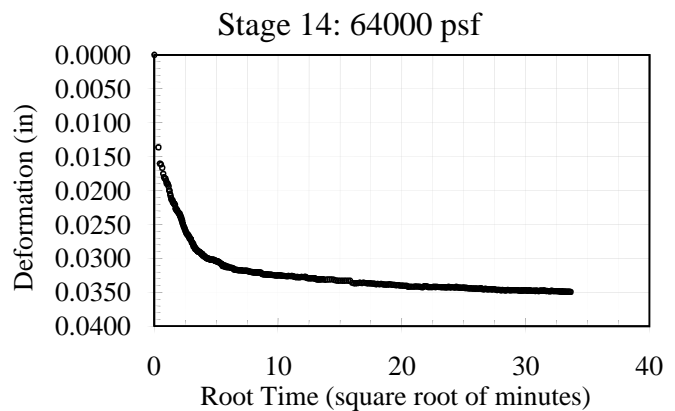
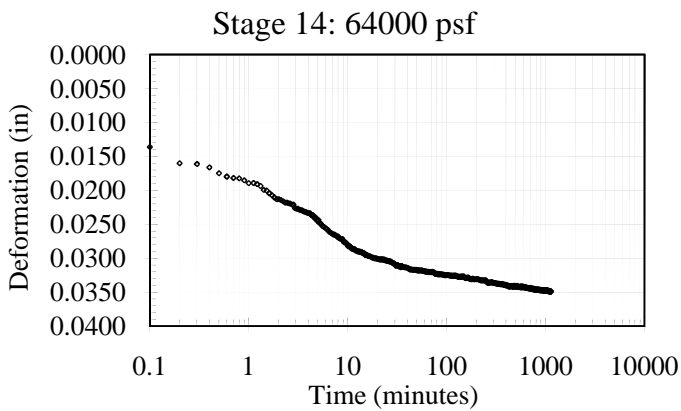
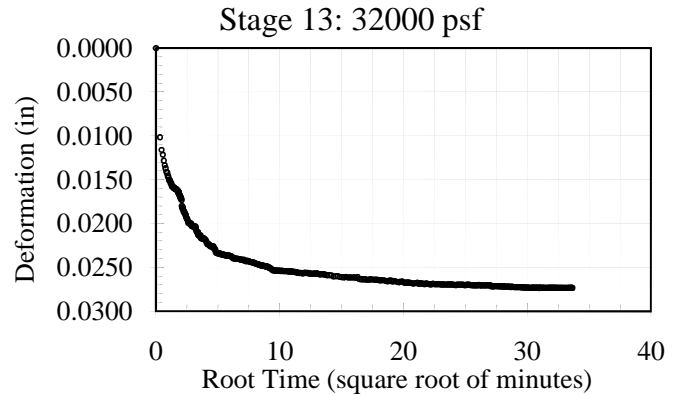
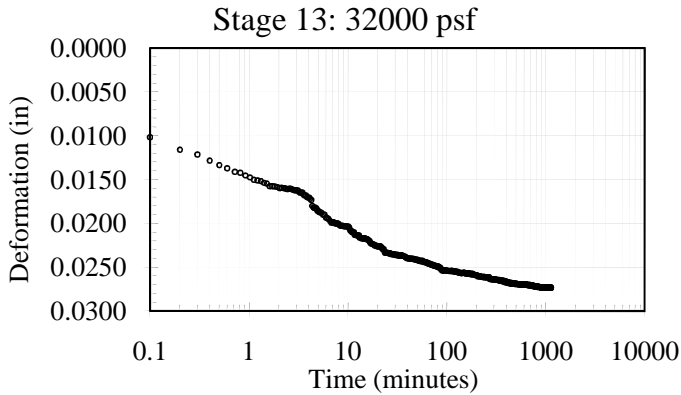
TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B



One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

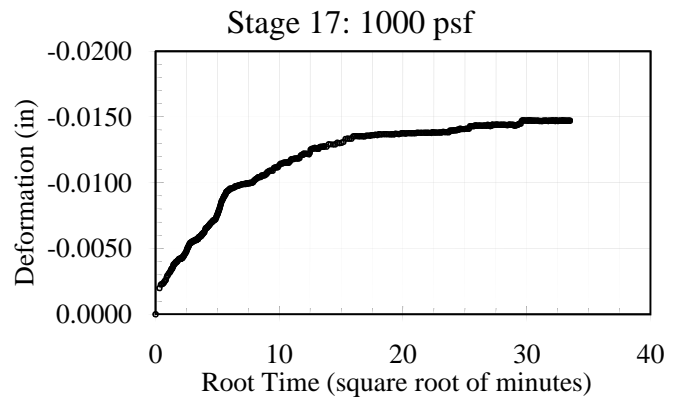
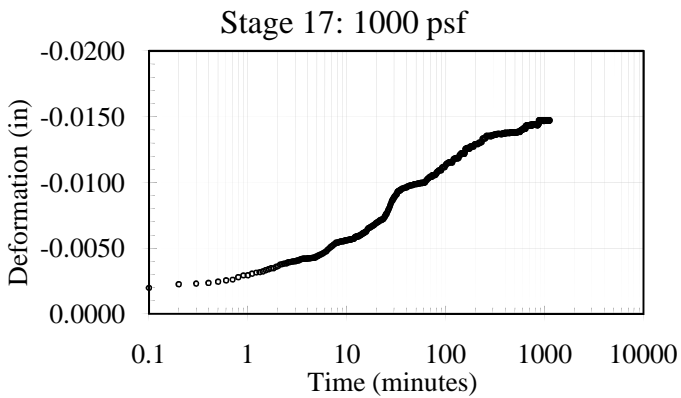
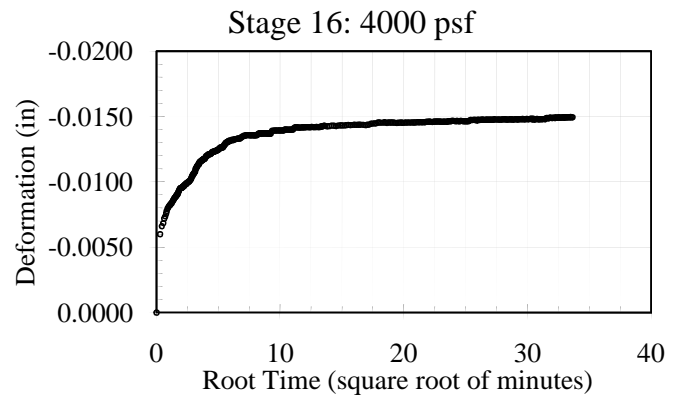
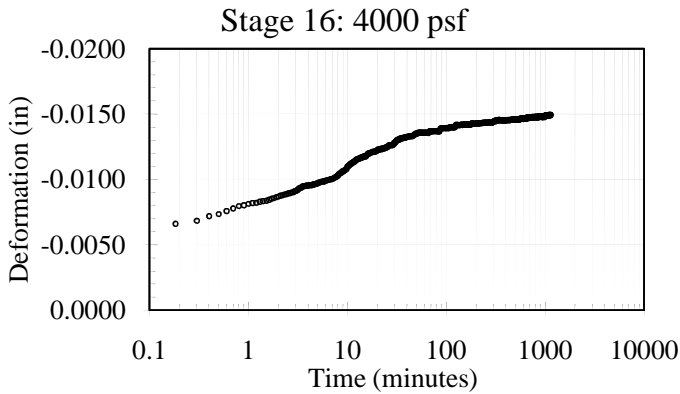
TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B



One-Dimensional Consolidation Properties of Soil

Client: Advanced Disposal Services
 Project: Orchard Hills Landfill
 Specimen: Clay

TRI Log No.: 32368.2
 Test Method: ASTM D 2435, Method B



ADS Large Scale Puncture Resistance Study

Representative Cell Construction
Pictures













Placement of granular drainage layer



Placement of 6 ounce per square yard filter fabric





ADS Large Scale Puncture Resistance Study

Representative Geomembrane
Exhuming / Inspection Pictures





Removal of nonwoven cushion geotextile



Exposed geomembrane panels

12/04/2016 10:00





Exhumed geomembrane panels



Electrical (arc) leak location testing of exhumed geomembrane panels

12/04/2018 10:53



Appendix C

Accelerated Bench-Scale Puncture Resistance and Geomembrane Compressive Creep Testing ASTM D7361, Modified (using site-specific sample profile)

26 February 2019

Timothy D. Curry

Advanced Disposal

232 Vance Road, Suite 208

Valley Park, Missouri 63088

Phone: 636 529 1974

tim.curry@advanceddisposal.com

RE: Geomembrane Protection Study: Accelerated “Bench Scale” Puncture Resistance Testing

Dear Mr. Curry,

Thank you for consulting TRI Environmental, Inc. (TRI) for your geosynthetics testing and research needs. TRI is pleased to submit this final report for laboratory testing. This report details accelerated testing to determine the affect of long-term loading of a geomembrane lined leachate collection trench system. Specifically, the impact on the puncture resistance of the HDPE geomembrane barrier in the system was of interest.

All testing was intended to evaluate the geomembrane puncture resistance based on site specific materials from Orchard Hills landfill (detailed in TRI report 32368) which is owned and operated by Advanced Disposal. Please find below details of the tests and analysis carried out.

Accelerated “Bench Scale” Puncture Resistance Testing

In order to determine the time-dependent loss of geomembrane thickness and potential for puncture of the geomembrane in the Orchard Hills leachate collection trench, a series of compression creep tests were performed using ASTM D 7361, *Standard Test Method for Accelerated Compressive Creep of Geosynthetic Materials Based on Time-Temperature Superposition Using the Stepped Isothermal Method*. In simple terms, the stepped isotherm method (SIM) test, developed by TRI and standardized through ASTM International, is a way to quickly perform what would otherwise be an exhaustive, very long test effort by using a much faster methodology of accelerated testing. The test involved taking a single test

specimen, or in this case, a test profile, and loading it at a starting or initial temperature while monitoring it for thickness change. At some test duration, the load is kept constant but the temperature is changed to affect a new test environment. This creates a new rate of thickness change, or creep response, which in turn is measured and recorded. This same procedure is followed for several more temperature steps, each generating a unique compression creep response specific to the new test temperature.

The resulting complete thickness vs time data is collected and then shifted based on time-temperature-superposition (TTS) theory. This involves first re-plotting the strain (or thickness) vs time data for each temperature step in log time. The strain data is then shifted in log time until the creep rate observed at the very end of any individual temperature step matches the creep rate at the next, adjacent temperature step. This slope matching exercise results in a single master curve that can be expressed at a single reference temperature, in this case, 20°C. Instead of performing numerous creep tests, each at a single temperature for subsequent TTS shifting, a SIM test serves to visit a portion of each creep curve using a single specimen. This testing technology is now used throughout the geosynthetic reinforcement and geosynthetic drainage industries,.

The SIM test performed for this project was modified using a test box to hold representative site materials to be included with the geomembrane in the evaluation. Specifically the leachate collection rock was used in the test to present the puncture challenge to the geomembrane/cushion geotextile composite test specimen. A smooth geomembrane material was used to represent the most conservative, puncture challenged, case.

The first effort in this test was to establish the relationship between temperature steps and shifted time to determine what acceleration could be achieved at each elevated temperature step. For this effort a geomembrane was tested between two steel loading platens to determine the temperature response profile of the geomembrane and to determine the highest test temperature subsequent tests could accommodate.

Next, a compressive SIM test was performed on a profile consisting of a loading platen/neoprene (to represent clay)/HDPE geomembrane/nonwoven geotextile/ drainage

rock/platen). The test was performed to include five temperature steps equating to, in shifted time, 2,958,012 hours (337 years). After each SIM test, geomembrane thickness (5 readings) was measured to determine the maximum deformation resulting from the test. Additional SIM tests then conducted but were terminated after 2, 3 and 4 temperature steps which equated to shifted times of 37, 489 and 7499 hours (0.004, 0.06 and 0.86 years). Five geomembrane thickness measurements were recorded each time at the minimum thickness location taken at the end of each test. In this way a plot of minimum thickness vs time was developed and a loss of thickness per unit time could be articulated. All thickness measurements secured post testing were performed immediately upon test disassembly or within approximately 45 seconds of load removal.

Test Results:

The smooth HDPE geomembrane material was used in the testing as this presented the most conservative case (most susceptible to out-of-plane strain and perhaps time-dependent thinning to rupture. SIM testing was performed under a compressive load of 130 psi. As referenced earlier the first effort was to establish the relationship between temperature steps and shifted time. The results of this test are presented in Figure 1.

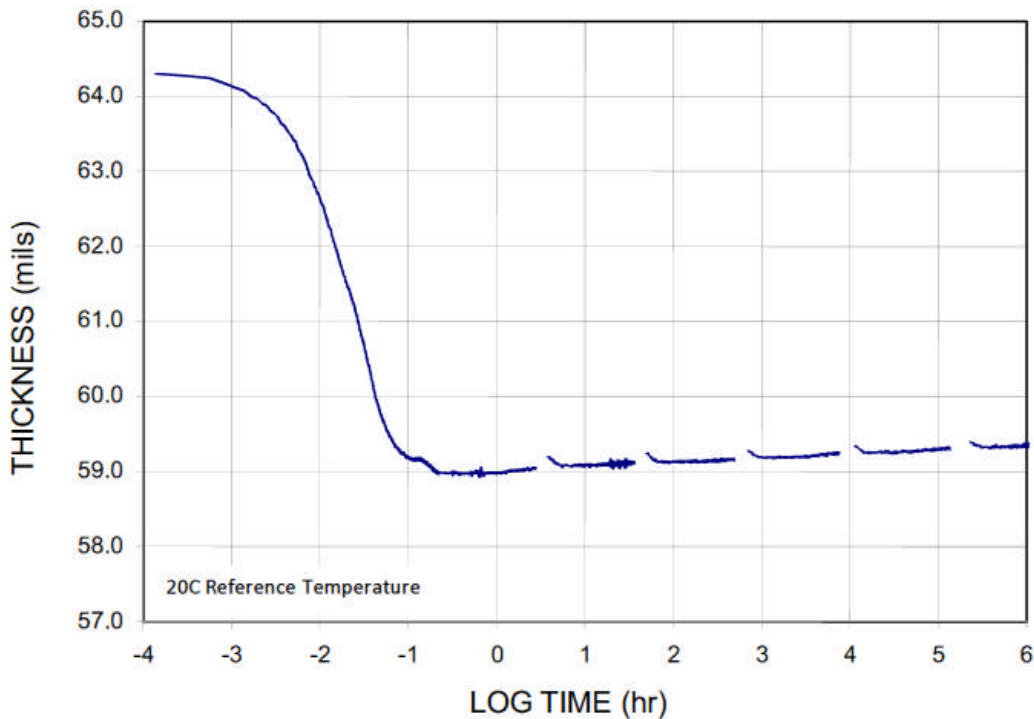


Figure 1: Compressive creep curve of geomembrane thickness



This plot shows a time-temperature shifted data plot of thickness vs log time. It also shows that a reference temperature of 20° C was used and that five temperature steps were employed during testing after test initiation at 20°C. The slight “increase” in thickness between Log 0 and Log 6 hours is believed to be related to test system expansion during testing. The time “reach” of this test via TTS shifting of the five temperature steps was over 300 years. As the test was performed between two heated platens no out-of-plane deformation was observed in the geomembrane during post-test inspection. The next SIM test was performed on an inverted representative profile of the leachate collection trench materials. The geomembrane, leachate collection gravel and nonwoven cushion geotextile materials were all site specific. The clay was represented by a closed cell soft neoprene pad.



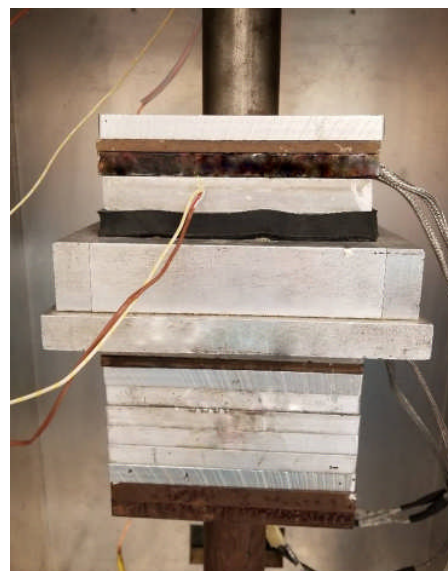
Leachate collection aggregate in SIM test box



Cushion GT over aggregate



Neoprene Pad to simulate clay



Full test set-up with materials inside box

Figure 2: SIM test set-up with site materials and synthetic clay (neoprene pad)



Cushion GT after SIM test



Geomembrane after SIM test

Figure 3: Post test geotextile and geomembrane

The geomembrane again demonstrated out-of-plane strain, however, the thickness of the geomembrane, after losing some thickness upon loading, reached steady state. Specific test results are as follows.

Log Time (hr)	Shifted Time (hr)	Shifted time (yrs)	Thickness (mils)					Average	Min
-3	0	0	68.6	67.6	67.7	66.9	67.6	67.7	66.9
1.564	37	0.004	59.0	62.6	61.0	63.1	64.2	62.0	59.0
2.689	489	0.06	58.5	61.6	63.2	63.3	60.7	61.5	58.5
3.875	7499	0.86	58.0	60.5	63.1	63.2	61.2	61.2	58.0
6.471	2958012	337	57.7	62.2	63.0	62.9	63.8	61.9	57.7

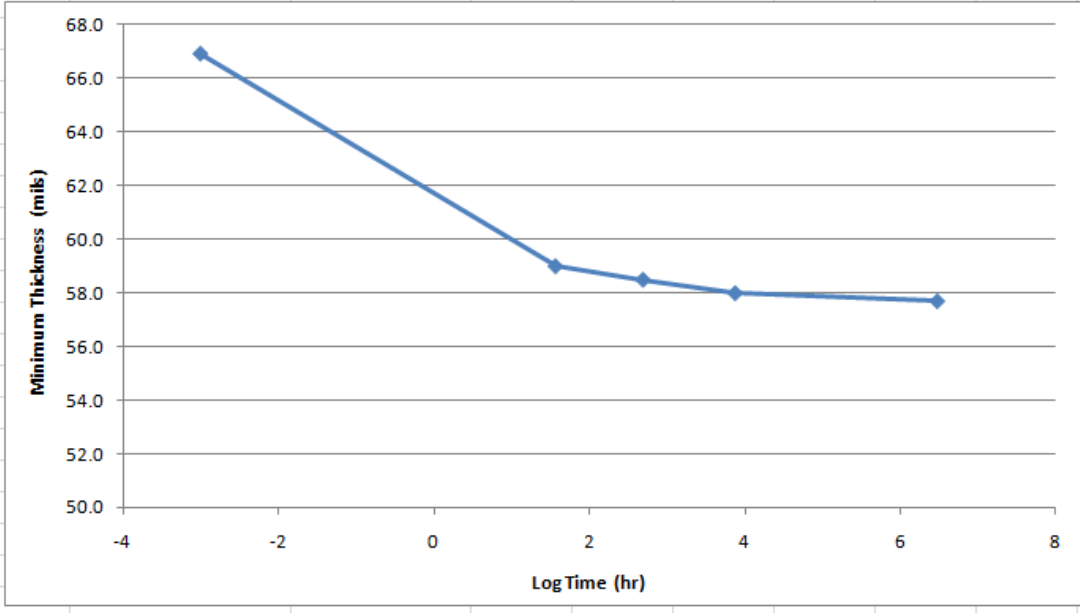


Figure 4: Geomembrane thickness reduction vs log time for site-profile SIM tests.

The thicknesses recorded were measured at the locations of highest out-of-plane deformation. Note that the first data point in the figure 4 graph at log time -3 along the x-axis represents initial pre-test geomembrane thickness. The last 4 data points are minimum thickness measurements recorded during post test inspection of the geomembrane as described above.

Conclusion

The SIM tests performed indicate the geomembrane thickness at the location of maximum deformation lost a total of 0.092 inches of thickness over 2,958,012 hours ($10^{6.471} = 337$ years) shifted time, or 2.73×10^{-4} inches per year under 130 psi loading in the test profile.



TRI is pleased to be of service to ADS Waste and trusts the reporting herein is useful for liner system evaluations. Please contact me if you have any questions or if I may provide any additional information.

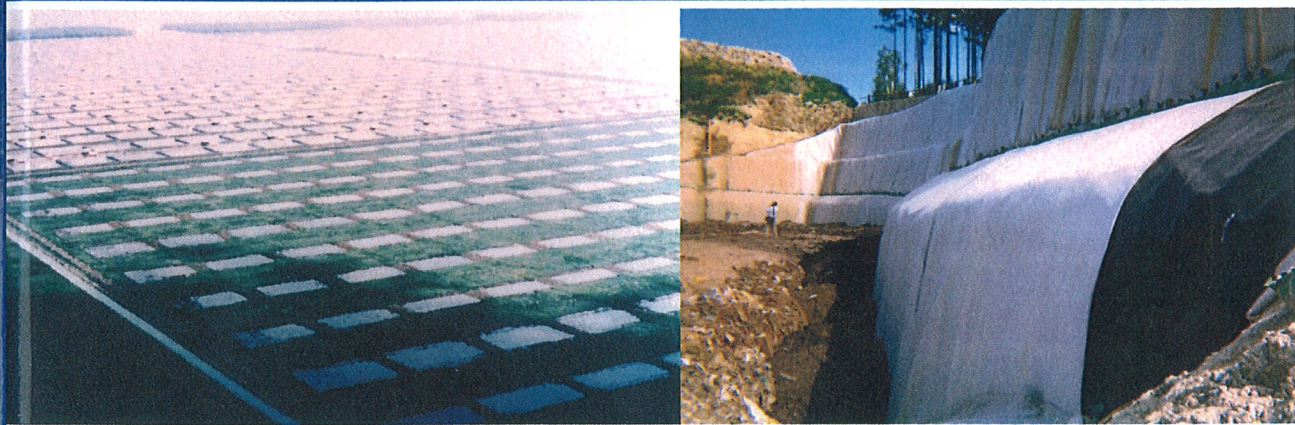
Sincerely,

A handwritten signature in black ink that reads "Sam R. Allen". The signature is written in a cursive, slightly slanted style.

Sam Allen
Vice President

DESIGNING WITH GEOSYNTHETICS

FIFTH EDITION



ROBERT M. KOERNER

022

- b = width of stone void, and
- y = deformation into stone void.

Example 2.9

Given a truck with 700 kPa tire inflation pressure on a stone base course consisting of 50 mm maximum-sized stone with a geotextile beneath it, calculate (a) the required grab tensile stress on the geotextile, and (b) the factor of safety for a geotextile whose maximum grab strength is 500 N with cumulative reduction factors of 2.5. Use a value of $f(\epsilon) = 0.52$.

Solution: (a) Using an empirical relationship that $d_v = 0.33 d_a$ and the value of $f(\epsilon) = 0.52$, the required grab tensile strength is as follows:

$$\begin{aligned}
 T_{\text{reqd}} &= p'(d_v)^2(0.52) \\
 &= p'(0.33 d_a)^2(0.52) \\
 &= 0.057 p' d_a^2 \\
 &= 0.057(700)(1000)(0.050)^2 \\
 T_{\text{reqd}} &= 100 \text{ N}
 \end{aligned}$$

(b) The factor of safety on a 500 N maximum grab tensile geotextile with reduction factors of 2.5, is as follows:

$$\begin{aligned}
 \text{FS} &= \frac{T_{\text{allow}}}{T_{\text{reqd}}} \\
 &= \frac{500/2.5}{100} \\
 \text{FS} &= 2.0, \text{ which is acceptable.}
 \end{aligned}$$

2.5.4 Puncture Resistance

The geotextile must always survive the installation process. This is not just related to the roadway separation function; indeed, fabric survivability is critical in all types of applications; without it the best of designs are futile (recall Figure 2.20). In this regard, sharp stones, tree stumps, roots, miscellaneous debris, and other items, either on the ground surface beneath the geotextile or placed above it, could puncture through the geotextile during backfilling and when traffic loads are imposed. The design method suggested for this situation is shown schematically in Figure 2.32. For these conditions, the vertical force exerted on the geotextile (which is gradually tightening around the protruding object) is as follows:

$$F_{\text{reqd}} = p' d_a^2 S_1 S_2 S_3 \tag{2.30}$$

where

- F_{reqd} = required vertical puncturing force to be resisted,
- d_a = average diameter of the puncturing aggregate or sharp object,

↑

geotextile

XXX

ice pressure is

dependent of particle
theoretical assumptions
; and the strain value
mobilized is related to

(2.29)

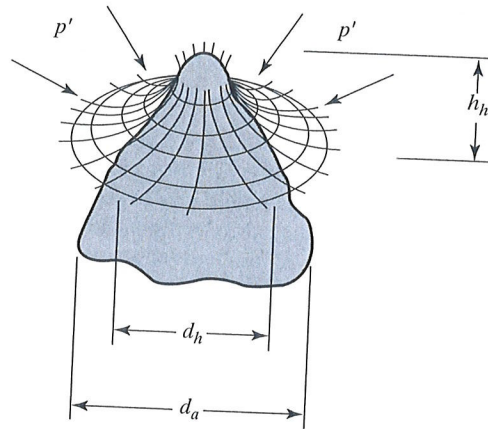


Figure 2.32 Visualization of a stone puncturing a geotextile as pressure is applied from above.

- p' = pressure exerted on the geotextile (approximately 100% of tire inflation pressure at the ground surface for thin covering thicknesses),
- S_1 = protrusion factor of the puncturing object (see Table 2.13),
- S_2 = scale factor to adjust the ASTM D4833 puncture test value that uses a 8.0 mm diameter puncture probe to the actual puncturing object (see Table 2.13), and
- S_3 = shape factor to adjust the ASTM D4833 flat puncture probe to the actual shape of the puncturing object (see Table 2.13).

Example 2.10

What is the factor of safety against puncture of a geotextile from a subrounded 25 mm diameter stone on the ground surface mobilized by a loaded truck with tire inflation pressure of 550 kPa traveling on the surface of the base course? The geotextile has an ultimate puncture strength of 300 N according to ASTM D4833.

TABLE 2.13 RECOMMENDED VALUES FOR FACTORS USED IN PUNCTURE ANALYSIS (DIMENSIONLESS)

Puncturing Object	S_1	S_2	S_3
Angular and relatively large	0.9	0.8	0.9
Angular and relatively small	0.6	0.6	0.7
Subrounded and relatively large	0.7	0.6	0.6
Subrounded and relatively small	0.4	0.4	0.5
Rounded and relatively large	0.5	0.4	0.4
Rounded and relatively small	0.2	0.2	0.3

S_1 = protrusion factor
 S_2 = scale factor
 S_3 = shape factor
 } see equation (2.30)

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2.5.5 Impa

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geotextiles.

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as follows:

where

E =
 m =
 g =

Solution: Using the full stress on the geotextile of 550 kPa and factors from Table 2.13 of 0.55, 0.50, and 0.55 for S_1 , S_2 , and S_3 respectively, we see that

$$\begin{aligned}
 F_{\text{reqd}} &= p' d_a^2 S_1 S_2 S_3 \\
 &= (550)(1000)(25 \times 0.001)^2 (0.55)(0.50)(0.55) \\
 F_{\text{reqd}} &= 52 \text{ N}
 \end{aligned}$$

Assuming that the cumulative reduction factors are 2.0, the factor of safety is as follows:

$$\begin{aligned}
 \text{FS} &= \frac{F_{\text{allow}}}{F_{\text{reqd}}} \\
 &= \frac{300/2.0}{52} \\
 \text{FS} &= 2.9, \text{ which is acceptable}
 \end{aligned}$$

2.5.5 Impact (Tear) Resistance

As with the puncture requirement just described, the resistance of a geotextile to impact is as much a survivability criterion as it is a separation function. Yet in many instances of separation the geotextile must resist the impact of various objects. The most obvious one is that of a rock falling on it, but there are also situations in which construction equipment and materials can cause or contribute to impact damage on geotextiles.

The problem addresses the energy mobilized by a free-falling object of known weight and height of drop. Rarely will an object be intentionally impelled onto an exposed geotextile with additional force, so only gravitational energy will be assumed.

To develop a design procedure, we assume a free-falling rock of specific gravity of 2.60, varying in diameter from 25 to 600 mm and falling from heights of 0.5 to 5 m. Using this data, the design curves in Figure 2.33 are developed. The relationship used is as follows:

$$\begin{aligned}
 E &= mgh \\
 &= (V \times \rho)gh \\
 &= [V \times (\rho_w G_s)]gh \\
 &= \left(\frac{\pi(d_a/1000)^3}{6} \right) \left(\frac{1000 \text{ kg}}{m^3} \right) (2.6)(9.81)h \\
 E &= 13.35 \times 10^{-6} d_a^3 h \tag{2.31}
 \end{aligned}$$

where

- E = energy developed (Joules),
- m = mass of the falling object (kg),
- g = acceleration due to gravity (m/sec²),

pressure is applied

100% of tire inflation thicknesses), Table 2.13),

test value that uses a puncturing object (see

figure probe to the actu-

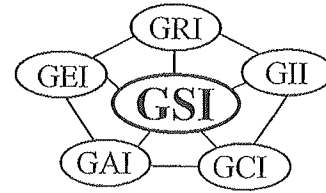
a subrounded 25 mm disk with tire inflation pressure geotextile has an ultimate

USED

S_3
0.9
0.7
0.6
0.5
0.4
0.3

Geosynthetic Institute

475 Kedron Avenue
Folsom, PA 19033-1208 USA
TEL (610) 522-8440
FAX (610) 522-8441



GRI White Paper #14

Modification to the “GRI-Method” for the RF_{CR} -Factor Used in the Design of Geotextiles for Puncture Protection of Geomembranes

by

**Robert M. Koerner
Geosynthetic Institute
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Folsom, PA 19033 USA**

**Phone: 610-522-8440
Fax: 610-522-8441**

**E-mail:
robert.koerner@coe.drexel.edu**

November 24, 2008

GRI White Paper #14

Modification to the “GRI-Method” for the RF_{CR} -Factor Used in the Design of Geotextiles for Puncture Protection of Geomembranes

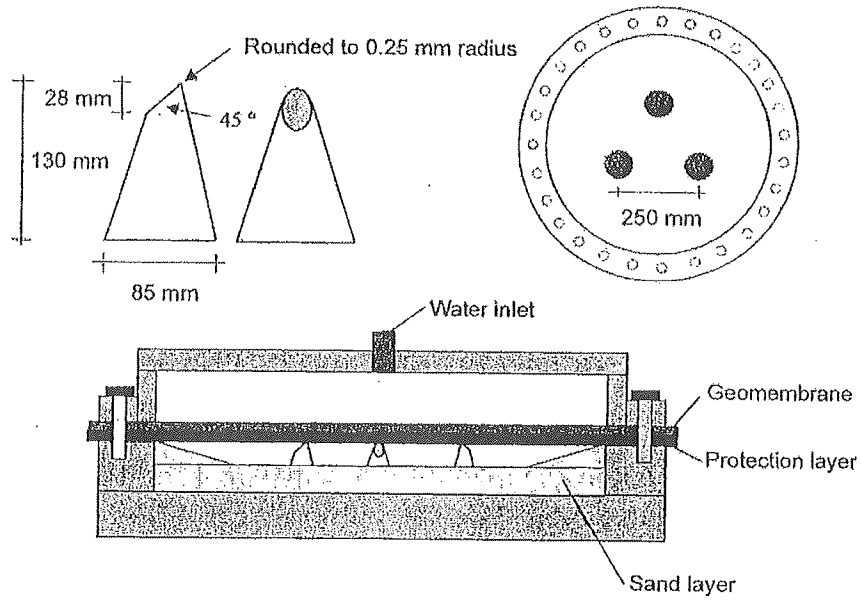
1.0 Background

In 1991, we published our first paper eventually leading to a geotextile design method for protecting geomembranes against puncture. Through the work of a number of colleagues (George Koerner, Grace Hsuan, Ragui Wilson-Fahmy) and graduate students (Don Hullings, Dhani Narejo, Mike Montelone, Bao-Lin Hwu) this method has been used worldwide for such geotextile design for about twelve-years.

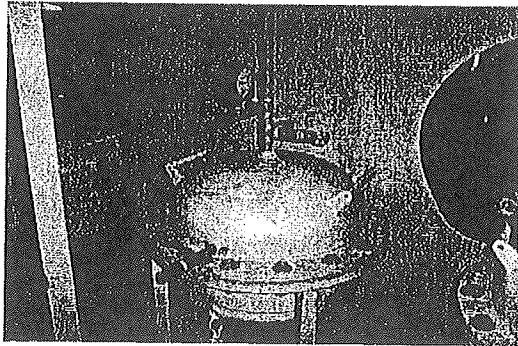
The data from which the method was developed, however, was based on short-term laboratory testing. To extend it into a long-term prediction, tables for biological/chemical degradation and long-term creep were presented largely on the basis of intuition rather than by experiments. Of these two mechanisms, creep is by far the more important. Now, after ten-years of creep puncture testing we are in a position of verifying, or not, the originally proposed table. This verification issue is the focus of this White Paper.

2.0 The “GRI-Method” for Designing Geotextiles to Prevent Geomembrane Puncture

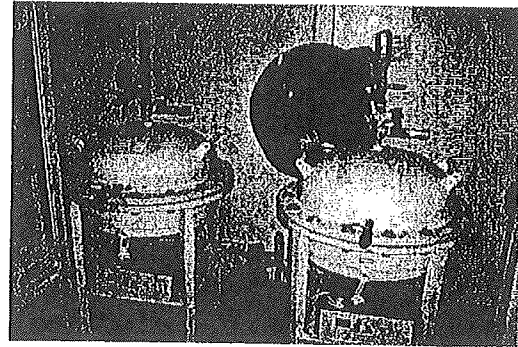
Figure 1 shows details of the truncated plastic cones used as worst-case puncturing objects to a geomembrane. The containment vessels are used to apply hydrostatic pressure to the geomembrane test specimen (1.5 mm smooth HDPE was used throughout the various studies), in turn to the protection geotextile (the “variable” in all tests), and then onto the stationary array of three cones. A well graded concrete sand was used to backfill portions of the cones allowing for a known height-of-cone to be evaluated.



(a) Sketches of truncated cones, their arrangement, and test vessel cross section



(b) Single pressure vessel



(c) Two of four identical pressure vessels with readout boxes

Fig. 1. Geosynthetic Research Institute (GRI) test vessel(s) used to evaluate geotextile protection materials; one vessel was used in the short-term tests, four were used for the long-term tests.

While many theses and technical papers have been written using this experimental test setup, a series of three papers captures the entire program; Wilson-Fahmy, et al. (1997), Narejo, et al. (1997) and Koerner, et al. (1997). The resulting design formula uses a conventional factor of safety as follows:

$$FS = p_{allow} / p_{act} \quad (1)$$

where

FS = factor of safety (against geomembrane puncture),

p_{act} = actual pressure due to the applied normal stress, e.g., landfill contents or surface impoundments, and

p_{allow} = allowable pressure using different types of geotextiles and site-specific conditions

Based on the experimental test results an empirical relationship for “ p_{allow} ” was obtained. It is given as Equation 2. Its use, however, requires the use of modification factors and reduction factors as given in Table 1. Note that in this table, all MF values ≤ 1.0 and all RF values ≥ 1.0 .

$$p_{allow} = \left(50 + 0.00045 \frac{M}{H^2} \right) \left[\frac{1}{MF_s \times MF_{PD} \times MF_A} \right] \left[\frac{1}{RF_{CBD} \times RF_{CR}} \right] \quad (2)$$

where

p_{allow} = allowable pressure (kPa),

M = geotextile mass per unit area (g/m^2),

H = protrusion height (m),

MF_s = modification factor for protrusion shape,

MF_{PD} = modification factor for packing density,

MF_A = modification factor for arching in solids,

RF_{CBD} = reduction factor for long-term chemical/biological degradation, and

RF_{CR} = reduction factor for long-term creep.

Table 1. Modification factors and reduction factors for geotextile protection material design using Equation 2, i.e., the “GRI-Method”.

(a) Modification factors (all ≤ 1.0)					
MF_s		MF_{PD}		MF_A	
Angular	1.0	Isolated	1.0	Hydrostatic	1.0
Subrounded	0.5	Dense, 38 mm	0.83	Geostatic, shallow	0.75
Rounded	0.25	Dense, 25 mm	0.67	Geostatic, mod.	0.50
		Dense, 12 mm	0.50	Geostatic, deep	0.25

(b) Reduction factors (all ≥ 1.0)					
RF_{CBD}		Mass per unit area (gm/m^2)	RF_{CR}		
			Protrusion height (mm)		
			38	25	12
Mild leachate	1.1	Geomembrane alone	N/R	N/R	N/R
Moderate leachate	1.3	270	N/R	N/R	>1.5
Harsh leachate	1.5	550	N/R	1.5	1.3
		1100	1.3	1.2	1.1
		>1100	$\cong 1.2$	$\cong 1.1$	$\cong 1.0$

Abbreviation: N/R = Not recommended

The design situation can be approached by using a given mass per unit area geotextile to determine the unknown FS-value, or from using a given FS-value to determine the unknown mass per unit area geotextile. Koerner (2005) gives numeric examples, and Valero and Austin (1999) present design charts for the many variables contained in the design equation. It might be noted that this method is the only design method that allows for direct selection of a geotextile protection material without the need for large scale trial-and-error experimental testing.

In Equation 2 the two terms “ RF_{CBD} ” and “ RF_{CR} ” are intended to extend the short term test results into a simulated long term performance behavior. Since HDPE is quite resistant to chemical and biological degradation, the term RF_{CBD} is comparatively small. The term RF_{CR} , however, is not small and in many cases a “not recommended” decision is suggested. Due to its importance in the overall design, a series of long term creep tests using this same methodology,

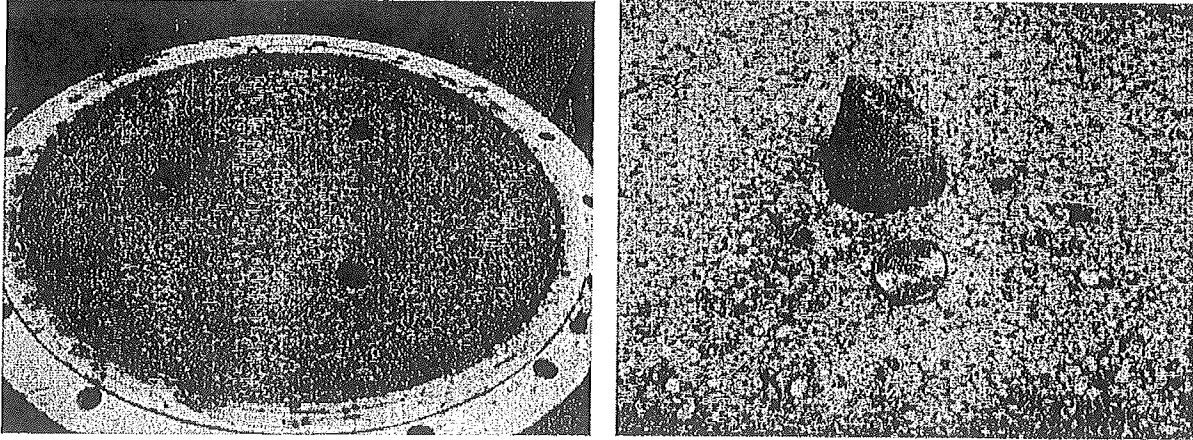
i.e., truncated cones, has been undertaken for the past ten years. This White Paper presents these new results which will be seen to lead to a revised table for the RF_{CR} -values.

3.0 Creep Puncture Results After Ten-Years

There are four identical test vessels used in this creep study, each containing three identical truncated cones shaped and configured as shown in Figure 1. In all cases the geomembranes being evaluated are 1.5 mm nominal thickness smooth HDPE which conform to the GRI-GM13 specification insofar as their physical, mechanical, and endurance properties are concerned. Also common to all four setups is the geotextile cushioning materials. They consisted of three layers of 200 g/m^2 needle-punched nonwoven (continuous filament) polyester geotextiles. They will be collectively referred to as 600 g/m^2 protection materials.

The differences in the four test vessels are the heights of the truncated cones causing the puncture to occur and the applied hydrostatic pressures.

Regarding the cone heights, sand is placed and compacted in the vessels leaving a protrusion height rising above the sand level; see Figure 2. As placed, two vessels had initial cone heights of 12 mm and the other two had initial cone heights of 38 mm. It was recognized that 38 mm was unacceptable, e.g., in Table 1b, “not recommended” is listed, but this limit was in need of being verified. Regarding the 12 mm cone heights, Table 1b indicates that it should be acceptable providing a $FS_{CR} = 1.3$ is used in the design procedure of Equation 2. This, of course, had to be verified as well.



(a) Array of three cones rising above sand level (b) An individual cone height of 38 mm

Fig 2. Photographs indicating the truncated cone protrusions producing the puncturing action.

Regarding the applied hydrostatic pressure, there was considerable uncertainty. The design procedure using Eqs. 1 and 2 does not address a maximum pressure. As a result high hydrostatic pressures were used for the two 12 mm cone heights (430 and 580 kPa) and low hydrostatic pressures were used for the two 38 mm cone heights (52 and 34 kPa). It is worth mentioning that hydrostatic pressure represents surface impoundment (liquid) stresses but overestimates solid waste stresses due to the arching that occurs within the solid waste as deformation occurs. The MF_A -term in Table 1a attempts to take such geostatic stresses into account.

Table 2 presents the results of this creep puncture study. Note that the cone heights varied somewhat due to shifting sand as pressure was applied and maintained. It should also be mentioned that the applied hydrostatic pressure represent 10 to 28% of the short-term failure stresses. As noted, all twelve of the truncated cones experienced yield in the geomembranes, with one having an actual break. The thickness reductions in the yield regions are also noted with the 38 mm cone heights resulting in the greatest reductions.

ated Cone Ten-Year Creep Puncture Tests
 W-PET geotextile protecting a 1.5 mm smooth HDPE geomembrane)

1)	1) Cone Heights ¹		Applied Puncture Stress ³		Final Description of Geomembrane ⁵	
	Final (mm) ²	(kPa)	(%) ⁴	Visual	Thickness (mm) ⁶	
	12.1	430	23	3-subtile yields – no breaks	1.16 (30% reduction)	
	11.5	580	28	3-subtile yields – no breaks	0.80 (52% reduction)	
	29.7	52	15	3-pronounced yields; one break	0.32 (80% reduction)	
	31.1	34	10	3-pronounced yields; no breaks	0.34 (78% reduction)	

ad three identical truncated cones beneath the geomembrane; see Figure 4.

nged during, or after, pressurization due to movement of the initially placed sand layers. i.e, the sand was around the stationary cones.

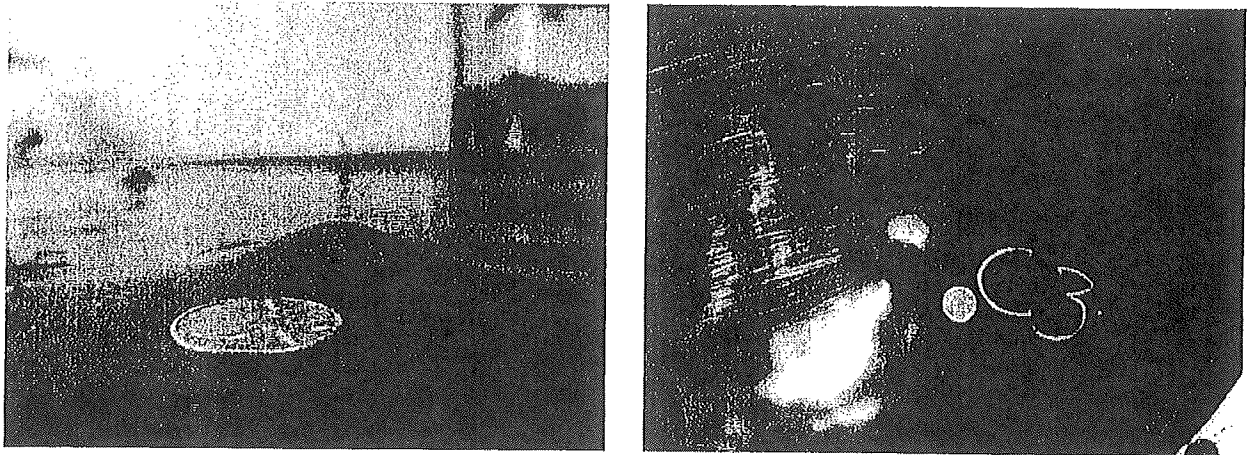
plied to geomembrane; beneath which is the geotextile and then the three puncturing cones.

arm failure stress using Equation 2 for the calculations.

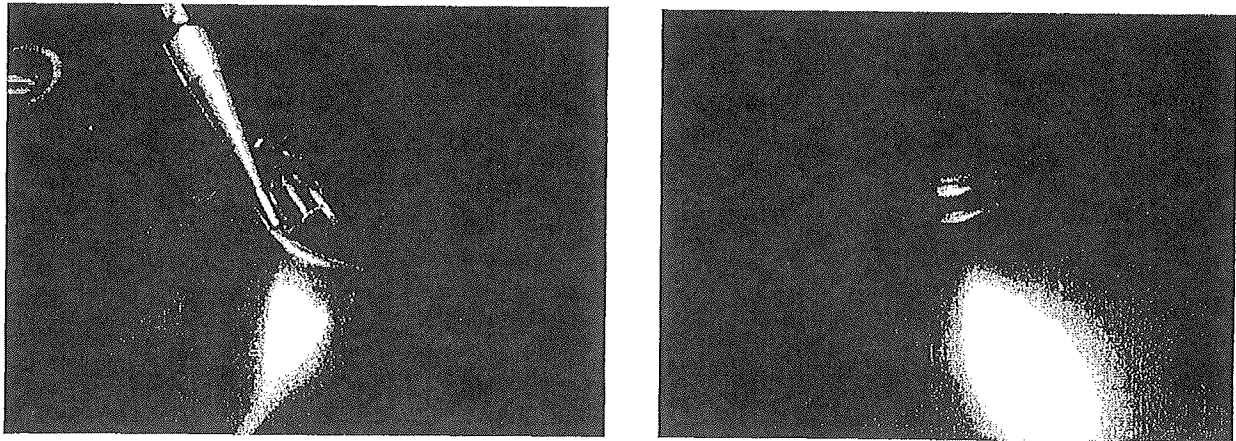
me response after ten-years of hydrostatic stress.

minimum geomembrane thickness above the cone tips.

Regarding the type and amount of yield it was very subtle for the 12 mm cone heights and very pronounced for the 38 mm cone heights (there was a small break in the yield zone for one of the six cones); see Figure 3. An analytic analyses of the resulting geomembrane strains in the yield regions was attempted but the crescent moon shape of the yield regions was not amendable to standard tensile strain calculations. On the other hand thickness strains were tractable and Table 2 gives these results; see Koerner, et al. (2009) for complete details.



(a) Deformations from 12 mm cone heights; Vessels No. 1 and 2



(b) Deformation from 38 mm cone heights; Vessels No. 3 and 4

Fig. 3. Photographs of several of the yield zones caused by truncated cone deformations of geomembrane test specimens.

4.0 Summary and Conclusions

The need for geomembrane protection against puncture by objects such as stones and gravel has been apparent for many years. Commonly used for this purpose are relatively thick needle-punched nonwoven geotextiles. In essence, such geotextiles provide a cushion in blunting the inherent aggressiveness of the puncturing object against the geomembrane. Even further, geomembranes used as liner materials beneath solid waste landfills are commonplace and for large landfills the normal stresses on such geomembranes are very high, i.e., the puncture situation is greatly exacerbated.

The type of geomembrane is also an issue. By virtue of its good chemical resistance and long anticipated lifetime, HDPE geomembranes are routinely used to line solid waste landfills. Many countries even sole source this type of geomembrane. That said, HDPE is (other than scrim reinforced geomembranes) the most sensitive geomembrane type to out-of-plane deformation, as is the situation arising from a puncturing stone located above or below the geomembrane; Nosko and Touze-Folz (2000).

As a result of the above issues, several approaches toward selecting a proper geotextile protection material are in the literature. This White Paper has focused on the “GRI-Method” which was the result of a large short-term testing project that has been published and widely used for about twelve-years. Needed, however, is the projection of the short-term testing results into long-term behavior. This was done in the past using empirical tables for both degradation (RF_{CBD}) and creep (RF_{CR}) reduction factors; recall Equation 2. The degradation by chemical and biological agents is the lesser of the two reduction factors and, as a result, this present effort is focused entirely on the validity of the RF_{CR} -values. The original values are given in Table 1b. To verify or refute the given values, this ten-year creep study of HDPE geomembranes and their

associated geotextile protection materials against puncture has been concluded and this White Paper presents the results.

The same type of pressure vessel and truncated puncturing cones as used in the short-term tests were used for these long-term tests. In all cases, 1.5 mm thick smooth HDPE geomembranes were used and protected by 600 g/m² needle-punched nonwoven PET geotextiles. Three truncated cones were used in each of four pressure vessels, the differences being the protruding cone heights (six at \simeq 12 mm and six at \simeq 38 mm) and applied hydrostatic pressures (varying from 34 to 580 kPa).

After ten-years of pressurization the vessels were dismantled and it was found that all six of the high cone heights (\simeq 38 mm) had pronounced yield zones in the geomembranes and one of the six had a small break within its yield zone. Clearly, such high cone heights with this type of geotextile are unacceptable. *The entry in Table 1b in this regard mentions “not recommended” and this comment is hereby substantiated.*

However, the creep test results at low cone heights (\simeq 12 mm) provide a different conclusion. Table 1b indicates that a 12 mm cone height with a 550 g/m² protection geotextile is acceptable with a $RF_{CR} = 1.3$. Since all six of these cases resulted in geomembrane yield (albeit small yields in comparison to the higher cone heights), *the Table 1b values must be changed and made more conservative in their design guidance.*

To be noted in Table 1b for RF_{CR} , a not recommended (N/R) comment exists for the various protrusion heights in descending order as the cone heights decrease and the protection geotextile mass increases. By virtue of these long-term creep test results, the N/R comments must be extended. Thus, our conclusion as a result of this creep testing program is to replace the existing RF_{CR} -values with Table 3 following:

Table 3. Revised values for “RF_{CR}” to be used in Equation 2 for geotextile protection materials design.

Mass per unit area (g/m ²)	“RF _{CR} ”-Values		
	Protrusion Height (mm)		
	38	25	12
Geomembrane alone	N/R	N/R	N/R
270	N/R	N/R	N/R
550	N/R	N/R	>1.5
1100	N/R	1.5	1.3
>1100	1.3	1.2	1.1

Abbreviation: N/R = Not recommended

Lastly, the entry of “>1.5” for a 12 mm cone height associated with a 550 g/m² geotextile is felt to be appropriate considering the following items.

- The geotextiles used at present are made from polypropylene fibers versus the tested geotextiles which were made from polyester fibers. Since the specific gravity of PP is 0.91 and that of PET is between 1.22 and 1.38, one has from 25% to 34% more filaments in an equivalent mass per unit area geotextile using polypropylene fibers. This provides for considerably greater protection capability.
- The area of yield for the six \approx 12 mm cone heights was extremely small and the thicknesses of the remaining geomembrane was such that considerable deformation could still be sustained before break is even close to occurring.
- The “>1.5” recommendation is precisely for additional conservatism and safety and if a designer wishes to be more conservative than the new recommended table suggests he/she is free to do so.

5.0 References

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Geomembrane Protection **Design Manual**

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First Edition

Chapter 2

DESIGN VARIABLES

2.1 Effective Protrusion Height

The effective protrusion height is the maximum dimension of the largest soil, aggregate, or any other object, to which a geomembrane liner may be exposed. The effective protrusion height depends, primarily, on two factors: size and arrangement. Two types of arrangements are possible for objects in the vicinity of a geomembrane: isolated and grouped. Each of these scenarios is discussed in detail in the following sections.

2.1.1 Isolated Protrusions

Isolated protrusions act more or less alone when interacting with a geomembrane in the puncture mode. An example is a stone underlying a geomembrane placed on a relatively level surface as indicated in Figure 2.1. The full exposed size of the stone in this case can challenge the geomembrane. The effective protrusion height, H' , in this case is equal to the maximum exposed object dimension as indicated in the figure. Conservatively, H' may be assumed to be equal to the diameter of the largest size particle in the subgrade as determining the exposed dimensions of particles over the life of a project may be difficult.

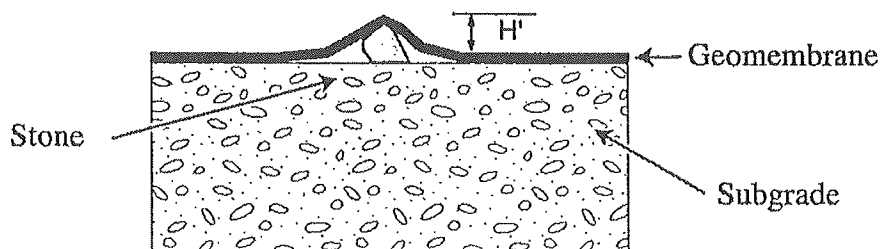


Figure 2.1 Effective Protrusion Height for the Case of Isolated Objects.

2.1.2 Grouped Protrusions

Grouped objects are close enough to interact with each other when challenging a geomembrane. An example is a drainage aggregate layer placed on top of a geomembrane. In such cases only a limited part of the full size of a particle can interact with the geomembrane because of the proximity of the surrounding particles. Practically, the adjoining particles “hide” part of each other. This is explained graphically in Figure 2.2. For the case of grouped particles, the effective protrusion height, H' , is approximated as half of the maximum particle size. This approximation is based on performance puncture testing with isolated and grouped protrusions. Particle size analysis according to ASTM Method D 422 can be used to determine the maximum dimensions of protrusions.

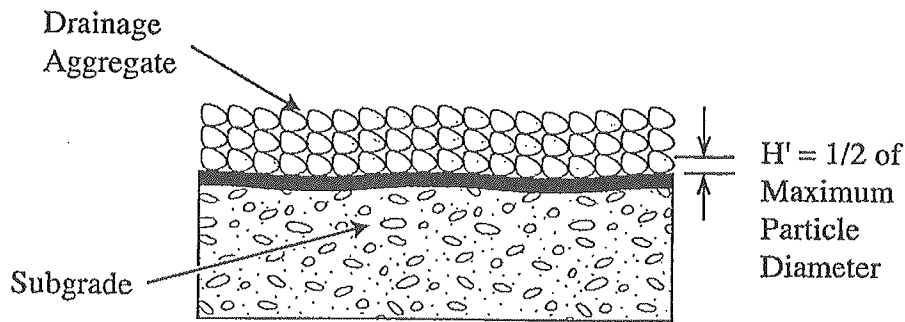


Figure 2.2 Effective Protrusion Height in the Case of Grouped Objects.

2.2 Protrusion Shape

For the purpose of geomembrane puncture protection, particle shape can be described by angularity. Angularity is a measure of sharpness of corners of a particle. Although a quantitative measurement of angularity is possible (Krumbein, 1941), for the design method presented here it is adequate to obtain a qualitative description as provided in Figure 2.3.

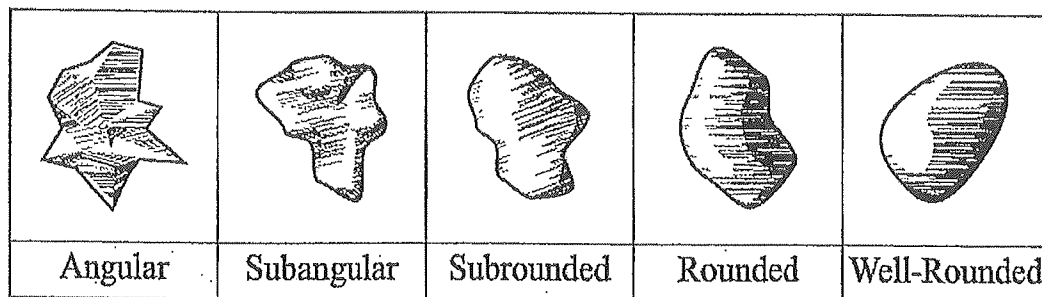


Figure 2.3 Angularity of Soil Particles (Sowers, 1979).

On the basis of the effect of shape on geomembrane protection requirements, soil particles can be placed into the following three categories:

Group I – Angular

Group II – Subangular and Subrounded

Group III – Rounded and Well Rounded

The design engineer should consider the angularity carefully as well rounded aggregate may become angular during installation, handling or excavation. When in doubt, it may be desirable to perform the calculations assuming angular stone.



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GSE Nonwoven “Heavyweight” Geotextile

GSE Nonwoven “Heavyweight” Geotextile is a family of staple fiber needlepunched geotextiles. The geotextile is manufactured using an advanced manufacturing and quality system to produce the most uniform and consistent nonwoven needlepunched geotextile currently available in the industry. GSE combines a fiber selection and approval system with in-line quality control and a state-of-the-art laboratory to ensure that every roll shipped meets customer specifications and for various applications.

Product Specifications

These product specifications meet or exceed GRI GT12 and AASHTO M288

TESTED PROPERTY	TEST METHOD	FREQUENCY	MINIMUM AVERAGE VALUE			
			NW20	NW24	NW28	NW32
Mass per Unit Area, oz/yd ² (g/m ²)	ASTM D 5261	90,000 ft ²	20 (675)	24 (810)	28 (950)	32 (1,080)
Grab Tensile Strength, lb (N)	ASTM D 4632	90,000 ft ²	450 (1,980)	500 (2,200)	550 (2,420)	600 (2,640)
Grab Elongation, %	ASTM D 4632	90,000 ft ²	50	50	50	50
Puncture Strength, lb (N)	ASTM D 4833	90,000 ft ²	200 (880)	250 (1,100)	300 (1,320)	350 (1,540)
Trapezoidal Tear Strength, lb (N)	ASTM D 4533	90,000 ft ²	125 (550)	200 (880)	250 (1,100)	270 (1,190)
UV Resistance (% retained after 500 hours)	ASTM D 4355	per formulation	70	70	70	70
NOMINAL ROLL DIMENSIONS						
Roll Length ⁽¹⁾ , ft (m)			200 (61)	200 (61)	150 (45.7)	150 (45.7)
Roll Width ⁽¹⁾ , ft (m)			15 (4.6)	15 (4.6)	15 (4.6)	15 (4.6)
Roll Area, ft ² (m ²)			3,000 (281)	3,000 (281)	2,250 (209)	2,250 (209)

NOTES:

- The property values listed are in weaker principal direction. All values listed are Minimum Average Values except UV resistance which is a typical value.
- ⁽¹⁾Roll lengths and widths have a tolerance of ±1%.

TEXTURED HDPE GEOMEMBRANE

ENGLISH UNITS



Minimum Average Values

Property	Test Method	40 mil	60 mil	80 mil	100 mil
Thickness, mils	ASTM D 5994				
minimum average		38	57	76	95
lowest individual of 8 of 10 readings		36	54	72	90
lowest individual of 10 readings		34	51	68	85
Asperity Height ¹ , mils	ASTM D 7466	10	10	10	10
Sheet Density, g/cc	ASTM D 1505/D 792	0.940	0.940	0.940	0.940
Tensile Properties²	ASTM D 6693				
1. Yield Strength, lb/in		84	126	168	210
2. Break Strength, lb/in		60	90	120	150
3. Yield Elongation, %		12	12	12	12
4. Break Elongation, %		100	100	100	100
Tear Resistance, lb	ASTM D 1004	28	42	56	70
Puncture Resistance, lb	ASTM D 4833	60	90	120	150
Stress Crack Resistance ³ , hrs	ASTM D 5397 (App.)	300	300	300	300
Carbon Black Content ⁴ , %	ASTM D 1603	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596	--Note 5--			
Oxidative Induction Time (OIT)					
Standard OIT, minutes	ASTM D 3895	100	100	100	100
Oven Aging at 85°C	ASTM D 5721				
High Pressure OIT - % retained after 90 days	ASTM D 5885	80	80	80	80
UV Resistance ⁶	ASTM D 7238				
High Pressure OIT ⁷ - % retained after 1600 hrs	ASTM D 5885	50	50	50	50
Roll Dimensions					
1. Width (feet):		23	23	23	23
2. Length (feet)		750	500	375	300
3. Area (square feet):		17,250	11,500	8,625	6,900
4. Gross weight (pounds, approx.)		3,500	3,500	3,470	3,470

- 1 Of 10 readings; 8 must be ≥ 7 mils and lowest individual reading must be ≥ 5 mils.
- 2 Machine direction (MD) and cross machine direction (XMD) average values should be on the basis of 5 test specimens each direction. Yield elongation is calculated using a gauge length of 1.3 inches; Break elongation is calculated using a gauge length of 2.0 inches.
- 3 The yield stress used to calculate the applied load for the SP-NCTL test should be the mean value via MQC testing.
- 4 Other methods such as ASTM D 4218 or microwave methods are acceptable if an appropriate correlation can be established.
- 5 Carbon black dispersion for 10 different views: Nine in Categories 1 and 2 with one allowed in Category 3.
- 6 The condition of the test should be 20 hr. UV cycle at 75°C followed by 4 hr. condensation at 60°C.
- 7 UV resistance is based on percent retained value regardless of the original HP-OIT value.

These data are provided for informational purposes only and are not intended as a warranty or guarantee. Poly-America, L.P. assumes no responsibility in connection with the use of these data. Suitability for a particular use shall be determined by and is the sole responsibility of the end user. These values are subject to change without notice. REV. 03/14



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GSE Nonwoven Geotextile

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Product Specifications

These product specifications meet or exceed GRI GT12, GRI GT13 and AASHTO M288.

TESTED PROPERTY	TEST METHOD	FREQUENCY	MINIMUM AVERAGE VALUE					
			NW4	NW6	NW8	NW10	NW12	NW16
AASHTO M288 Class			3	2	1	>1	>>1	>>>1
Mass per Unit Area, oz/yd ² (g/m ²)	ASTM D 5261	90,000 ft ²	4 (135)	6 (200)	8 (270)	10 (335)	12 (405)	16 (540)
Grab Tensile Strength, lb (N)	ASTM D 4632	90,000 ft ²	120 (530)	160 (710)	220 (975)	260 (1,155)	320 (1,420)	390 (1,735)
Grab Elongation, %	ASTM D 4632	90,000 ft ²	50	50	50	50	50	50
Puncture Strength, lb (N)	ASTM D 4833	90,000 ft ²	60 (265)	90 (395)	120 (525)	165 (725)	190 (835)	240 (1,055)
Trapezoidal Tear Strength, lb (N)	ASTM D 4533	90,000 ft ²	50 (220)	65 (290)	90 (395)	100 (445)	125 (555)	150 (665)
Apparent Opening Size, Sieve No. (mm)	ASTM D 4751	540,000 ft ²	70 (0.212)	70 (0.212)	80 (0.180)	100 (0.150)	100 (0.150)	100 (0.150)
Permittivity, sec ⁻¹	ASTM D 4491	540,000 ft ²	1.80	1.50	1.30	1.00	0.80	0.60
Water Flow Rate, gpm/ft ² (l/min/m ²)	ASTM D 4491	540,000 ft ²	135 (5,495)	110 (4,480)	95 (3,865)	75 (3,050)	60 (2,440)	45 (1,830)
UV Resistance (% retained after 500 hours)	ASTM D 4355	per formulation	70	70	70	70	70	70
NOMINAL ROLL DIMENSIONS								
Roll Length ⁽¹⁾ , ft (m)			850 (259)	850 (259)	600 (182)	500 (152)	400 (122)	300 (91)
Roll Width ⁽¹⁾ , ft (m)			15 (4.5)	15 (4.5)	15 (4.5)	15 (4.5)	15 (4.5)	15 (4.5)
Roll Area, ft ² (m ²)			12,750 (1,185)	12,750 (1,185)	9,000 (836)	7,500 (698)	6,000 (557)	4,500 (418)

NOTES:

- The property values listed are in weaker principal direction. All values listed are Minimum Average Values except apparent opening size in mm and UV resistance. Apparent opening size (mm) is a Maximum Value. UV is a typical value.
- ⁽¹⁾Roll lengths and widths have a tolerance of ±1%.

J.5 – Final Cover Evaluations

- J.5-A Waste Settlement
- J.5-B Final Cover Geomembrane Strain
- J.5-C Final Cover Geocomposite Transmissivity
- J.5-D Toe Drain Capacity
- J.5-E Terrace Berms

J.5-A Waste Settlement



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF WASTE MASS

Problem Statement

Differential settlement is a measure of the difference in settlement that occurs between different locations based on location-specific loads. Differential settlement has the potential to impact landfills when the variation of settlement is greater than the design slopes of design features. This calculation determines the maximum settlement that is anticipated to occur within the waste mass at multiple locations to ensure that the plateau area of the final cover will maintain positive drainage after settlement occurs.

Note: The maximum differential settlement of the foundation was previously determined in a calculation in Appendix J.3-B and is incorporated into this calculation.

Given

- Drawings of the “Final Landform Grades” and “Excavation Grades” contained in this Application.
- Section 2.2**, Hydrogeologic Report, in this Application.
- Appendix J.1** “Summary of Geotechnical Parameters” contained in this Application.
- Microsoft Excel settlement calculation spreadsheets (refer to attached pages).
- Yee, K., Menard Geosystems Sdn Bhd, Lumpur, K., Ugrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect, Master Builders Journal, September, 1999 (Refer to attached pages).
- Personal Communication, Research Reference Notes, Craig H. Benson, University of Wisconsin-Madison, 2009.
- Budhu, Muni, Foundations and Earth Retaining Structures (refer to attached pages).



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF WASTE MASS

Background

Consolidation of materials that will lead to settlement is divided into two categories:

1. Primary Consolidation Settlement

Occurs in saturated cohesive soils occurs due to the expulsion of water in response to an increase in effective stress. Primary settlement is determined using the following equation for normally consolidated soils.

$$S_P = \left((C_c) \left(\frac{H_o}{1 + e_o} \right) \log \left(\frac{\sigma'_{v_o} + \Delta\sigma_v}{\sigma'_{v_o}} \right) \right)$$

Where,

- S_p = Primary Settlement, feet
- C_c = Compression Index
- H = Thickness of the layer, feet
- e_o = Initial void ratio = $n/(1-n)$
- $\Delta\sigma'_v$ = Change in vertical effective stress, psf
- σ'_{v_o} = Initial vertical effective stress, psf

2. Secondary Consolidation Settlement

Occurs only in saturated cohesive soils and is the result of the plastic adjustment of soil fabrics. Secondary consolidation is calculated using the following equation.

$$S_S = \left(\left(\frac{C_\alpha}{1 + e_p} \right) H \log \left(\frac{T_2}{T_1} \right) \right)$$

Where:

- S_S = Secondary settlement, feet
- C_α = Secondary compression index
- H = Thickness of Layer, feet
- e_p = Void Ratio at end of primary consolidation
= e_o (to be conservative)
- T_1 = Time at start of secondary compression, years
- T_2 = Time at end of observation period, years

Waste does not experience primary consolidation in the manner of a saturated soil. Waste will undergo initial and primary compression. Both types of compression occur rapidly and are grouped together.



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Date: 05/2022

TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF WASTE MASS
Assumptions and Calculations – Final Cover / Waste Settlement

In addition to foundation settlement, the waste mass will also settle under its weight. Therefore, the differential settlement of the waste mass is also analyzed to ensure that the final cover maintains positive drainage after settlement. For the purpose of this analysis, the maximum differential settlement is determined for the plateau (top) of the landfill in the horizontal and vertical expansions. The design of the plateau is 10H:1V (5.71 degrees).

Three points were selected for the waste settlement analysis, as described below:

- W1: This point represents the thickest waste column in the horizontal expansion and the maximum waste height in the horizontal expansion (located in Cell 11).
- W2: This point represents the thickest waste column in the vertical expansion area and the maximum waste height in the vertical expansion (located in Cell 7).
- W3: This point represents the thickest waste column in the vertical expansion area over an LCS pipe (located in Cell 7).
- W4: This point represents the thinnest waste column in the horizontal and vertical expansion, the minimum waste elevation on the plateau, and the mid-point between W1 and W2/W3 (located between Cell 7 and Cell 11).
- W5: This point represents the thinnest waste column in the horizontal and vertical expansion, the minimum waste elevation on the plateau, and the mid-point between W1 and W2/W3 (located between Cell 7 and Cell 11).

The locations of the waste settlement points are presented on **Figures 1-3** (see attached pages). **Table J.5A-1** below provides the elevations of the settlement points, and elevations and thicknesses of the landfill system layers.

Location / Point	Final Cover Thickness (ft)	Waste Thickness (ft)	LCS Drainage Layer Thickness (ft)	Low Permeable Earth Liner Thickness (ft)	Wadsworth Till Thickness (ft)
W1	5	198	1	5	33.8
W2	5	206	1	5	26.3
W3	5	206	1	5	22.5
W4	5	136.7	1	5	85.1
W5	5	136.7	1	5	83.7

The distances between each point are as follows:

- W1 to W2: 338 ft
- W1 to W3: 346 ft
- W1 to W4: 196 ft



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF WASTE MASS

- W1 to W5: 193 ft
- W2 to W3: 44 ft
- W2 to W4: 214 ft
- W2 to W5: 220 ft
- W3 to W4: 245 ft
- W3 to W5: 205 ft
- W4 to W5: 235 ft

For the purpose of this calculation, settlement is calculated incrementally for 15-ft thick fill lifts of waste and one lift for the final cover placement for one landfill cell. It is assumed that each lift of waste will take 3 months to complete. The time of primary compression is estimated to be completed within 2 to 30 days following loading (Benson, see the attached reference). From this calculation, it is assumed that the final cover will only be subjected to the primary settlement from the final lift of the landfill plus secondary settlement that will occur during post-construction.

The secondary settlement considers secondary settlement that occurs by the self-weight of each fill lift based on Terzaghi's time-settlement relationship. Secondary settlement is calculated for each lift individually, and then summed to provide a total value for secondary settlement.

The attached spreadsheets show the results of each waste settlement location, which are also summarized in **Table J.5A-2**.

Point	Incremental Primary Settlement of Last Lift (ft)	Secondary Settlement (ft)	Total Settlement (ft)
W1	2.01	23.93	25.94
W2	1.93	24.89	26.82
W3	1.93	24.89	26.82
W4	1.76	16.56	18.32
W5	1.76	16.56	18.32



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TITLE: DIFFERENTIAL SETTLEMENT EVALUATION OF WASTE MASS*Differential Settlement*

The differential settlement between points are calculated as follows:

$$S_{\text{diff}} = \frac{|S_{S1} - S_{S2}|}{\text{Distance}_{S1 \text{ to } S2}} \times 100\%$$

Point 1	Point 2	Settlement 1 (ft)	Settlement 2 (ft)	Distance Between Points (ft)	Differential Settlement (%)
W1	W2	25.94	26.82	338	0.26%
W1	W3	25.94	26.82	346	0.26%
W1	W4	25.94	18.32	196	3.89%
W1	W5	25.94	18.32	193	3.95%
W2	W3	26.82	26.82	44	0.00%
W2	W4	26.82	18.32	214	3.97%
W2	W5	26.82	18.32	220	3.87%
W3	W4	26.82	18.32	245	3.47%
W3	W5	26.82	18.32	205	4.15%
W4	W5	18.32	18.32	235	0.00%

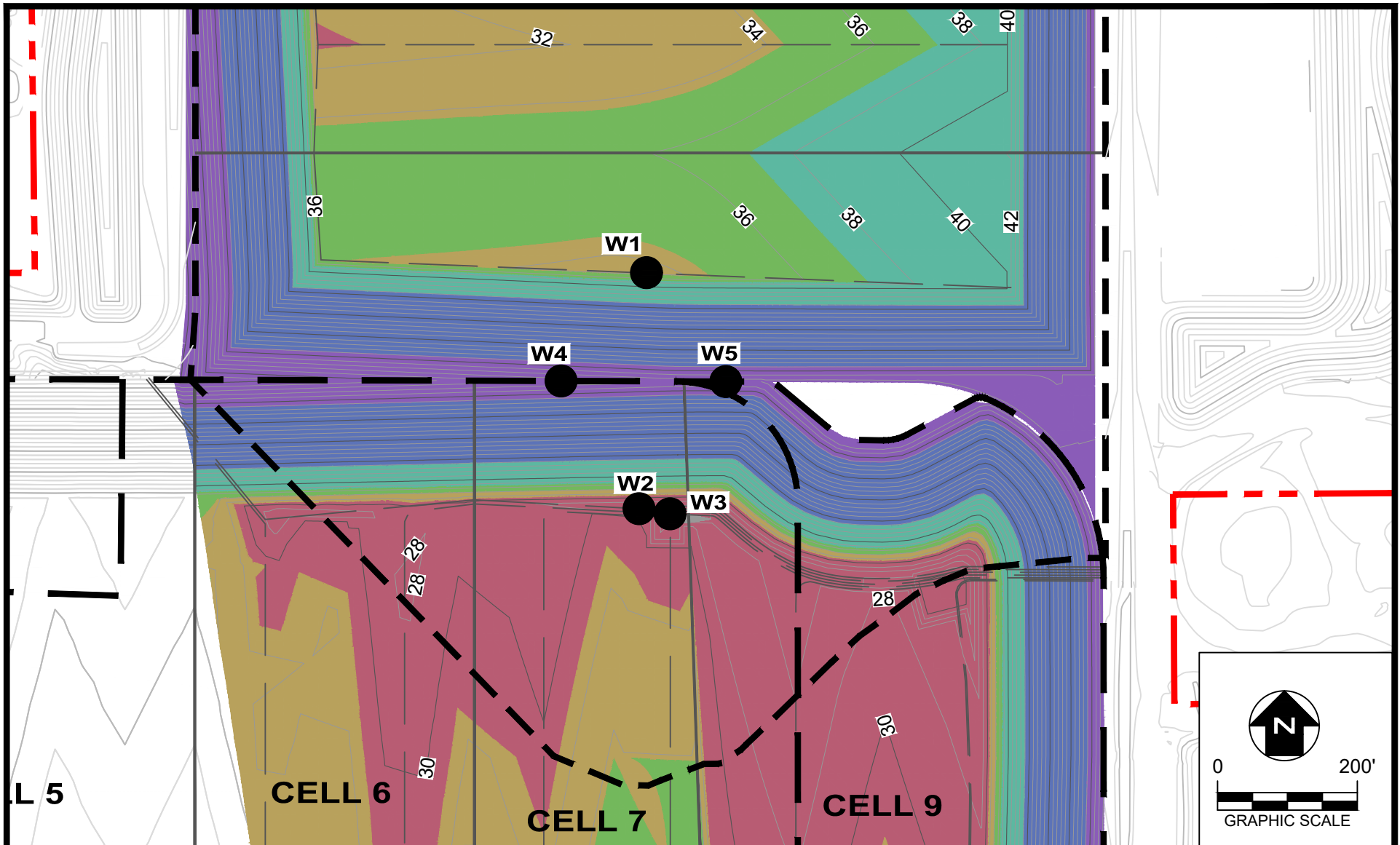
In addition to the waste settlement calculated above, the foundation soils have been determined to experience a maximum differential settlement along the foundation of approximately 0.89% (**Appendix J.3-B**). Therefore, the cumulative impact of foundation and waste settlement is estimated to be 5.04% (4.15% + 0.89%).

Resulting Slopes of Final Cover System After Differential Settlement

The maximum differential settlement across the expansion's plateau is calculated to be approximately 5.04 percent (located in the vertical expansion). The design slope of the plateau is 10H:1V (approximately 5.71 degrees). Therefore, the resulting slope after differential settlement is anticipated to be approximately 0.67 degrees. This slope is acceptable, as the final cover will maintain positive drainage.

Summary of Results

The leachate collection layer in both the horizontal expansion area and vertical expansion area will remain free draining, even after differential settlement occurs. The final cover of the plateau area will remain positive drainage, even after differential settlement occurs. Therefore, the proposed expansion is appropriately designed with respect to differential settlement.



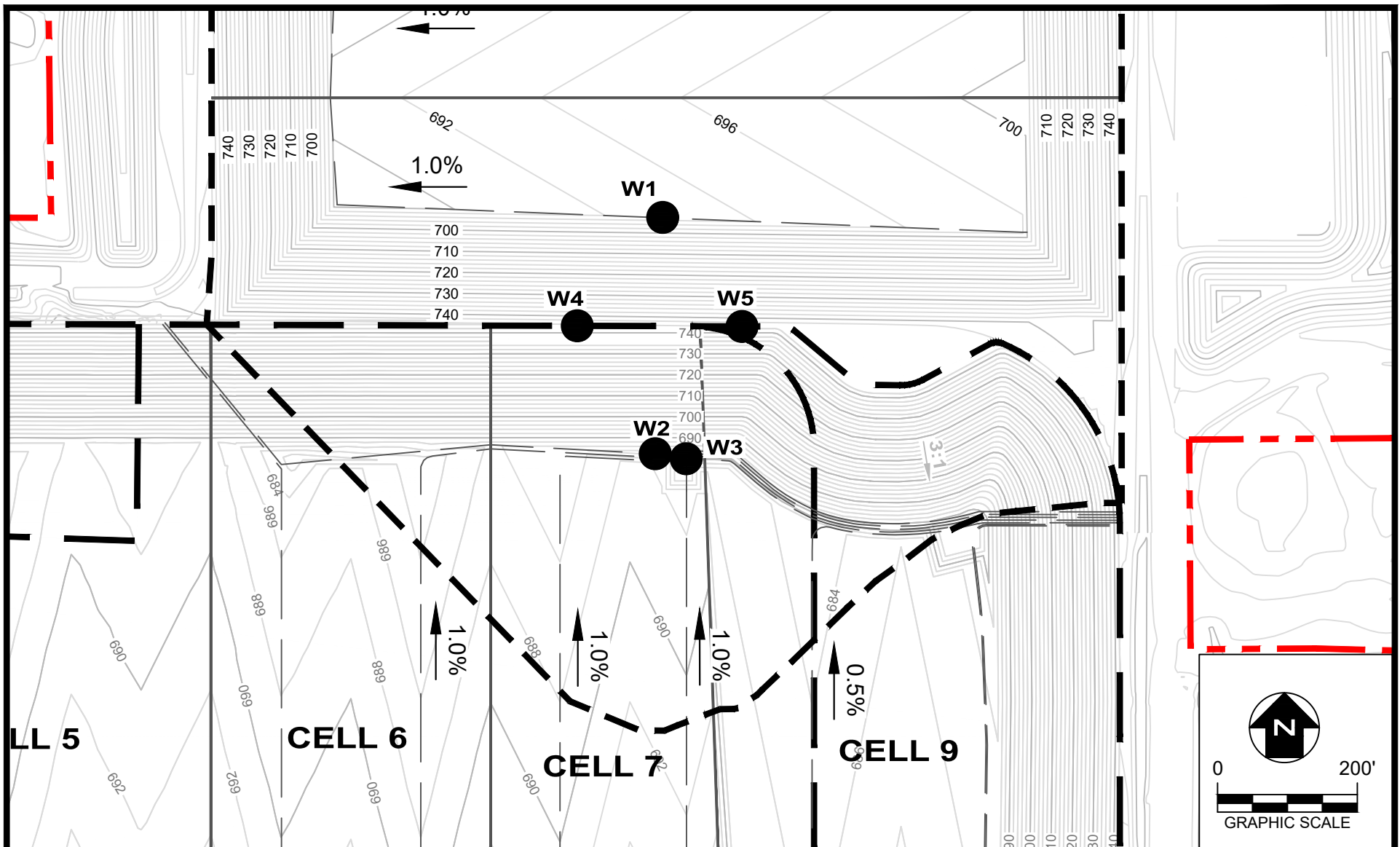
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**FIGURE 1
LOCATION OF WASTE SETTLEMENT ANALYSIS POINTS
THICKNESS OF WADSWORTH TILL TO REMAIN IN PLACE**

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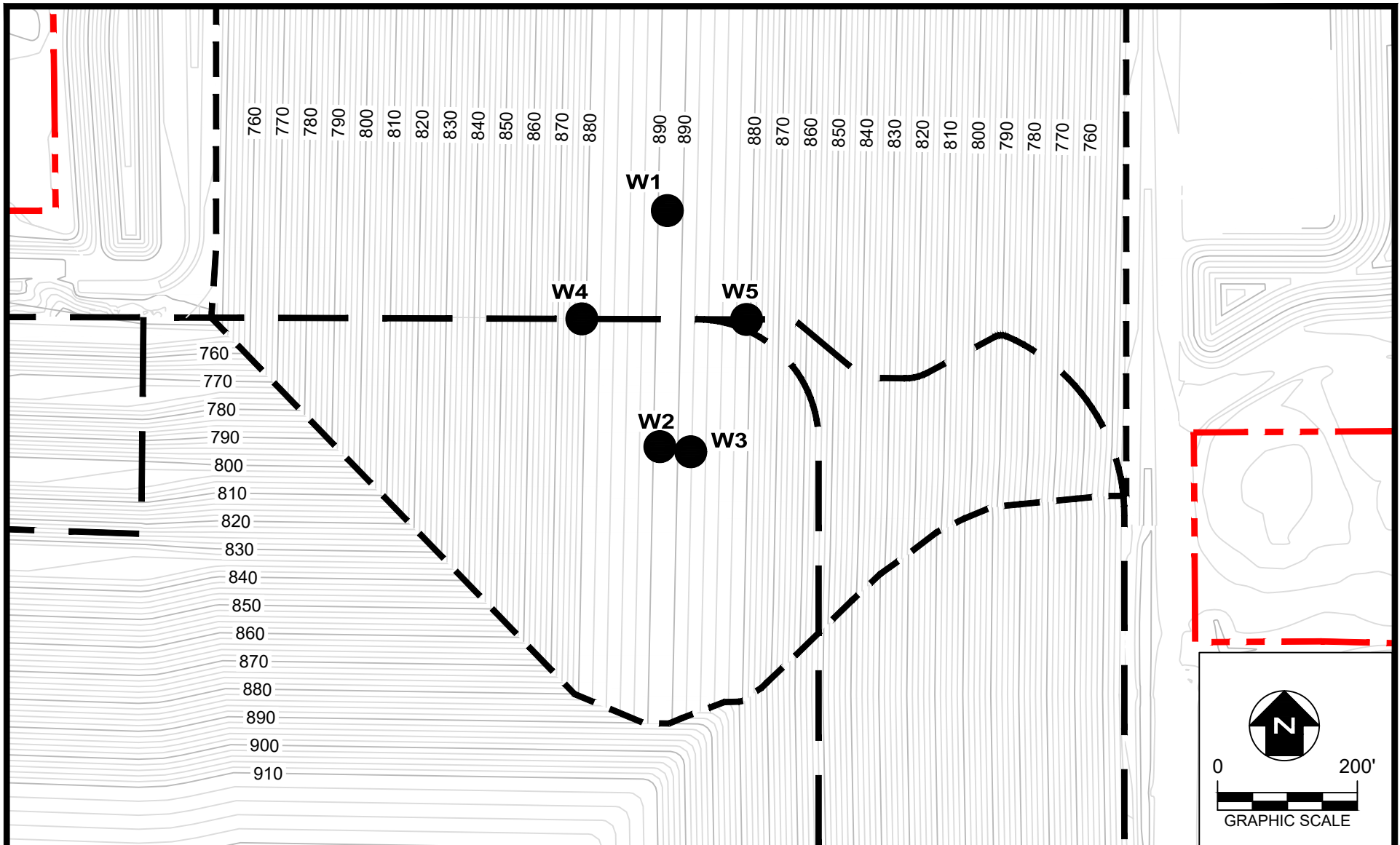
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**FIGURE 2
LOCATION OF WASTE SETTLEMENT ANALYSIS POINTS
TOP OF LINER**

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**FIGURE 3
LOCATION OF WASTE SETTLEMENT ANALYSIS POINTS
TOP OF WASTE**

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Zion Landfill - Site 2 North Expansion - Primary Settlement at Point "W1"
May 2022

Given:	
Primary Settlement Eqtn.	$S_p = [H \times C_c (\log (\sigma'_{z0} + \sigma'_{zf}) / \sigma'_{z0})] / (1 + e_0)$ $C_c' = C_c / (1 + e_0)$ $C_c' = 0.172$ <p>H_{waste} = height of waste fill lift Maximum waste height = 198 feet Cell is divided into fourteen (14) waste lifts: 13-lifts at 15-ft ea., and 14th lift at 3-ft V_{waste (pcf)} = 75 Each lift takes 3 months to complete</p>
Final Cover	H _{final cover (ft)} = 5 V _{final cover (pcf)} = 130.3 Assume 3 months to complete construction of final cover
Stresses	σ'_{z0} = initial effective stress (psf) σ'_{zf} = final effective stress (psf)
Other Information	Each lift takes 3 months to complete (conservative) Life of landfill is assumed to be 30 years

Placement of Lift (mos.)	Lift No.	Depth of Waste Fill Lift (ft)	Total Depth of Waste Fill (ft)	Mid-Lift Stresses (psf)																								Total Primary Settlement "S _p " (ft)	Incremental Primary Settlement "S _p " (ft)						
				Lift 1		Lift 2		Lift 3		Lift 4		Lift 5		Lift 6		Lift 7		Lift 8		Lift 9		Lift 10		Lift 11		Lift 12				Lift 13		Lift 14		Final Cover	
				σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}			σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}
3	1	15	15	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.00	0.78 ft
6	2	15	30	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.78	1.23 ft
9	3	15	45	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.01	1.55 ft
12	4	15	60	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3.56	1.80 ft
15	5	15	75	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5.36	2.01 ft
18	6	15	90	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7.37	2.18 ft
21	7	15	105	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9.55	2.33 ft
24	8	15	120	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11.88	2.46 ft
27	9	15	135	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	14.34	2.58 ft
30	10	15	150	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	16.92	2.69 ft
33	11	15	165	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	19.61	2.78 ft
36	12	15	180	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	22.40	2.87 ft
39	13	15	195	563	7,313	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	25.27	0.68 ft
41	14	3	198	563	7,425	563	6,863	563	6,300	563	5,738	563	5,175	563	4,613	563	4,050	563	3,488	563	2,925	563	2,363	563	1,800	563	1,238	563	675	113	113	0	0	25.95	2.01 ft
44	final cover	5	203	563	7,751	563	7,188	563	6,626	563	6,063	563	5,501	563	4,938	563	4,376	563	3,813	563	3,251	563	2,688	563	2,126	563	1,563	563	1,001	113	438	326	326	27.96	(Σ Sp = 27.96 ft)

Notes:

Primary settlement equation and methodology taken from reference below.
 Incremental settlement is the difference of the total primary settlement number and the previous total primary settlement number.

Reference:

Coduto, D.P., *Geotechnical Engineering Principles and Practices*, 1998, pp.435-446.

**Zion Landfill - Site 2 North Expansion - Secondary Settlement at Point "W1"
May 2022**

Given:	
Waste	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$C_{\alpha'} = C_{\alpha} / (1+e_o) = 0.051$, and $e_o = 0.650$
	$C_{\alpha'} = 0.051$
	Maximum waste height = 198 ft. (waste) + 5 ft. (cover) = 203 ft. Cell is divided into fourteen (14) lifts: 13-lifts at 15-ft and 1-lift at 3-ft $H_o =$ height of lifts 1-13 = 15 ft. lift 14 = 3.0 ft. Assume 3 months to complete each lift: $t_1 =$ 0.25 yrs
Final Cover	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$e_o = 0.432$
	$C_{\alpha'} = 0.0032$
	$H_o =$ height of final cover = 5 feet Assume 3 months to complete construction of final cover
Other Information	Landfill life conservatively assumed = 30 years
	Post Closure monitoring period = 30 years
	$t_1 =$ time of pseudo-primary settlement to occur after completion of fill (years)
	$t_2 =$ time after placed fill and post-closure (years) = (30 + 30 - ($\sum t_x$))

Lift No.	(A) Total Time in Months to Complete Filling of Lifts	(B) Total Time in Years to Complete Filling of Lifts ($\sum t_x$)	(C) t_1 (yrs)	(D) t_2 (yrs)	t_2 / t_1	S_s (ft)
1	3	0.25	0.25	59.75	239	1.819
2	6	0.50	0.25	59.50	238	1.818
3	9	0.75	0.25	59.25	237	1.817
4	12	1.00	0.25	59.00	236	1.815
5	15	1.25	0.25	58.75	235	1.814
6	18	1.50	0.25	58.50	234	1.812
7	21	1.75	0.25	58.25	233	1.811
8	24	2.00	0.25	58.00	232	1.810
9	27	2.25	0.25	57.75	231	1.808
10	30	2.50	0.25	57.50	230	1.807
11	33	2.75	0.25	57.25	229	1.805
12	36	3.00	0.25	57.00	228	1.804
13	39	3.25	0.25	56.75	227	1.802
14	42	3.50	0.25	56.50	226	0.360
final cover	45	3.75	0.25	56.25	225	0.026
$\sum S_s = 23.93$ ft						

Notes:

(A) = 3 months + time for filling previous lifts

(B) = Col.(A) / 12

(C) = (3 mos.) x (1 yr./12mos.) = 0.25

(D) = 30 + 30 - Col.(B)

Secondary settlement equation for waste taken from reference below.

Values for final cover " $c_{\alpha'}$ " and e_o determined from laboratory test data.

Reference:

Yee, K., Menard Geosystems, Lumpur, K., *Upgrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect*, Technical Paper in Master Builders Journal, Sept.1999.

Zion Landfill - Site 2 North Expansion - Primary Settlement at Point "W2"
May 2022

Given:	
Primary Settlement Eqn.	$S_p = [H \times C_c (\log (\sigma'_{z0} + \sigma'_{zf}) / \sigma'_{z0})] / (1 + e_0)$
	$C'_c = C_c / (1 + e_0)$ $C'_c = 0.172$ H_{waste} = height of waste fill lift Maximum waste height = 206 feet Cell is divided into fourteen (14) waste lifts: 13-lifts at 15-ft ea., and 14th lift at 11-ft Y_{waste} (pcf) = 75 Each lift takes 3 months to complete
Final Cover	$H_{final\ cover}$ (ft) = 5 $Y_{final\ cover}$ (pcf) = 130.3 Assume 3 months to complete construction of final cover
Stresses	σ'_{z0} = initial effective stress (psf) σ'_{zf} = final effective stress (psf)
Other Information	Each lift takes 3 months to complete (conservative) Life of landfill is assumed to be 30 years

Placement of Lift (mos.)	Lift No.	Depth of Waste Fill Lift (ft)	Total Depth of Waste Fill (ft)	Mid-Lift Stresses (psf)																												Total Primary Settlement "S _p " (ft)	Incremental Primary Settlement "S _p " (ft)		
				Lift 1		Lift 2		Lift 3		Lift 4		Lift 5		Lift 6		Lift 7		Lift 8		Lift 9		Lift 10		Lift 11		Lift 12		Lift 13		Lift 14				Final Cover	
				σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}			σ'_{z0}	σ'_{zf}
3	1	15	15	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.00	
6	2	15	30	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.78	0.78 ft
9	3	15	45	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.01	1.23 ft
12	4	15	60	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3.56	1.55 ft
15	5	15	75	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5.36	1.80 ft
18	6	15	90	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7.37	2.01 ft
21	7	15	105	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9.55	2.18 ft
24	8	15	120	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11.88	2.33 ft
27	9	15	135	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	14.34	2.46 ft
30	10	15	150	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	16.92	2.58 ft
33	11	15	165	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	19.61	2.69 ft
36	12	15	180	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	22.40	2.78 ft
39	13	15	195	563	7,313	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	25.27	2.87 ft
41	14	11	206	563	7,725	563	7,163	563	6,600	563	6,038	563	5,475	563	4,913	563	4,350	563	3,788	563	3,225	563	2,663	563	2,100	563	1,538	563	975	413	413	0	0	27.53	2.26 ft
44	final cover	5	211	563	8,051	563	7,488	563	6,926	563	6,363	563	5,801	563	5,238	563	4,676	563	4,113	563	3,551	563	2,988	563	2,426	563	1,863	563	1,301	413	738	326	326	29.46	1.93 ft
																																			(Σ Sp = 29.46 ft)

Notes:
 Primary settlement equation and methodology taken from reference below.
 Incremental settlement is the difference of the total primary settlement number and the previous total primary settlement number.
 Reference:
 Coduto, D.P., *Geotechnical Engineering Principles and Practices*, 1998, pp.435-446.



**Zion Landfill - Site 2 North Expansion - Secondary Settlement at Point "W2"
May 2022**

Given:	
Waste	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$C_{\alpha'} = C_{\alpha} / (1+e_o) = 0.051$, and $e_o = 0.650$ $C_{\alpha'} = 0.051$
	Maximum waste height = 206 ft. (waste) + 5 ft. (cover) = 211 ft. Cell is divided into fourteen (14) lifts: 13-lifts at 15-ft and 1-lift at 11-ft $H_o =$ height of lifts 1-13 = 15 ft. lift 14 = 11.0 ft. Assume 3 months to complete each lift: $t_1 =$ 0.25 yrs
Final Cover	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$e_o = 0.432$ $C_{\alpha} = 0.0032$ $H_o =$ height of final cover = 5 feet Assume 3 months to complete construction of final cover
Other Information	Landfill life conservatively assumed = 30 years Post Closure monitoring period = 30 years $t_1 =$ time of pseudo-primary settlement to occur after completion of fill (years) $t_2 =$ time after placed fill and post-closure (years) = (30 + 30 - ($\sum t_x$))

Lift No.	(A) Total Time in Months to Complete Filling of Lifts	(B) Total Time in Years to Complete Filling of Lifts ($\sum t_x$)	(C) t_1 (yrs)	(D) t_2 (yrs)	t_2 / t_1	S_s (ft)
1	3	0.25	0.25	59.75	239	1.819
2	6	0.50	0.25	59.50	238	1.818
3	9	0.75	0.25	59.25	237	1.817
4	12	1.00	0.25	59.00	236	1.815
5	15	1.25	0.25	58.75	235	1.814
6	18	1.50	0.25	58.50	234	1.812
7	21	1.75	0.25	58.25	233	1.811
8	24	2.00	0.25	58.00	232	1.810
9	27	2.25	0.25	57.75	231	1.808
10	30	2.50	0.25	57.50	230	1.807
11	33	2.75	0.25	57.25	229	1.805
12	36	3.00	0.25	57.00	228	1.804
13	39	3.25	0.25	56.75	227	1.802
14	42	3.50	0.25	56.50	226	1.321
final cover	45	3.75	0.25	56.25	225	0.026
$\sum S_s =$ 24.89 ft						

Notes:

(A) = 3 months + time for filling previous lifts

(B) = Col.(A) / 12

(C) = (3 mos.) x (1 yr./12mos.) = 0.25

(D) = 30 + 30 - Col.(B)

Secondary settlement equation for waste taken from reference below.

Values for final cover " c_{α} " and e_o determined from laboratory test data.

Reference:

Yee, K., Menard Geosystems, Lumpur, K., *Upgrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect*, Technical Paper in Master Builders Journal, Sept.1999.

Zion Landfill - Site 2 North Expansion - Primary Settlement at Point "W3"
May 2022

Given:	
Primary Settlement Eqtn.	$S_p = [H \times C_c (\log (\sigma'_{z0} + \sigma'_{zf}) / \sigma'_{z0})] / (1 + e_0)$ $C_c' = C_c / (1 + e_0)$ $C_c' = 0.172$ <p>H_{waste} = height of waste fill lift Maximum waste height = 206 feet Cell is divided into fourteen (14) waste lifts: 13-lifts at 15-ft ea., and 14th lift at 11-ft V_{waste (pcf)} = 75 Each lift takes 3 months to complete</p>
Final Cover	H _{final cover (ft)} = 5 V _{final cover (pcf)} = 130.3 Assume 3 months to complete construction of final cover
Stresses	σ'_{z0} = initial effective stress (psf) σ'_{zf} = final effective stress (psf)
Other Information	Each lift takes 3 months to complete (conservative) Life of landfill is assumed to be 30 years

Placement of Lift (mos.)	Lift No.	Depth of Waste Fill Lift (ft)	Total Depth of Waste Fill (ft)	Mid-Lift Stresses (psf)																								Total Primary Settlement "S _p " (ft)	Incremental Primary Settlement "S _p " (ft)						
				Lift 1		Lift 2		Lift 3		Lift 4		Lift 5		Lift 6		Lift 7		Lift 8		Lift 9		Lift 10		Lift 11		Lift 12				Lift 13		Lift 14		Final Cover	
				σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}			σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}	σ'_{z0}	σ'_{zf}
3	1	15	15	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.00	0.78 ft
6	2	15	30	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.78	1.23 ft
9	3	15	45	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.01	1.55 ft
12	4	15	60	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3.56	1.80 ft
15	5	15	75	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5.36	2.01 ft
18	6	15	90	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7.37	2.18 ft
21	7	15	105	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9.55	2.33 ft
24	8	15	120	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11.88	2.46 ft
27	9	15	135	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	14.34	2.58 ft
30	10	15	150	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	16.92	2.69 ft
33	11	15	165	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	19.61	2.78 ft
36	12	15	180	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	22.40	2.87 ft
39	13	15	195	563	7,313	563	6,750	563	6,188	563	5,625	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	25.27	2.26 ft
41	14	11	206	563	7,725	563	7,163	563	6,600	563	6,038	563	5,475	563	4,913	563	4,350	563	3,788	563	3,225	563	2,663	563	2,100	563	1,538	563	975	413	413	0	0	27.53	1.93 ft
44	final cover	5	211	563	8,051	563	7,488	563	6,926	563	6,363	563	5,801	563	5,238	563	4,676	563	4,113	563	3,551	563	2,988	563	2,426	563	1,863	563	1,301	413	738	326	326	29.46	(Σ Sp = 29.50 ft)

Notes:

Primary settlement equation and methodology taken from reference below.
 Incremental settlement is the difference of the total primary settlement number and the previous total primary settlement number.

Reference:

Coduto, D.P., *Geotechnical Engineering Principles and Practices*, 1998, pp.435-446.

**Zion Landfill - Site 2 North Expansion - Secondary Settlement at Point "W3"
May 2022**

Given:	
Waste	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$C_{\alpha'} = C_{\alpha} / (1+e_o) = 0.051$, and $e_o = 0.650$ $C_{\alpha'} = 0.051$
	Maximum waste height = 206 ft. (waste) + 5 ft. (cover) = 211 ft. Cell is divided into fourteen (14) lifts: 13-lifts at 15-ft and 1-lift at 14-ft $H_o =$ height of lifts 1-13 = 15 ft. lift 14 = 11.0 ft. Assume 3 months to complete each lift: $t_1 = 0.25$ yrs
Final Cover	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$e_o = 0.432$ $C_{\alpha} = 0.0032$ $H_o =$ height of final cover = 5 feet Assume 3 months to complete construction of final cover
Other Information	Landfill life conservatively assumed = 30 years Post Closure monitoring period = 30 years $t_1 =$ time of pseudo-primary settlement to occur after completion of fill (years) $t_2 =$ time after placed fill and post-closure (years) = (30 + 30 - ($\sum t_x$))

Lift No.	(A) Total Time in Months to Complete Filling of Lifts	(B) Total Time in Years to Complete Filling of Lifts ($\sum t_x$)	(C) t_1 (yrs)	(D) t_2 (yrs)	t_2 / t_1	S_s (ft)
1	3	0.25	0.25	59.75	239	1.819
2	6	0.50	0.25	59.50	238	1.818
3	9	0.75	0.25	59.25	237	1.817
4	12	1.00	0.25	59.00	236	1.815
5	15	1.25	0.25	58.75	235	1.814
6	18	1.50	0.25	58.50	234	1.812
7	21	1.75	0.25	58.25	233	1.811
8	24	2.00	0.25	58.00	232	1.810
9	27	2.25	0.25	57.75	231	1.808
10	30	2.50	0.25	57.50	230	1.807
11	33	2.75	0.25	57.25	229	1.805
12	36	3.00	0.25	57.00	228	1.804
13	39	3.25	0.25	56.75	227	1.802
14	42	3.50	0.25	56.50	226	1.321
final cover	45	3.75	0.25	56.25	225	0.026
$\sum S_s = 24.89$ ft						

Notes:

(A) = 3 months + time for filling previous lifts

(B) = Col.(A) / 12

(C) = (3 mos.) x (1 yr./12mos.) = 0.25

(D) = 30 + 30 - Col.(B)

Secondary settlement equation for waste taken from reference below.

Values for final cover " c_{α} " and e_o determined from laboratory test data.

Reference:

Yee, K., Menard Geosystems, Lumpur, K., *Upgrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect*, Technical Paper in Master Builders Journal, Sept.1999.

Zion Landfill - Site 2 North Expansion - Primary Settlement at Point "W4"
May 2022

Given:	
Primary Settlement Eqtn.	$S_p = [H \times C_c (\log (\sigma'_{zo} + \sigma'_{zf}) / \sigma'_{zo})] / (1+e_o)$
	$C_c' = C_c / (1+e_o)$ $C_c' = 0.172$ H_{waste} = height of waste fill lift Maximum waste height = 136.7 feet Cell is divided into ten (10) waste lifts: 9-lifts at 15-ft ea., and 10th lift at 1.7-ft V_{waste} (pcf) = 75 Each lift takes 3 months to complete
Final Cover	$H_{final\ cover}$ (ft) = 5 $V_{final\ cover}$ (pcf) = 130.3 Assume 3 months to complete construction of final cover
Stresses	σ'_{zo} = initial effective stress (psf) σ'_{zf} = final effective stress (psf)
Other Information	Each lift takes 3 months to complete (conservative) Life of landfill is assumed to be 30 years

Placement of Lift (mos.)	Lift No.	Depth of Waste Fill Lift (ft)	Total Depth of Waste Fill (ft)	Mid-Lift Stresses (psf)																				Total Primary Settlement "S _p " (ft)	Incremental Primary Settlement "S _p " (ft)		
				Lift 1		Lift 2		Lift 3		Lift 4		Lift 5		Lift 6		Lift 7		Lift 8		Lift 9		Final Cover					
				σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}			σ'_{zo}	σ'_{zf}
3	1	15	15	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.00	
																											0.78 ft
6	2	15	30	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.78	1.23 ft
																											1.55 ft
9	3	15	45	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.01	1.80 ft
																											3.56
12	4	15	60	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5.36	2.01 ft
																											7.37
15	5	15	75	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	9.55	2.18 ft
																											11.88
18	6	15	90	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	14.34	2.46 ft
																											11.88
21	7	15	105	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	14.34	0.35 ft
																											14.69
24	8	15	120	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	14.69	1.76 ft
																											16.46
27	9	15	135	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	14.34	
																											16.46
30	10	1.7	136.7	563	5,126	563	4,564	563	4,001	563	3,439	563	2,876	563	2,314	563	1,751	563	1,189	563	626	64	64	0	0	14.69	
																											16.46
33	final cover	5	141.7	563	5,452	563	4,890	563	4,327	563	3,765	563	3,202	563	2,640	563	2,077	563	1,515	563	952	64	390	326	326	16.46	(Σ S _p = 16.46ft)

Notes:

Primary settlement equation and methodology taken from reference below.
Incremental settlement is the difference of the total primary settlement number and the previous total primary settlement number.

Reference:

Coduto, D.P., *Geotechnical Engineering Principles and Practices*, 1998, pp.435-446.



Zion Landfill - Site 2 North Expansion - Secondary Settlement at Point "W4"
May 2022

Given:	
Waste	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$C_{\alpha'} = C_{\alpha} / (1+e_o) = 0.051$, and $e_o = 0.650$
	$C_{\alpha'} = 0.051$
	Maximum waste height = 136.7 ft. (waste) + 5 ft. (cover) = 141.7 ft. Cell is divided into ten (10) lifts: 9-lifts at 15-ft and 1-lift at 1.7-ft $H_o =$ height of lifts 1-9 = 15 ft. lift 10 = 1.7 ft. Assume 3 months to complete each lift: $t_1 = 0.25$ yrs
Final Cover	Secondary Settlement Eqtn: $S_s = [(C_{\alpha'}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$e_o = 0.432$ $C_{\alpha} = 0.0032$
	$H_o =$ height of final cover = 5 feet Assume 3 months to complete construction of final cover
Other Information	Landfill life conservatively assumed = 30 years
	Post Closure monitoring period = 30 years
	$t_1 =$ time of pseudo-primary settlement to occur after completion of fill (years) $t_2 =$ time after placed fill and post-closure (years) = (30 + 30 - ($\sum t_x$))

Lift No.	(A)	(B)	(C)	(D)	t_2 / t_1	S_s (ft)
	Total Time in Months to Complete Filling of Lifts	Total Time in Years to Complete Filling of Lifts ($\sum t_x$)	t_1 (yrs)	t_2 (yrs)		
1	3	0.25	0.25	59.75	239	1.819
2	6	0.50	0.25	59.50	238	1.818
3	9	0.75	0.25	59.25	237	1.817
4	12	1.00	0.25	59.00	236	1.815
5	15	1.25	0.25	58.75	235	1.814
6	18	1.50	0.25	58.50	234	1.812
7	21	1.75	0.25	58.25	233	1.811
8	24	2.00	0.25	58.00	232	1.810
9	27	2.25	0.25	57.75	231	1.808
10	30	2.50	0.25	57.50	230	0.205
final cover	33	2.75	0.25	57.25	229	0.026
$\sum S_s = 16.56$ ft						

Notes:

- (A) = 3 months + time for filling previous lifts
- (B) = Col.(A) / 12
- (C) = (3 mos.) x (1 yr./12mos.) = 0.25
- (D) = 30 + 30 - Col.(B)

Secondary settlement equation for waste taken from reference below.
 Values for final cover " c_{α} " and e_o determined from laboratory test data.

Reference:

Yee, K., Menard Geosystems, Lumpur, K., *Upgrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect*, Technical Paper in Master Builders Journal, Sept. 1999.



Zion Landfill - Site 2 North Expansion - Primary Settlement at Point "W5"
May 2022

Given:	
Primary Settlement Eqtn.	$S_p = [H \times C_c (\log (\sigma'_{zo} + \sigma'_{zf}) / \sigma'_{zo})] / (1+e_o)$
	$C_c' = C_c / (1+e_o)$ $C_c' = 0.172$ H_{waste} = height of waste fill lift Maximum waste height = 136.7 feet Cell is divided into ten (10) waste lifts: 9-lifts at 15-ft ea., and 10th lift at 1.7-ft V_{waste} (pcf) = 75 Each lift takes 3 months to complete
Final Cover	$H_{final\ cover}$ (ft) = 5 $V_{final\ cover}$ (pcf) = 130.3 Assume 3 months to complete construction of final cover
Stresses	σ'_{zo} = initial effective stress (psf) σ'_{zf} = final effective stress (psf)
Other Information	Each lift takes 3 months to complete (conservative) Life of landfill is assumed to be 30 years

Placement of Lift (mos.)	Lift No.	Depth of Waste Fill Lift (ft)	Total Depth of Waste Fill (ft)	Mid-Lift Stresses (psf)																				Total Primary Settlement "S _p " (ft)	Incremental Primary Settlement "S _p " (ft)			
				Lift 1		Lift 2		Lift 3		Lift 4		Lift 5		Lift 6		Lift 7		Lift 8		Lift 9		Lift 10				Final Cover		
				σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}	σ'_{zo}	σ'_{zf}			σ'_{zo}	σ'_{zf}	σ'_{zo}
3	1	15	15	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.00	
6	2	15	30	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.78	0.78 ft
9	3	15	45	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.01	1.23 ft
12	4	15	60	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3.56	1.55 ft
15	5	15	75	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	0	0	5.36	1.80 ft
18	6	15	90	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	0	0	7.37	2.01 ft
21	7	15	105	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	0	0	9.55	2.18 ft
24	8	15	120	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	0	0	11.88	2.33 ft
27	9	15	135	563	5,063	563	4,500	563	3,938	563	3,375	563	2,813	563	2,250	563	1,688	563	1,125	563	563	0	0	0	0	0	14.34	2.46 ft
30	10	1.7	136.7	563	5,126	563	4,564	563	4,001	563	3,439	563	2,876	563	2,314	563	1,751	563	1,189	563	626	64	64	0	0	14.69	0.35 ft	
39	final cover	5	141.7	563	5,452	563	4,890	563	4,327	563	3,765	563	3,202	563	2,640	563	2,077	563	1,515	563	952	64	390	326	326	16.46	1.76 ft	
																									(Σ S _p = 16.46 ft)			

Notes:

Primary settlement equation and methodology taken from reference below.
Incremental settlement is the difference of the total primary settlement number and the previous total primary settlement number.

Reference:

Coduto, D.P., *Geotechnical Engineering Principles and Practices*, 1998, pp.435-446.

Zion Landfill - Site 2 North Expansion - Secondary Settlement at Point "W5"
May 2022

Given:	
Waste	Secondary Settlement Eqtn: $S_s = [(C_{\alpha}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$C_{\alpha}' = C_{\alpha} / (1+e_o) = 0.051$, and $e_o = 0.650$
	$C_{\alpha}' = 0.051$
	Maximum waste height = 136.7 ft. (waste) + 5 ft. (cover) = 141.7 ft. Cell is divided into ten (10) lifts: 9-lifts at 15-ft and 1-lift at 1.7-ft $H_o =$ height of lifts 1-9 = 15 ft. lift 10 = 1.7 ft. Assume 3 months to complete each lift: $t_1 = 0.25$ yrs
Final Cover	Secondary Settlement Eqtn: $S_s = [(C_{\alpha}) / (1+e_o)] * (H_o) * (\log (t_2 / t_1))$
	$e_o = 0.432$ $C_{\alpha} = 0.0032$
	$H_o =$ height of final cover = 5 feet Assume 3 months to complete construction of final cover
Other Information	Landfill life conservatively assumed = 30 years
	Post Closure monitoring period = 30 years
	$t_1 =$ time of pseudo-primary settlement to occur after completion of fill (years)
	$t_2 =$ time after placed fill and post-closure (years) = (30 + 30 - ($\sum t_x$))

Lift No.	(A)	(B)	(C)	(D)	t_2 / t_1	S_s (ft)
	Total Time in Months to Complete Filling of Lifts	Total Time in Years to Complete Filling of Lifts ($\sum t_x$)	t_1 (yrs)	t_2 (yrs)		
1	3	0.25	0.25	59.75	239	1.819
2	6	0.50	0.25	59.50	238	1.818
3	9	0.75	0.25	59.25	237	1.817
4	12	1.00	0.25	59.00	236	1.815
5	15	1.25	0.25	58.75	235	1.814
6	18	1.50	0.25	58.50	234	1.812
7	21	1.75	0.25	58.25	233	1.811
8	24	2.00	0.25	58.00	232	1.810
9	27	2.25	0.25	57.75	231	1.808
10	30	2.50	0.25	57.50	230	0.205
final cover	33	2.75	0.25	57.25	229	0.026
$\sum S_s = 16.56$ ft						

Notes:
 (A) = 3 months + time for filling previous lifts
 (B) = Col.(A) / 12
 (C) = (3 mos.) x (1 yr./12mos.) = 0.25
 (D) = 30 + 30 - Col.(B)
 Secondary settlement equation for waste taken from reference below.
 Values for final cover " c_{α} " and e_o determined from laboratory test data.

Reference:
 Yee, K., Menard Geosystems, Lumpur, K., *Upgrading of Existing Landfills by Dynamic Consolidation - A Geotechnical Aspect*, Technical Paper in Master Builders Journal, Sept. 1999.



Published Technical References

UPGRADING OF EXISTING LANDFILLS BY DYNAMIC CONSOLIDATION A GEOTECHNICAL ASPECT

Ir Kenny Yee, Menard Geosystems Sdn Bhd, Kuala Lumpur

ABSTRACT: In recent years, the scarcity of land space available for new urban development has prompted a renewed interest from local authorities in the end use of various landfills or in the extension of the life of existing landfills. Rehabilitation of closed landfills for urban developments has received considerable interest. Likewise, the extension of landfill life to allow for more waste storage is also receiving equal attention. In both cases, ground improvement is required.

Dynamic consolidation (also known as dynamic compaction) is a ground improvement technique. The process involves dropping heavy weights (15ton - 20tons) on to the surface of the fill from a considerable height (15m - 20m) following a selected grid pattern. These high-energy impacts produce sufficient compaction effort to reduce void space, increase density and reduce long-term settlement of the fill. By increasing the density, it increases the storage capacity of the landfill. Beside, it also increases the bearing capacity. Reducing the long-term settlement, roads, parking bays and lighter structures can be designed on shallow foundations on closed landfills.

In this paper, the subject of settlement of waste fills is addressed. A case study concerning a housing development over a landfill is also presented.

1.0 INTRODUCTION

Landfilling is one of the most economic and feasible means of disposing municipal solid waste in Malaysia and other countries in Southeast Asia. In the past, the disposal of waste fills was carried out by uncontrolled dumping into ex-mining ponds and low-lying areas close to housing estates. With increasing scarcity of land in urban areas, it is increasingly difficult to find new landfill sites for future dumping. This has prompted the local authorities and privatized companies (operators of landfill) to find solution to extend the life of the landfill to allow for more waste storage.

Typical landfills may occupy an area ranging from several acres to hundreds of acres. Settlement estimation is a topic of concern. From the operator's viewpoint, landfill capacity will be increased if most settlement occurs during the stage of filling. Unfortunately, the landfill settlement continues over an extended period of time with a final settlement that can be as large as 30%-40% of the initial fill height (H.I.Ling, et.al. 1998). Hence, it is imperative that a solution is needed to increase the rate of settlement to recover the additional space.

Dynamic consolidation is a good method of compacting refuse and waste fill. This technique

involves dropping heavy weights (15 – 20 tons) on to the surface of the fill from a height of 10 to 20m following a selected grid pattern. The high-energy impacts produce shock waves that propagate to great depths (figure 1). As a result, the density of the waste fill is increased and hence, the storage capacity of the landfill is also increased.

With the increase in the density of the waste fill, the overall bearing capacity is improved. The long-term settlement is reduced and hence, the differential settlement is also reduced which is important for the integrity of the cover system when the landfill is closed. In the past such landfills have been considered suitable only for green areas. With the increasing scarcity of land in urban areas, it is making it necessary to build structures above such fills. Charles et.al. (1981) report several case histories of construction on old refuse tips, which include construction of a 2-storey hospital, roads and highways. Welsh (1983) cites a roadway site with 6m to 12m of waste fills. Menard (1984) cites a case for a warehouse designed with floor loads of 20 kN/m² and spread footing with 145 kN/m² with 6m to 17m of refuse waste. There are many other recorded and published case studies on such developments (e.g. Aziz & Mohd. Raihan (992), Downie & Treharne (1979), Faisal, K.Yee & Varaksin (1997), Fryman & Baker (1987),

Lewis & Langer (1994), Mappleback & Fraser (1993), Steinberg & Lukas (1984), etc.).

In this paper, the subject of settlement of waste fills and rehabilitation of landfill for housing development is presented. Only the geotechnical aspect is covered. The related environmental issue has been intentionally left out due to space constraint.

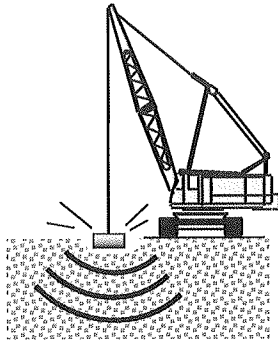


Figure 1

2.0 COMPOSITION OF LANDFILL

Most landfills are heterogeneous and they exhibit anisotropic material properties that are difficult to characterize. Typically, a landfill consists of food and garden wastes, paper products, plastics and rubber, textiles, wood, ashes and the soils used as cover material. Table 1 shows the various components of waste fills with their range of unit weights. The unit weight and void ratio vary with the types of waste, composition, depth, method of compaction and the rate of decomposition, among other factors. The rate of decomposition is further complicated by several factors including the effects of time, temperature and environmental conditions. In short, it is a combination of all of the problems of soft clay, uncompacted fill, organic consolidation and decomposition and even collapse of cavities and erosion of soil into cavities. It is as heterogeneous as the modern industrial-urban complex that produces it. Hence, its composition varies from community to community and from nation to nation. Thus, the waste properties can be considered as site-specific.

Two different forms of landfill can be defined. The uncontrolled dump is of random composition, dumped loosely from trucks,

accumulated without control or compaction, and sometimes covered with a thin layer of soil when it reached its capacity (see figure 2a). At the other end, it is the well-managed sanitary landfill. The materials are spread in layers and compacted by bulldozers and compactors. In some cases, certain wastes such as tires are segregated from others (see figure 2b). Most of the old landfills are the uncontrolled dumps. Until recently, through privatization scheme the landfill operation follows the engineered landfill scheme. Thus, it is expected that developments over old landfills will require more engineering effort.

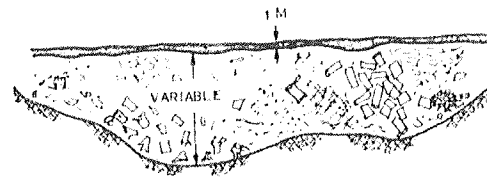


Figure 2(a)

Uncontrolled Landfill (No controlled placement and no compaction)

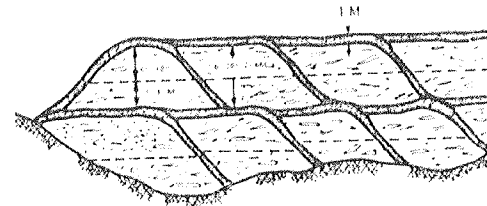


Figure 2(b)

Controlled Sanitary Landfill (Spread and compacted in layers of 2-3m thick; encapsulated with soil in cells of 2-6m thick)

3.0 SETTLEMENT CHARACTERISTICS

Settlement is the major problem with landfills. Sowers (1972) cites a case of a small shopping center built over a landfill. The buildings are on piles driven through the waste fills. The building walls and roof have remained intact. However, floor slabs supported directly on the fill surface have settled as much as 75mm. The floor slabs were connected to the pile-supported exterior grade beams, but was not connected to the interior columns. As a result, the floor drapes downward from the exterior walls toward the interior of the building. Small interior partitions resting directly on the floor have cracked badly and doorframes have been wrecked out of shape.

Table 1 (source: After Tchobanoglous et. al. 1977)

Waste Component	Uncompacted Unit Weight (kN/m ³)	Water Content	Ratio of Compacted to Uncompacted Unit Weight	
			Normal Compaction	Well Compacted
Food waste	1.3 – 4.7	50 – 80	2.9	3.0
Paper / paper board	0.3 – 1.3	4 – 10	4.5	6.2
Plastics	0.3 – 1.3	1 – 4	6.7	10
Textiles	0.3 – 0.9	6 – 15	5.6	6.7
Rubber and leather	0.9 – 2.5	1 – 12	3.3	3.3
Yard waste	0.6 – 2.2	30 – 80	4	5
Wood	1.3 – 3.1	15 – 40	3.3	3.3
Glass	1.6 – 4.7	1 – 4	1.7	2.5
Metals	0.5 – 11.0	2 – 6	4.3	5.3
Ash, brick, dirt	3.1 – 9.4	6 – 12	1.2	1.3

Furthermore, settlement has increased since then, probably due to a change in the moisture environment from leaking sewers in the fill.

There are two possible approaches to the assessment of settlement:

- (a) Extrapolation of monitored data obtained specifically for the given fill
 - 1) By graphical method
 - 2) By analytical method
- (b) Estimation from existing published data on similar type of fills
 - 1) By graphical method
 - 2) By analytical method

Method (a) is the most reliable but requires time for monitoring. This method relies on the approximately linear relationship between settlement and logarithm of time elapsed since placement of waste fill. Method (b) relies on published data for other fills of similar type, and gives approximate answers quickly. However, the results are less dependable since the published data are rarely likely to apply exactly to a specific given fill. Preliminary estimates obtained by method (b) should be checked by monitoring. We shall address the different categories of settlement as follow:

3.1 Settlement Under Self-Weight

One of the contributing factors to the overall settlement is caused by the self-weight of the fill. The time-settlement relationship under self-weight is analogous to the secondary compression of soils after a short period of pseudo-primary settlement, typically, 1 to 4 months long. Measurements taken from past records indicate a coefficient of secondary compression ranging from 0.1 to 0.4 (NAVFAC, 1983). Thus, settlement of the waste fills under its self-weight after completion of filling can be estimated by equation (1) below.

$$(\Delta H)_{sw} = H C_{\alpha} \log (t_2 / t_1) \dots\dots\dots (1)$$

where

- (ΔH)_{sw} = self-weight settlement at time t₂ (m)
- H = thickness of waste fill (m)
- t₁ = time pseudo-primary settlement to occur after completion of fill (years)
- t₂ = time after completion of fill (years)
- C_α = coefficient of secondary compression

Table 2 below suggests typical self-weight settlements. According to Leach & Goodger (1991), a good compaction can reduce the self-weight settlement potential by between 50% and 75%.

Typical unit weights for municipal waste are summarized in Table 3.

Table 4 below shows the unit weights obtained from various landfills sites.

3.2 Settlement Under External Loads

The time-settlement behavior of an old waste fills under an applied load is analogous to the behavior of peat. As load is placed large primary (mechanical) settlements occur rapidly with little or no pore pressure build up. This is followed by secondary compression, which occurs over a long period of time.

The relation of the imposed stress to settlement can be expressed as follow:

$$(\Delta H)_p = H C_r \log (\{ \sigma'_o + \Delta \sigma' \} / \sigma'_o) \dots\dots\dots (2)$$

where

- (ΔH)_p = primary (or mechanical) settlement (m)
- H = thickness of waste fill (m)
- e_o = initial void ratio

Table 2 (Source: Leach & Goodger (1991) – CIRIA Special Publication 78)

Material	Potential Self-Weight Settlement (expressed as % of depth of fill)
Well-compacted, well-graded sand and gravel	0.5
Well-compacted shale and rockfill	0.5
Medium-compacted rockfill	1
Well-compacted clay	0.5
Lightly compacted clay	1.5
Lightly compacted clay placed in deep layers	1 – 2
Nominally compacted opencast backfill	1.2
Uncompacted sand	3.5
Uncompacted (pumped) clay	12
Well-compacted mixed refuse (waste fill)	30
Well-controlled domestic refuse (waste fill) placed in layers and well compacted	10

Table 3

Description	Average Total Unit Weight γ_T (kN/m ³)	Source
Sanitary Landfill		Tchobanoglous et.al. (1977)
• Poor compaction	2.8 – 4.7	
• Moderate to good compaction	4.7 – 7.1	
• Good to excellent compaction	7.1 – 9.4	
• Baled waste	5.5 – 10.5	
• Shredded and compacted	6.4 – 10.5	
• In situ density	5.5 – 6.9	
Active landfill with leachate mound	6.6	
Household Trash Can	1.1	NAVFAC (1983)
Delivery Truck	2.4	
Sanitary Landfill (a) Not shredded		
• Poor compaction	3.1	
• Good compaction	6.3	
• Best compaction	9.4	
Shredded	8.6	
Sanitary Landfill		NSWMA (1985)
• In a landfill	6.9 – 7.5	
After degradation and settlement	9.9 – 11.0	

Table 4

Landfill Sites	Waste Density (kN/m ³)
Old Klang Road, Kuala Lumpur	7.0
Kelana Jaya, Kuala Lumpur	6.0
Merrylands, Sydney ¹	9.4
Thornleigh, Sydney ¹	8.4
Lucas Heights, Sydney ¹	11.3
Albany, New York ²	7 – 16
Fayetteville, Arkansas ³	4.8
Richmond, California ⁴	7.2

Note: 1 – data obtained from Hausmann et.al (1993)

2 – data obtained from Gifford et.al. (1992)

Note: This is a construction and demolition debris landfill

3 – data obtained from Welsh (1983)

4 – data obtained from Sharma et.al. (1989)

σ'_o = effective overburden pressure (kN/m²)
 $\Delta\sigma'$ = effective imposed stress (kN/m²)
 C_r = compression ratio (= $C_c / (1 + e_0)$)
 C_c = compression index

NAVFAC (1983) reports that the primary compression ratio (C_r) ranges from 0.1 to 0.4. Sowers (1972) reports that the compression index (C_c) is related to the initial void ratio as shown in figure 3. The relation can be expressed as follow:

For fills low in organic matters $C_c = 0.15e_0$
 For fills high in organic matters $C_c = 0.55e_0$

It is interesting to note that the maximum C_c for peat is about one-third greater than the maximum observed for waste fills.

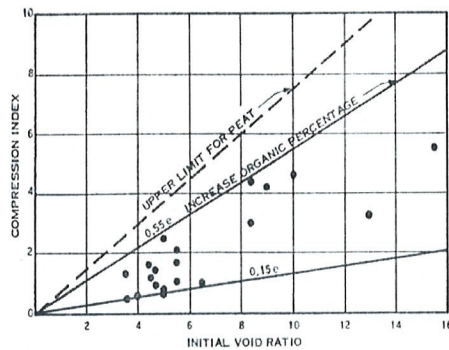


Figure 3

Environmental conditions as well as the composition of the waste fills determine the amount of long-term settlement. This long-term settlement is a combination of mechanical secondary compression, physico-chemical action, and bio-chemical decay. When there is no drastic change in the environment the settlement-log time relationship is more or less linear, similar to secondary compression of soils. The settlement can be expressed by the same equation (1) above. NAVFAC(1983) reported the coefficient of secondary compression (C_α) ranged from 0.02 to 0.07. These values are for fills, which have undergone decomposition for about 10-15 years. Higher compressibility is usually associated with high organic content. It is also true for advanced degree of decomposition.

Sowers (1972) introduces a factor “ α ” for the long-term settlement. He suggested “ α ” as a function of the initial void ratio (e_0). This “ α ” value is high if the organic content subject to decay is large and the environment is favorable (i.e. warm and moist, with fluctuating water table that pumps fresh air into the fill). This value is low for more inert materials and under non-favorable environments. Nonetheless, for any given void ratio there is a large range of values for “ α ” (see figure 4). The relation can be expressed as follow:

For favorable condition to decay $\alpha = 0.03e_0$
 For unfavorable condition to decay $\alpha = 0.09e_0$

This “ α ” value can be translated to the classical C_α by dividing “ α ” by $\{1 + e_0\}$ i.e. $C_\alpha = \alpha / \{1 + e_0\}$.

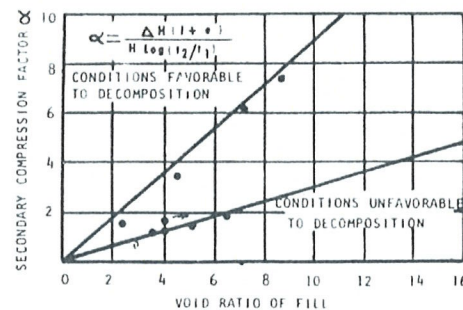


Figure 4

Other calculation methods include the use of a rheological model as presented in the Gibson and Lo theory or the power creep law. The power creep law provides a better representation of the field measured settlement data than the rheological model. However, the rheological model has parameters that can be assigned physical meaning and reflect the effects of certain refuse placement conditions. The details are not presented in this paper.

4.0 DIFFERENTIAL SETTLEMENT & DESIGN MEASURES

There are too many uncertainties for accurate prediction of differential settlement on waste fills. In this case, recourse should be made to the generally accepted rule in engineering practice that, in uniform ground, differential movement will not exceed 75% of the total overall

NOTES ON PRIMARY/INITIAL COMPRESSION OF MSW

Settlement of MSW generally occurs in three phases: initial compression, primary compression, and secondary compression. Initial and primary compression occur rapidly due to the open structure and high hydraulic conductivity of MSW (10^{-3} to 10^{-5} cm/s) and are difficult to separate. Consequently, initial and primary compression usually are grouped together. Secondary compression occurs over longer time frames and is caused by mechanical creep and biological decay. From the perspective of operations and maintenance planning after closure, secondary compression is the most important settlement process. Sowers (1973) indicates that primary compression is normally complete within 30 d and Sharma and De (2007) indicate that 3-4 months is typically assumed in practice. Bjarngard and Edgers (1990) analyzed 24 case histories and reported that primary compression was complete within the first two days following loading.

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FOUNDATIONS
AND
EARTH
RETAINING
STRUCTURES

MUNI BUDHU



Layer 2

$$E_u \text{ at top of layer} = 10,000 \text{ kPa}, E_u \text{ at bottom of layer} = 30,000 \text{ kPa}$$

$$(E_u)_{\text{avg}} = \frac{10,000 + 30,000}{2} = 20,000 \text{ kPa}$$

Step 3: Find weighted harmonic mean E_u .

$$E_u = \frac{2(6000) + 1(20,000)}{3} = 10,667 \text{ kPa}$$

Step 4: Find shape parameter $\frac{A_b}{L_1^2}$

$$A_b = (3 \times 4) + (8 \times 6) + (3 \times 4) = 72 \text{ m}^2, \quad \frac{A_b}{L_1^2} = \frac{72}{12^2} = 0.5$$

Step 5: Find shape, embedment, and wall factors.

$$\mu_s = 0.45(0.5)^{-0.38} = 0.59, \quad \mu_{\text{emb}} = 1 - 0.08 \frac{4}{10} \left(1 + \frac{4}{3} \times 0.5\right) = 0.94$$

$$A_w = \text{perimeter} \times \text{depth} = (3 + 4 + 3 + 4 + 4 + 3 + 4 + 4 + 5 + 6 + 8) \times 4 = 176 \text{ m}$$

$$\frac{A_w}{A_b} = \frac{172}{72} = 2.44, \quad \mu_{\text{wall}} = 1 - 0.16(2.44)^{0.54} = 0.74$$

Step 6: Calculate immediate settlement.

$$\rho_e = \frac{2Q_a}{E_u L_1} (1 - \nu_u^2) \mu_s \mu_{\text{emb}} \mu_{\text{wall}} = \frac{2 \times 5000}{10667 \times 12} (1 - 0.45^2) \times 0.59 \times 0.94 \times 0.74 = 0.026 \text{ m} = 26 \text{ mm}.$$

7.7.2 Primary Consolidation Settlement

Primary consolidation settlement for fine-grained soils is normally estimated from parameters deduced from the one-dimensional consolidation test. In using the parameters from the one-dimensional consolidation test, you are assuming (1) vertical uniform strain only (lateral strain is zero), (2) no settlement occurs from shear stresses, (3) saturated soil, (4) the initial excess porewater pressure is equal to the change in applied stress at the instant the load is applied, and (5) excess porewater pressures are dissipated only vertically. In practice, lateral strains are significant except when the ratio of the layer thickness to lateral dimension of the loaded area is small (approaches zero). Shear stresses also cause settlement and excess porewater pressures can dissipate not only in the vertical but in any direction.

The initial excess porewater pressure is equal to the change in vertical stress at the instant the vertical stress is applied is possible only if the lateral stresses are equal to the vertical stresses. If the lateral strains are zero, then under undrained conditions (at the instant the load is applied), the vertical settlement is zero.

Normally Consolidated Clays The primary consolidation settlement for normally consolidated soils ($\text{OCR} = 1$) occurs solely along the compression line (NCL) and is given as

$$\rho_{pc} = H \frac{\Delta e}{1 + e_0} = \frac{H}{1 + e_0} C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}; \quad \text{OCR} = 1 \quad (7.49)$$

where H is the thickness of the soil layer, $\Delta e = C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}$, C_c is the compression index, e_0 ($e_0 = wG_s$) is the initial void load ratio, $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma_z$, σ'_{z_0} is the preconsolidation vertical effective stress, and $\Delta\sigma_z$ is the increase in vertical stress at the center of the layer.

Overconsolidated Clays If the soil were overconsolidated, we would have to consider two cases depending on the magnitude of $\Delta\sigma_z$ (Fig. 7.16). In the first case (Fig. 7.16a), the increase in $\Delta\sigma_z$ is such that $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma_z$ is less than the preconsolidation vertical effective stress, σ'_{zc} . In this case, consolidation occurs along the unloading/reloading line (URL), and

$$\rho_{pc} = \frac{H}{1 + e_0} C_r \log \frac{\sigma'_{\text{fin}}}{\sigma'_{z_0}}; \quad \sigma'_{\text{fin}} < \sigma'_{zc} \quad (7.50)$$

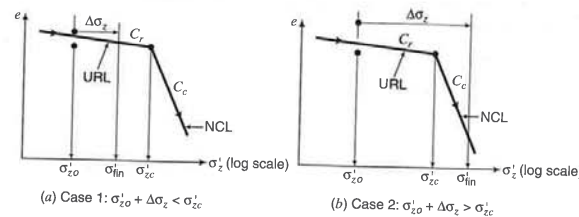


FIGURE 7.16 Two cases to consider for calculating primary consolidation settlement of overconsolidated soils.

In case 2 (Fig. 7.16b), the increase in $\Delta\sigma_z$ is such that $\sigma'_{\text{fin}} = \sigma'_{z_0} + \Delta\sigma'_z$ is greater than σ'_{zc} . In this case, we have to consider two components of settlement—one along the URL and the other along the NCL. The primary consolidation for this case is

$$\rho_{pc} = \frac{H}{1 + e_0} \left(C_r \log \frac{\sigma'_{zc}}{\sigma'_{z_0}} + C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{zc}} \right); \quad \sigma'_{\text{fin}} > \sigma'_{zc} \quad (7.51)$$

or

$$\rho_{pc} = \frac{H}{1 + e_0} \left\{ C_r \log(\text{OCR}) + C_c \log \frac{\sigma'_{\text{fin}}}{\sigma'_{zc}} \right\}; \quad \sigma'_{\text{fin}} > \sigma'_{zc} \quad (7.52)$$

Primary Consolidation Settlement Using m_v . You can also calculate the primary consolidation settlement using the modulus of compressibility, m_v . However, unlike C_c , which is constant, m_v varies with vertical effective stress levels. You should compute an average value of m_v over the stress range σ'_{z_0}

J.5-B Final Cover Geomembrane Strain



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Proj. #: 631020105

Calculated By: ORC

Date: 05/2022

Checked By: DAM

Date: 05/2022

TITLE: FINAL COVER GEOMEMBRANE STRAIN

Problem Statement:

Determine whether the final cover geomembrane has the required strength to withstand the normal stresses imposed by the waste stabilization process as required by 35 Ill. Admin. Code Section 811.314(b)(3)(B)(ii).

Given:

- The final cover system design includes slopes of 4H:1V and 10H:1V slopes.
- Poly-Flex (Manufacturer) product information for 40-mil textured LLDPE geomembrane (refer to attached pages).

Assumptions:

- The allowable strain for the final cover geomembrane is 30% (manufacturer's specifications).

Calculations:

AutoCAD 2018 was used to determine the maximum differential settlement dimensions that occur based on the initial design final cover slopes and maximum 30% allowable strain. The maximum allowable strain was then calculated using the following equation:

$$\text{Strain } (\varepsilon) = \frac{|\Delta H|}{L_o}$$

Please see the attached page for the supporting calculations.

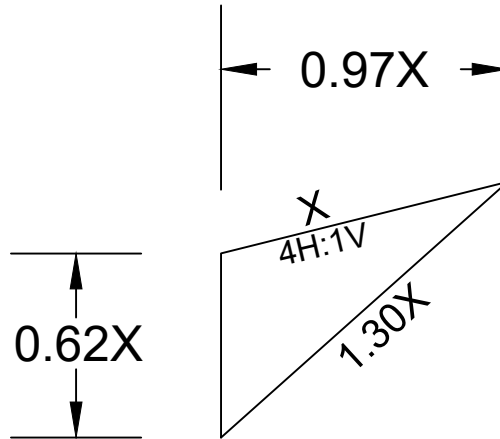
Results:

The results indicate that the geomembrane can accommodate a differential settlement of 64% for 4H:1V slopes before reaching its allowable strain limit. A differential settlement of 64% far exceeds the maximum differential settlement that was calculated for the final cover slopes due to waste settlement (please refer to **Appendix J.5-A**). However, the final cover will be routinely observed for differential settlement. The geomembrane will be evaluated for over-stressing in locations where differential settlement exceeds 64%.

INITIAL 4H:1V SLOPE

MAXIMUM DIFFERENTIAL SETTLEMENT:

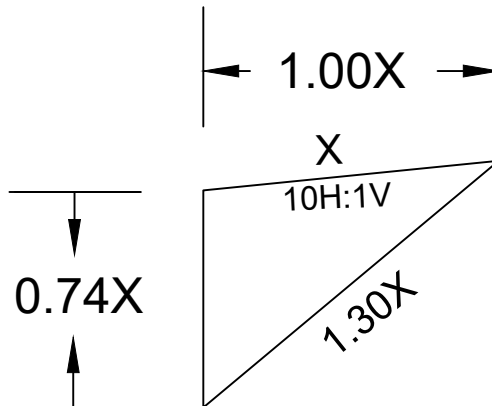
$$dH/dL * 100\% = (0.62X) / (0.97X) * 100\% = 64\%$$



INITIAL 10H:1V SLOPE

MAXIMUM DIFFERENTIAL SETTLEMENT:

$$dH/dL * 100\% = (0.74X) / (1.00X) * 100\% = 74\%$$



NOTES

MAXIMUM DIFFERENTIAL SETTLEMENT IS CALCULATED ASSUMING A MAXIMUM ALLOWABLE GEOMEMBRANE STRAIN OF 30% THIS VALUE IS BASED ON MANUFACTURER'S RECOMMENDATIONS.



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**GFL EVERGLADES HOLDINGS, LLC
ZION LANDFILL - SITE 2 NORTH**

**MAX. ALLOWABLE DIFFERENTIAL SETTLEMENT
BASED ON ALLOWABLE GEOMEMBRANE STRAIN**

APPROVED BY: RDS	PROJ. NO.:	003211	DATE: FEB. 2020
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Published Technical References

TEXTURED LLDPE GEOMEMBRANE

ENGLISH UNITS



Minimum Average Values

Property	Test Method	40 Mil	60 Mil	80 Mil
Thickness, mils	ASTM D 5994			
minimum average		38	57	76
lowest individual of 8 of 10 readings		36	54	72
lowest individual of 10 readings		34	51	68
Asperity Height ¹ , mils	ASTM D 7466	10	10	10
Sheet Density, g/cc (max.)	ASTM D 1505/D 792	0.939	0.939	0.939
Tensile Properties²	ASTM D 6693			
1. Break Strength, lb/in		60	90	120
2. Break Elongation, %		250	250	250
2% Modulus, lb/in ² (max.)	ASTM D 5323	60,000	60,000	60,000
Tear Resistance, lb	ASTM D 1004	22	33	44
Puncture Resistance, lb	ASTM D 4833	44	66	88
Axi-Symmetric Break Strain, %	ASTM D 5617	30	30	30
Carbon Black Content ³ , %	ASTM D 1603	2.0 - 3.0	2.0 - 3.0	2.0 - 3.0
Carbon Black Dispersion	ASTM D 5596		--Note 4--	
Oxidative Induction Time (OIT)				
Standard OIT, minutes	ASTM D 3895	100	100	100
Oven Aging at 85°C	ASTM D 5721			
High Pressure OIT - % retained after 90 days	ASTM D 5885	60	60	60
UV Resistance ⁵	ASTM D 7238			
High Pressure OIT ⁶ - % retained after 1600 hrs	ASTM D 5885	35	35	35
Roll Dimensions				
1. Width (feet):		23	23	23
2. Length (feet):		750	500	375
3. Area (square feet):		17,250	11,500	8,625
4. Gross weight (pounds, approx.):		3,465	3,465	3,435

1 Of 10 readings; 8 must be ≥ 7 mils and lowest individual reading must be ≥ 5 mils.
2 Machine direction (MD) and cross machine direction (XMD) average values should be on the basis of 5 test specimens each direction. Break elongation is calculated using a gauge length of 2.0 inches.
3 Other methods such as ASTM D 4218 or microwave methods are acceptable if an appropriate correlation can be established.
4 Carbon black dispersion for 10 different views: Nine in Categories 1 and 2 with one allowed in Category 3.
5 The condition of the test should be 20 hr. UV cycle at 75°C followed by 4 hr. condensation at 60°C.
6 UV resistance is based on percent retained value regardless of the original HP-OIT value.
These data are provided for informational purposes only and are not intended as a warranty or guarantee. Poly-America, L.P. assumes no responsibility in connection with the use of these data. Suitability for a particular use shall be determined by and is the sole responsibility of the end user. These values are subject to change without notice. REV. 03/14

J.5-C Final Cover Geocomposite Transmissivity



Client: Zion Landfill, Inc.
 Project: Zion Landfill – Site 2 North Expansion
 Project #: 631020105
 Calculated By: ORC
 Checked By: DAM

Date: 05/2022

Date: 05/2022

TITLE: FINAL COVER GEOCOMPOSITE TRANSMISSIVITY

Problem Statement

Determine whether the 6-oz/yd² final cover geocomposite will remain free-draining based on stormwater impingement rates through the final cover.

Given

- Qian, Xuede et. al, Geotechnical Aspects of Landfill Design and Construction. (Refer to attached pages).
- Product information sheet from GSE FabriNet 200-mil Geocomposite. (Refer to attached pages).
- Hydrogeologic Evaluation of Landfill Performance (HELP) model. (see **Appendix K**).
- Landfill design specifications for layer types and thicknesses.

Assumptions

- The final cover system includes the following components, from top to bottom:
 - 3-foot protective soil cover (6-inch vegetative soil layer and 2.5-foot protective soil layer)
 - 6-oz/yd² double-sided geocomposite drainage layer
 - 40-mil textured LLDPE geomembrane
 - 2-foot final cover barrier soil ($k \leq 1 \times 10^{-5}$ cm/sec)
- The minimum slope of final landform is 10H:1V (0.10 ft/ft)
- The maximum daily peak head within final cover over geomembrane is 0.075 in, or 0.00625 ft (see HELP model results for Model 3: 70-years after post-closure care period).
- The hydraulic conductivity of the final cover soils is assumed to be 4.2×10^{-5} cm/sec (see HELP).
- The transmissivity of the geocomposite is 0.5 gal/(min*ft) (see attached pages)
- The total final cover thickness above the geocomposite is 3 feet.
- Flow rate is determined using Darcy's Law:

$$Q_{FC} = k_s(i)(A)$$

where:

Q_{FC} = flow rate through the final cover soils, ft³/sec

k_s = hydraulic conductivity of the final cover, ft/sec

i = hydraulic gradient = $(h + D) / D$

h = head of final cover



Client: Zion Landfill, Inc.
 Project: Zion Landfill – Site 2 North Expansion
 Project #: 631020105
 Calculated By: ORC
 Checked By: DAM

Date: 05/2022
 Date: 05/2022

TITLE: FINAL COVER GEOCOMPOSITE TRANSMISSIVITY

D = thickness of the final cover above the geocomposite
 A = area over which flow occurs, ft² (unit area = 1 ft²)

- Assume that the final cover is saturated and there is no soil suction. Then hydraulic gradient is given by:

$$i = (h + D) / D$$

where:

i = hydraulic gradient
 h = head within final cover above geocomposite
 D = thickness of the final cover soils above the geocomposite

- The flow rate of the geocomposite may be lower than manufacturer specifications due to clogging and deformation.

$$Q_{\text{allow}} = \frac{Q_{\text{GC}}}{RF_{\text{IN}} * RF_{\text{CR}} * RF_{\text{CC}} * RF_{\text{BC}}}$$

Where:

Q_{allow} = Allowable flow rate after reduction factors
 Q_{GC} = Calculated geocomposite flow rate from above rate
 RF_{IN} = Reduction factor for elastic deformation
 RF_{CR} = reduction factor for creep deformation
 RF_{CC} = reduction factor for chemical clogging
 RF_{BC} = Reduction factor for biological clogging

The following reduction factors are assumed (RF_{IN} = 1.5, RF_{CR} = 1.4, RF_{CC} = 1.2, and RF_{BC} = 1.5). These value are the upper end values based on testing on geocomposites under normal stresses.

Calculations

1. Determine maximum flow rate through final cover:

$$Q_{\text{FC}} = k_s(i)(A) = \frac{k_s A (h+D)}{D} = \frac{(4.2 \times 10^{-5} \text{ cm/sec} \times 0.033 \text{ ft/cm})(1 \text{ ft}^2)(0.00625 \text{ ft} + 3 \text{ ft})}{3 \text{ ft}} = 1.4 \times 10^{-6} \text{ ft}^3/\text{sec}$$

2. Convert transmissivity of geocomposite to ft³/sec:

$$\text{Transmissivity} = \frac{0.5 \text{ gal}}{\text{min} \cdot \text{ft}} \times \frac{1 \text{ ft}^3}{7.48 \text{ gal}} \times \frac{1 \text{ min}}{60 \text{ sec}} = 0.001 \frac{\text{ft}^2}{\text{sec}}$$

3. Determine maximum flow rate through the geocomposite based on manufacturer data:

$$Q_{\text{GC}} = k_{\text{GC}}(i)(A) = k_{\text{GC}}(i)(wt) = (k_{\text{GC}}t)(i)(w) = \theta_{\text{GC}}iw$$



Client: Zion Landfill, Inc.
 Project: Zion Landfill – Site 2 North Expansion
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 Date: 05/2022

TITLE: FINAL COVER GEOCOMPOSITE TRANSMISSIVITY

$$Q_{GC} = \theta_{GC}(i)(A) = (0.001 \frac{\text{ft}^2}{\text{sec}}) (0.10 \frac{\text{ft}}{\text{ft}})(1\text{ft}) = 1.0 \times 10^{-4} \text{ ft}^3/\text{sec}$$

where:

Q_{GC} = flow rate through the geocomposite, cm^3/sec

k_{GC} = hydraulic conductivity of the geocomposite, cm/sec

θ_{GC} = Transmissivity of Geocomposite

i = hydraulic gradient based on slope (ft/ft)

w = flow width (unit width = 1 ft)

4. Apply reduction factors to determine allowable field geocomposite flow rate:

$$Q_{\text{allow}} = \frac{Q_{GC}}{RF_{IN} * RF_{CR} * RF_{CC} * RF_{BC}}$$

$$Q_{\text{allow}} = \frac{1.0 \times 10^{-4} \text{ ft}^3/\text{sec}}{1.5 * 1.4 * 1.2 * 1.5} = 2.6 \times 10^{-5} \text{ ft}^3/\text{sec}$$

5. Verify that geocomposite has a higher flow rate than the impingement rate through the final cover:

If $Q_{FC} < Q_{\text{allow}} \rightarrow$ Free draining conditions

$1.4 \times 10^{-6} \text{ ft}^3/\text{sec} < 2.6 \times 10^{-5} \text{ ft}^3/\text{sec} \rightarrow$ Free draining conditions

Results

The GSE 6 oz/yd² double-sided Fabrinet 200-mil Geocomposite will remain free-draining based on the maximum impingement rates through the final cover.

Published Technical References

GEOTECHNICAL ASPECTS *of* LANDFILL DESIGN *and* CONSTRUCTION



Xuede Qian • Robert M. Koerner • Donald H. Gray

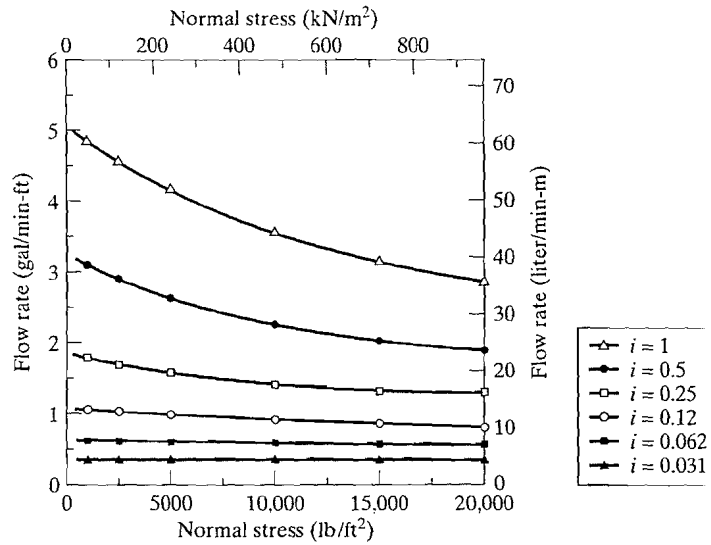


FIGURE 8.10 Flow Rate Behavior of a 0.25-inch (6.3-mm) Geonet Sandwiched between a 16-oz/yd² (540-g/m²) Nonwoven Needle-Punched Geotextile with Clay above and a 60-mil (1.5-mm) HDPE Geomembrane below (USEPA, 1989)

geotextile-geonet system also must be capable of sustaining these loads over time, suggesting that long-term tests (or reduction factors) be required to adequately assess such situations as well.

8.4.3 Allowable Geonet Flow Rate

An allowable flow rate must be extracted from hydraulic testing of the type just described. Accordingly, it is necessary to assess how realistic the test setup is in contrast to the actual field conditions. If it does not model site-specific conditions adequately, then some adjustments to the laboratory value must be made. This is often the case. As mentioned previously, the laboratory-generated index value is treated as an ultimate value, which means that when using ASTM D4716 for flow rate determination, the value must be reduced before used in design. In other words,

$$q_{\text{allowable}} < q_{\text{ultimate}}$$

The recommended way of doing this is to prescribe reduction factors on each of the items not assessed adequately in the laboratory test. For example (Koerner, 1998),

$$q_{\text{allow}} = \frac{q_{\text{ult}}}{RF_{\text{IN}} \times RF_{\text{CR}} \times RF_{\text{CC}} \times RF_{\text{BC}}} \tag{8.17}$$

where q_{ult} = ultimate flow rate determined from ASTM D4716 for short-term index tests between solid plates using water as the transported liquid under laboratory test temperatures;

q_{allow} = allowable flow rate for final design purposes;

TABLE 8.7 Flow Rates and Reduction from Curves of Figures 8.9 and 8.10 (modified from Koerner, 1994)

Normal Stress	Cross Section		Hydraulic Gradient (<i>i</i>)					
			0.03	0.06	0.12	0.25	0.50	1.00
1,000 lb/ft ² (48 kN/m ²)	HDPE (both sides)	gal/min-m	0.5	0.9	1.5	2.5	4.4	7.5
		liter/min-m	6.2	11.2	18.6	31.1	54.6	93.2
	GT/Clay (one side)	gal/min-m	0.4	0.7	1.1	1.8	3.2	5.0
		liter/min-m	5.0	8.7	13.7	22.4	39.7	62.1
	Difference	gal/min-m	0.1	0.2	0.4	0.7	1.2	2.5
		liter/min-m	1.2	2.5	4.9	8.7	14.9	31.1
Reduction			20%	22%	30%	28%	28%	33%
5,000 lb/ft ² (240 kN/m ²)	HDPE (both sides)	gal/min-m	0.5	0.8	1.4	2.3	3.9	6.2
		liter/min-m	6.2	9.9	17.4	28.6	48.4	77.0
	GT/Clay (one side)	gal/min-m	0.4	0.6	1.0	1.6	2.7	4.3
		liter/min-m	5.0	7.5	12.4	19.9	33.5	53.4
	Difference	gal/min-m	0.1	0.2	0.4	0.7	1.2	1.9
		liter/min-m	1.2	2.4	5.0	8.7	14.9	23.6
Reduction			20%	25%	29%	30%	31%	31%
10,000 lb/ft ² (480 kN/m ²)	HDPE (both sides)	gal/min-m	0.4	0.8	1.3	2.2	3.8	5.7
		liter/min-m	5.0	9.9	16.1	27.3	47.2	70.8
	GT/Clay (one side)	gal/min-m	0.3	0.6	0.9	1.4	2.3	3.6
		liter/min-m	3.7	7.5	11.2	17.4	28.6	44.7
	Difference	gal/min-m	0.1	0.2	0.4	0.8	1.5	2.1
		liter/min-m	1.3	2.4	4.9	9.9	18.6	26.1
Reduction			25%	25%	31%	36%	39%	37%
20,000 lb/ft ² (960 kN/m ²)	HDPE (both sides)	gal/min-m	0.4	0.6	0.9	1.6	2.7	4.3
		liter/min-m	5.0	7.5	11.2	19.9	33.5	53.4
	GT/Clay (one side)	gal/min-m	0.3	0.5	0.8	1.3	1.9	2.8
		liter/min-m	3.7	6.2	9.9	16.1	23.6	34.8
	Difference	gal/min-m	0.1	0.1	0.1	0.3	0.8	1.5
		liter/min-m	1.3	1.3	1.3	3.8	9.9	18.6
Reduction			25%	17%	11%	19%	30%	35%

RF_{IN} = reduction factor for elastic deformation, or intrusion, of the adjacent geosynthetics into the geonet's core space;

RF_{CR} = reduction factor for creep deformation of the geonet and adjacent geosynthetics into geonet's core space;

RF_{CC} = reduction factor for chemical clogging and/or precipitation of chemicals in the geonet's core space; and

RF_{BC} = reduction factor for biological clogging in the geonet's core space.

Some guidelines as to various reduction factors to be used in different situations are given in Table 8.8. Note that these values are based on preliminary and relatively sparse information. Other reduction factors, such as installation damage, viscosity effects and temperature effects, could also have been incorporated. If needed, they can be included on a site-specific basis. An example problem follows, which illustrates the use of geonets and points out that high factors of safety are warranted in critical situations.

EXAMPLE 8.2

What is the allowable geonet flow rate to be used in the design of a secondary leachate collection (i.e., leak detection) system? Assume that laboratory testing at proper design load and proper hydraulic gradient gave a short-term between-rigid-plate index value of 1.2 gal/min-ft (14.9 liter/min-m).

Solution: Average values from Table 8.8 are used (however, note the large resulting reduction):

$$\begin{aligned} q_{\text{allow}} &= \frac{q_{\text{ult}}}{RF_{IN} \times RF_{CR} \times RF_{CC} \times RF_{BC}} & (8.17) \\ &= \frac{1.2}{1.75 \times 1.7 \times 1.75 \times 1.75} \\ &= \frac{1.2}{9.11} = 0.13 \text{ gal/min-ft (1.6 liter/min-m)} \end{aligned}$$

TABLE 8.8 Recommended Preliminary Reduction Factors for Determining Allowable Flow Rate or Transmissivity of Biplanar Geonets (Koerner, 1998)

Application Area	Reduction Factor Values			
	RF_{IN}	RF_{CR}^*	RF_{CC}	RF_{BC}
Sport fields	1.0 to 1.2	1.0 to 1.5	1.0 to 1.2	1.1 to 1.3
Capillary break	1.1 to 1.3	1.0 to 1.2	1.1 to 1.5	1.1 to 1.3
Roof and plaza decks	1.2 to 1.4	1.0 to 1.2	1.0 to 1.2	1.1 to 1.3
Retaining walls, seeping rock and soil slopes	1.3 to 1.5	1.2 to 1.4	1.1 to 1.5	1.0 to 1.5
Drainage blankets	1.3 to 1.5	1.2 to 1.4	1.0 to 1.2	1.0 to 1.2
Surface water drains for landfill caps	1.3 to 1.5	1.1 to 1.4	1.0 to 1.2	1.2 to 1.5
Secondary leachate collection (landfill)	1.5 to 2.0	1.4 to 2.0	1.5 to 2.0	1.5 to 2.0
Primary leachate collection (landfill)	1.5 to 2.0	1.4 to 2.0	1.5 to 2.0	1.5 to 2.0

*These values are sensitive to the density of the resin used in the geonet's manufacture. The higher the density, the lower the reduction factor. Creep of the covering geotextile(s) is a product-specific issue.

8.4.4 Designing with Geonets for Drainage

Several design problems and their solutions are presented in this section. The necessary theory for proper understanding of the example problems is summarized at the beginning.

8.4.4.1 Flow Rate Factor of Safety. "Design by Function" requires the formulation of a factor of safety as follows:

$$FS = \frac{\text{Allowable (test) value}}{\text{Required (design) value}}$$

For geonets serving as a drainage medium, the targeted value is flow rate and the above equation can be written as

$$FS = q_{\text{allow}}/q_{\text{reqd}} \quad (8.18)$$

where q_{allow} = the allowable flow rate; and
 q_{reqd} = the required flow rate.

8.4.4.2 Transmissivity. For saturated systems under laminar flow conditions, Darcy's formula can be used to arrive at an alternative to flow rate, namely, the transmissivity. Darcy's formula states that

$$Q = k \cdot i \cdot A = k \cdot i \cdot (w \cdot t) \quad (8.19)$$

$$Q = (k \cdot t) \cdot i \cdot w = \theta \cdot i \cdot w \text{ and} \quad (8.20)$$

$$\theta = k \cdot t = Q/(i \cdot w) \quad (8.21)$$

where Q = the volumetric flow rate, cm³/sec;
 k = the coefficient of permeability, cm/sec;
 i = the hydraulic gradient,
 A = the flow cross-sectional area, cm²;
 θ = the transmissivity, cm²/sec;
 w = the width, cm; and
 t = the thickness, cm.

In a similar manner as the preceding, a factor of safety can be formulated using transmissivity (viz., $FS = \theta_{\text{allow}}/\theta_{\text{reqd}}$). Thus, Q/w and transmissivity carry the same units and are directly related to one another by means of the hydraulic gradient i . At a hydraulic gradient of 1.0, they are numerically identical. At other values of hydraulic gradient, they are not equal. Also note that the system must be saturated and flow must be laminar in order to use transmissivity. When in doubt, it is usually best to use flow rate per unit width.

8.4.4.3 Required Flow Rate. The required flow rate for the leachate drainage layer can be calculated from the equations

$$q_{\text{reqd}} = (r \cdot w \cdot L_H)/w \quad (\text{suitable only for square or rectangular cell}) \quad (8.22)$$

and

$$q_{\text{reqd}} = [r \cdot (L_H)_{\text{max}} \cdot dw]/dw \quad (8.23)$$

GSE FabriNet 200 mil Geocomposite

GSE FabriNet geocomposite consists of a 200 mil thick GSE HyperNet geonet heat-laminated on one or both sides with a GSE nonwoven needle-punched geotextile. The geotextile is available in mass per unit area range of 6 oz/yd² to 16 oz/yd². The geocomposite is designed and formulated to perform drainage function under a range of anticipated site loads, gradients and boundary conditions.



AT THE CORE:
A 200 mil thick HyperNet geonet heat-laminated on one or both sides with a nonwoven needlepunched geotextile.

Product Specifications

Tested Property	Test Method	Frequency	Minimum Average Roll Value ⁽¹⁾		
			6 oz/yd ²	8 oz/yd ²	10 oz/yd ²
Geocomposite					
Transmissivity ⁽²⁾ , gal/min/ft, (m ² /sec) Double-Sided Composite Single-Sided Composite	ASTM D 4716	1/540,000 ft ²	0.5 (1x10 ⁻⁴) 4.8 (1x10 ⁻³)	0.5 (1x10 ⁻⁴) 4.8 (1x10 ⁻³)	0.4 (9x10 ⁻⁵) 4.3 (9x10 ⁻⁴)
Ply Adhesion, lb/in			ASTM D 7005	1/50,000 ft ²	1.0
Geonet Core^(1,3) - GSE HyperNet					
Geonet Core Thickness, mil	ASTM D 5199	1/50,000 ft ²	200	200	200
Transmissivity ⁽²⁾ , gal/min/ft (m ² /sec)	ASTM D 4716		9.6 (2 x 10 ⁻³)	9.6 (2 x 10 ⁻³)	9.6 (2 x 10 ⁻³)
Density, g/cm ³	ASTM D 1505	1/50,000 ft ²	0.94	0.94	0.94
Tensile Strength (MD), lb/in	ASTM D 7179	1/50,000 ft ²	45	45	45
Carbon Black Content, %	ASTM D 4218	1/50,000 ft ²	2.0	2.0	2.0
Geotextile^(1,3)					
Mass per Unit Area, oz/yd ²	ASTM D 5261	1/90,000 ft ²	6	8	10
Grab Tensile Strength, lb	ASTM D 4632	1/90,000 ft ²	160	220	260
Grab Elongation	ASTM D 4632	1/90,000 ft ²	50%	50%	50%
CBR Puncture Strength, lb	ASTM D 6241	1/540,000 ft ²	435	575	725
Trapezoidal Tear Strength, lb	ASTM D 4533	1/90,000 ft ²	65	90	100
AOS, US sieve ⁽¹⁾ , (mm)	ASTM D 4751	1/540,000 ft ²	70 (0.212)	80 (0.180)	100 (0.150)
Permittivity, sec ⁻¹	ASTM D 4491	1/540,000 ft ²	1.5	1.3	1.0
Water Flow Rate, gpm/ft ²	ASTM D 4491	1/540,000 ft ²	110	95	75
UV Resistance, % retained	ASTM D 4355 (after 500 hours)	per formulation	70	70	70
NOMINAL ROLL DIMENSIONS⁽⁴⁾					
Roll Width, ft			14.75	14.75	14.75
Roll Length, ft	Double-Sided Composite		270	260	230
	Single-Sided Composite		300	300	290
Roll Area, ft ²	Double-Sided Composite		3,982	3,835	3,392
	Single-Sided Composite		4,425	4,425	4,277

NOTES:

- ⁽¹⁾ All geotextile properties are minimum average roll values except AOS which is maximum average roll value and UV resistance is typical value. Geonet core thickness is nominal value.
- ⁽²⁾ Gradient of 0.1, normal load of 10,000 psf, water at 70°F between steel plates for 15 minutes. Contact GSE for performance transmissivity value for use in design.
- ⁽³⁾ Component properties prior to lamination.
- ⁽⁴⁾ Roll widths and lengths have a tolerance of ±1%.

GSE is a leading manufacturer and marketer of geosynthetic lining products and services. We've built a reputation of reliability through our dedication to providing consistency of product, price and protection to our global customers.

Our commitment to innovation, our focus on quality and our industry expertise allow us the flexibility to collaborate with our clients to develop a custom, purpose-fit solution.



[DURABILITY RUNS DEEP] For more information on this product and others, please visit us at GSEworld.com, call 800.435.2008 or contact your local sales office.

J.5-D Toe Drain Capacity



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: TOE DRAIN CAPACITY**Problem Statement**

Determine whether the 4-inch toe drains (discharge pipe) are sized to drain water that percolates through the final cover and is transmitted downslope through the 6-oz geocomposite.

Given

- Toe drains are 4-inch diameter polyethylene pipes with smooth interior, sloped at 3%.
- The toe drains are spaced at 200-foot intervals at the toe of the final cover slope (see **Design Drawings**).
- The geocomposite is a 6-oz/yd² double-sided geocomposite.
- Product information sheet from GSE FabriNet 200-mil Geocomposite. (Refer to attached pages).

Assumptions

- The toe drains (smooth pipes) have a Manning's coefficient of 0.012.
- The geocomposite has a transmissivity of 0.5 gal/(min*ft) (see attached product information sheet).
- No flow reduction factors due to deformation of clogging will be applied to the geocomposite (conservative assumption).

Calculations

- 1) Calculate the maximum flow rate converging on the toe drain based on manufacturer transmissivity values, assuming that flow across the 200-foot wide spacing converges at the same time:

$$\text{Transmissivity} = \frac{0.5 \text{ gal}}{\text{min} \cdot \text{ft}} \times \frac{1 \text{ ft}^3}{7.48 \text{ gal}} \times \frac{1 \text{ min}}{60 \text{ sec}} = 0.001 \frac{\text{ft}^2}{\text{sec}}$$

$$\text{Max Flow Rate} = 200 \text{ ft} \times 0.001 \frac{\text{ft}^2}{\text{sec}} = 0.20 \frac{\text{ft}^3}{\text{sec}}$$

- 2) Determine maximum flow rate with the full flow capacity rate for the 4-inch pipes:

$$\text{Cross Sectional Area of the 4" diameter pipe: } A = \pi \left(\frac{D^2}{4} \right) = \pi \left(\frac{\left(\frac{4 \text{ inches}}{12 \text{ inches/ft}} \right)^2}{4} \right) = 0.09 \text{ ft}^2$$

$$\text{Wetted Perimeter of pipe: } P = 2\pi \left(\frac{D}{2} \right) = 2\pi \left(\frac{\frac{4 \text{ inches}}{12 \text{ inches/ft}}}{2} \right) = 1.05 \text{ ft}$$

$$\text{Hydraulic Radius: } R_h = \frac{A}{P} = \frac{0.09 \text{ ft}^2}{1.05 \text{ ft}} = 0.09 \text{ ft}$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

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Date: 05/2022

TITLE: TOE DRAIN CAPACITY

Cross Sectional Velocity (Manning Equation):

$$v = \left(\frac{k_n}{n} \right) (R_h)^{\frac{2}{3}} (S)^{\frac{1}{2}}$$

Where,

 $k_n = 1.49$ for English units $n =$ Manning's Coefficient of Roughness, 0.012 for a smooth pipe $S =$ slope, 0.03 ft/ft

$$v = \left(\frac{1.49}{0.012} \right) (0.09 \text{ ft})^{\frac{2}{3}} (0.03 \text{ ft/ft})^{\frac{1}{2}} = 4.32 \text{ ft/sec}$$

$$Q = vA = (4.32 \text{ ft/sec})(0.09 \text{ ft}^2) = 0.39 \text{ ft}^3/\text{sec}$$

- 3) Compare the maximum rate of flow converging on the toe drain with the full flow capacity of the pipe.

$$0.20 \frac{\text{ft}^3}{\text{sec}} < 0.39 \frac{\text{ft}^3}{\text{sec}}$$

Results

The Zion Landfill Site 2 North Expansion has been designed with a toe drain spacing of approximately 200 feet. The results of this calculation indicate that the toe drains will pass the maximum discharge rate through the geocomposite.

Published Technical References

GSE FabriNet 200 mil Geocomposite

GSE FabriNet geocomposite consists of a 200 mil thick GSE HyperNet geonet heat-laminated on one or both sides with a GSE nonwoven needle-punched geotextile. The geotextile is available in mass per unit area range of 6 oz/yd² to 16 oz/yd². The geocomposite is designed and formulated to perform drainage function under a range of anticipated site loads, gradients and boundary conditions.



AT THE CORE:
A 200 mil thick HyperNet geonet heat-laminated on one or both sides with a nonwoven needlepunched geotextile.

Product Specifications

Tested Property	Test Method	Frequency	Minimum Average Roll Value ⁽¹⁾		
			6 oz/yd ²	8 oz/yd ²	10 oz/yd ²
Geocomposite					
Transmissivity ⁽²⁾ , gal/min/ft, (m ² /sec) Double-Sided Composite	ASTM D 4716	1/540,000 ft ²	0.5 (1x10 ⁻³)	0.5 (1x10 ⁻⁴)	0.4 (9x10 ⁻⁵)
Single-Sided Composite			4.8 (1x10 ⁻³)	4.8 (1x10 ⁻³)	4.3 (9x10 ⁻⁴)
Ply Adhesion, lb/in	ASTM D 7005	1/50,000 ft ²	1.0	1.0	1.0
Geonet Core^(1,3) - GSE HyperNet					
Geonet Core Thickness, mil	ASTM D 5199	1/50,000 ft ²	200	200	200
Transmissivity ⁽²⁾ , gal/min/ft (m ² /sec)	ASTM D 4716		9.6 (2 x 10 ⁻³)	9.6 (2 x 10 ⁻³)	9.6 (2 x 10 ⁻³)
Density, g/cm ³	ASTM D 1505	1/50,000 ft ²	0.94	0.94	0.94
Tensile Strength (MD), lb/in	ASTM D 7179	1/50,000 ft ²	45	45	45
Carbon Black Content, %	ASTM D 4218	1/50,000 ft ²	2.0	2.0	2.0
Geotextile^(1,3)					
Mass per Unit Area, oz/yd ²	ASTM D 5261	1/90,000 ft ²	6	8	10
Grab Tensile Strength, lb	ASTM D 4632	1/90,000 ft ²	160	220	260
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Permittivity, sec ⁻¹	ASTM D 4491	1/540,000 ft ²	1.5	1.3	1.0
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NOMINAL ROLL DIMENSIONS⁽⁴⁾					
Roll Width, ft			14.75	14.75	14.75
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Roll Area, ft ²	Double-Sided Composite		3,982	3,835	3,392
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NOTES:

- ⁽¹⁾ All geotextile properties are minimum average roll values except AOS which is maximum average roll value and UV resistance is typical value. Geonet core thickness is nominal value.
- ⁽²⁾ Gradient of 0.1, normal load of 10,000 psf, water at 70°F between steel plates for 15 minutes. Contact GSE for performance transmissivity value for use in design.
- ⁽³⁾ Component properties prior to lamination.
- ⁽⁴⁾ Roll widths and lengths have a tolerance of ±1%.

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J.5-E Terrace Berms



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: STABILITY OF TERRACE BERM

Problem Statement

Determine the factor of safety (FS) against slope failure of the terrace berms on the final cover for static and seismic conditions. 35 Ill. Admin. Code Section 811.304 (d) requires that “the waste disposal unit shall be designed to achieve a factor of safety against slope failure of at least: 1.5 for static conditions and 1.3 under seismic conditions.”

Given

- Das, Braja M., *Principles of Geotechnical Engineering*. PWS-Kent, Second Edition, 1990. (Please see attached pages).
- Design detail of typical terrace berm and final cover included in the design drawings contained in this application.
- Appendix J.1** “Summary of Geotechnical Design Parameters” contained in this Application.

Assumptions

- Assume berm will typically be constructed from the same materials as the final cover soil, which has the following properties:

<u>Short-term Conditions</u>		<u>Long-term Conditions</u>	
γ_{soil}	= 130.3 pcf	γ'_{soil}	= 130.3 pcf
Φ	= 11.8 degrees	Φ'	= 34.3 degrees
c	= 1,465 psf	c'	= 0 psf

- The terrace berms for the proposed final cover typically have a 2H:1V slope and will rise approximately 2-feet above the highest common point of the slope.
- The slope of the final cover side slopes is 4H:1V, therefore $\beta = 14.04$ degrees.
- The horizontal acceleration is 0.0461g.



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: STABILITY OF TERRACE BERM**Calculations**Volume of Terrace Berm per Unit Width

Refer to the attached diagram of the typical terrace berm for dimensions.

$$\text{Volume of Terrace} = (0.5)(20 \text{ ft})(10 \text{ ft}) - (0.5)(20 \text{ ft})(5 \text{ ft}) - (0.5)(4 \text{ ft})(2 \text{ ft}) - (0.5)(6 \text{ ft})(3 \text{ ft})$$

$$\text{Volume of Terrace} = 37.0 \text{ ft}^3 \text{ per foot of berm}$$

$$\text{Weight of Terrace (W)} = 37.0 \frac{\text{ft}^3}{\text{ft}} (130.3 \text{ pcf}) = 4,821.1 \text{ lb/ft}$$

Static Conditions: Short-term Conditions

The factor of safety under static conditions using the Culmann's Method.

Normal Force (N):

$$N = W \cos\beta = \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\cos 14.04^\circ) = 4,677.1 \text{ lb/ft}$$

Length of Sliding Surface (\overline{AB}):

$$\overline{AB} = \frac{H}{\sin\beta} = \frac{5.0 \text{ ft}}{\sin(14.04^\circ)} = 20.6 \text{ ft}$$

N per length of sliding surface:

$$N' = \frac{N}{\overline{AB}} = \frac{4,677.1 \text{ lb/ft}}{20.6 \text{ ft}} = 227.0 \text{ psf}$$

Resisting Force (F_R):

$$F_R = N' \times \tan(\varphi) + c = (227.0 \text{ psf})(\tan 11.8^\circ) + 1,465 \text{ psf} = 1,512.4 \text{ psf}$$

Driving Force (F_D):

$$F_D = \frac{W \sin\beta}{\overline{AB}} = \frac{\left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\sin 14.04^\circ)}{20.6 \text{ ft}} = 56.8 \text{ psf}$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

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TITLE: STABILITY OF TERRACE BERM*Factor of Safety (F.S.):*

$$F.S. = \frac{F_R}{F_D} = \frac{1,512.4 \text{ psf}}{56.8 \text{ psf}} = 26.6$$

Static Conditions: Long-term Conditions

The factor of safety under static conditions using the Culmann's Method.

Normal Force (N):

$$N = W \cos\beta = \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\cos 14.04^\circ) = 4,677.1 \text{ lb/ft}$$

Length of Sliding Surface (\overline{AB}):

$$\overline{AB} = \frac{H}{\sin\beta} = \frac{5.0 \text{ ft}}{\sin(14.04^\circ)} = 20.6 \text{ ft}$$

N per length of sliding surface:

$$N' = \frac{N}{\overline{AB}} = \frac{4,677.1 \text{ lb/ft}}{20.6 \text{ ft}} = 227.0 \text{ psf}$$

Resisting Force (F_R):

$$F_R = N' \times \tan(\phi) + c = (227.0 \text{ psf})(\tan 34.3^\circ) + 0 \text{ psf} = 154.8 \text{ psf}$$

Driving Force (F_D):

$$F_D = \frac{W \sin\beta}{\overline{AB}} = \frac{\left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\sin 14.04^\circ)}{20.6 \text{ ft}} = 56.8 \text{ psf}$$

Factor of Safety (F.S.):

$$F.S. = \frac{F_R}{F_D} = \frac{154.8 \text{ psf}}{56.8 \text{ psf}} = 2.73$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: STABILITY OF TERRACE BERMSeismic Conditions: Short-term Conditions

The factor of safety under seismic conditions using the Culmann's Method.

Normal Force (N):

$$\begin{aligned}
 N &= W \cos \beta - 0.0461 W \cos \beta \\
 N &= \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \cos (14.04^\circ) - 0.0461 \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \cos (14.04^\circ) \\
 N &= 4,461.5 \text{ lb/ft}
 \end{aligned}$$

N per length of sliding surface:

$$N' = \frac{N}{AB} = \frac{4,461.5 \text{ lb/ft}}{20.6 \text{ ft}} = 216.6 \text{ psf}$$

Resisting force (F_R):

$$F_R = N' \tan(\varphi) + c = (216.6 \text{ psf})(\tan 11.8^\circ) + 1,465 \text{ psf} = 1,510.3 \text{ psf}$$

Driving Force (F_D):

$$\begin{aligned}
 F_D &= \frac{W \sin \beta + 0.0461 W \cos \beta}{AB} \\
 F_D &= \frac{\left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \sin (14.04^\circ) + 0.0461 \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \cos (14.04^\circ)}{20.6 \text{ ft}} \\
 F_D &= 67.2 \text{ psf}
 \end{aligned}$$

Calculation of Safety Factor ($F.S.$):

$$F.S. = \frac{F_R}{F_D} = \frac{1,510.3 \text{ psf}}{67.2 \text{ psf}} = 22.5$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: STABILITY OF TERRACE BERMSeismic Conditions: Long-term Conditions

The factor of safety under seismic conditions using the Culmann's Method.

Normal Force (N):

$$N = W \cos \beta - 0.0461 W \cos \beta$$

$$N = \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\cos 14.04^\circ) - 0.0461 \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) (\cos 14.04^\circ)$$

$$N = 4,461.5 \text{ lb/ft}$$

N per length of sliding surface:

$$N' = \frac{N}{AB} = \frac{4,461.5 \text{ lb/ft}}{20.6 \text{ ft}} = 216.6 \text{ psf}$$

Resisting force (F_R):

$$F_R = N' \tan(\varphi) + c = (216.6 \text{ psf})(\tan 34.3^\circ) + 0 \text{ psf} = 147.8 \text{ psf}$$

Driving Force (F_D):

$$F_D = \frac{W \sin \beta + 0.0461 W \cos \beta}{AB}$$

$$F_D = \frac{\left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \sin (14.04^\circ) + 0.0461 \left(4,821.1 \frac{\text{lb}}{\text{ft}}\right) \cos (14.04^\circ)}{20.6 \text{ ft}}$$

$$F_D = 67.2 \text{ psf}$$

Calculation of Safety Factor (F.S.):

$$F.S. = \frac{F_R}{F_D} = \frac{147.8 \text{ psf}}{67.2 \text{ psf}} = 2.20$$



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

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Date: 05/2022

Checked by: DAM

Date: 05/2022

TITLE: STABILITY OF TERRACE BERM

Results

The terrace berms have been designed to meet the required factor of safety for both static (at least 1.5) and seismic conditions (at least 1.3). In summary:

FACTORS OF SAFETY	
Short-term Conditions	
Static Conditions	Seismic Conditions
26.6	22.5
Long-term Conditions	
Static Conditions	Seismic Conditions
2.73	2.20



Client: Zion Landfill, Inc.

Project: Zion Landfill – Site 2 North Expansion

Project #: 631020105

Calculated By: ORC

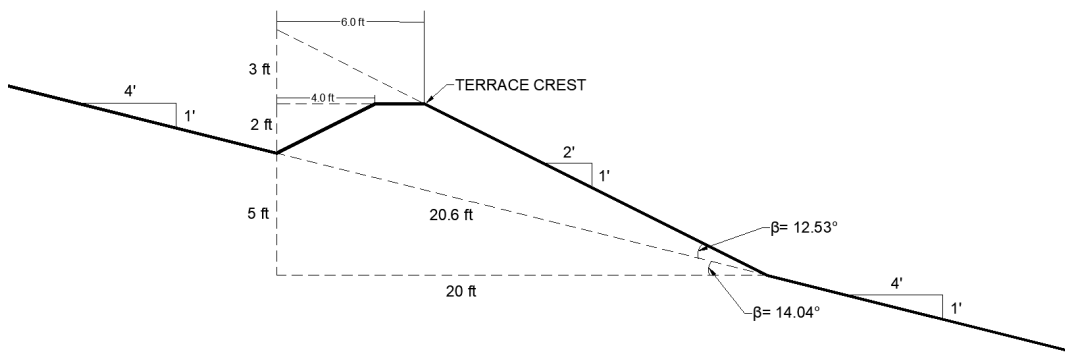
Date: 05/2022

Checked by: DAM

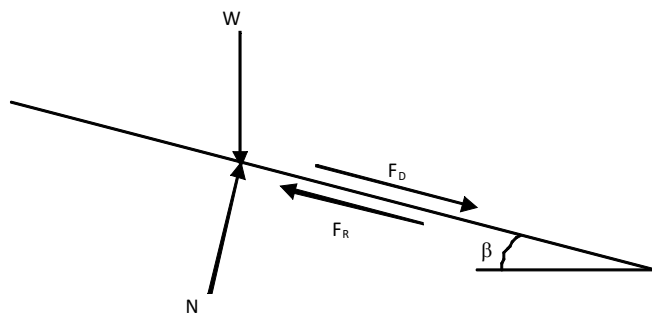
Date: 05/2022

TITLE: STABILITY OF TERRACE BERM

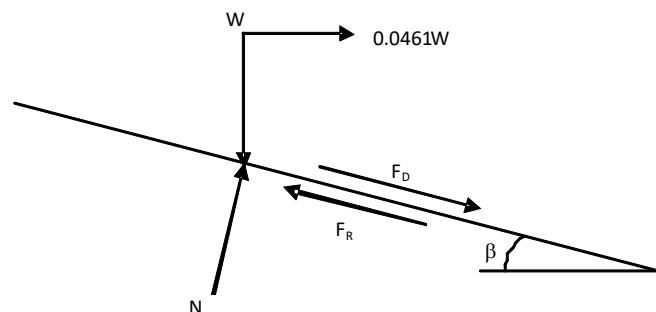
Design Detail for Terrace Berm:



Static Conditions:



Seismic Conditions:

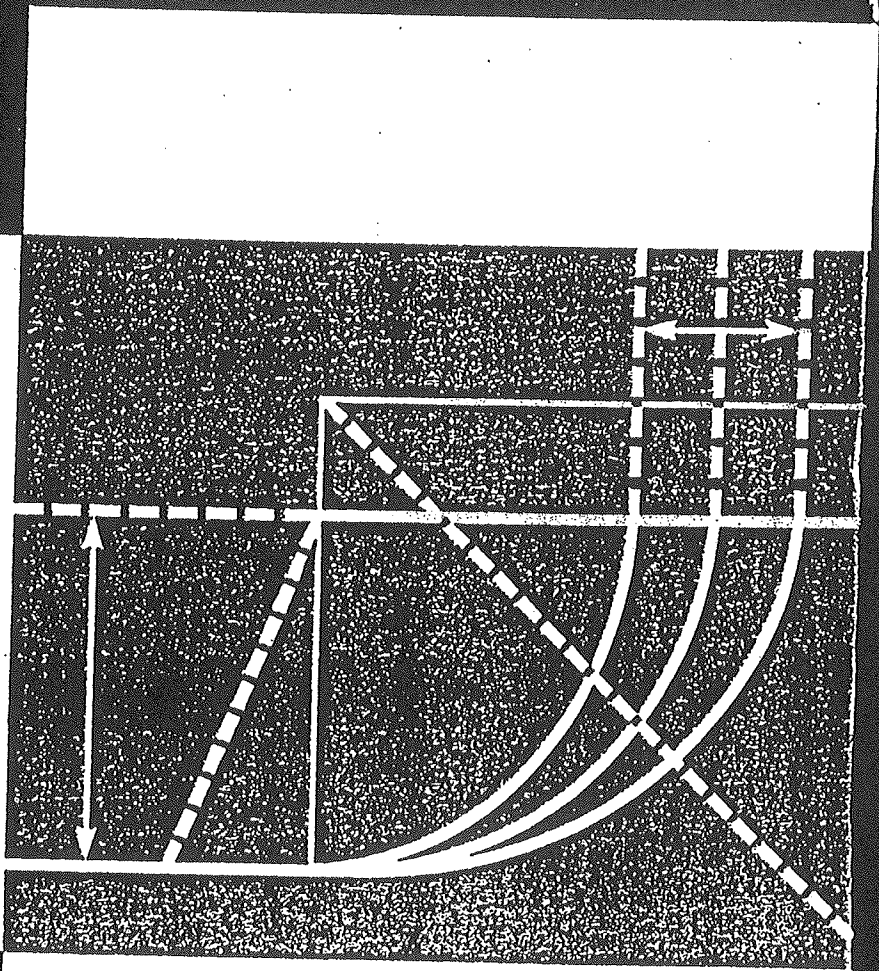


Published Technical References

B R A J A M . D A S

**Principles of
Geotechnical Engineering**

Third Edition



Analysis of Finite Slope with Plane Failure Surface (Culmann's Method)

This analysis is based on the assumption that the failure of a slope occurs along a plane when the average shearing stress tending to cause the slip is more than the shear strength of the soil. Also, the most critical plane is the one that has a minimum ratio of the average shearing stress that tends to cause failure to the shear strength of soil.

Figure 12.5 shows a slope of height H . The slope rises at an angle β with the horizontal. AC is a trial failure plane. Considering a unit length perpendicular to the section of the slope, the weight of the wedge $ABC = W$:

$$\begin{aligned} W &= \frac{1}{2} (H)(\overline{BC})(1)(\gamma) \\ &= \frac{1}{2} H(H \cot \theta - H \cot \beta) \gamma \\ &= \frac{1}{2} \gamma H^2 \left[\frac{\sin(\beta - \theta)}{\sin \beta \sin \theta} \right] \end{aligned} \quad (12.29)$$

The normal and tangential components of W with respect to the plane AC are as follows:

$$\begin{aligned} N_a &= \text{normal component} = W \cos \theta \\ &= \frac{1}{2} \gamma H^2 \left[\frac{\sin(\beta - \theta)}{\sin \beta \sin \theta} \right] \cos \theta \end{aligned} \quad (12.30)$$

$$\begin{aligned} T_a &= \text{tangential component} = W \sin \theta \\ &= \frac{1}{2} \gamma H^2 \left[\frac{\sin(\beta - \theta)}{\sin \beta \sin \theta} \right] \sin \theta \end{aligned} \quad (12.31)$$

The average normal stress and shear stress on the plane AC may be given by

$$\begin{aligned} \sigma &= \text{average normal stress} \\ &= \frac{N_a}{(AC)(1)} = \frac{N_a}{\left(\frac{H}{\sin \theta} \right)} \\ &= \frac{1}{2} \gamma H \left[\frac{\sin(\beta - \theta)}{\sin \beta \sin \theta} \right] \cos \theta \sin \theta \end{aligned} \quad (12.32)$$

and

$$\begin{aligned} \tau &= \text{average shear stress} \\ &= \frac{T_a}{(AC)(1)} = \frac{T_a}{\left(\frac{H}{\sin \theta} \right)} \\ &= \frac{1}{2} \gamma H \left[\frac{\sin(\beta - \theta)}{\sin \beta \sin \theta} \right] \sin^2 \theta \end{aligned} \quad (12.33)$$

